## AROMA JOES

# Site Plan Application \#08-21 <br> STAFF REPORT 

August 25, 2021

SITE: 56 Derry Street; Map 173 Lot 029-000
Zoning: Business (B)
Purpose of Plans: Propose an Aroma Joe's drive-thru coffee shop at 56 Derry Street with associated parking and drives.

## Plans Under Review:

Non-Residential Site Plan, Aroma Joe's; prepared by Keach-Nordstrom Associates, Inc., 10 Commerce Park North, Suite 3B, Bedford, NH 03110; prepared for owner: Steve S. \& Hsiang Hwa W. Pan, 13 King Henry Drive, Londonderry, NH 03053 and owner/applicant: Scott Ziefelder, 169 Cannan Back Road, Barrington, NH 03825; consisting of 16 sheets including a cover sheet, with general notes 1-32 on Sheet 1; dated June 22, 2021, last revised August 12, 2021. [Plan set attached hereto]

## Attachments:

A. Second Round of Peer Review by Fuss \& O'Neill, dated August 16, 2021
B. Traffic Impact and Access Study, prepared by TEPP, received July 20, 2021
C. Adjusted CAP Fee Worksheet

## Application Tracking:

- June 23, 2021 - Application received.
- July 20, 2021 - Traffic Impact and Access Study received.
- July 28, 2021 - Application accepted, public hearing held, waiver granted for relief from residential buffer, continued to 8/25/21.
- August 17, 2021 - Revised Plans received.
- August 25, 2021 - Continuance scheduled.


## Comments \& Recommendations:

## Status Update

Please note that since the revised plan set was received on August 17, 2021, and this report was authored on August 18, 2021, a full review by town staff and the peer reviewer remains pending.

Since the last iteration of the plan set the applicant has made the following modifications:

1. Hours of operation (Sheet 1, note 15) have been revised to 5:15 AM to 9:00 PM, Monday through Sunday.
2. The proposed freestanding sign shows on Sheet 1 and the detail of the sign is on Sheet 10.
3. A crosswalk has been added per request at previous meeting.
4. The proposed sidewalk easement shows on Sheet E1.
5. The loading space and dumpster location are clearly identified on the plan

## Peer Review

Attachment A is the second round of peer review comments, distributed on Monday, August 16, 2021. For that reason, the applicant has not had time to provide a response. Peer review of the Traffic Study (Attachment B) is ongoing but expected soon.

1. The outstanding comments primarily focus on the stormwater/drainage design. These comments are under review by the applicant as well as the Town Engineer.
2. Some comments related to items being reviewed in the Traffic Study
3. Other items were administrative in nature and have been addressed by the applicant.

## TRAFFIC STUDY

Attachment B is the Traffic Impact and Access Study prepared by Kim Hazarvartian of TEPP
LLC, currently under peer review. The study concludes that no significant traffic impact results from this application.

The study reports:
2022 total vehicle trips:

- weekday daily, 629 (total of in and out)
- weekday AM-street-peak hour*, 106 (53 in and 53 out)
- weekday PM-street-peak hour, 40 (20 in and 20 out)

2032 total vehicle-trips are:

- weekday daily, 694 (total of in and out)
- weekday AM-street-peak hour, 117 (58 in and 539 out) [Staff assumes 539 is a typo]
- weekday PM-street-peak hour, 44 (22 in and 22 out)

Primary trips ${ }^{* *}$ are added to Derry Road near the site. 2022 primary vehicle-trips are:

- weekday daily, 69 (total of in and out)
- weekday AM-street-peak hour, 12 (6 in and 6 out)
- weekday PM-street-peak hour, 4 ( 2 in and 2 out)

2032 primary vehicle-trips are:

- weekday daily, 78 (total of in and out)
- weekday AM-street-peak hour, 13 ( 6 in and 7 out)
- weekday PM-street-peak hour, 6 ( 3 in and 3 out)
*Peak hours are reported to be 7:00 A.M. to 8:00 P.M., and 4:00 P.M to 5:00 P.M.
**Primary trips are vehicles drawn to the site by the coffee shop rather vehicles that are already travelling by the site. Peer review has indicated this is an acceptable approach with one caveat described in the CAP fee discussion below.

Staff has asked the Town's peer review consultant to examine how the traffic study lines up with the excepted operations of Aroma Joe's. For example, the study reports 53 vehicles in 2022 and 58 vehicles in 2032 during the morning peak hour. This requires customer turnover to be about 1 minute per vehicle. The operator of Aroma Joe's indicated a typical turnover time ranging from a minute to a few. This matters in determining the adequacy of the queue length to avoid the familiar problem of drive-thru customers spilling out onto public roads.

## CAP FEE AdJustment

While Fuss \& O'Neill serves as the Town's peer review consultant, including the traffic study in this application, VHB establishes the rates and methodology used in the Town's impact fee system, or Cost Allocation Procedure (CAP). Staff consulted VHB on the appropriate calculation of the CAP fee for this application, suspecting that using the square footage of the building was not an appropriate representation of its impact to traffic. In agreement with Staff's query, VHB recommended using the alternate method for unique categories, which is to multiply the number of new daily trips (or, Primary Trips) by $\$ 199$. VHB noted that the Traffic Study appears to allocate $10 \%$ of total trips as Primary Trips ( 69 of 692), whereas their standard is 15\%, or 104 Primary Trips. See Attachment C for the updated CAP Fee worksheet.

As it relates to the overall traffic impact, the total number of vehicles remains the same. Please note that the Traffic Study remains under peer review where the discrepancy between the two methodologies will be addressed.

## Plan Notes

## Suggested Changes to Plan/Notes:

1. Note \#29: Staff suggests amending the note to state "All signs are subject to approval by the Zoning Administrator prior to installation." The note provided is consistent with §276.11.1.B (13), however, this regulation is inconsistent with actual sign permit practices.
2. Note \#19 \& \#27: Staff suggests amending instances of "Community Development Department" to "Planning Department"
3. A plan note should address the hours of refuse removal / garbage pick-up. The typical hours are 7:00 A.M. and 7:00 P.M., Monday through Friday only.
4. References to Derry Road should be revised to Derry Street,

## Draft Motions

## CONTINUE the public hearing to a date certain:

I move to continue the public hearing for the site plan application for Aroma Joes at 56 Derry Street; Map 173 Lot 029-000 to date certain, $\qquad$ , 2021.

Motion by: $\qquad$ Second: $\qquad$ Carried/Failed: $\qquad$

## APPROVE the site plan application:

I move to approve the Site Plan: Non-Residential Site Plan, Aroma Joe's; prepared by KeachNordstrom Associates, Inc., 10 Commerce Park North, Suite 3B, Bedford, NH 03110; prepared for owner: Steve S. \& Hsiang Hwa W. Pan, 13 King Henry Drive, Londonderry, NH 03053 and owner/applicant: Scott Ziefelder, 169 Cannan Back Road, Barrington, NH 03825; consisting of 16 sheets including a cover sheet, with general notes 1-32 on Sheet 1; dated June 22, 2021, last revised August 12, 2021; subject to, and revised per, the following stipulations:

1. All stipulations of approval shall be incorporated into the Development Agreement, which shall be recorded at the HCRD, together with the Plan.
2. All improvements shown on the Plan shall be completed in their entirety and at the expense of the applicant or the applicant's assigns.
3. Prior to the issuance of a Certificate of Occupancy, an L.L.S. Certified "as-built" site plan shall be provided to the Town of Hudson Land Use Development, confirming that the site conforms to the Plan approved by the Planning Board.
4. A cost allocation procedure (CAP) amount of $\$ 20,696.00$ shall be paid prior to the issuance of a Certificate of Occupancy
5. The onsite drainage system shall be constructed and maintained in compliance with NHDES requirements for such systems.
6. Prior to the Planning Board endorsement of the Plan, it shall be subject to final administrative review by Town Planner and Town Engineer.
7. The applicant shall schedule a pre-construction meeting with the Town Engineer prior to beginning work on the site.
8. Hours of refuse removal shall be exclusive to the hours between 7:00 A.M. and 7:00 P.M., Monday through Friday only.

Motion by: $\qquad$ Second: $\qquad$ Carried/Failed: $\qquad$

## FUSS \& O'NEILL

August 16, 2021

Mr. Brian Groth<br>Town Planner<br>Town of Hudson<br>12 School Street<br>Hudson, NH 03051

Re: Town of Hudson Planning Board Review
Aroma Joe's Site Plan, 56 Derry Street
Tax Map 173 Lot 29; Acct. \#1350-970
Reference No. 20030249.2040

Dear Mr. Groth:

Fuss \& O’Neill (F\&O) has reviewed the second submission of the materials received on June 23, 2021, related to the above-referenced project. Authorization to proceed was received on August 10, 2021. A list of items reviewed is enclosed. The scope of our review is based on the Site Plan Review Codes, Stormwater Codes, Driveway Review Codes, Sewer Use Ordinance 77, Zoning Regulations, and criteria outlined in the CLD Consulting Engineers Proposal approved September 16, 2003, revised September 20, 2004, June 4, 2007, September 3, 2008, and October 2015.

The project appears to consist of the development of a drive-thru coffee shop on a previously undeveloped site. Proposed improvements to the site also include the construction of a driveway, parking areas, drainage improvements, landscaping, lighting and other associated site improvements. The proposed buildings will be serviced by public water and sewer.

The following items have outstanding issues:

1. Site Plan Review Codes (HR 275)
g. Former Fuss \& O'Neill Comment: HR 275-8.C.(6). The applicant has not provided any off-street loading spaces on the plan set.
Current Fuss \& O'Neill Comment: The applicant has added a loading space to the plan set. We note that the space shown is 40 feet long instead of the standard 60 feet. The applicant should confirm that the size of anticipated delivery vehicles will fit in this location.
h. Former Fuss \& O'Neill Comment: HR 275-9.C.(11). The applicant has provided one handicap space for the site which meets the one space required. We recommend that the applicant add spot grades to the parking lot and sidewalk area to ensure that it is constructed as intended. This is especially important in the area of the bandicap space and ramp.
Current Fuss \& O’Neill Comment: The applicant has provided spot grades for the parking lot ramp locations. We continue to recommend spot grades be provided for the sidewalk ramps as well.

Mr. Brian Groth
August 16, 2021
Page 2 of 8
i. Former Fuss \& O Neill Comment: HR 275-9.F. The applicant did not provide copies of any easements or deeds as part of the package received for review, and has not shown any existing or proposed easements on the plans.
Current Fuss \& O’Neill Comment: The applicant has stated that no easements or deeds are required. We continue to recommend a sidewalk easement be provided to the Town for the relocated sidewalk.

## 3. Driveway Review Codes (HR 275-8.B. (34)/Chapter 193)

d. New Fuss \& O'Neill Comment: We understand that the Town has requested a crosswalk be installed from the Derry Street sidewalk to the sidewalk at the proposed building. We recommend that the applicant provide appropriate pedestrian signage both internally for this crosswalk and for vehicles approaching Derry Street at the right turn lane.

## 6. Drainage Design/Stormwater Management (HR 275-9.A./Chapter 290)

a. Former Fuss \& O'Neill Comment: HR 275-9.A.(3). The applicant should provide test pits within the footprint of the infiltration basin area, as required by NHDES and common engineering practice.
Current Fuss \& O'Neill Comment: The applicant has noted that test pits are consistent within the site and within close proximity to the infiltration basin areas. We note test pit \#2 has an existing elevation of 160.0, with 66" depth to ESHWT as noted within the Test Pit data and BMP worksheet for Infiltration Pond 2. Applying this test pit data to the entire Infiltration Pond\#2: Bottom of basin is 159.0, existing elevation at bottom of basin is 164.0. Calculating ESHW'T to be 66" below 164.0, computes to 158.5 and not 156.0 as noted within the BMP worksheet. This does not meet the 3.0 ' required separation from ESHWT. The applicant should provide additional test pit information to support the use of an infiltration basin situated upon the site in respect to existing elevations.
b. Former Fuss \& O’Neill Comment: HR 290-5.A.9. \& 290-5.A.11. The applicant should provide NHDES BMP worksheets and an Infiltration Feasibility Report to illustrate the ESHWT is accounted for within the BMP design, as well as overall Stormwater Design meets NHDES standards.
Current Fuss \& O’Neill Comment: The applicant has provided BMP worksheets. We continue to recommend the applicant provide the required Infiltration Feasibility Report.
c. Former Fuss \& O'Neill Comment: HR 290-7.B.14. Although the property has been partially developed in the past, the contours illustrate a low point near the existing CBs to be removed, as well as the close proximity of the abutting wetland discharge point. The applicant should provide a letter from a wetland scientist stating wetlands do not exist upon the site.
Current Fuss \& O'Neill Comment: The applicant has stated that a letter was provided under separate cover. We note that a letter was not provided as part of the package received for review.
d. Former Fuss \& O’Neill Comment: HR 290-7.B.16. The applicant should label snow storage areas upon the plan set. Due to the close proximity of wetlands and proposed infiltration systems, we suggest reviewing the need for onsite signage or fencing to ensure proper snow storage/removal occurs.
Current Fuss \& O'Neill Comment: The applicant has illustrated two snow storage areas upon the landscaping plan. The applicant should review the need for fencing or signage to

Mr. Brian Groth
August 16, 2021
Page 3 of 8
ensure snow storage does not occur within the footprint of the infiltration basins.
e. Former Fuss \& O'Neill Comment: Engineering Technical Guideline \& Typical Details (ETGTD) ETGTD 910.8. The HydroCAD analysis illustrates that the proposed conditions utilize an infiltration rate of $6.00 \mathrm{in} / \mathrm{hr}$. The applicant should provide additional information and/ or conversion calculations to support the use of the infiltration rate. Does this rate utilize a factor of safety, does it follow typical current engineering practice as outlined within Env-W q 1504.14(c), does the soil need to be amended, etc.
Current Fuss \& O'Neill Comment: The applicant has updated the infiltration rate to be $3.0 \mathrm{in} / \mathrm{hr}$, from the previously utilized $6.0 \mathrm{in} / \mathrm{hr}$. The Test Pit information provided within the Stormwater Management and Erosion Control Plan illustrates a perc rate of 2.0 $\mathrm{min} / \mathrm{inch}$, which is equivalent to the utilized $3.0 \mathrm{in} / \mathrm{hr}$. The applicant should provide additional information as to why this rate was utilized and a factor of safety is not being accounted for within the calculations.
g. Former Fuss \& O'Neill Comment: ETGTD 920.6. The applicant should provide rip rap outlet calculations within the Stormwater Management Report.
Current Fuss \& O'Neill Comment: The applicant has provided rip rap calculations. The applicant should also provide all outlet protection apron calculations, as illustrated upon Plan Sheet 10 of the Plan Set.
h. Former Fuss \& O'Neill Comment: ETGTD 930.4. We note that the majority of the stormwater design utilizes pipe slopes of less than the required $2.0 \%$. The applicant should review these pipe slopes and provide calculations showing that the drain line velocities are self-cleaning.
Current Fuss \& O'Neill Comment: The applicant has provided information demonstrating that the pipes are self-cleaning within the HydroCAD 2-year report. The applicant should review this with the Town to ensure this is an acceptable variation from the Regulations.
9. Landscaping (HR 275-8.C.(7) \& 276-11.1.B.(20)) and Lighting (HR 276-11.1.B.(14))
d. Former Fuss \& O'Neill Comment: HR 276-11.1.B.(14). The applicant has noted that the hours of operation for the facility are 6:00 am to $6: 00 \mathrm{pm}$. The applicant should provide additional information regarding whether the lights are intended to be in operation during non-working hours.
Current Fuss \& O'Neill Comment: The applicant has stated that the lights will be on a timer and scheduled around the operating hours of the business. We note that applicant should update the business hours on the plan set per the Planning Board meeting on July 28, 2021.

## 11. Other

d. New Fuss \& O'Neill Comment: The applicant should update Derry Road plan references to Derry Street.

The following items require Town evaluation or input:

1. Site Plan Review Codes (HR 275)
a. Former/Current Fuss \& O'Neill Comment: Hudson Regulation (HR) 275-6.C. The applicant has proposed to realign the existing sidewalk along Derry Street to accommodate

Mr. Brian Groth
August 16, 2021
Page 4 of 8
a turning lane and the site entrance. The Town should review a need for an easement for the new sidewalk areas that are out of the Right-of-Way.

The following items are resolved or have no further Fuss \& O’Neill input:

## 1. Site Plan Review Codes (HR 275)

b. Former Fuss \& O'Neill Comment: HR 275-6.I. The scope of this review does not include the adequacy of any fire protection provisions for the proposed building. No fire service connections to the buildings are shown.
Current Fuss \& O’Neill Comment: The applicant has added a fire service to the plan set. No further Fuss \& O'Neill comment.
c. Former Fuss \& O Neill Comment: HR 275-6.T. The applicant is proposing the construction of a 10 foot wide right turn lane on Derry Street southbound to access the site. We note that no grading was provided for this right turn lane area. The applicant should review and provide spot grades on the plans to ensure positive drainage will exist in this area.
Current Fuss \& O’Neill Comment: The applicant has added spot grades to the turn lane. No further Fuss \& O’Neill comment.
d. Former Fuss \& O'Neill Comment: HR 275-6.T. The applicant has shown a 50 foot long right turn lane with a 50' long 10:1 taper. The applicant should confirm that these turn lane dimensions meet Town standards and the turn lane is long enough to accommodate expected traffic entering the site.
Current Fuss \& O’Neill Comment: The applicant has increased the lane to an 11 foot width and will be reviewed as part of the traffic review. No further Fuss \& O'Neill comment.
e. Former Fuss \& O 'Neill Comment: HR 275-8.C.(2) and Zoning Ordinance (ZO) 334-15.A. The applicant has provided parking calculations on the plan set. The applicant has noted that 9 parking are required for the 900 square foot facility and that 9 spaces are provided.
f. Former Fuss \& O'Neill Comment: HR 275-8.C.(2).(c).[5]. The applicant should show the required stacking spaces in the drive thru area. We note that the Regulation requires a minimum of 12 stacking spaces in the drive thru, or a number of stacking spaces determined appropriate by the Planning Board for the use served.

Current Fuss \& O’Neill Comment: The applicant has noted that the 12 stacking spaces required have been added to the plan set. The plan shows a 240 feet of stacking length for the drive thru location. No further Fuss \& O’Neill comment.

## 2. Administrative Review Codes (HR 276)

a. Former Fuss \& O 'Neill Comment: HR 276-11.1.B.(12).(c). The applicant should review and confirm that a 100 foot buffer exists between the residential use to the west of the site and the proposed development of this commercial site.
Current Fuss \& O’Neill Comment: The applicant has included a waiver application as part of the package received for review. We also note that waiver was approved by the Planning Board on June 28, 2021. No further Fuss \& O’Neill comment.

Mr. Brian Groth
August 16, 2021
Page 5 of 8
b. Former Fuss \& O'Neill Comment: HR 276-11.1.B.(13). The applicant has not included details for any proposed site signage other than traffic signs. The applicant has included a note stating that, "All signs are subject to approval by the Hudson Planning Board prior to installation."
c. Former Fuss \& O'Neill Comment: HR 276-11.1.B.(16).The applicant has not provided the locations of all driveways and travel ways within 200 feet of the site.
Current Fuss \& O'Neill Comment: The applicant has provided the required information. No further Fuss \& O'Neill comment.
d. Former Fuss \& O Neill Comment: HR 276-11.1.B.(23). The applicant has not noted any pertinent highway projects on the plan set.

## 3. Driveway Review Codes (HR 275-8.B. (34)/Chapter 193)

a. Former Fuss \& O Neill Comment: HR 193.10.C. The applicant has not provided grading at the driveway connection to Derry Street so we are unable to confirm that the proposed driveway grading conforms to the Regulation and Town standards.
Current Fuss \& O’Neill Comment: The applicant has added grading to the driveway. No further Fuss \& O'Neill comment.
b. Former Fuss \& O Neill Comment: HR 193.10.E. The applicant has not provided any sight distances for the proposed driveway location on the plan set.
Current Fuss \& O’Neill Comment: The applicant has added sight distance information to the plan set. No further Fuss \& O’Neill comment.
c. Former Fuss \& O'Neill Comment: The driveway layout at the entrance and the parking lot doesn't appear to allow for larger trucks to access the site. The applicant should confirm that these are not anticipated, and review the need for signage to prevent such trucks from attempting to access the site. The applicant should also provide information as to the types of delivery trucks expected to access the site.
Current Fuss \& O’Neill Comment: The applicant has stated that Aroma Joe's sixes trucks according to site constraints. They do not anticipate larger trucks will try to access the site. No further Fuss \& O'Neill comment.
d. Former Fuss \& O 'Neill Comment: The applicant has called for vertical granite curb on the plan set and provided a detail for bituminous curb only. The applicant should coordinate the plans and details.
Current Fuss \& O’Neill Comment: The applicant has added the vertical granite curb detail to the plan set. No further Fuss \& O’Neill comment.

## 4. Traffic

a. Former Fuss \& O'Neill Comment: HR 275-9.B. The applicant has not provided any traffic information as part of their review package.

Current Fuss \& O’Neill Comment: The applicant has submitted a Traffic Impact and Access Study which Fuss \& O'Neill will review and provide comments for separately. No further Fuss \& O'Neill comment.

## 5. Utility Design/Conflicts

a. Former Fuss \& O 'Neill Comment: Engineering Technical Guideline \& Typical Details (ETGTD)

Mr. Brian Groth
August 16, 2021
Page 6 of 8
Section 720.8.3.The applicant has not provided a cleanout for the proposed sewver service. This should be located at the property line.
Current Fuss \& O’Neill Comment: The applicant has added a sewer manhole to the site next to the property line per the request of the Town Engineer. No further Fuss \& O’Neill comment.
b. Former Fuss \& O Neill Comment: The applicant should provide a water/sewer crossing detail for the sewer service crossing the water main in Derry Street, and crossing details for the service piping at the drain line in the driveway.
Current Fuss \& O’Neill Comment: The applicant has provided the recommended detail on the plan set. No further Fuss \& O’Neill comment.

## 6. Drainage Design/Stormwater Management (HR 275-9.A./Chapter 290)

f. Former Fuss \& O’Neill Comment: ETGTD 920.4.18. \& 920.4.11. The applicant should state on the plan that the responsibility of maintaining the stormwater features are solely the owner's.
Current Fuss \& O’Neill Comment: The applicant has added a note to the plan set. No further Fuss \& O'Neill comment.
i. Former Fuss \& O'Neill Comment: ETGTD 930.12. The applicant should review the use of curb cuts on this private site. Snow storage and snow melt could reduce the effectiveness of this drainage design, leading to unwanted flooding.

Current Fuss \& O’Neill Comment: The applicant has revised the curb cuts to curb inlets. No further Fuss \& O'Neill comment.
j. Former Fuss \& O'Neill Comment: The applicant will be required to comply with all provisions of the Town of Hudson's MS4 permit, including but not limited to annual reporting requirements, construction site stormwater runoff control, and record keeping requirements.
k. Former Fuss \& O'Neill Comment: Please note that this review was carried out in accordance with applicable regulations and standards in place in New Hampshire at this time. Note that conditions at the site, including average weather conditions, patterns and trends, and design storm characteristics, may change in the future. In addition, future changes in federal, state or local laws, rules or regulations, or in generally accepted scientific or industry information concerning environmental, atmospheric and geotechnical conditions and developments may affect the information and conclusions set forth in this review. In no way shall Fuss \& O'Neill be liable for any of these changed conditions that may impact the review, regardless of the source of or reason for such changed conditions. Other than as described herein, no other investigation or analysis has been requested by the Client or performed by Fuss \& O'Neill in preparing this review.

## 7. Zoning (ZO 334)

a. Former Fuss \& O’Neill Comment: ZO 334-14. A. The applicant has not provided the proposed building beight on the plan set.
Current Fuss \& O'Neill Comment: The applicant has added the proposed building height on the plan set. No further Fuss \& O'Neill comment.
b. Former Fuss \& O’Neill Comment: ZO 334-17 \& 334-21. The applicant has noted that the subject parcel is located within the Business (B) zoning district. The proposed use is permitted by the Ordinance within the Business district.
c. Former Fuss \& O'Neill Comment: ZO 334-33. The applicant has not shown any wetlands on the plan set.

Mr. Brian Groth
August 16, 2021
Page 7 of 8
d. Former Fuss \& O'Neill Comment: ZO 334-60. The applicant has not provided any information for any proposed signs on site, except traffic and parking signage. The applicant has noted that signs are subject to Planning Board approval prior to installation.
e. Former Fuss \& O’Neill Comment: ZO 334-83 and HR 218-4.E. The applicant has noted that the site is not located within a designated flood hazard area.

## 8. Erosion Control/Wetland Impacts

a. Former Fuss \& O'Neill Comment: The Town of Hudson should reserve the right to require any additional erosion control measures as needed. The applicant has noted this on the plans.

## 9. Landscaping (HR 275-8.C.(7) \& 276-11.1.B.(20)) and Lighting (HR 276-11.1.B.(14))

a. Former Fuss \& O'Neill Comment: HR 275-8.C.(7). The applicant has met the parking lot landscaping requirements.
b. Former Fuss \& O'Neill Comment: HR 275-8.C.(8). The applicant has provided screening for the residential use to the west by using the existing tree line.
c. Former Fuss \& O Neill Comment: HR 276-11.1.B.(14). The applicant has shown lighting fixture locations on the plans with details and photometric information.

## 10. State and Local Permits (HR 275-9.G.)

a. Former Fuss \& O'Neill Comment: HR 275-9.G. The applicant has listed required permits and statuses on the plan set.
b. Former Fuss \& O'Neill Comment: HR 275-9.G. The applicant did not provide copies of any applicable Town, State or Federal approvals or permits in the review package.
c. Former Fuss \& O 'Neill Comment: Additional local and state permitting may be required.

## 11. Other

a. Former Fuss \& O'Neill Comment: The applicant should review the circles shown on sheets 8 and 9. They appear to be a drafting error.
Current Fuss \& O’Neill Comment: The applicant has corrected the plan. No further Fuss \& O'Neill comment.
b. Former Fuss \& O Neill Comment: The applicant has proposed retaining walls adjacent to the parking lot. The applicant has provided a typical detail for the walls but individual designs were not provided. We note that a portion of this wall appears to be nearly 10 feet tall but specific wall grades are not provided. The applicant should provide detailed design drawings for the proposed wall, stamped by an Engineer licensed in the State of New Hampshire, for Town review prior to construction.
Current Fuss \& O’Neill Comment: The applicant has stated that a detailed design will be provided prior to construction. No further Fuss \& O’Neill comment.
c. Former Fuss \& O'Neill Comment: ETGTD Section 565.1.1. The applicant is reminded of Town of Hudson requirements for the importing of off-site fill materials for use in constructing this project. It is recommended that these requirements be stated on the plans for the Contractors attention.
Current Fuss \& O’Neill Comment: The applicant has noted this on the plan set. No further Fuss \& O'Neill comment.

Mr. Brian Groth
August 16, 2021
Page 8 of 8

Please feel free to call if you have any questions.
Very truly yours,


Steven W. Reichert, P.E.

SWR:
Enclosure
cc: Town of Hudson Engineering Division - File
Keach- Nordstrom Associates, Inc. - alewis@keachnordstrom.com

# 08/25/21, SP \#08-21, Attachment B 

TRAFFIC IMPACT AND ACCESS STUDY

56 DERRY ROAD Hudson, New Hampshire

July 1, 2021

Prepared for Keach-Nordstrom Associates, Inc.

# TRAFFIC-IMPACT AND ACCESS STUDY 

56 DERRY ROAD<br>Hudson, New Hampshire

July 1, 2021


Prepared for Keach-Nordstrom Associates, Inc.

## CONTENTS

SUMMARY ..... 1
Project Description .....  1
Study Scope ..... 1
Trip Generation ..... 1
Capacity Analysis ..... 2
Traffic Impacts ..... 3
INTRODUCTION ..... 5
Project Description ..... 5
Study Approach ..... 5
EXISTING CONDITIONS ..... 7
Introduction ..... 7
Physical Conditions ..... 7
Traffic Volumes ..... 8
Vehicle Speeds ..... 9
Sight Distances. ..... 11
FUTURE CONDITIONS ..... 13
Introduction ..... 13
Planned Road Improvements ..... 13
Background-Traffic Growth ..... 13
No-Build Traffic Volumes ..... 13
tRIP Generation ..... 16
Trip Distribution and Network Assignment ..... 18
Build Traffic Volumes ..... 18
Traffic-Volume Changes ..... 18
CAPACITY ANALYSIS ..... 24
Introduction ..... 24
Methods ..... 24
Results ..... 25
CONCLUSION ..... 29
Project Description. ..... 29
Trip Generation ..... 29
Capacity Analysis ..... 30
Traffic Impacts ..... 30
APPENDIX
Appendix A: Project Plan
Appendix B: Traffic Counts
Appendix C: Monthly Traffic Volumes
Appendix D: Vehicle Speeds
Appendix E: Capacity-Analysis Worksheets

## TABLES

Table 1. 2021 existing traffic volumes ..... 9
Table 2. Vehicle speeds. ..... 11
Table 3. Sight distances. ..... 12
Table 4. Calculated weekday vehicle-trip generation ..... 17
Table 5. Trip distribution and network assignment. ..... 18
Table 6. Traffic-volume changes. ..... 23
Table 7. Level-of-service criteria for intersections ..... 25
Table 8. Capacity-analysis summary. ..... 27

## FIGURES

Figure 1. Site location. ..... 6
Figure 2. 2021 existing traffic volumes ..... 10
Figure 3. 2022 no-build traffic volumes ..... 14
Figure 4. 2032 no-build traffic volumes ..... 15
Figure 5. 2022 site traffic volumes ..... 19
Figure 6. 2032 site traffic volumes ..... 20
Figure 7. 2022 build traffic volumes. ..... 21
Figure 8. 2032 build traffic volumes. ..... 22

## SUMMARY

## PROJECT DESCRIPTION

Keach-Nordstrom Associates, Inc. (KNA) has retained TEPP LLC to prepare this traffic impact and access study (TIAS) for a proposed commercial redevelopment in the Town of Hudson, New Hampshire.

The proposed redevelopment will:

- be at 56 Derry Road
- provide one drive-through coffee shop
- have one driveway to the west side of Derry Road, with a one-lane entrance and a twolane exit


## STUDY SCOPE

The TIAS study area includes the following unsignalized intersections:

- Derry Road/Ledge Road
- Derry Road/driveway

This TIAS analyzes the following conditions as applicable:

- 2021 existing
- 2022 and 2032 no-build, with background-traffic growth
- 2022 and 2032 build, with background-traffic growth and the proposed redevelopment

This TIAS analyzes traffic operations for the following hours as applicable:

- weekday AM street-peak hour
- weekday PM street-peak hour


## TRIP GENERATION

Total trips appear on the site driveway but not all are added to Derry Road near the site. 2022 total vehicle-trips are:

- weekday daily, 629 (total of in and out)
- weekday AM-street-peak hour, 106 (53 in and 53 out)
- weekday PM-street-peak hour, 40 (20 in and 20 out)

2032 total vehicle-trips are:

- weekday daily, 694 (total of in and out)
- weekday AM-street-peak hour, 117 (58 in and 539 out)
- weekday PM-street-peak hour, 44 (22 in and 22 out)

Primary trips are added to Derry Road near the site. 2022 primary vehicle-trips are:

- weekday daily, 69 (total of in and out)
- weekday AM-street-peak hour, 12 (6 in and 6 out)
- weekday PM-street-peak hour, 4 ( 2 in and 2 out)

2032 primary vehicle-trips are:

- weekday daily, 78 (total of in and out)
- weekday AM-street-peak hour, 13 (6 in and 7 out)
- weekday PM-street-peak hour, 6 ( 3 in and 3 out)


## CAPACITY ANALYSIS

Capacity analysis shows, for the Derry Road/Ledge Road intersection

- low delays for left turns from Derry Road
- moderate delays or delayed operations for movements from Ledge Road
- insignificant project impacts

Capacity analysis shows, for the Derry Road/driveway intersection:

- low delays for left turns from Derry Road
- moderate delays or delayed operations for movements from the driveway

Delayed operations on minor-street approaches to high-volume arterials are typical and acceptable.

TRAFFIC IMPACTS
Analysis indicates no significant area impact due to the proposed redevelopment.

## INTRODUCTION

## PROJECT DESCRIPTION

KNA has retained TEPP LLC to prepare this TIAS for a proposed commercial redevelopment in the Town of Hudson, New Hampshire.

The proposed redevelopment will:

- be at 56 Derry Road
- provide one drive-through coffee shop
- have one driveway to the west side of Derry Road, with a one-lane entrance and a twolane exit

Figure 1 shows site location. The project plan is in Appendix A.

## STUDY APPROACH

This TIAS assesses traffic impacts and access for the proposed redevelopment.
The TIAS study area includes the following unsignalized intersections:

- Derry Road/Ledge Road
- Derry Road/driveway

This TIAS analyzes the following conditions as applicable:

- 2021 existing
- 2022 and 2032 no-build, with background-traffic growth
- 2022 and 2032 build, with background-traffic growth and the proposed redevelopment

This TIAS analyzes traffic operations for the following hours as applicable:

- weekday AM street-peak hour
- weekday PM street-peak hour

Differences in traffic operations between the no-build and build conditions approximate traffic impacts of the proposed redevelopment.

## TEPP



Figure 1. Site location.

## EXISTING CONDITIONS

## INTRODUCTION

Existing conditions include:

- physical conditions of the transportation network, roads, and intersections
- traffic volumes
- other relevant information


## PHYSICAL CONDITIONS

## INTRODUCTION

Figure 1 shows the transportation network.
The TIAS study area includes the following existing unsignalized intersection: Derry Road/Ledge Road.

Description of the TIAS study area follows.

## DERRY ROAD

Derry Road:

- is oriented approximately north-south
- functions as an arterial street
- is also known as New Hampshire Routes (NH) 3A and 102
- to the south, connects with the Town Center and New Hampshire Route 111 (NH 111), an arterial highway that leads to the City of Nashua and Towns of Windham and Salem
- to the north, connects with NH 102, an arterial highway that leads to the Towns of Londonderry and Derry, and NH 3A, an arterial highway that leads to the Town of Litchfield and the City of Manchester
- has a horizontal alignment includes minor to moderate horizontal curvature, but is essentially tangent at the proposed driveway location
- has a near-level vertical alignment
- has a three-lane cross-section with one travel lane per direction, a center-two-way-leftturn lane (TWLTL), and paved shoulders
- has asphaltic-cement concrete (ACC) pavement in overall good condition
- has curb and sidewalk along both sides
- includes utility poles along the west side, some with luminaires
- has a posted speed limit of 30 miles per hour (mph)
- has nearby commercial and residential development
- is under the jurisdiction of the Town


## DERRY ROAD/LEDGE ROAD INTERSECTION

The intersection:

- is three legged
- has Derry Road as the major north-south street
- has Ledge Road as the minor east leg
- on Derry Road, has one travel lane per direction and one center TWLTL
- on the Ledge Road approach, has one lane
- has a STOP sign on the Ledge Road approach
- is illuminated
- has commercial and residential development nearby


## TRAFFIC VOLUMES

## TRAFFIC COUNTS

TEPP LLC obtained an automatic traffic counter (ATR) count:

- on Derry Road along the site frontage
- from Wednesday, June 2, to Thursday, June 3, 2021

The ATR data are in Appendix B.

## ADJUSTMENTS

The June 2021 traffic counts were adjusted to reflect peak-month and non-pandemic conditions.

The increase to peak month was 2.0 percent, based on based on NHDOT 2019 monthly volumes for Group 4 (Urban Highways) averages in Appendix C,

The increase to pre-pandemic was 5.6 percent. NHDOT continuous count station 82229031 , on Daniel Webster Highway north of Hilton Drive, in the Town of Merrimack showed May 2021 two-way average-daily traffic (ADT) of 15,404 vehicles. The station showed May 2019 prepandemic two-way ADT of 16,260 vehicles, which is 5.6 percent greater.

The combined increase was 7.7 percent.

## RESULTS

Table 1 and Figure 2 show 2021 existing traffic volumes.

## Table 1. 2021 existing traffic volumes.

| Location and Time Period | Vehicles $^{\mathrm{a}}$ | K-factor $^{\mathrm{b}}$ | Percent Direction |
| :--- | :---: | :---: | :---: |
| Derry Road near Site Frontage |  |  |  |
| Weekday Daily | 28,667 | --- | --- |
| Weekday AM-Street-Peak Hour | 2,157 | 7.5 | 58 Southbound |
| Weekday PM-Street-Peak Hour | 2,290 | 8.0 | 54 Northbound |

${ }^{\text {a }}$ Two-way-total volumes.
${ }^{\mathrm{b}} \mathrm{K}=$ hour volume as a percent of daily volume.

Derry Road near the site frontage showed about:

- 28,667 weekday-daily vehicles
- 2,157 vehicles during the weekday AM street-peak hour, predominantly southbound
- 2,290 vehicles during the weekday PM street-peak hour, predominantly northbound


## VEHICLE SPEEDS

The ATR collected vehicle speeds:

- on Derry Road along the site frontage
- from Wednesday, June 2, to Thursday, June 3, 2021


Weekday AM-Street-Peak Hour


Weekday PM-Street-Peak Hour
Not to Scale

Figure 2. 2021 existing traffic volumes.

The data are in Appendix D and are summarized in Table 2.
Table 2 indicates that on Derry Road:

| Table 2. Vehicle speeds. |  |  |  |
| :--- | :--- | :---: | :---: |
|  | Speeds (mph) |  |  |
| Location and Direction | Speed Limit | Mean $^{\mathrm{a}}$ | $85^{\text {th }}$ Percentile |
| Derry Road along Site Frontage |  |  |  |
| Northbound | 30 |  |  |
| Southbound | 30 | 35.3 | 39.0 |

${ }^{\text {a }}$ From ATR conducted from Wednesday, June 2, to Thursday, June 3, 2021.

- the posted speed limit was 30 mph
- the northbound the mean speed was 35.3 mph and the $85^{\text {th }}$ percentile speed was 39.0 mph
- for southbound the mean speed was 33.6 mph and the $85^{\text {th }}$ percentile speed was 37.2 mph


## SIGHT DISTANCES

The American Association of State Highway and Transportation Officials (AASHTO) has established authoritative policy for sight distances at unsignalized intersections ${ }^{1}$ in terms of:

- stopping sight distance (SSD)
- optional intersection sight distance (ISD)

SSD: ${ }^{2}$

- provides for safety
- enables a driver, on the major road, to perceive and react accordingly to a vehicle entering the major road from a minor road
- is conservative because it encompasses a wide range of brake-reaction times and deceleration rates

[^0]
## Optional ISD: ${ }^{3}$

- is ordinarily greater than SSD and may enhance traffic operations
- is not required for safety

Table 3 shows relevant available sight distances that are at least 400 ft , per NHDOT practice, and are adequate.

| Table 3. Sight distances. |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
| Intersection, Movements, and View | Available Sight <br> Distance (ft) ${ }^{\text {a }}$ | Speeds (miles per hour) |  |  |
|  |  | Limit | SSD Provides For | ISD Provides For |
| Portland Street/Proposed Road for Proposed Road Movements |  |  |  |  |
| Portland Street to/from South | 400 | 30 | 45+ | 36+ |
| Portland Street to/from North | 400 | 30 | 45+ | 36+ |

${ }^{\text {a }}$ With appropriate roadside and vegetation maintenance.

[^1]
## INTRODUCTION

Future conditions include:

- planned road improvements independent of the proposed redevelopment
- future no-build traffic volumes, with background-traffic growth and without the proposed redevelopment
- future build traffic volumes, with background-traffic growth and with the proposed redevelopment


## PLANNED ROAD IMPROVEMENTS

TEPP LLC identified no significant planned road improvement in the study area independent of the project.

## BACKGROUND-TRAFFIC GROWTH

Background-traffic growth:

- is independent of the proposed redevelopment
- is related to land development in the immediate area, population and economic development in the region, and changes in travel patterns in the region
- typically considers two factors: a general traffic-growth rate and specific planned land developments in the immediate area

This TIAS uses a 1.0-percent annual growth rate. This yields about 11.6-percent growth between 2021 and 2032.

## NO-BUILD TRAFFIC VOLUMES

The background-traffic growth described above was applied to 2021 existing traffic volumes. Figures 3 and 4 show 2022 and 2032 no-build traffic volumes.


Weekday AM-Street-Peak Hour

$\mathrm{N} \rightarrow$

Not to Scale

Weekday PM-Street-Peak Hour

Figure 3. 2022 no-build traffic volumes.


Weekday AM-Street-Peak Hour


## Weekday PM-Street-Peak Hour

Not to Scale

Figure 4. 2032 no-build traffic volumes.

## TRIP GENERATION

## BASIC TRIP GENERATION

The Institute of Transportation Engineers (ITE) compiles and publishes trip-generation information for a variety of land uses in Trip Generation Manual. ${ }^{4}$ This guide for estimating site traffic includes coffee/donut shop with drive-through window and no indoor seating, land use 938, based on floor area. ${ }^{5}$ However, this information is based on sites with floor areas of 90 square feet (sf) and is not applicable to the proposed land use, with a floor area of about 900 sf .

Stephen G. Pernaw \& Company, Inc. has published appropriate and applicable trip-generation information specific to this land use, which estimates trip generation based on traffic volumes passing the site. ${ }^{6}$ Basic trip generation is based on this information.

## TRIP TYPES

Total trips appear on site driveways but not all are added to roads near the site. Accordingly, ITE compiles information on three trip types, based on empirical data for many land uses, in the authoritative Hooper, Trip Generation Handbook. ${ }^{7}$ These three trip types are:

- primary trips that are added to the area and are primarily for visiting the site
- diverted trips that not added to the general area; these trips are from existing traffic on roads near the site
- pass-by trips that are not added to the general area; these trips are from existing traffic passing the site ${ }^{8}$


## RESULTS

Table 4 shows calculated weekday vehicle-trip generation for the site.

[^2]|  |  | AM-Street-Peak Hour |  |  | PM-Street-Peak Hour |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Daily ${ }^{\text {a }}$ | Total ${ }^{\text {b }}$ | In | Out | Total ${ }^{\text {c }}$ | In | Out |
| 2022 Vehicle-Trips |  |  |  |  |  |  |  |
| Primary | 69 | 12 | 6 | 6 | 4 | 2 | 2 |
| $\underline{\text { Pass-By }{ }^{\text {d }}}$ | $\underline{560}$ | $\underline{94}$ | $\underline{47}$ | 47 | $\underline{36}$ | $\underline{18}$ | $\underline{18}$ |
| Total | 629 | 106 | 53 | 53 | 40 | 20 | 20 |
| 2032 Vehicle-Trips |  |  |  |  |  |  |  |
| Primary | 78 | 13 | 6 | 7 | 6 | 3 | 3 |
| $\underline{\text { Pass-By }}$ d | $\underline{616}$ | $\underline{104}$ | $\underline{52}$ | 52 | 38 | $\underline{19}$ | $\underline{19}$ |
| Total | 694 | 117 | 58 | 59 | 44 | 22 | 22 |

[^3]Total trips appear on the site driveway but not all are added to Derry Road near the site. 2022 total vehicle-trips are:

- weekday daily, 629 (total of in and out)
- weekday AM-street-peak hour, 106 (53 in and 53 out)
- weekday PM-street-peak hour, 40 (20 in and 20 out)

2032 total vehicle-trips are:

- weekday daily, 694 (total of in and out)
- weekday AM-street-peak hour, 117 (58 in and 539 out)
- weekday PM-street-peak hour, 44 (22 in and 22 out)

Primary trips are added to Derry Road near the site. 2022 primary vehicle-trips are:

- weekday daily, 69 (total of in and out)
- weekday AM-street-peak hour, 12 (6 in and 6 out)
- weekday PM-street-peak hour, 4 (2 in and 2 out)

2032 primary vehicle-trips are:

- weekday daily, 78 (total of in and out)
- weekday AM-street-peak hour, 13 ( 6 in and 7 out)
- weekday PM-street-peak hour, 6 ( 3 in and 3 out)


## TRIP DISTRIBUTION AND NETWORK ASSIGNMENT

Trip distribution and network assignment of vehicle-trips to and from the site may consider such factors as existing site distribution, travel patterns, population, regional land development, and site access. Trip distribution and network assignment for this TIAS considered the 2021 existing volumes.

Table 5 shows trip distribution and network assignment for primary trips. Pass-by trips were assigned reflecting peak-hour directional distributions on Derry Road: 58-percent southbound for the weekday AM-street-peak hour and 54-percent northbound for the weekday PM-street-peak hour. Figures 5 and 6 show site traffic volumes.

| Table 5. | Trip distribution and network assignment. |
| :--- | :---: |
| Road and Direction (To/From) | Approximate Percent |
| Derry Road to/from South | 45 |
| Derry Road to/from South | $\underline{55}$ |
| Total | 100 |

## BUILD TRAFFIC VOLUMES

Site traffic volumes were superimposed on the no-build traffic volumes to estimate build traffic volumes. Figures 7 and 8 show the resulting 2022 and 2032 build traffic volumes.

TRAFFIC-VOLUME CHANGES
Table 6 presents calculated traffic-volume changes due to the proposed redevelopment for the:

- weekday AM-street-peak hour
- weekday PM-street-peak hour


Weekday AM-Street-Peak Hour


Weekday PM-Street-Peak Hour

Figure 5. 2022 site traffic volumes.


## Weekday AM-Street-Peak Hour



## Weekday PM-Street-Peak Hour

[^4]

## Weekday AM-Street-Peak Hour



Figure 7. 2022 build traffic volumes.


## Weekday AM-Street-Peak Hour



## Weekday PM-Street-Peak Hour

Not to Scale

Figure 8. 2032 build traffic volumes.

|  | 2022 Traffic Volumes (vph) ${ }^{\text {a }}$ |  |  | 2032 Traffic Volumes (vph) |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Location and Time Period | No-Build | Build | Change | No-Build | Build | Change |
| Derry Road North of Driveway |  |  |  |  |  |  |
| Weekday AM-Street-Peak Hour | 2,178 | 2,184 | 6 | 2,407 | 2.413 | 6 |
| Weekday PM-Street-Peak Hour | 2,312 | 2,314 | 2 | 2,555 | 2,557 | 2 |
| Derry Road South of Driveway |  |  |  |  |  |  |
| Weekday AM-Street-Peak Hour | 2,178 | 2,184 | 6 | 2,407 | 2,414 | 7 |
| Weekday PM-Street-Peak Hour | 2,312 | 2,314 | 2 | 2,555 | 2,559 | 4 |

${ }^{\text {a }}$ Two-way total volumes.

Table 6 shows peak-hour-traffic-volume increases:

- of 2 to 7 vehicle-trips
- constituting averages about one vehicle-trip per 8 to 30 minutes
- that are further split by northbound and southbound direction on Derry Road


## INTRODUCTION

This TIAS has quantified existing, future-no-build and future-build traffic volumes. Capacity analysis models the quality of traffic operations. Comparing build conditions to the no-build conditions indicates impacts of the proposed redevelopment on quality of traffic operations.

## METHODS

Capacity analysis calculates LOS for transportation facilities. LOS indicates the quality of traffic operations based on delay and other measures. The six LOS are designated A to F. LOS A represents the best or highest operating conditions. LOS F is the lowest, but does not necessarily connote failure.

LOS is a function of traffic volumes and traffic control. Because these volumes can vary, LOS of a transportation facility can differ by time of day, day of the week, or month. For example, a transportation facility with a low LOS during peak hours may have a high LOS during other hours. The operational analysis methods of the Transportation Research Board (TRB) ${ }^{9}$ models LOS for intersections based on calculated delay per vehicle, as shown in Table 7. Synchro analysis software was used.

Method inputs include:

- intersection geometry
- traffic control, such as YIELD sign, two-way STOP sign, all-way STOP sign, roundabout, or signal (including phasing, timing, and progression)
- traffic volumes
- vehicle composition, such as passenger cars and trucks

The methods are all approximate. In particular, the method for two-way STOP-sign control can be conservative, with observed delays and queuing shorter than those modeled.

[^5]Table 7. Level-of-service criteria for intersections.
Control Delay (seconds/vehicle)

| Level of Service | Unsignalized Intersections $^{\mathrm{a}}$ | Signalized Intersections |
| :---: | :---: | :---: |
| A | $\leq 10.0$ | $\leq 10.0$ |
| B | $>10.0$ and $\leq 15.0$ | $>10.0$ and $\leq 20.0$ |
| C | $>15.0$ and $\leq 25.0$ | $>20.0$ and $\leq 35.0$ |
| D | $>25.0$ and $\leq 35.0$ | $>35.0$ and $\leq 55.0$ |
| E | $>35.0$ and $\leq 50.0$ | $>55.0$ and $\leq 80.0$ |
| F | $>50$ | $>80$ |

From Transportation Research Board, Highway Capacity Manual 2010 (Washington D.C., 2010).
${ }^{\text {a }}$ For YIELD sign, two-way STOP sign or all-way STOP sign, control delay defines LOS. For roundabout approaches and overall intersection, control delay defines LOS. For roundabout lanes with volume/capacity ratio $\leq 1.0$, control delay defines LOS. For roundabout lanes with volume/capacity ratio $>1.0$, LOS is F regardless of control delay.

## RESULTS

Table 8 shows computed LOS, delays, and queues at study-area intersections for the:

- weekday AM-street-peak hour
- weekday PM-street-peak hour

The analysis is under the following conditions, as applicable:

- 2021 existing
- 2022 and 2032 no build
- 2022 and 2032 build

Capacity-analysis worksheets that give detail and explanation are in Appendix E.
Table 8 shows, for the Derry Road/Ledge Road intersection

- low delays for left turns from Derry Road
- moderate delays or delayed operations for movements from Ledge Road
- insignificant project impacts

Table 8 shows, for the Derry Road/driveway intersection:

TEPP

- low delays for left turns from Derry Road
- moderate delays or delayed operations for movements from the driveway

Delayed operations on minor-street approaches to high-volume arterials are typical and acceptable.

| Intersection, Control, Hour and Movement | 2021 Existing |  |  |  | 2022 No Build |  |  |  | 2032 No Build |  |  |  | 2022 Build |  |  |  | 2032 No Build |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | LOS $^{\text {a }}$ | Delay ${ }^{\text {b }}$ | V/C ${ }^{\text {c }}$ | Queue ${ }^{\text {d }}$ | Los | Delay | V/C | Queue | LOS | Delay | V/C | Queue | LOS | Delay | V/C | Queue | LOS | Delay | V/C | Queue |
| Derry Road/Ledge Road Intersection, Unsignalized, Weekday AM-Street-Peak Hour |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Derry Road SB L | B | 10.7 | 0.019 | 0.1 | B | 10.9 | 0.020 | 0.1 | B | 11.6 | 0.024 | 0.1 | B | 11.0 | 0.020 | 0.1 | B | 11.6 | 0.024 | 0.1 |
| Ledge Road WB LR | D | 33.3 | 0.370 | 1.6 | D | 31.8 | 0.307 | 1.2 | E | 41.2 | 0.402 | 1.8 | E | 35.2 | 0.333 | 1.4 | E | 41.6 | 0.405 | 1.8 |
| Derry Road/Ledge Road Intersection, Unsignalized, Weekday PM-Street-Peak Hour |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Derry Road SB L | B | 11.7 | 0.037 | 0.1 | B | 12.6 | 0.045 | 0.1 | B | 13.7 | 0.056 | 0.2 | B | 12.6 | 0.045 | 0.1 | B | 13.7 | 0.056 | 0.2 |
| Ledge Road WB LR | D | 29.6 | 0.250 | 1.0 | D | 33.4 | 0.225 | 0.8 | E | 42.9 | 0.305 | 1.2 | D | 33.6 | 0.226 | 0.8 | E | 43.3 | 0.307 | 1.2 |
| Derry Road/Driveway Intersection, Unsignalized, Weekday AM-Street-Peak Hour |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Derry Road NB L | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | B | 12.9 | 0.053 | 0.2 | B | 14.1 | 0.2 | 0.066 |
| Driveway EB L | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | E | 37.5 | 0.188 | 0.7 | E | 46.9 | 0.246 | 0.9 |
| Driveway EB R | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | D | 30.5 | 0.192 | 0.7 | E | 39.3 | 0.266 | 1.0 |
| Derry Road/Driveway Intersection, Unsignalized, Weekday PM-Street-Peak Hour |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Derry Road NB L | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | B | 11.2 | 0.021 | 0.1 | B | 11.9 | 0.025 | 0.1 |
| Driveway EB L | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | E | 36.1 | 0.095 | 0.3 | E | 42.5 | 0.113 | 0.4 |
| Driveway EB R | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | --- | C | 21.0 | 0.043 | 0.1 | C | 24.3 | 0.061 | 0.2 |

${ }^{\text {a }}$ LOS $=$ level of service.
b Delay = average delay in seconds per vehicle
c $\mathrm{V} / \mathrm{C}=$ volume/capacity ratio.
${ }^{\mathrm{d}} 95^{\text {th }}$ percentile queue in vehicles.
$\mathrm{EB}=$ eastbound, $\mathrm{WB}=$ westbound, $\mathrm{SB}=$ southbound, $\mathrm{NB}=$ northbound, $\mathrm{L}=$ left, $\mathrm{T}=$ through, $\mathrm{R}=$ right.

## CONCLUSION

## PROJECT DESCRIPTION

The proposed redevelopment will:

- be at 56 Derry Road
- provide one drive-through coffee shop
- have one driveway to the west side of Derry Road, with a one-lane entrance and a twolane exit


## TRIP GENERATION

Total trips appear on the site driveway but not all are added to Derry Road near the site. 2022 total vehicle-trips are:

- weekday daily, 629 (total of in and out)
- weekday AM-street-peak hour, 106 (53 in and 53 out)
- weekday PM-street-peak hour, 40 (20 in and 20 out)

2032 total vehicle-trips are:

- weekday daily, 694 (total of in and out)
- weekday AM-street-peak hour, 117 (58 in and 539 out)
- weekday PM-street-peak hour, 44 (22 in and 22 out)

Primary trips are added to Derry Road near the site. 2022 primary vehicle-trips are:

- weekday daily, 69 (total of in and out)
- weekday AM-street-peak hour, 12 (6 in and 6 out)
- weekday PM-street-peak hour, 4 (2 in and 2 out)

2032 primary vehicle-trips are:

- weekday daily, 78 (total of in and out)
- weekday AM-street-peak hour, 13 (6 in and 7 out)
- weekday PM-street-peak hour, 6 ( 3 in and 3 out)


## CAPACITY ANALYSIS

Capacity analysis shows, for the Derry Road/Ledge Road intersection

- low delays for left turns from Derry Road
- moderate delays or delayed operations for movements from Ledge Road
- insignificant project impacts

Capacity analysis shows, for the Derry Road/driveway intersection:

- low delays for left turns from Derry Road
- moderate delays or delayed operations for movements from the driveway

Delayed operations on minor-street approaches to high-volume arterials are typical and acceptable.

## TRAFFIC IMPACTS

Analysis indicates no significant area impact due to the proposed redevelopment.

APPENDIX

Appendix A: Project Plan


Appendix B: Traffic Counts


## Accurate Counts

## 978-664-2565

N/S Street : Derry Road
E/W Street : Ledge Road
City/State : Hudson, NH
Weather : Clear

File Name : 15530001
Site Code: 15530001
Start Date : 6/2/2021
Page No : 1

Groups Printed- Cars - Trucks

|  | Derry Rd From North |  | Ledge Rd From East |  | Derry Rd From South |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Left | Thru | Left | Right | Thru | Right | Int. Total |
| 07:00 AM | 3 | 246 | 7 | 10 | 211 | 3 | 480 |
| 07:15 AM | 3 | 297 | 7 | 5 | 225 | 0 | 537 |
| 07:30 AM | 1 | 319 | 5 | 4 | 191 | 3 | 523 |
| 07:45 AM | 3 | 278 | 3 | 8 | 204 | 3 | 499 |
| Total | 10 | 1140 | 22 | 27 | 831 | 9 | 2039 |
| 08:00 AM | 2 | 251 | 3 | 3 | 162 | 4 | 425 |
| 08:15 AM | 2 | 250 | 2 | 4 | 153 | 3 | 414 |
| 08:30 AM | 2 | 288 | 7 | 2 | 175 | 2 | 476 |
| 08:45 AM | 2 | 240 | 3 | 5 | 212 | 6 | 468 |
| Total | 8 | 1029 | 15 | 14 | 702 | 15 | 1783 |
| Grand Total | 18 | 2169 | 37 | 41 | 1533 | 24 | 3822 |
| Apprch \% | 0.8 | 99.2 | 47.4 | 52.6 | 98.5 | 1.5 |  |
| Total \% | 0.5 | 56.8 | 1 | 1.1 | 40.1 | 0.6 |  |
| Cars | 16 | 2092 | 34 | 40 | 1492 | 23 | 3697 |
| \% Cars | 88.9 | 96.4 | 91.9 | 97.6 | 97.3 | 95.8 | 96.7 |
| Trucks | 2 | 77 | 3 | 1 | 41 | 1 | 125 |
| \% Trucks | 11.1 | 3.6 | 8.1 | 2.4 | 2.7 | 4.2 | 3.3 |


|  | Derry Rd From North |  |  | Ledge Rd From East |  |  | Derry Rd From South |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Left | Thru | App. Total | Left | Right | App. Total | Thru | Right | App. Total | Int. Total |
| Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1 |  |  |  |  |  |  |  |  |  |  |
| Peak Hour for Entire Intersection Begins at 07:00 AM |  |  |  |  |  |  |  |  |  |  |
| 07:00 AM | 3 | 246 | 249 | 7 | 10 | 17 | 211 | 3 | 214 | 480 |
| 07:15 AM | 3 | 297 | 300 | 7 | 5 | 12 | 225 | 0 | 225 | 537 |
| 07:30 AM | 1 | 319 | 320 | 5 | 4 | 9 | 191 | 3 | 194 | 523 |
| 07:45 AM | 3 | 278 | 281 | 3 | 8 | 11 | 204 | 3 | 207 | 499 |
| Total Volume | 10 | 1140 | 1150 | 22 | 27 | 49 | 831 | 9 | 840 | 2039 |
| \% App. Total | 0.9 | 99.1 |  | 44.9 | 55.1 |  | 98.9 | 1.1 |  |  |
| PHF | . 833 | . 893 | . 898 | . 786 | . 675 | . 721 | 923 | . 750 | . 933 | 949 |
| Cars | 8 | 1113 | 1121 | 20 | 27 | 47 | 808 | 9 | 817 | 1985 |
| \% Cars | 80.0 | 97.6 | 97.5 | 90.9 | 100 | 95.9 | 97.2 | 100 | 97.3 | 97.4 |
| Trucks | 2 | 27 | 29 | 2 | 0 | 2 | 23 | 0 | 23 | 54 |
| \% Trucks | 20.0 | 2.4 | 2.5 | 9.1 | 0 | 4.1 | 2.8 | 0 | 2.7 | 2.6 |

## Accurate Counts

978-664-2565

N/S Street : Derry Road E/W Street : Ledge Road City/State : Hudson, NH Weather : Clear

File Name : 15530001
Site Code : 15530001 Start Date : 6/2/2021
Page No : 2


Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1
Peak Hour for Each Approach Begins at:

|  | 07:15 AM |  |  | 07:00 AM |  |  | 07:00 AM |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| +0 mins. | 3 | 297 | 300 | 7 | 10 | 17 | 211 | 3 | 214 |
| +15 mins. | 1 | 319 | 320 | 7 | 5 | 12 | 225 | 0 | 225 |
| +30 mins. | 3 | 278 | 281 | 5 | 4 | 9 | 191 | 3 | 194 |
| +45 mins. | 2 | 251 | 253 | 3 | 8 | 11 | 204 | 3 | 207 |
| Total Volume | 9 | 1145 | 1154 | 22 | 27 | 49 | 831 | 9 | 840 |
| \% App. Total | 0.8 | 99.2 |  | 44.9 | 55.1 |  | 98.9 | 1.1 |  |
| PHF | 750 | . 897 | . 902 | . 786 | . 675 | 721 | . 923 | . 750 | . 933 |
| Cars | 8 | 1120 | 1128 | 20 | 27 | 47 | 808 | 9 | 817 |
| \% Cars | 88.9 | 97.8 | 97.7 | 90.9 | 100 | 95.9 | 97.2 | 100 | 97.3 |
| Trucks | 1 | 25 | 26 | 2 | 0 | 2 | 23 | 0 | 23 |
| \% Trucks | 11.1 | 2.2 | 2.3 | 9.1 | 0 | 4.1 | 2.8 | 0 | 2.7 |

## Accurate Counts

978-664-2565

N/S Street : Derry Road
E/W Street : Ledge Road
City/State : Hudson, NH
Weather : Clear

File Name : 15530001
Site Code : 15530001
Start Date : 6/2/2021
Page No : 10

|  | Derry Rd From North |  |  | Ledge Rd <br> From East |  |  | Derry Rd From South |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Left | Thru | Peds | Left | Right | Peds | Thru | Right | Peds | Exclu. Total | Inclu. Total | Int. Total |
| 07:00 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 07:15 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 07:30 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 07:45 AM | 0 | 0 | 0 | 0 | 0 | 0 | 2 | 0 | 0 | 0 | 2 | 2 |
| Total | 0 | 0 | 0 | 0 | 0 | 0 | 2 | 0 | 0 | 0 | 2 | 2 |
| 08:00 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 08:15 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 08:30 AM | 0 | 1 | 0 | 0 | 2 | 1 | 0 | 0 | 0 | 1 | 3 | 4 |
| 08:45 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Total | 0 | 1 | 0 | 0 | 2 | 1 | 0 | 0 | 0 | 1 | 3 | 4 |
| Grand Total | 0 | 1 | 0 | 0 | 2 | 1 | 2 | 0 | 0 | 1 | 5 | 6 |
| Apprch \% | 0 | 100 |  | 0 | 100 |  | 100 | 0 |  |  |  |  |
| Total \% | 0 | 20 |  | 0 | 40 |  | 40 | 0 |  | 16.7 | 83.3 |  |


|  | Derry Rd From North |  |  | Ledge Rd From East |  |  | Derry Rd From South |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Left | Thru | App. Total | Left | Right | App. Total | Thru | Right | App. Total | Int. Total |
| Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1 |  |  |  |  |  |  |  |  |  |  |
| Peak Hour for Entire Inter | Beg | 7:45 |  |  |  |  |  |  |  |  |
| 07:45 AM | 0 | 0 | 0 | 0 | 0 | 0 | 2 | 0 | 2 | 2 |
| 08:00 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 08:15 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 08:30 AM | 0 | 1 | 1 | 0 | 2 | 2 | 0 | 0 | 0 | 3 |
| Total Volume | 0 | 1 | 1 | 0 | 2 | 2 | 2 | 0 | 2 | 5 |
| \% App. Total | 0 | 100 |  | 0 | 100 |  | 100 | 0 |  |  |
| PHF | . 000 | . 250 | . 250 | . 000 | . 250 | . 250 | . 250 | . 000 | . 250 | . 417 |

## Accurate Counts

978-664-2565

N/S Street : Derry Road E/W Street : Ledge Road City/State : Hudson, NH Weather : Clear

File Name : 15530001
Site Code : 15530001
Start Date : 6/2/2021
Page No : 11

|  | Peak Hour Data <br> Peak Hour Begins at 07:45 AM <br> Bikes Peds |  |
| :---: | :---: | :---: |

Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1
Peak Hour for Each Approach Begins at:

|  | 07:45 AM |  |  | 07:45 AM |  |  | 07:00 AM |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| +0 mins. | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| +15 mins. | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| +30 mins. | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| +45 mins. | 0 | 1 | 1 | 0 | 2 | 2 | 2 | 0 | 2 |
| Total Volume | 0 | 1 | 1 | 0 | 2 | 2 | 2 | 0 | 2 |
| \% App. Total | 0 | 100 |  | 0 | 100 |  | 100 | 0 |  |
| PHF | . 000 | . 250 | 250 | . 000 | . 250 | . 250 | . 250 | . 000 | 250 |

## Accurate Counts

## 978-664-2565

N/S Street : Derry Road
E/W Street : Ledge Road
City/State : Hudson, NH
Weather : Clear

File Name : 15530001
Site Code : 15530001
Start Date : 6/2/2021
Page No : 1

Groups Printed- Cars - Trucks

| Groups Printed- Cars - Trucks |  |  |  |  |  |  | Int. Total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Derry Rd From North |  | Ledge Rd From East |  | Derry Rd From South |  |  |
| Start Time | Left | Thru | Left | Right | Thru | Right |  |
| 04:00 PM | 2 | 251 | 2 | 6 | 284 | 8 | 553 |
| 04:15 PM | 2 | 239 | 1 | 6 | 277 | 6 | 531 |
| 04:30 PM | 6 | 227 | 1 | 3 | 287 | 5 | 529 |
| 04:45 PM | 9 | 248 | 3 | 8 | 282 | 5 | 555 |
| Total | 19 | 965 | 7 | 23 | 1130 | 24 | 2168 |
| 05:00 PM | 3 | 237 | 1 | 6 | 258 | 6 | 511 |
| 05:15 PM | 6 | 269 | 4 | 5 | 282 | 6 | 572 |
| 05:30 PM | 8 | 220 | 2 | 8 | 261 | 6 | 505 |
| 05:45 PM | 3 | 244 | 1 | 5 | 277 | 4 | 534 |
| Total | 20 | 970 | 8 | 24 | 1078 | 22 | 2122 |
| Grand Total | 39 | 1935 | 15 | 47 | 2208 | 46 | 4290 |
| Apprch \% | 2 | 98 | 24.2 | 75.8 | 98 | 2 |  |
| Total \% | 0.9 | 45.1 | 0.3 | 1.1 | 51.5 | 1.1 |  |
| Cars | 39 | 1913 | 15 | 47 | 2188 | 45 | 4247 |
| \% Cars | 100 | 98.9 | 100 | 100 | 99.1 | 97.8 | 99 |
| Trucks | 0 | 22 | 0 | 0 | 20 | 1 | 43 |
| \% Trucks | 0 | 1.1 | 0 | 0 | 0.9 | 2.2 | 1 |


|  | Derry Rd From North |  |  | Ledge Rd From East |  |  | Derry Rd From South |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Left | Thru | App. Total | Left | Right | App. Total | Thru | Right | App. Total | Int. Total |
| Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1 |  |  |  |  |  |  |  |  |  |  |
| Peak Hour for Entire Inter | Beg | 04:00 P |  |  |  |  |  |  |  |  |
| 04:00 PM | 2 | 251 | 253 | 2 | 6 | 8 | 284 | 8 | 292 | 553 |
| 04:15 PM | 2 | 239 | 241 | 1 | 6 | 7 | 277 | 6 | 283 | 531 |
| 04:30 PM | 6 | 227 | 233 | 1 | 3 | 4 | 287 | 5 | 292 | 529 |
| 04:45 PM | 9 | 248 | 257 | 3 | 8 | 11 | 282 | 5 | 287 | 555 |
| Total Volume | 19 | 965 | 984 | 7 | 23 | 30 | 1130 | 24 | 1154 | 2168 |
| \% App. Total | 1.9 | 98.1 |  | 23.3 | 76.7 |  | 97.9 | 2.1 |  |  |
| PHF | . 528 | . 961 | . 957 | 583 | . 719 | . 682 | . 984 | 750 | 988 | . 977 |
| Cars | 19 | 952 | 971 | 7 | 23 | 30 | 1113 | 23 | 1136 | 2137 |
| \% Cars | 100 | 98.7 | 98.7 | 100 | 100 | 100 | 98.5 | 95.8 | 98.4 | 98.6 |
| Trucks | 0 | 13 | 13 | 0 | 0 | 0 | 17 | 1 | 18 | 31 |
| \% Trucks | 0 | 1.3 | 1.3 | 0 | 0 | 0 | 1.5 | 4.2 | 1.6 | 1.4 |

N/S Street : Derry Road E/W Street : Ledge Road City/State : Hudson, NH Weather : Clear

File Name : 15530001
Site Code : 15530001
Start Date : 6/2/2021
Page No : 2


Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1
Peak Hour for Each Approach Begins at:

|  | 04:30 PM |  |  | 04:45 PM |  |  | 04:00 PM |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| +0 mins. | 6 | 227 | 233 | 3 | 8 | 11 | 284 | 8 | 292 |
| +15 mins. | 9 | 248 | 257 | 1 | 6 | 7 | 277 | 6 | 283 |
| +30 mins. | 3 | 237 | 240 | 4 | 5 | 9 | 287 | 5 | 292 |
| +45 mins. | 6 | 269 | 275 | 2 | 8 | 10 | 282 | 5 | 287 |
| Total Volume | 24 | 981 | 1005 | 10 | 27 | 37 | 1130 | 24 | 1154 |
| \% App. Total | 2.4 | 97.6 |  | 27 | 73 |  | 97.9 | 2.1 |  |
| PHF | . 667 | . 912 | . 914 | . 625 | . 844 | . 841 | 984 | . 750 | . 988 |
| Cars | 24 | 975 | 999 | 10 | 27 | 37 | 1113 | 23 | 1136 |
| \% Cars | 100 | 99.4 | 99.4 | 100 | 100 | 100 | 98.5 | 95.8 | 98.4 |
| Trucks | 0 | 6 | 6 | 0 | 0 | 0 | 17 | 1 | 18 |
| \% Trucks |  | 0.6 | 0.6 | 0 | 0 | 0 | 1.5 | 4.2 | 1.6 |

## Accurate Counts

978-664-2565


E/W Street : Ledge Road
City/State : Hudson, NH
Weather : Clear

File Name : 15530001
Site Code : 15530001
Start Date : 6/2/2021
Page No : 10

| Groups Printed- Bikes Peds |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Derry Rd From North |  |  | Ledge Rd From East |  |  | Derry Rd From South |  |  |  |  |  |
| Start Time | Left | Thru | Peds | Left | Right | Peds | Thru | Right | Peds | Exclu. Total | Inclu. Total | Int. Total |
| 04:00 PM | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 1 | 1 | 1 | 2 |
| 04:15 PM | 0 | 2 | 1 | 0 | 1 | 0 | 1 | 1 | 0 | 1 | 5 | 6 |
| 04:30 PM | 1 | 1 | 0 | 0 | 0 | 0 | 3 | 2 | 0 | 0 | 7 | 7 |
| 04:45 PM | 0 | 0 | 0 | 2 | 1 | 0 | 0 | 0 | 0 | 0 | 3 | 3 |
| Total | 1 | 3 | 1 | 2 | 2 | 0 | 5 | 3 | 1 | 2 | 16 | 18 |
| 05:00 PM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 1 | 1 |
| 05:15 PM | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 1 |
| 05:30 PM | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 1 |
| 05:45 PM | 0 | 0 | 0 | 4 | 0 | 2 | 0 | 0 | 0 | 2 | 4 | 6 |
| Total | 0 | 2 | 0 | 4 | 0 | 2 | 0 | 1 | 0 | 2 | 7 | 9 |
| Grand Total | 1 | 5 | 1 | 6 | 2 | 2 | 5 | 4 | 1 | 4 | 23 | 27 |
| Apprch \% | 16.7 | 83.3 |  | 75 | 25 |  | 55.6 | 44.4 |  |  |  |  |
| Total \% | 4.3 | 21.7 |  | 26.1 | 8.7 |  | 21.7 | 17.4 |  | 14.8 | 85.2 |  |


|  | Derry Rd From North |  |  | Ledge Rd From East |  |  | Derry Rd From South |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Left | Thru | App. Total | Left | Right | App. Total | Thru | Right | App. Total | Int. Total |
| Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1 |  |  |  |  |  |  |  |  |  |  |
| Peak Hour for Entire Inter | Beg | 4:00 P |  |  |  |  |  |  |  |  |
| 04:00 PM | 0 | 0 | 0 | 0 | 0 | 0 | , | 0 | 1 | 1 |
| 04:15 PM | 0 | 2 | 2 | 0 | 1 | 1 | 1 | 1 | 2 | 5 |
| 04:30 PM | 1 | 1 | 2 | 0 | 0 | 0 | 3 | 2 | 5 | 7 |
| 04:45 PM | 0 | 0 | 0 | 2 | 1 | 3 | 0 | 0 | 0 | 3 |
| Total Volume | 1 | 3 | 4 | 2 | 2 | 4 | 5 | 3 | 8 | 16 |
| \% App. Total | 25 | 75 |  | 50 | 50 |  | 62.5 | 37.5 |  |  |
| PHF | . 250 | . 375 | . 500 | . 250 | . 500 | . 333 | . 417 | . 375 | . 400 | . 571 |

## Accurate Counts

N/S Street : Derry Road E/W Street : Ledge Road City/State : Hudson, NH Weather : Clear

File Name : 15530001
Site Code : 15530001
Start Date : 6/2/2021
Page No : 11

|  | Peak Hour Data <br> Peak Hour Begins at 04:00 PM <br> Bikes Peds |  |
| :---: | :---: | :---: |

Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1
Peak Hour for Each Approach Begins at:

|  | 04:00 PM |  |  | 04:00 PM |  |  | 04:00 PM |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| +0 mins. | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 1 |
| +15 mins. | 0 | 2 | 2 | 0 | 1 | 1 | 1 | 1 | 2 |
| +30 mins. | 1 | 1 | 2 | 0 | 0 | 0 | 3 | 2 | 5 |
| +45 mins. | 0 | 0 | 0 | 2 | 1 | 3 | 0 | 0 | 0 |
| Total Volume | 1 | 3 | 4 | 2 | 2 | 4 | 5 | 3 | 8 |
| \% App. Total | 25 | 75 |  | 50 | 50 |  | 62.5 | 37.5 |  |
| PHF | . 250 | . 375 | . 500 | . 250 | . 500 | . 333 | . 417 | . 375 | . 400 |

Appendix C: Monthly Traffic Volumes

Year 2019 Monthly Data

Group 4 Averages:

| Month | ADT | Adjustment to Average | Adjustmen to Peak |
| :---: | :---: | :---: | :---: |
| January | 11,431 | 1.12 | 1.23 |
| February | 11,848 | 1.08 | 1.18 |
| March | 12,141 | 1.06 | 1.15 |
| April | 12,860 | 1.00 | 1.09 |
| May | 13,551 | 0.95 | 1.03 |
| June | 13,785 | 0.93 | 1.02 |
| July | 13,942 | 0.92 | 1.01 |
| August | 14,016 | 0.92 | 1.00 |
| September | 13,379 | 0.96 | 1.05 |
| October | 13,339 | 0.96 | 1.05 |
| November | 12,265 | 1.05 | 1.14 |
| December | 11,496 | 1.12 | 1.22 |

Urban Highways

Adjustment Adjustment
o Average to Peak
1.23
1.15
1.09
1.03
1.01
1.00
1.05
1.22

Average ADT: 12,838
Peak ADT: 14,016

Appendix D: Vehicle Speeds

Location: South of Ledge Road
City/State: Hudson, NH
Direction: SB,

| $\begin{array}{r} \hline 6 / 2 / 2021 \\ \text { Time } \\ \hline \end{array}$ | $\begin{aligned} & \hline 0-15 \\ & \mathrm{MPH} \end{aligned}$ | $\begin{gathered} >15- \\ 20 \mathrm{MPH} \end{gathered}$ | $\begin{gathered} >20- \\ 25 \mathrm{MPH} \end{gathered}$ | $\begin{gathered} >25- \\ 30 \mathrm{MPH} \end{gathered}$ | $\begin{gathered} \quad>30- \\ 35 \mathrm{MPH} \end{gathered}$ | $\begin{gathered} >35- \\ 40 \mathrm{MPH} \end{gathered}$ | $\begin{gathered} >40- \\ 45 \mathrm{MPH} \end{gathered}$ | $\begin{gathered} >45- \\ 50 \mathrm{MPH} \end{gathered}$ | $\begin{gathered} >50- \\ 55 \mathrm{MPH} \end{gathered}$ | $\begin{gathered} >55- \\ 60 \mathrm{MPH} \end{gathered}$ | $\begin{gathered} >60- \\ 65 \mathrm{MPH} \end{gathered}$ | $\begin{gathered} >65- \\ 70 \mathrm{MPH} \end{gathered}$ | $\begin{aligned} & \hline>70 \\ & \text { MPH } \end{aligned}$ | Total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 12:00 AM | 0 | 0 | 0 | 3 | 16 | 9 | 3 | 1 | 1 | 0 | 0 | 0 | 0 | 33 |
| 1:00 | 0 | 0 | 0 | 2 | 13 | 8 | 4 | 0 | 0 | 0 | 0 | 0 | 0 | 27 |
| 2:00 | 0 | 00 | 0 | 4 | 8 | 6 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 18 |
| 3:00 |  | 0 | 0 | 4 | 17 | 15 | 8 | 1 | 0 | 1 | 0 | 0 | 0 | 46 |
| 4:00 | 0 | 0 | 2 | 8 | 37 | 73 | 31 | 3 | 0 | 0 | 0 | 0 | 0 | 155 |
| 5:00 |  | 02 | 1 | 9 | 108 | 237 | 59 | 12 | 0 | 0 | 0 | 0 | 0 | 428 |
| 6:00 | 0 | 00 | 17 | 69 | 257 | 350 | 63 | 3 | 0 | 0 | 0 | 0 | 0 | 759 |
| 7:00 | 0 | 02 | 3 | 154 | 618 | 244 | 25 | 2 | 0 | 0 | 0 | 0 | 0 | 1048 |
| 8:00 |  | - 8 | 24 | 255 | 435 | 206 | 26 | 2 | 0 | 0 | 0 | 0 | 0 | 956 |
| 9:00 | 2 | 27 | 8 | 133 | 363 | 227 | 27 | 0 | 0 | 0 | 0 | 0 | 0 | 767 |
| 10:00 | 0 | 08 | 5 | 101 | 400 | 264 | 33 | 1 | 1 | 0 | 0 | 0 | 0 | 813 |
| 11:00 | 0 | 06 | 10 | 104 | 364 | 219 | 33 | 2 | 0 | 0 | 0 | 0 | 0 | 738 |
| 12:00 PM |  | 0 | 15 | 134 | 397 | 255 | 31 | 3 | 0 | 0 | 0 | 0 | 0 | 840 |
| 1:00 |  | 19 | 14 | 121 | 427 | 197 | 25 | 4 | 0 | 0 | 0 | 0 | 0 | 798 |
| 2:00 | 0 | 010 | 27 | 182 | 566 | 215 | 12 | 1 | 0 | 0 | 0 | 0 | 0 | 1013 |
| 3:00 | 1 | 13 | 29 | 222 | 475 | 228 | 20 | 3 | 0 | 0 | 0 | 0 | 0 | 981 |
| 4:00 | 2 | 24 | 11 | 119 | 498 | 253 | 37 | 3 | 0 | 0 | 0 | 0 | 0 | 927 |
| 5:00 | 1 | 14 | 38 | 102 | 438 | 313 | 45 | 2 | 0 | 0 | 0 | 0 | 0 | 943 |
| 6:00 | 0 | 5 | 8 | 77 | 334 | 298 | 59 | 2 | 0 | 0 | 0 | 0 | 0 | 783 |
| 7:00 | 0 | 03 | 2 | 38 | 298 | 221 | 39 | 3 | 1 | 0 | 0 | 0 | 0 | 605 |
| 8:00 | 0 | 02 | 1 | 27 | 206 | 180 | 30 | 2 | 0 | 0 | 0 | 0 | 0 | 448 |
| 9:00 |  | 0 | 1 | 21 | 129 | 89 | 21 | 3 | 1 | 0 | 0 | 0 | 0 | 267 |
| 10:00 |  | 0 | 0 | 15 | 69 | 54 | 14 | 2 | 0 | 0 | 0 | 0 | 0 | 154 |
| 11:00 | 0 | 0 | 0 | 7 | 26 | 30 | 7 | 0 | 0 | 0 | 0 | 0 | 0 | 72 |
| Total |  | 73 | 216 | 1911 | 6499 | 4191 | 652 | 55 | 4 | 1 | 0 | 0 | 0 | 13619 |
| Percentile |  |  |  | 15th | 50th | 85th | 95th |  |  |  |  |  |  |  |
| Speed |  |  |  | 29.7 | 33.5 | 37.8 | 40.3 |  |  |  |  |  |  |  |
| Mean Speed (Average) |  |  |  | 33.7 |  |  |  |  |  |  |  |  |  |  |
| 10 MPH Pace Speed |  |  |  | 30-39 |  |  |  |  |  |  |  |  |  |  |
| Number in Pace |  |  |  | 10644 |  |  |  |  |  |  |  |  |  |  |
| Percent in Pace |  |  |  | 78.2\% |  |  |  |  |  |  |  |  |  |  |
| Number > 30 MPH |  |  |  | 11402 |  |  |  |  |  |  |  |  |  |  |
| Percent > 30 MPH |  |  |  | 83.7\% |  |  |  |  |  |  |  |  |  |  |

## Accurate Counts

978-664-2565
Location : Derry Road
15530001
Location: South of Ledge Road
City/State: Hudson, NH
Direction: SB,

| $\begin{array}{r} \hline 6 / 3 / 2021 \\ \text { Time } \\ \hline \end{array}$ | $\begin{aligned} & \hline 0-15 \\ & \text { MPH } \end{aligned}$ | $\begin{gathered} >15- \\ 20 \mathrm{MPH} \end{gathered}$ | $\begin{gathered} >20- \\ 25 \mathrm{MPH} \end{gathered}$ | $\begin{gathered} >25- \\ 30 \mathrm{MPH} \end{gathered}$ | $\begin{gathered} >30- \\ 35 \mathrm{MPH} \end{gathered}$ | $\begin{gathered} >35- \\ 40 \mathrm{MPH} \end{gathered}$ | $\begin{gathered} >40- \\ 45 \mathrm{MPH} \end{gathered}$ | $\begin{gathered} \quad>45- \\ 50 \mathrm{MPH} \end{gathered}$ | $\begin{gathered} \hline>50- \\ 55 \mathrm{MPH} \end{gathered}$ | $\begin{gathered} >55- \\ 60 \mathrm{MPH} \end{gathered}$ | $\begin{gathered} >60- \\ 65 \mathrm{MPH} \end{gathered}$ | $\begin{gathered} >65- \\ 70 \mathrm{MPH} \end{gathered}$ | $\begin{aligned} & \hline 70 \\ & \text { MPH } \end{aligned}$ | Total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 12:00 AM | 0 | 0 | 0 | 8 | 10 | 21 | 4 | 1 | 0 | 0 | 0 | 0 | 0 | 44 |
| 1:00 | 0 | 1 | 0 | 8 | 10 | 4 | 3 | 1 | 1 | 0 | 0 | 0 | 0 | 28 |
| 2:00 | 0 | 0 | 2 | 5 | 13 | 13 | 2 | 1 | 0 | 0 | 0 | 0 | 0 | 36 |
| 3:00 | 0 | 0 | 0 | 1 | 19 | 19 | 8 | 1 | 1 | 0 | 0 | 0 | 0 | 49 |
| 4:00 | 0 | 0 | 2 | 4 | 38 | 71 | 22 | 1 | 0 | 0 | 0 | 0 | 0 | 138 |
| 5:00 | 0 | 1 | 1 | 15 | 154 | 193 | 50 | 7 | 0 | 0 | 0 | 0 | 0 | 421 |
| 6:00 | 0 | 2 | 0 | 61 | 308 | 335 | 36 | 2 | 0 | 0 | 0 | 0 | 0 | 744 |
| 7:00 | 1 | 12 | 20 | 195 | 534 | 248 | 10 | 5 | 1 | 0 | 0 | 0 | 0 | 1026 |
| 8:00 | 0 | 3 | 23 | 252 | 463 | 180 | 23 | 1 | 0 | 0 | 0 | 0 | 0 | 945 |
| 9:00 | 0 | 3 | 8 | 87 | 352 | 241 | 28 | 2 | 0 | 0 | 0 | 0 | 0 | 721 |
| 10:00 | 0 | 7 | 11 | 107 | 401 | 204 | 16 | 1 | 0 | 0 | 1 | 0 | 0 | 748 |
| 11:00 | 0 | 8 | 22 | 163 | 428 | 169 | 15 | 5 | 0 | 0 | 0 | 0 | 0 | 810 |
| 12:00 PM | 0 | 6 | 20 | 91 | 438 | 235 | 29 | 0 | 0 | 0 | 0 | 0 | 0 | 819 |
| 1:00 | 0 | 6 | 11 | 132 | 434 | 216 | 26 | 0 | 0 | 0 | 0 | 0 | 0 | 825 |
| 2:00 | 0 | 8 | 42 | 233 | 498 | 194 | 32 | 1 | 0 | 0 | 0 | 0 | 0 | 1008 |
| 3:00 | 1 | 6 | 18 | 251 | 476 | 200 | 25 | 1 | 1 | 0 | 0 | 0 | 0 | 979 |
| 4:00 | 10 | 17 | 56 | 222 | 400 | 265 | 37 | 1 | 0 | 1 | 0 | 0 | 0 | 1009 |
| 5:00 | 0 | 3 | 9 | 97 | 480 | 327 | 32 | 2 | 0 | 0 | 0 | 0 | 0 | 950 |
| 6:00 | 0 | 1 | 2 | 30 | 297 | 306 | 58 | 5 | 1 | 0 | 0 | 0 | 0 | 700 |
| 7:00 | 0 | 0 | 1 | 44 | 226 | 210 | 42 | 5 | 0 | 0 | 0 | 0 | 0 | 528 |
| 8:00 | 1 | 3 | 5 | 37 | 141 | 166 | 39 | 2 | 0 | 2 | 0 | 0 | 0 | 396 |
| 9:00 | 1 | 1 | 0 | 13 | 85 | 129 | 30 | 5 | 0 | 0 | 0 | 0 | 0 | 264 |
| 10:00 | 1 | 2 | 0 | 10 | 62 | 58 | 25 | 6 | 0 | 0 | 0 | 0 | 0 | 164 |
| 11:00 | 0 | 1 | 0 | 10 | 28 | 30 | 7 | 2 | 0 | 0 | 0 | 0 | 0 | 78 |
| Total | 15 | 91 | 253 | 2076 | 6295 | 4034 | 599 | 58 | 5 | 3 | 1 | 0 | 0 | 13430 |
|  |  |  | Percentile | 15th | 50th | 85th | 95th |  |  |  |  |  |  |  |
|  |  |  | Speed | 29.7 | 33.5 | 37.2 | 39.7 |  |  |  |  |  |  |  |
|  | Mea | an Speed ( | Average) | 33.5 |  |  |  |  |  |  |  |  |  |  |
|  |  | MPH Pa | ce Speed | 30-39 |  |  |  |  |  |  |  |  |  |  |
|  |  | Numbe | r in Pace | 10290 |  |  |  |  |  |  |  |  |  |  |
|  |  | Percen | t in Pace | 76.6\% |  |  |  |  |  |  |  |  |  |  |
|  |  | Number > | 30 MPH | 10995 |  |  |  |  |  |  |  |  |  |  |
|  |  | Percent $>$ | 30 MPH | 81.9\% |  |  |  |  |  |  |  |  |  |  |
| Grand Total | 22 | 174 | 469 | 3987 | 12794 | 8225 | 1251 | 113 | 9 | 4 | 1 | 0 | 0 | 27049 |
| Stats |  |  | Percentile | 15th | 50th | 85th | 95th |  |  |  |  |  |  |  |
|  |  |  | Speed | 29.7 | 33.5 | 37.2 | 39.7 |  |  |  |  |  |  |  |
|  |  | S Speed ( | Average) | 33.6 |  |  |  |  |  |  |  |  |  |  |
|  |  | MPH Pa | ce Speed | 30-39 |  |  |  |  |  |  |  |  |  |  |
|  |  | Numbe | in Pace | 20934 |  |  |  |  |  |  |  |  |  |  |
|  |  | Percen | t in Pace | 77.4\% |  |  |  |  |  |  |  |  |  |  |
|  |  | Number > | 30 MPH | 22397 |  |  |  |  |  |  |  |  |  |  |
|  |  | Percent > | 30 MPH | 82.8\% |  |  |  |  |  |  |  |  |  |  |

Location: South of Ledge Road
City/State: Hudson, NH
Direction: NB,

| $\begin{array}{r} \hline 6 / 2 / 2021 \\ \text { Time } \end{array}$ | $\begin{aligned} & \hline 0-15 \\ & \text { MPH } \end{aligned}$ | $\begin{gathered} >15- \\ 20 \mathrm{MPH} \end{gathered}$ | $\begin{gathered} >20- \\ 25 \mathrm{MPH} \end{gathered}$ | $\begin{gathered} >25- \\ 30 \mathrm{MPH} \end{gathered}$ | $\begin{gathered} >30- \\ 35 \mathrm{MPH} \end{gathered}$ | $\begin{gathered} >35- \\ 40 \mathrm{MPH} \end{gathered}$ | $\begin{gathered} >40- \\ 45 \mathrm{MPH} \end{gathered}$ | $\begin{gathered} >45- \\ 50 \mathrm{MPH} \end{gathered}$ | $\begin{gathered} >50- \\ 55 \mathrm{MPH} \end{gathered}$ | $\begin{gathered} \hline>55- \\ 60 \mathrm{MPH} \end{gathered}$ | $\begin{gathered} >60- \\ 65 \mathrm{MPH} \end{gathered}$ | $\begin{gathered} >65- \\ 70 \mathrm{MPH} \end{gathered}$ | $\begin{aligned} & >70 \\ & \mathrm{MPH} \end{aligned}$ | Total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 12:00 AM | 0 | 0 | 1 | 1 | 7 | 23 | 7 | 2 | 0 | 0 | 0 | 0 | 0 | 41 |
| 1:00 | 0 | 0 | 0 | 0 | 10 | 10 | 2 | 0 | 0 | 0 | 0 | 0 | 0 | 22 |
| 2:00 | 0 | 0 | 0 | 2 | 4 | 6 | 5 | 2 | 0 | 0 | 0 | 0 | 0 | 19 |
| 3:00 | 0 | 0 | 0 | 0 | 7 | 18 | 7 | 7 | 0 | 0 | 0 | 0 | 1 | 40 |
| 4:00 | 0 | 0 | 2 | 0 | 11 | 29 | 27 | 4 | 0 | 0 | 0 | 0 | 2 | 75 |
| 5:00 | 0 | 0 | 2 | 3 | 38 | 102 | 77 | 17 | 0 | 1 | 0 | 0 | 1 | 241 |
| 6:00 | 0 | 5 | 21 | 61 | 133 | 217 | 94 | 11 | 2 | 0 | 0 | 0 | 0 | 544 |
| 7:00 | 3 | 1 | 17 | 87 | 235 | 404 | 86 | 7 | 1 | 0 | 0 | 0 | 1 | 842 |
| 8:00 | 3 | 1 | 24 | 63 | 221 | 352 | 68 | 1 | 0 | 0 | 0 | 0 | 2 | 735 |
| 9:00 | 1 | 2 | 12 | 34 | 204 | 323 | 70 | 5 | 0 | 0 | 0 | 1 | 2 | 654 |
| 10:00 | 3 | 6 | 8 | 30 | 238 | 274 | 72 | 7 | 0 | 0 | 0 | 0 | 2 | 640 |
| 11:00 | 2 | 6 | 8 | 63 | 275 | 312 | 80 | 6 | 0 | 0 | 0 | 1 | 2 | 755 |
| 12:00 PM | 2 | 7 | 16 | 39 | 296 | 362 | 72 | 5 | 0 | 0 | 0 | 0 | 0 | 799 |
| 1:00 | 6 | 2 | 17 | 90 | 312 | 317 | 60 | 3 | 1 | 0 | 0 | 0 | 0 | 808 |
| 2:00 | 3 | 2 | 4 | 75 | 349 | 411 | 70 | 5 | 0 | 0 | 0 | 1 | 1 | 921 |
| 3:00 | 4 | 4 | 20 | 86 | 423 | 431 | 94 | 3 | 0 | 0 | 0 | 0 | 0 | 1065 |
| 4:00 | 6 | 8 | 24 | 204 | 498 | 389 | 32 | 1 | 0 | 0 | 0 | 0 | 1 | 1163 |
| 5:00 | 0 | 3 | 6 | 105 | 408 | 493 | 86 | 5 | 0 | 0 | 0 | 0 | 0 | 1106 |
| 6:00 | 3 | 5 | 3 | 45 | 293 | 418 | 104 | 11 | 0 | 0 | 0 | 0 | 1 | 883 |
| 7:00 | 0 | 3 | 3 | 28 | 179 | 348 | 70 | 9 | 2 | 0 | 0 | 0 | 0 | 642 |
| 8:00 | 0 | 1 | 5 | 41 | 189 | 222 | 42 | 7 | 0 | 0 | 0 | 0 | 0 | 507 |
| 9:00 | 1 | 0 | 0 | 13 | 103 | 159 | 50 | 3 | 0 | 0 | 0 | 0 | 0 | 329 |
| 10:00 | 0 | 0 | 2 | 10 | 44 | 81 | 25 | 2 | 0 | 1 | 0 | 0 | 0 | 165 |
| 11:00 | 0 | 0 | 3 | 3 | 23 | 51 | 15 | 4 | 0 | 0 | 0 | 0 | 0 | 99 |
| Total | 37 | 56 | 198 | 1083 | 4500 | 5752 | 1315 | 127 | 6 | 2 | 0 | 3 | 16 | 13095 |
| Percentile |  |  |  | 15th | 50th | 85th | 95th |  |  |  |  |  |  |  |
| Speed |  |  |  | 31 | 35.3 | 39 | 41.5 |  |  |  |  |  |  |  |
| Mean Speed (Average) |  |  |  | 35.4 |  |  |  |  |  |  |  |  |  |  |
| 10 MPH Pace Speed |  |  |  | 30-39 |  |  |  |  |  |  |  |  |  |  |
| Number in Pace |  |  |  | 10159 |  |  |  |  |  |  |  |  |  |  |
| Percent in Pace |  |  |  | 77.6\% |  |  |  |  |  |  |  |  |  |  |
| Number > 30 MPH |  |  |  | 11721 |  |  |  |  |  |  |  |  |  |  |
| Percent > 30 MPH |  |  |  | 89.5\% |  |  |  |  |  |  |  |  |  |  |

Location: South of Ledge Road
City/State: Hudson, NH

| $\begin{array}{r} \hline 6 / 3 / 2021 \\ \text { Time } \end{array}$ | $\begin{aligned} & \hline 0-15 \\ & \mathrm{MPH} \end{aligned}$ | $\begin{gathered} >15- \\ 20 \mathrm{MPH} \end{gathered}$ | $\begin{gathered} >20- \\ 25 \mathrm{MPH} \end{gathered}$ | $\begin{gathered} \hline>25- \\ 30 \mathrm{MPH} \end{gathered}$ | $\begin{gathered} \quad>30- \\ 35 \mathrm{MPH} \end{gathered}$ | $\begin{gathered} >35- \\ 40 \mathrm{MPH} \end{gathered}$ | $\begin{gathered} >40- \\ 45 \mathrm{MPH} \end{gathered}$ | $\begin{gathered} \quad>45- \\ 50 \mathrm{MPH} \end{gathered}$ | $\begin{gathered} >50- \\ 55 \mathrm{MPH} \end{gathered}$ | $\begin{gathered} >55- \\ 60 \mathrm{MPH} \end{gathered}$ | $\begin{gathered} >60- \\ 65 \mathrm{MPH} \end{gathered}$ | $\begin{gathered} >65- \\ 70 \mathrm{MPH} \end{gathered}$ | $\begin{aligned} & \hline>70 \\ & \text { MPH } \end{aligned}$ | Total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 12:00 AM | 0 | 1 | 0 | 5 | 9 | 27 | 7 | 0 | 0 | 0 | 1 | 1 | 0 | 51 |
| 1:00 | 0 | 0 | 0 | 2 | 7 | 8 | 8 | 1 | 1 | 0 | 0 | 0 | 0 | 27 |
| 2:00 | 0 | 0 | 1 | 1 | 4 | 7 | 7 | 3 | 0 | 0 | 0 | 0 | 0 | 23 |
| 3:00 | 0 | 0 | 0 | 1 | 5 | 9 | 7 | 0 | 1 | 0 | 0 | 0 | 0 | 23 |
| 4:00 | 0 | 0 | 0 | 4 | 11 | 45 | 25 | 6 | 0 | 0 | 0 | 0 | 0 | 91 |
| 5:00 | 0 | 0 | 5 | 9 | 52 | 108 | 68 | 21 | 0 | 0 | 0 | 0 | 1 | 264 |
| 6:00 | 1 | 2 | 18 | 66 | 104 | 199 | 85 | 11 | 1 | 1 | 0 | 0 | 2 | 490 |
| 7:00 | 4 | 2 | 16 | 103 | 315 | 359 | 62 | 6 | 1 | 0 | 0 | 2 | 1 | 871 |
| 8:00 | 7 | 2 | 20 | 56 | 261 | 326 | 85 | 7 | 1 | 0 | 0 | 0 | 9 | 774 |
| 9:00 | 1 | 1 | 3 | 48 | 218 | 316 | 65 | 9 | 1 | 0 | 0 | 0 | 2 | 664 |
| 10:00 | 1 | 4 | 3 | 51 | 199 | 318 | 90 | 7 | 0 | 0 | 0 | 0 | 0 | 673 |
| 11:00 | 2 | 5 | 9 | 71 | 232 | 270 | 95 | 1 | 0 | 0 | 0 | 3 | 2 | 690 |
| 12:00 PM | 3 | 3 | 7 | 50 | 310 | 358 | 75 | 8 | 0 | 0 | 0 | 0 | 0 | 814 |
| 1:00 | 2 | 4 | 16 | 74 | 373 | 349 | 56 | 3 | 0 | 0 | 0 | 0 | 0 | 877 |
| 2:00 | 5 | 7 | 15 | 129 | 431 | 347 | 47 | 2 | 0 | 0 | 1 | 2 | 0 | 986 |
| 3:00 | 5 | 2 | 17 | 180 | 447 | 366 | 53 | 1 | 1 | 0 | 0 | 0 | 0 | 1072 |
| 4:00 | 15 | 16 | 39 | 121 | 396 | 460 | 73 | 0 | 0 | 0 | 0 | 0 | 0 | 1120 |
| 5:00 | 0 | 5 | 12 | 68 | 433 | 471 | 104 | 5 | 0 | 0 | 0 | 0 | 0 | 1098 |
| 6:00 | 0 | 0 | 2 | 46 | 317 | 359 | 103 | 13 | 1 | 0 | 0 | 0 | 0 | 841 |
| 7:00 | 3 | 0 | 6 | 38 | 195 | 301 | 89 | 6 | 0 | 0 | 0 | 0 | 0 | 638 |
| 8:00 | 0 | 0 | 1 | 24 | 160 | 215 | 44 | 2 | 1 | 0 | 0 | 0 | 0 | 447 |
| 9:00 | 0 | 3 | 4 | 16 | 99 | 119 | 62 | 1 | 0 | 0 | 0 | 0 | 0 | 304 |
| 10:00 | 0 | 0 | 1 | 13 | 38 | 81 | 30 | 7 | 1 | 0 | 0 | 0 | 0 | 171 |
| 11:00 | 0 | 0 | 1 | 2 | 22 | 53 | 25 | 1 | 0 | 0 | 0 | 0 | 0 | 104 |
| Total | 49 | 57 | 196 | 1178 | 4638 | 5471 | 1365 | 121 | 10 | 1 | 2 | 8 | 17 | 13113 |
|  |  |  | Percentile | 15th | 50th | 85th | 95th |  |  |  |  |  |  |  |
|  |  |  | Speed | 31 | 35.3 | 39 | 41.5 |  |  |  |  |  |  |  |
|  | Mea | Speed | Average) | 35.3 |  |  |  |  |  |  |  |  |  |  |
|  |  | MPH Pa | ce Speed | 30-39 |  |  |  |  |  |  |  |  |  |  |
|  |  | Numbe | in Pace | 10023 |  |  |  |  |  |  |  |  |  |  |
|  |  | Percen | t in Pace | 76.4\% |  |  |  |  |  |  |  |  |  |  |
|  |  | Number $>$ | 30 MPH | 11633 |  |  |  |  |  |  |  |  |  |  |
|  |  | Percent $>$ | 30 MPH | 88.7\% |  |  |  |  |  |  |  |  |  |  |
| Grand Total | 86 | 113 | 394 | 2261 | 9138 | 11223 | 2680 | 248 | 16 | 3 | 2 | 11 | 33 | 26208 |
| Stats |  |  | Percentile | 15th | 50th | 85th | 95th |  |  |  |  |  |  |  |
|  |  |  | Speed | 31 | 35.3 | 39 | 41.5 |  |  |  |  |  |  |  |
|  |  | Speed | Average) | 35.3 |  |  |  |  |  |  |  |  |  |  |
|  |  | MPH Pa | ce Speed | 30-39 |  |  |  |  |  |  |  |  |  |  |
|  |  | Numbe | in Pace | 20182 |  |  |  |  |  |  |  |  |  |  |
|  |  | Percen | t in Pace | 77.0\% |  |  |  |  |  |  |  |  |  |  |
|  |  | Number > | 30 MPH | 23354 |  |  |  |  |  |  |  |  |  |  |
|  |  | Percent $>$ | 30 MPH | 89.1\% |  |  |  |  |  |  |  |  |  |  |

Appendix E: Capacity-Analysis Worksheets

| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 1.1 |  |  |  |  |  |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | Mr |  | $\mathbf{F}$ |  | a | 4 |
| Traffic Vol, veh/h | 24 | 29 | 895 | 10 | 11 | 1228 |
| Future Vol, veh/h | 24 | 29 | 895 | 10 | 11 | 1228 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | 0 | - |
| Veh in Median Storage, \# | 0 | - | 0 | - | - | 0 |
| Grade, \% | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 72 | 72 | 93 | 93 | 90 | 90 |
| Heavy Vehicles, \% | 9 | 0 | 3 | 0 | 20 | 2 |
| Mvmt Flow | 33 | 40 | 962 | 11 | 12 | 1364 |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 0.7 |  |  |  |  |  |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | Mr |  | $\mathbf{F}$ |  | 1 | 4 |
| Traffic Vol, veh/h | 8 | 25 | 1217 | 26 | 20 | 1039 |
| Future Vol, veh/h | 8 | 25 | 1217 | 26 | 20 | 1039 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | 0 | - |
| Veh in Median Storage, \# | 0 | - | 0 | - | - | 0 |
| Grade, \% | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 68 | 68 | 99 | 99 | 96 | 96 |
| Heavy Vehicles, \% | 0 | 0 | 2 | 4 | 0 | 1 |
| Mvmt Flow | 12 | 37 | 1229 | 26 | 21 | 1082 |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 0.8 |  |  |  |  |  |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | Mr |  | $\mathbf{F}$ |  | a | 4 |
| Traffic Vol, veh/h | 24 | 29 | 904 | 10 | 11 | 1240 |
| Future Vol, veh/h | 24 | 29 | 904 | 10 | 11 | 1240 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | 0 | - |
| Veh in Median Storage, \# | 0 | - | 0 | - | - | 0 |
| Grade, \% | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 90 | 90 | 90 | 90 | 90 | 90 |
| Heavy Vehicles, \% | 9 | 0 | 3 | 0 | 20 | 2 |
| Mvmt Flow | 27 | 32 | 1004 | 11 | 12 | 1378 |


| Major/Minor M | Minor1 |  | Major1 |  | Major2 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 2412 | 1010 | 0 | 0 | 1015 | 0 |
| Stage 1 | 1010 | - | - | - | - | - |
| Stage 2 | 1402 | - | - | - | - | - |
| Critical Hdwy | 6.49 | 6.2 | - | - | 4.3 | - |
| Critical Hdwy Stg 1 | 5.49 | - | - | - | - | - |
| Critical Hdwy Stg 2 | 5.49 | - | - | - | - | - |
| Follow-up Hdwy | 3.581 | 3.3 | - | - | 2.38 | - |
| Pot Cap-1 Maneuver | 34 | 294 | - | - | 618 | - |
| Stage 1 | 342 | - | - | - | - | - |
| Stage 2 | 220 | - | - | - | - | - |
| Platoon blocked, \% |  |  | - | - |  | - |
| Mov Cap-1 Maneuver | 33 | 294 | - | - | 618 | - |
| Mov Cap-2 Maneuver | 135 | - | - | - | - | - |
| Stage 1 | 342 | - | - | - | - | - |
| Stage 2 | 216 | - | - | - | - | - |
|  |  |  |  |  |  |  |
| Approach | WB |  | NB |  | SB |  |
| HCM Control Delay, s | 31.8 |  | 0 |  | 0.1 |  |
| HCM LOS | D |  |  |  |  |  |
|  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | NBT | NBRWBLn1 |  | SBL | SBT |
| Capacity (veh/h) |  | - | - | 192 | 618 | - |
| HCM Lane V/C Ratio |  | - | - | 0.307 | 0.02 | - |
| HCM Control Delay (s) |  | - | - | 31.8 | 10.9 | - |
| HCM Lane LOS |  | - | - | D | B | - |
| HCM 95th \%tile Q(veh) |  | - | - | 1.2 | 0.1 | - |


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 0.6 |  |  |  |  |  |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | Mr |  | $\mathbf{F}$ |  | 1 | 4 |
| Traffic Vol, veh/h | 8 | 25 | 1229 | 26 | 20 | 1049 |
| Future Vol, veh/h | 8 | 25 | 1229 | 26 | 20 | 1049 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | 0 | - |
| Veh in Median Storage, \# | 0 | - | 0 | - | - | 0 |
| Grade, \% | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 90 | 90 | 90 | 90 | 90 | 90 |
| Heavy Vehicles, \% | 0 | 0 | 2 | 4 | 0 | 1 |
| Mvmt Flow | 9 | 28 | 1366 | 29 | 22 | 1166 |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |


| Major/Minor | Minor1 | Major1 |  | Major2 |  |
| :--- | ---: | ---: | ---: | ---: | :--- |
| Conflicting Flow All | 2664 | 1116 | 0 | 0 | 1122 |
| Stage 1 | 1116 | - | - | - | - |
| $\quad$ Stage 2 | 1548 | - | - | - | - |


| Minor Lane/Major Mvmt | NBT | NBRWBLn1 | SBL | SBT |  |
| :--- | ---: | ---: | ---: | ---: | :--- |
| Capacity (veh/h) | - | -163 | 561 | - |  |
| HCM Lane V/C Ratio | - | -0.402 | 0.024 | - |  |
| HCM Control Delay (s) | - | -41.2 | 11.6 | - |  |
| HCM Lane LOS | - | - | E | B | - |
| HCM 95th \%tile Q(veh) | - | - | 1.8 | 0.1 | - |

## Notes

$\sim$ : Volume exceeds capacity $\$$ : Delay exceeds $300 s \quad+$ : Computation Not Defined $\quad$ : All major volume in platoon

| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 0.7 |  |  |  |  |  |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | Mr |  | $\mathbf{F}$ |  | a | 4 |
| Traffic Vol, veh/h | 9 | 28 | 1358 | 29 | 22 | 1159 |
| Future Vol, veh/h | 9 | 28 | 1358 | 29 | 22 | 1159 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | 0 | - |
| Veh in Median Storage, \# | 0 | - | 0 | - | - | 0 |
| Grade, \% | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 90 | 90 | 90 | 90 | 90 | 90 |
| Heavy Vehicles, \% | 0 | 0 | 2 | 4 | 0 | 1 |
| Mvmt Flow | 10 | 31 | 1509 | 32 | 24 | 1288 |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 0.9 |  |  |  |  |  |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | Mr |  | $\uparrow$ |  | i | 4 |
| Traffic Vol, veh/h | 29 | 24 | 907 | 10 | 11 | 1243 |
| Future Vol, veh/h | 29 | 24 | 907 | 10 | 11 | 1243 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | 0 | - |
| Veh in Median Storage, \# | 0 | - | 0 | - | - | 0 |
| Grade, \% | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 90 | 90 | 90 | 90 | 90 | 90 |
| Heavy Vehicles, $\%$ | 9 | 0 | 3 | 0 | 20 | 2 |
| Mvmt Flow | 32 | 27 | 1008 | 11 | 12 | 1381 |


| Major/Minor | Minor1 |  | Major1 |  | Major2 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 2419 | 1014 | 0 | 0 | 1019 | 0 |
| Stage 1 | 1014 | - | - | - | - | - |
| Stage 2 | 1405 | - | - | - | - | - |
| Critical Hdwy | 6.49 | 6.2 | - | - | 4.3 | - |
| Critical Hdwy Stg 1 | 5.49 | - | - | - | - | - |
| Critical Hdwy Stg 2 | 5.49 | - | - | - | - | - |
| Follow-up Hdwy | 3.581 | 3.3 | - | - | 2.38 | - |
| Pot Cap-1 Maneuver | 34 | 292 | - | - | 616 | - |
| Stage 1 | 340 | - | - | - | - | - |
| Stage 2 | 219 | - | - | - | - | - |
| Platoon blocked, \% |  |  | - | - |  | - |
| Mov Cap-1 Maneuver | 33 | 292 | - | - | 616 | - |
| Mov Cap-2 Maneuver | 134 | - | - | - | - | - |
| Stage 1 | 340 | - | - | - | - | - |
| Stage 2 | 215 | - | - | - | - | - |
|  |  |  |  |  |  |  |
| Approach | WB |  | NB |  | SB |  |
| HCM Control Delay, s | 35.2 |  | 0 |  | 0.1 |  |
| HCM LOS | E |  |  |  |  |  |
|  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | NBT | NBRWBLn1 |  | SBL | SBT |
| Capacity (veh/h) |  | - | - | 177 | 616 | - |
| HCM Lane V/C Ratio |  | - | - | 0.333 | 0.02 | - |
| HCM Control Delay (s) |  | - | - | 35.2 | 11 | - |
| HCM Lane LOS |  | - | - | E | B | - |
| HCM 95th \%tile Q(veh) |  | - | - | 1.4 | 0.1 | - |


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 0.6 |  |  |  |  |  |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | Mr |  | $\uparrow$ |  | i | 4 |
| Traffic Vol, veh/h | 8 | 25 | 1230 | 26 | 20 | 1050 |
| Future Vol, veh/h | 8 | 25 | 1230 | 26 | 20 | 1050 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | 0 | - |
| Veh in Median Storage, \# | 0 | - | 0 | - | - | 0 |
| Grade, \% | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 90 | 90 | 90 | 90 | 90 | 90 |
| Heavy Vehicles, $\%$ | 0 | 0 | 2 | 4 | 0 | 1 |
| Mvmt Flow | 9 | 28 | 1367 | 29 | 22 | 1167 |


| Major/Minor | Minor1 |  | Major1 |  | Major2 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 2593 | 1382 | 0 | 0 | 1396 | 0 |
| Stage 1 | 1382 | - | - | - | - | - |
| Stage 2 | 1211 | - | - | - | - | - |
| Critical Hdwy | 6.4 | 6.2 | - | - | 4.1 | - |
| Critical Hdwy Stg 1 | 5.4 | - | - | - | - | - |
| Critical Hdwy Stg 2 | 5.4 | - | - | - | - | - |
| Follow-up Hdwy | 3.5 | 3.3 | - | - | 2.2 | - |
| Pot Cap-1 Maneuver | 28 | 178 | - | - | 496 | - |
| Stage 1 | 235 | - | - | - | - | - |
| Stage 2 | 285 | - | - | - | - | - |
| Platoon blocked, \% |  |  | - | - |  | - |
| Mov Cap-1 Maneuver | 27 | 178 | - | - | 496 | - |
| Mov Cap-2 Maneuver | 127 | - | - | - | - | - |
| Stage 1 | 235 | - | - | - | - | - |
| Stage 2 | 272 | - | - | - | - | - |
|  |  |  |  |  |  |  |
| Approach | WB |  | NB |  | SB |  |
| HCM Control Delay, s | 33.6 |  | 0 |  | 0.2 |  |
| HCM LOS | D |  |  |  |  |  |
|  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | NBT | NBRWBLn1 |  | SBL | SBT |
| Capacity (veh/h) |  | - | - | 162 | 496 | - |
| HCM Lane V/C Ratio |  | - | - | 0.226 | 0.045 | - |
| HCM Control Delay (s) |  | - | - | 33.6 | 12.6 | - |
| HCM Lane LOS |  | - | - | D | B | - |
| HCM 95th \%tile Q(veh) |  | - | - | 0.8 | 0.1 | - |




| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |


| Major/Minor | Minor1 | Major1 |  | Major2 |  |
| :--- | ---: | ---: | ---: | ---: | :--- |
| Conflicting Flow All | 2671 | 1119 | 0 | 0 | 1125 |
| Stage 1 | 1119 | - | - | - | - |
| $\quad$ Stage 2 | 1552 | - | - | - | - |


| Minor Lane/Major Mvmt | NBT | NBRWBLn1 | SBL | SBT |
| :--- | ---: | ---: | ---: | ---: |
| Capacity (veh/h) | - | -162 | 559 | - |
| HCM Lane V/C Ratio | - | -0.405 | 0.024 | - |
| HCM Control Delay (s) | - | -41.6 | 11.6 | - |
| HCM Lane LOS | - | - | E | B |
| HCM 95th \%tile Q(veh) | - | - | 1.8 | 0.1 |
| (ven | - |  |  |  |

## Notes

$\sim$ : Volume exceeds capacity $\$$ : Delay exceeds $300 s \quad+$ : Computation Not Defined $\quad$ : All major volume in platoon



HCM LOS E

| Minor Lane/Major Mvmt | NBL | NBT EBLn1 EBLn2 | SBT | SBR |  |
| :--- | ---: | ---: | ---: | ---: | :--- |
| Capacity (veh/h) | 422 | -113 | 142 | - | - |
| HCM Lane V/C Ratio | 0.066 | -0.246 | 0.266 | - | - |
| HCM Control Delay (s) | 14.1 | -46.9 | 39.3 | - | - |
| HCM Lane LOS | B | - | E | E | - |
| HCM 95th \%tile Q(veh) | 0.2 | - | 0.9 | 1 | - |

## Notes

$\sim$ : Volume exceeds capacity $\$$ : Delay exceeds $300 \mathrm{~s} \quad+$ : Computation Not Defined *: All major volume in platoon

| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 0.8 |  |  |  |  |  |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | Mr |  | $\mathbf{F}$ |  | a | 4 |
| Traffic Vol, veh/h | 9 | 28 | 1359 | 29 | 22 | 1160 |
| Future Vol, veh/h | 9 | 28 | 1359 | 29 | 22 | 1160 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | 0 | - |
| Veh in Median Storage, \# | 0 | - | 0 | - | - | 0 |
| Grade, \% | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 90 | 90 | 90 | 90 | 90 | 90 |
| Heavy Vehicles, \% | 0 | 0 | 2 | 4 | 0 | 1 |
| Mvmt Flow | 10 | 31 | 1510 | 32 | 24 | 1289 |


| Major/Minor M | Minor1 |  | Major1 |  | Major2 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 2863 | 1526 | 0 | 0 | 1542 | 0 |
| Stage 1 | 1526 | - | - | - | - | - |
| Stage 2 | 1337 | - | - | - | - | - |
| Critical Hdwy | 6.4 | 6.2 | - | - | 4.1 | - |
| Critical Hdwy Stg 1 | 5.4 | - | - | - | - | - |
| Critical Hdwy Stg 2 | 5.4 | - | - | - | - | - |
| Follow-up Hdwy | 3.5 | 3.3 | - | - | 2.2 | - |
| Pot Cap-1 Maneuver | 19 | 146 | - | - | 436 | - |
| Stage 1 | 200 | - | - | - | - | - |
| Stage 2 | 247 | - | - | - | - | - |
| Platoon blocked, \% |  |  | - | - |  | - |
| Mov Cap-1 Maneuver | 18 | 146 | - | - | 436 | - |
| Mov Cap-2 Maneuver | 106 | - | - | - | - | - |
| Stage 1 | 200 | - | - | - | - | - |
| Stage 2 | 233 | - | - | - | - | - |
|  |  |  |  |  |  |  |
| Approach | WB |  | NB |  | SB |  |
| HCM Control Delay, s | 43.3 |  | 0 |  | 0.3 |  |
| HCM LOS | E |  |  |  |  |  |
|  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | NBT | NBRWBLn1 |  | SBL | SBT |
| Capacity (veh/h) |  | - | - | 134 | 436 | - |
| HCM Lane V/C Ratio |  | - | - | 0.307 | 0.056 | - |
| HCM Control Delay (s) |  | - | - | 43.3 | 13.7 | - |
| HCM Lane LOS |  | - | - | E | B | - |
| HCM 95th \%tile Q(veh) |  | - | - | 1.2 | 0.2 | - |


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |




CAP FEE WORKSHEET - 2021

Date:___07-21-21_Z_ Zone \#__ Map/Lot: $\frac{173 / 029-000}{56 \text { Derry Street }}$
Project Name: $\qquad$
Proposed ITE Use \#1:__ Commercial - Coffee Shop
New Daily Trips (15\% of Total): $\qquad$ 104 trips.

CAP FEES: (ONE CHECK NEEDED)

1. (Bank 09)

| 2070-701 | Coffee/Donut Shop <br> (104 trips @ \$199 per trip) |
| :--- | :--- | :--- | | $\$ \quad 20,696.00$ |
| :--- |

Total CAP Fee
\$_20,696.00

Check should be made payable to the Town of Hudson.
Thank you,
Brian Groth
Town Planner


OWNER:
STEVE S. \& HSIANG HWA W. PAN 13 KING HENRY DRIVE LONDONDERRY, NH 03053

## OWNER/APPLICANT: <br> SCOTT ZIEFELDER

169 CANAAN BACK ROAD
BARINGTON, NH 03825

## PREPARED BY:

KEACH-NORDSTROM ASSOCIATES, INC 10 COMMERCE PARK NORTH, SUITE 3B BEDFORD, NEW HAMPSHIRE 03110
(603) 627-2881


| SHEET TITLE | SHEET No. |
| :--- | :---: |
| MASTER PLAN | 1 |
| EXISTING CONDITIONS/REMOVALS PLAN | 2 |
| NON-RESIDENTIAL SITE PLAN | 3 |
| GRADING, DRAINAGE, AND UTILITY PLAN | 4 |
| EROSION CONTROL PLAN | 5 |
| LANDSCAPE PLAN | 6 |
| LIGHTING PLAN | 7 |
| SIGHT DISTANCE PLAN | 8 |
| CONSTRUCTION DETAILS | $9-14$ |
| EASEMENT PLAN | E1 |


















[^0]:    ${ }^{1}$ AASHTO, A Policy on Geometric Design of Highways and Streets, 6th Edition (Washington, DC, 2011), pages 928 to 9-29.
    ${ }^{2}$ AASHTO, pages 3-2 to 3-6.

[^1]:    ${ }^{3}$ AASHTO, pages 9-22 to 9-55.

[^2]:    ${ }^{4}$ ITE, Trip Generation Manual, 10th edition (Washington DC, September 2017).
    ${ }^{5}$ ITE, Trip Generation Manual, V Volume 2, Data, Services (Land Uses 900-999), pages 250 and 251, pages 249 to 254.
    ${ }^{6}$ Stephen G. Pernaw \& Company, Inc., Traffic Impact Assessment, Proposed Drive-Thru Coffee Shop, Northwood, New Hampshire (Concord, New Hampshire, October 2019), page 10 and Appendix E.
    ${ }^{7}$ Kevin G. Hooper, P.E., Principal Editor, Trip Generation Handbook, $3^{\text {rd }}$ edition (Washington DC: Institute of Transportation Engineers, September 2017).
    ${ }^{8}$ Definitions of primary trips, diverted trips, and pass-by trips are in Hooper, page 93. Relevant data on primary trips, diverted trips and pass-by trips are in Hooper, $3^{\text {rd }}$ edition, page 216.

[^3]:    ${ }^{\text {a }}$ Estimated total weekday daily trips are 5.93 times weekday AM-street-peak hour trips, based on ITE, Trip Generation Manual, Volume 2, Data, Services (Land Uses 900-999), pages 250 and 251.
    b Total weekday AM-street-peak hour trips are 0.0488 times 2021 no-build weekday AM-street-peak hour volume on Derry Road along the site frontage. Stephen G. Pernaw \& Company, Inc., Appendix E.
    c Total weekday PM-street-peak hour trips are 0.0172 times 2021 no-build weekday PM-street-peak hour volume on Derry Road along the site frontage. Stephen G. Pernaw \& Company, Inc., Appendix E.
    d Pass-by trip percentage is 89 . Based on Hooper, Trip Generation Handbook, $3^{\text {rd }}$ edition, page 216, coffee/donut shop with drive-through window and no indoor seating, land use 938.

[^4]:    Figure 6. 2032 site traffic volumes.

[^5]:    ${ }^{9}$ TRB, Highway Capacity Manual 2000 (Washington DC 2000) and Highway Capacity Manual 2010 (Washington DC, 2010).

