AROMA JOES

SITE PLAN APPLICATION #08-21 STAFF REPORT

August 25, 2021

SITE: 56 Derry Street; Map 173 Lot 029-000

ZONING: Business (B)

PURPOSE OF PLANS: Propose an Aroma Joe's drive-thru coffee shop at 56 Derry Street with associated parking and drives.

PLANS UNDER REVIEW:

Non-Residential Site Plan, Aroma Joe's; prepared by Keach-Nordstrom Associates, Inc., 10 Commerce Park North, Suite 3B, Bedford, NH 03110; prepared for owner: Steve S. & Hsiang Hwa W. Pan, 13 King Henry Drive, Londonderry, NH 03053 and owner/applicant: Scott Ziefelder, 169 Cannan Back Road, Barrington, NH 03825; consisting of 16 sheets including a cover sheet, with general notes 1-32 on Sheet 1; dated June 22, 2021, last revised August 12, 2021. [Plan set attached hereto]

ATTACHMENTS:

- A. Second Round of Peer Review by Fuss & O'Neill, dated August 16, 2021
- B. Traffic Impact and Access Study, prepared by TEPP, received July 20, 2021
- C. Adjusted CAP Fee Worksheet

APPLICATION TRACKING:

- June 23, 2021 Application received.
- July 20, 2021 Traffic Impact and Access Study received.
- July 28, 2021 Application accepted, public hearing held, waiver granted for relief from residential buffer, continued to 8/25/21.
- August 17, 2021 Revised Plans received.
- August 25, 2021 Continuance scheduled.

COMMENTS & RECOMMENDATIONS:

STATUS UPDATE

Please note that since the revised plan set was received on August 17, 2021, and this report was authored on August 18, 2021, a full review by town staff and the peer reviewer remains pending.

Since the last iteration of the plan set the applicant has made the following modifications:

1. Hours of operation (Sheet 1, note 15) have been revised to 5:15 AM to 9:00 PM, Monday through Sunday.

- 2. The proposed freestanding sign shows on Sheet 1 and the detail of the sign is on Sheet 10.
- 3. A crosswalk has been added per request at previous meeting.
- 4. The proposed sidewalk easement shows on Sheet E1.
- 5. The loading space and dumpster location are clearly identified on the plan

PEER REVIEW

Attachment A is the second round of peer review comments, distributed on Monday, August 16, 2021. For that reason, the applicant has not had time to provide a response. Peer review of the Traffic Study (**Attachment B**) is ongoing but expected soon.

- 1. The outstanding comments primarily focus on the stormwater/drainage design. These comments are under review by the applicant as well as the Town Engineer.
- 2. Some comments related to items being reviewed in the Traffic Study
- 3. Other items were administrative in nature and have been addressed by the applicant.

TRAFFIC STUDY

Attachment B is the Traffic Impact and Access Study prepared by Kim Hazarvartian of TEPP LLC, currently under peer review. The study concludes that no significant traffic impact results from this application.

The study reports:

2022 total vehicle trips:

- weekday daily, 629 (total of in and out)
- weekday AM-street-peak hour*, 106 (53 in and 53 out)
- weekday PM-street-peak hour, 40 (20 in and 20 out)

2032 total vehicle-trips are:

- weekday daily, 694 (total of in and out)
- weekday AM-street-peak hour, 117 (58 in and 539 out) [Staff assumes 539 is a typo]
- weekday PM-street-peak hour, 44 (22 in and 22 out)

Primary trips** are added to Derry Road near the site. 2022 primary vehicle-trips are:

- weekday daily, 69 (total of in and out)
- weekday AM-street-peak hour, 12 (6 in and 6 out)
- weekday PM-street-peak hour, 4 (2 in and 2 out)

2032 primary vehicle-trips are:

- weekday daily, 78 (total of in and out)
- weekday AM-street-peak hour, 13 (6 in and 7 out)
- weekday PM-street-peak hour, 6 (3 in and 3 out)

**Primary trips are vehicles drawn to the site by the coffee shop rather vehicles that are already travelling by the site. Peer review has indicated this is an acceptable approach with one caveat described in the CAP fee discussion below.

Staff has asked the Town's peer review consultant to examine how the traffic study lines up with the excepted operations of Aroma Joe's. For example, the study reports 53 vehicles in 2022 and 58 vehicles in 2032 during the morning peak hour. This requires customer turnover to be about 1 minute per vehicle. The operator of Aroma Joe's indicated a typical turnover time ranging from a minute to a few. This matters in determining the adequacy of the queue length to avoid the familiar problem of drive-thru customers spilling out onto public roads.

CAP FEE ADJUSTMENT

While Fuss & O'Neill serves as the Town's peer review consultant, including the traffic study in this application, VHB establishes the rates and methodology used in the Town's impact fee system, or Cost Allocation Procedure (CAP). Staff consulted VHB on the appropriate calculation of the CAP fee for this application, suspecting that using the square footage of the building was not an appropriate representation of its impact to traffic. In agreement with Staff's query, VHB recommended using the alternate method for unique categories, which is to multiply the number of new daily trips (or, Primary Trips) by \$199. VHB noted that the Traffic Study appears to allocate 10% of total trips as Primary Trips (69 of 692), whereas their standard is 15%, or 104 Primary Trips. See **Attachment C** for the updated CAP Fee worksheet.

As it relates to the overall traffic impact, the total number of vehicles remains the same. Please note that the Traffic Study remains under peer review where the discrepancy between the two methodologies will be addressed.

PLAN NOTES

Suggested Changes to Plan/Notes:

- 1. Note #29: Staff suggests amending the note to state "All signs are subject to approval by the Zoning Administrator prior to installation." The note provided is consistent with \$276.11.1.B (13), however, this regulation is inconsistent with actual sign permit practices.
- 2. Note #19 & #27: Staff suggests amending instances of "Community Development Department" to "Planning Department"
- 3. A plan note should address the hours of refuse removal / garbage pick-up. The typical hours are 7:00 A.M. and 7:00 P.M., Monday through Friday only.
- 4. References to Derry Road should be revised to Derry Street,

DRAFT MOTIONS

^{*}Peak hours are reported to be 7:00 A.M. to 8:00 P.M., and 4:00 P.M to 5:00 P.M.

$\underline{CONTINUE}$ the public hearing to a date certain:

	continue the public hearing for the site place 173 Lot 029-000 to date certain,	an application for Aroma Joes at 56 Derry, 2021.
Motion by:	Second:	Carried/Failed:
<u>APPROV</u>	$\overline{ m VE}$ the site plan application:	
Nordstrom for owner: owner/appl 16 sheets in	Associates, Inc., 10 Commerce Park Nor Steve S. & Hsiang Hwa W. Pan, 13 King licant: Scott Ziefelder, 169 Cannan Back	te Plan, Aroma Joe's; prepared by Keach- rth, Suite 3B, Bedford, NH 03110; prepared g Henry Drive, Londonderry, NH 03053 and Road, Barrington, NH 03825; consisting of s 1-32 on Sheet 1; dated June 22, 2021, last the following stipulations:
1.	All stipulations of approval shall be incowhich shall be recorded at the HCRD, to	rporated into the Development Agreement, gether with the Plan.
	All improvements shown on the Plan sha expense of the applicant or the applicant	all be completed in their entirety and at the sassigns.
		occupancy, an L.L.S. Certified "as-built" site adson Land Use Development, confirming ed by the Planning Board.
	A cost allocation procedure (CAP) amou issuance of a Certificate of Occupancy	nt of \$20,696.00 shall be paid prior to the
	The onsite drainage system shall be cons NHDES requirements for such systems.	tructed and maintained in compliance with
6.	Prior to the Planning Board endorsement administrative review by Town Planner	
	The applicant shall schedule a pre-construction beginning work on the site.	ruction meeting with the Town Engineer prior
	Hours of refuse removal shall be exclusi P.M., Monday through Friday only.	ve to the hours between 7:00 A.M. and 7:00
Motion by:	Second:	Carried/Failed:



August 16, 2021

Mr. Brian Groth Town Planner Town of Hudson 12 School Street Hudson, NH 03051

Re: Town of Hudson Planning Board Review Aroma Joe's Site Plan, 56 Derry Street

Tax Map 173 Lot 29; Acct. #1350-970

Reference No. 20030249.2040

Dear Mr. Groth:

Fuss & O'Neill (F&O) has reviewed the second submission of the materials received on June 23, 2021, related to the above-referenced project. Authorization to proceed was received on August 10, 2021. A list of items reviewed is enclosed. The scope of our review is based on the Site Plan Review Codes, Stormwater Codes, Driveway Review Codes, Sewer Use Ordinance 77, Zoning Regulations, and criteria outlined in the CLD Consulting Engineers Proposal approved September 16, 2003, revised September 20, 2004, June 4, 2007, September 3, 2008, and October 2015.

The project appears to consist of the development of a drive-thru coffee shop on a previously undeveloped site. Proposed improvements to the site also include the construction of a driveway, parking areas, drainage improvements, landscaping, lighting and other associated site improvements. The proposed buildings will be serviced by public water and sewer.

The following items have outstanding issues:

1. Site Plan Review Codes (HR 275)

- g. Former Fuss & O'Neill Comment: HR 275-8.C.(6). The applicant has not provided any off-street loading spaces on the plan set.
 - **Current Fuss & O'Neill Comment:** The applicant has added a loading space to the plan set. We note that the space shown is 40 feet long instead of the standard 60 feet. The applicant should confirm that the size of anticipated delivery vehicles will fit in this location.
- h. Former Fuss & O'Neill Comment: HR 275-9.C.(11). The applicant has provided one handicap space for the site which meets the one space required. We recommend that the applicant add spot grades to the parking lot and sidewalk area to ensure that it is constructed as intended. This is especially important in the area of the handicap space and ramp.
 - **Current Fuss & O'Neill Comment:** The applicant has provided spot grades for the parking lot ramp locations. We continue to recommend spot grades be provided for the sidewalk ramps as well.

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i. Former Fuss & O'Neill Comment: HR 275-9.F. The applicant did not provide copies of any easements or deeds as part of the package received for review, and has not shown any existing or proposed easements on the plans.

Current Fuss & O'Neill Comment: The applicant has stated that no easements or deeds are required. We continue to recommend a sidewalk easement be provided to the Town for the relocated sidewalk.

3. Driveway Review Codes (HR 275-8.B. (34)/Chapter 193)

d. **New Fuss & O'Neill Comment:** We understand that the Town has requested a crosswalk be installed from the Derry Street sidewalk to the sidewalk at the proposed building. We recommend that the applicant provide appropriate pedestrian signage both internally for this crosswalk and for vehicles approaching Derry Street at the right turn lane.

6. Drainage Design/Stormwater Management (HR 275-9.A./Chapter 290)

- a. Former Fuss & O'Neill Comment: HR 275-9.A.(3). The applicant should provide test pits within the footprint of the infiltration basin area, as required by NHDES and common engineering practice.
 - Current Fuss & O'Neill Comment: The applicant has noted that test pits are consistent within the site and within close proximity to the infiltration basin areas. We note test pit #2 has an existing elevation of 160.0, with 66" depth to ESHWT as noted within the Test Pit data and BMP worksheet for Infiltration Pond 2. Applying this test pit data to the entire Infiltration Pond#2: Bottom of basin is 159.0, existing elevation at bottom of basin is 164.0. Calculating ESHWT to be 66" below 164.0, computes to 158.5 and not 156.0 as noted within the BMP worksheet. This does not meet the 3.0' required separation from ESHWT. The applicant should provide additional test pit information to support the use of an infiltration basin situated upon the site in respect to existing elevations.
- b. Former Fuss & O'Neill Comment: HR 290-5.A.9. & 290-5.A.11. The applicant should provide NHDES BMP worksheets and an Infiltration Feasibility Report to illustrate the ESHWT is accounted for within the BMP design, as well as overall Stormwater Design meets NHDES standards.
 - **Current Fuss & O'Neill Comment:** The applicant has provided BMP worksheets. We continue to recommend the applicant provide the required Infiltration Feasibility Report.
- c. Former Fuss & O'Neill Comment: HR 290-7.B.14. Although the property has been partially developed in the past, the contours illustrate a low point near the existing CBs to be removed, as well as the close proximity of the abutting wetland discharge point. The applicant should provide a letter from a wetland scientist stating wetlands do not exist upon the site.
 - **Current Fuss & O'Neill Comment:** The applicant has stated that a letter was provided under separate cover. We note that a letter was not provided as part of the package received for review.
- d. Former Fuss & O'Neill Comment: HR 290-7.B.16. The applicant should label snow storage areas upon the plan set. Due to the close proximity of wetlands and proposed infiltration systems, we suggest reviewing the need for onsite signage or fencing to ensure proper snow storage/removal occurs.
 - **Current Fuss & O'Neill Comment:** The applicant has illustrated two snow storage areas upon the landscaping plan. The applicant should review the need for fencing or signage to



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- ensure snow storage does not occur within the footprint of the infiltration basins.
- e. Former Fuss & O'Neill Comment: Engineering Technical Guideline & Typical Details (ETGTD) ETGTD 910.8. The HydroCAD analysis illustrates that the proposed conditions utilize an infiltration rate of 6.00in/hr. The applicant should provide additional information and/or conversion calculations to support the use of the infiltration rate. Does this rate utilize a factor of safety, does it follow typical current engineering practice as outlined within Env-Wq 1504.14(c), does the soil need to be amended, etc.
 - **Current Fuss & O'Neill Comment:** The applicant has updated the infiltration rate to be 3.0 in/hr, from the previously utilized 6.0 in/hr. The Test Pit information provided within the Stormwater Management and Erosion Control Plan illustrates a perc rate of 2.0 min/inch, which is equivalent to the utilized 3.0 in/hr. The applicant should provide additional information as to why this rate was utilized and a factor of safety is not being accounted for within the calculations.
- g. Former Fuss & O'Neill Comment: ETGTD 920.6. The applicant should provide rip rap outlet calculations within the Stormwater Management Report.
 - **Current Fuss & O'Neill Comment:** The applicant has provided rip rap calculations. The applicant should also provide all outlet protection apron calculations, as illustrated upon Plan Sheet 10 of the Plan Set.
- h. Former Fuss & O'Neill Comment: ETGTD 930.4. We note that the majority of the stormwater design utilizes pipe slopes of less than the required 2.0%. The applicant should review these pipe slopes and provide calculations showing that the drain line velocities are self-cleaning.
 - **Current Fuss & O'Neill Comment:** The applicant has provided information demonstrating that the pipes are self-cleaning within the HydroCAD 2-year report. The applicant should review this with the Town to ensure this is an acceptable variation from the Regulations.

9. Landscaping (HR 275-8.C.(7) & 276-11.1.B.(20)) and Lighting (HR 276-11.1.B.(14))

d. Former Fuss & O'Neill Comment: HR 276-11.1.B.(14). The applicant has noted that the hours of operation for the facility are 6:00 am to 6:00 pm. The applicant should provide additional information regarding whether the lights are intended to be in operation during non-working hours.

Current Fuss & O'Neill Comment: The applicant has stated that the lights will be on a timer and scheduled around the operating hours of the business. We note that applicant should update the business hours on the plan set per the Planning Board meeting on July 28, 2021.

11. Other

d. **New Fuss & O'Neill Comment:** The applicant should update Derry <u>Road</u> plan references to Derry <u>Street</u>.

The following items require Town evaluation or input:

- 1. Site Plan Review Codes (HR 275)
 - a. **Former/Current Fuss & O'Neill Comment:** Hudson Regulation (HR) 275-6.C. The applicant has proposed to realign the existing sidewalk along Derry Street to accommodate



Mr. Brian Groth August 16, 2021 Page 4 of 8

a turning lane and the site entrance. The Town should review a need for an easement for the new sidewalk areas that are out of the Right-of-Way.

The following items are resolved or have no further Fuss & O'Neill input:

1. Site Plan Review Codes (HR 275)

- b. Former Fuss & O'Neill Comment: HR 275-6.I. The scope of this review does not include the adequacy of any fire protection provisions for the proposed building. No fire service connections to the buildings are shown.
 - Current Fuss & O'Neill Comment: The applicant has added a fire service to the plan set. No further Fuss & O'Neill comment.
- c. Former Fuss & O'Neill Comment: HR 275-6.T. The applicant is proposing the construction of a 10 foot wide right turn lane on Derry Street southbound to access the site. We note that no grading was provided for this right turn lane area. The applicant should review and provide spot grades on the plans to ensure positive drainage will exist in this area.
 - Current Fuss & O'Neill Comment: The applicant has added spot grades to the turn lane. No further Fuss & O'Neill comment.
- d. Former Fuss & O'Neill Comment: HR 275-6.T. The applicant has shown a 50 foot long right turn lane with a 50' long 10:1 taper. The applicant should confirm that these turn lane dimensions meet Town standards and the turn lane is long enough to accommodate expected traffic entering the site.
 - Current Fuss & O'Neill Comment: The applicant has increased the lane to an 11 foot width and will be reviewed as part of the traffic review. No further Fuss & O'Neill comment.
- e. Former Fuss & O'Neill Comment: HR 275-8.C.(2) and Zoning Ordinance (ZO) 334-15.A. The applicant has provided parking calculations on the plan set. The applicant has noted that 9 parking are required for the 900 square foot facility and that 9 spaces are provided.
- f. Former Fuss & O'Neill Comment: HR 275-8.C.(2).(c).[5]. The applicant should show the required stacking spaces in the drive thru area. We note that the Regulation requires a minimum of 12 stacking spaces in the drive thru, or a number of stacking spaces determined appropriate by the Planning Board for the use served.
 - Current Fuss & O'Neill Comment: The applicant has noted that the 12 stacking spaces required have been added to the plan set. The plan shows a 240 feet of stacking length for the drive thru location. No further Fuss & O'Neill comment.

2. Administrative Review Codes (HR 276)

- a. Former Fuss & O'Neill Comment: HR 276-11.1.B.(12).(c). The applicant should review and confirm that a 100 foot buffer exists between the residential use to the west of the site and the proposed development of this commercial site.
 - Current Fuss & O'Neill Comment: The applicant has included a waiver application as part of the package received for review. We also note that waiver was approved by the Planning Board on June 28, 2021. No further Fuss & O'Neill comment.



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- b. Former Fuss & O'Neill Comment: HR 276-11.1.B.(13). The applicant has not included details for any proposed site signage other than traffic signs. The applicant has included a note stating that, "All signs are subject to approval by the Hudson Planning Board prior to installation."
- c. Former Fuss & O'Neill Comment: HR 276-11.1.B.(16). The applicant has not provided the locations of all driveways and travel ways within 200 feet of the site.
 - Current Fuss & O'Neill Comment: The applicant has provided the required information. No further Fuss & O'Neill comment.
- d. Former Fuss & O'Neill Comment: HR 276-11.1.B.(23). The applicant has not noted any pertinent highway projects on the plan set.

3. Driveway Review Codes (HR 275-8.B. (34)/Chapter 193)

site. No further Fuss & O'Neill comment.

- a. Former Fuss & O'Neill Comment: HR 193.10.C. The applicant has not provided grading at the driveway connection to Derry Street so we are unable to confirm that the proposed driveway grading conforms to the Regulation and Town standards.
 - Current Fuss & O'Neill Comment: The applicant has added grading to the driveway. No further Fuss & O'Neill comment.
- b. Former Fuss & O'Neill Comment: HR 193.10.E. The applicant has not provided any sight distances for the proposed driveway location on the plan set.
 - Current Fuss & O'Neill Comment: The applicant has added sight distance information to the plan set. No further Fuss & O'Neill comment.
- c. Former Fuss & O'Neill Comment: The driveway layout at the entrance and the parking lot doesn't appear to allow for larger trucks to access the site. The applicant should confirm that these are not anticipated, and review the need for signage to prevent such trucks from attempting to access the site. The applicant should also provide information as to the types of delivery trucks expected to access the site.

 Current Fuss & O'Neill Comment: The applicant has stated that Aroma Joe's sixes trucks according to site constraints. They do not anticipate larger trucks will try to access the
- d. Former Fuss & O'Neill Comment: The applicant has called for vertical granite curb on the plan set and provided a detail for bituminous curb only. The applicant should coordinate the plans and details.

 Current Fuss & O'Neill Comment: The applicant has added the vertical granite curb detail to the plan set. No further Fuss & O'Neill comment.

4. Traffic

- a. Former Fuss & O'Neill Comment: HR 275-9.B. The applicant has not provided any traffic information as part of their review package.
 - Current Fuss & O'Neill Comment: The applicant has submitted a Traffic Impact and Access Study which Fuss & O'Neill will review and provide comments for separately. No further Fuss & O'Neill comment.

5. Utility Design/Conflicts

a. Former Fuss & O'Neill Comment: Engineering Technical Guideline & Typical Details (ETGTD)



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Section 720.8.3. The applicant has not provided a cleanout for the proposed sewer service. This should be located at the property line.

Current Fuss & O'Neill Comment: The applicant has added a sewer manhole to the site next to the property line per the request of the Town Engineer. No further Fuss & O'Neill comment.

b. Former Fuss & O'Neill Comment: The applicant should provide a water/sewer crossing detail for the sewer service crossing the water main in Derry Street, and crossing details for the service piping at the drain line in the driveway.

Current Fuss & O'Neill Comment: The applicant has provided the recommended detail on the plan set. No further Fuss & O'Neill comment.

6. Drainage Design/Stormwater Management (HR 275-9.A./Chapter 290)

- f. Former Fuss & O'Neill Comment: ETGTD 920.4.18. & 920.4.11. The applicant should state on the plan that the responsibility of maintaining the stormwater features are solely the owner's.
 - Current Fuss & O'Neill Comment: The applicant has added a note to the plan set. No further Fuss & O'Neill comment.
- i. Former Fuss & O'Neill Comment: ETGTD 930.12. The applicant should review the use of curb cuts on this private site. Snow storage and snow melt could reduce the effectiveness of this drainage design, leading to unwanted flooding.
 - Current Fuss & O'Neill Comment: The applicant has revised the curb cuts to curb inlets. No further Fuss & O'Neill comment.
- j. Former Fuss & O'Neill Comment: The applicant will be required to comply with all provisions of the Town of Hudson's MS4 permit, including but not limited to annual reporting requirements, construction site stormwater runoff control, and record keeping requirements.
- k. Former Fuss & O'Neill Comment: Please note that this review was carried out in accordance with applicable regulations and standards in place in New Hampshire at this time. Note that conditions at the site, including average weather conditions, patterns and trends, and design storm characteristics, may change in the future. In addition, future changes in federal, state or local laws, rules or regulations, or in generally accepted scientific or industry information concerning environmental, atmospheric and geotechnical conditions and developments may affect the information and conclusions set forth in this review. In no way shall Fuss & O'Neill be liable for any of these changed conditions that may impact the review, regardless of the source of or reason for such changed conditions. Other than as described herein, no other investigation or analysis has been requested by the Client or performed by Fuss & O'Neill in preparing this review.

7. Zoning (ZO 334)

- a. Former Fuss & O'Neill Comment: ZO 334-14.A. The applicant has not provided the proposed building height on the plan set.
 - Current Fuss & O'Neill Comment: The applicant has added the proposed building height on the plan set. No further Fuss & O'Neill comment.
- b. Former Fuss & O'Neill Comment: ZO 334-17 & 334-21. The applicant has noted that the subject parcel is located within the Business (B) zoning district. The proposed use is permitted by the Ordinance within the Business district.
- c. Former Fuss & O'Neill Comment: ZO 334-33. The applicant has not shown any wetlands on the plan set.



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- d. Former Fuss & O'Neill Comment: ZO 334-60. The applicant has not provided any information for any proposed signs on site, except traffic and parking signage. The applicant has noted that signs are subject to Planning Board approval prior to installation.
- e. Former Fuss & O'Neill Comment: ZO 334-83 and HR 218-4.E. The applicant has noted that the site is not located within a designated flood hazard area.

8. Erosion Control/Wetland Impacts

a. Former Fuss & O'Neill Comment: The Town of Hudson should reserve the right to require any additional erosion control measures as needed. The applicant has noted this on the plans.

9. Landscaping (HR 275-8.C.(7) & 276-11.1.B.(20)) and Lighting (HR 276-11.1.B.(14))

- a. Former Fuss & O'Neill Comment: HR 275-8.C.(7). The applicant has met the parking lot landscaping requirements.
- b. Former Fuss & O'Neill Comment: HR 275-8.C.(8). The applicant has provided screening for the residential use to the west by using the existing tree line.
- c. Former Fuss & O'Neill Comment: HR 276-11.1.B.(14). The applicant has shown lighting fixture locations on the plans with details and photometric information.

10. State and Local Permits (HR 275-9.G.)

- a. Former Fuss & O'Neill Comment: HR 275-9.G. The applicant has listed required permits and statuses on the plan set.
- b. Former Fuss & O'Neill Comment: HR 275-9.G. The applicant did not provide copies of any applicable Town, State or Federal approvals or permits in the review package.
- c. Former Fuss & O'Neill Comment: Additional local and state permitting may be required.

11. Other

- a. Former Fuss & O'Neill Comment: The applicant should review the circles shown on sheets 8 and 9. They appear to be a drafting error.
 - Current Fuss & O'Neill Comment: The applicant has corrected the plan. No further Fuss & O'Neill comment.
- b. Former Fuss & O'Neill Comment: The applicant has proposed retaining walls adjacent to the parking lot. The applicant has provided a typical detail for the walls but individual designs were not provided. We note that a portion of this wall appears to be nearly 10 feet tall but specific wall grades are not provided. The applicant should provide detailed design drawings for the proposed wall, stamped by an Engineer licensed in the State of New Hampshire, for Town review prior to construction.
 - Current Fuss & O'Neill Comment: The applicant has stated that a detailed design will be provided prior to construction. No further Fuss & O'Neill comment.
- c. Former Fuss & O'Neill Comment: ETGTD Section 565.1.1. The applicant is reminded of Town of Hudson requirements for the importing of off-site fill materials for use in constructing this project. It is recommended that these requirements be stated on the plans for the Contractors attention.
 - Current Fuss & O'Neill Comment: The applicant has noted this on the plan set. No further Fuss & O'Neill comment.



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Please feel free to call if you have any questions.

Very truly yours,

Steven W. Biglially signed by Steven W. Reichert, PE. DN: cn=Steven W. Reichert, PE, c=US, o=Fus& O'Neil, Inc., ou=Fus& & O'Ne

Steven W. Reichert, P.E.

SWR: Enclosure

cc: Town of Hudson Engineering Division - File

Keach- Nordstrom Associates, Inc. - alewis@keachnordstrom.com

08/25/21, SP #08-21, Attachment	08/25/21.	SP #08-21,	Attachment I
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TRAFFIC IMPACT AND ACCESS STUDY

56 DERRY ROAD Hudson, New Hampshire

July 1, 2021

Prepared for Keach-Nordstrom Associates, Inc.

TRAFFIC-IMPACT AND ACCESS STUDY

56 DERRY ROAD Hudson, New Hampshire

July 1, 2021



Prepared for Keach-Nordstrom Associates, Inc.



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TEPP

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SUMMARY

PROJECT DESCRIPTION

Keach-Nordstrom Associates, Inc. (KNA) has retained TEPP LLC to prepare this traffic impact and access study (TIAS) for a proposed commercial redevelopment in the Town of Hudson, New Hampshire.

The proposed redevelopment will:

- be at 56 Derry Road
- provide one drive-through coffee shop
- have one driveway to the west side of Derry Road, with a one-lane entrance and a two-lane exit

STUDY SCOPE

The TIAS study area includes the following unsignalized intersections:

- Derry Road/Ledge Road
- Derry Road/driveway

This TIAS analyzes the following conditions as applicable:

- 2021 existing
- 2022 and 2032 no-build, with background-traffic growth
- 2022 and 2032 build, with background-traffic growth and the proposed redevelopment

This TIAS analyzes traffic operations for the following hours as applicable:

- weekday AM street-peak hour
- weekday PM street-peak hour

TRIP GENERATION

Total trips appear on the site driveway but not all are added to Derry Road near the site. 2022 total vehicle-trips are:

TEPP

- weekday daily, 629 (total of in and out)
- weekday AM-street-peak hour, 106 (53 in and 53 out)
- weekday PM-street-peak hour, 40 (20 in and 20 out)

2032 total vehicle-trips are:

- weekday daily, 694 (total of in and out)
- weekday AM-street-peak hour, 117 (58 in and 539 out)
- weekday PM-street-peak hour, 44 (22 in and 22 out)

Primary trips are added to Derry Road near the site. 2022 primary vehicle-trips are:

- weekday daily, 69 (total of in and out)
- weekday AM-street-peak hour, 12 (6 in and 6 out)
- weekday PM-street-peak hour, 4 (2 in and 2 out)

2032 primary vehicle-trips are:

- weekday daily, 78 (total of in and out)
- weekday AM-street-peak hour, 13 (6 in and 7 out)
- weekday PM-street-peak hour, 6 (3 in and 3 out)

CAPACITY ANALYSIS

Capacity analysis shows, for the Derry Road/Ledge Road intersection

- low delays for left turns from Derry Road
- moderate delays or delayed operations for movements from Ledge Road
- insignificant project impacts

Capacity analysis shows, for the Derry Road/driveway intersection:

- low delays for left turns from Derry Road
- moderate delays or delayed operations for movements from the driveway

Delayed operations on minor-street approaches to high-volume arterials are typical and acceptable.



TRAFFIC IMPACTS

Analysis indicates no significant area impact due to the proposed redevelopment.

INTRODUCTION

PROJECT DESCRIPTION

KNA has retained TEPP LLC to prepare this TIAS for a proposed commercial redevelopment in the Town of Hudson, New Hampshire.

The proposed redevelopment will:

- be at 56 Derry Road
- provide one drive-through coffee shop
- have one driveway to the west side of Derry Road, with a one-lane entrance and a two-lane exit

Figure 1 shows site location. The project plan is in Appendix A.

STUDY APPROACH

This TIAS assesses traffic impacts and access for the proposed redevelopment.

The TIAS study area includes the following unsignalized intersections:

- Derry Road/Ledge Road
- Derry Road/driveway

This TIAS analyzes the following conditions as applicable:

- 2021 existing
- 2022 and 2032 no-build, with background-traffic growth
- 2022 and 2032 build, with background-traffic growth and the proposed redevelopment

This TIAS analyzes traffic operations for the following hours as applicable:

- weekday AM street-peak hour
- weekday PM street-peak hour

Differences in traffic operations between the no-build and build conditions approximate traffic impacts of the proposed redevelopment.



Figure 1. Site location.

EXISTING CONDITIONS

INTRODUCTION

Existing conditions include:

- physical conditions of the transportation network, roads, and intersections
- traffic volumes
- other relevant information

PHYSICAL CONDITIONS

INTRODUCTION

Figure 1 shows the transportation network.

The TIAS study area includes the following existing unsignalized intersection: Derry Road/Ledge Road.

Description of the TIAS study area follows.

DERRY ROAD

Derry Road:

- is oriented approximately north-south
- functions as an arterial street
- is also known as New Hampshire Routes (NH) 3A and 102
- to the south, connects with the Town Center and New Hampshire Route 111 (NH 111), an arterial highway that leads to the City of Nashua and Towns of Windham and Salem
- to the north, connects with NH 102, an arterial highway that leads to the Towns of Londonderry and Derry, and NH 3A, an arterial highway that leads to the Town of Litchfield and the City of Manchester
- has a horizontal alignment includes minor to moderate horizontal curvature, but is essentially tangent at the proposed driveway location
- has a near-level vertical alignment

TEPP

- has a three-lane cross-section with one travel lane per direction, a center-two-way-left-turn lane (TWLTL), and paved shoulders
- has asphaltic-cement concrete (ACC) pavement in overall good condition
- has curb and sidewalk along both sides
- includes utility poles along the west side, some with luminaires
- has a posted speed limit of 30 miles per hour (mph)
- has nearby commercial and residential development
- is under the jurisdiction of the Town

DERRY ROAD/LEDGE ROAD INTERSECTION

The intersection:

- is three legged
- has Derry Road as the major north-south street
- has Ledge Road as the minor east leg
- on Derry Road, has one travel lane per direction and one center TWLTL
- on the Ledge Road approach, has one lane
- has a STOP sign on the Ledge Road approach
- is illuminated
- has commercial and residential development nearby

TRAFFIC VOLUMES

TRAFFIC COUNTS

TEPP LLC obtained an automatic traffic counter (ATR) count:

- on Derry Road along the site frontage
- from Wednesday, June 2, to Thursday, June 3, 2021

The ATR data are in Appendix B.

ADJUSTMENTS

The June 2021 traffic counts were adjusted to reflect peak-month and non-pandemic conditions.

The increase to peak month was 2.0 percent, based on based on NHDOT 2019 monthly volumes for Group 4 (Urban Highways) averages in Appendix C,

The increase to pre-pandemic was 5.6 percent. NHDOT continuous count station 82229031, on Daniel Webster Highway north of Hilton Drive, in the Town of Merrimack showed May 2021 two-way average-daily traffic (ADT) of 15,404 vehicles. The station showed May 2019 pre-pandemic two-way ADT of 16,260 vehicles, which is 5.6 percent greater.

The combined increase was 7.7 percent.

RESULTS

Table 1 and Figure 2 show 2021 existing traffic volumes.

Table 1. 2021 existing traffic v	olumes.			
Location and Time Period	Vehicles ^a	K-factor ^b	Percent Direction	
Derry Road near Site Frontage				
Weekday Daily	28,667			
Weekday AM-Street-Peak Hour	2,157	7.5	58 Southbound	
Weekday PM-Street-Peak Hour	2,290	8.0	54 Northbound	

^a Two-way-total volumes.

Derry Road near the site frontage showed about:

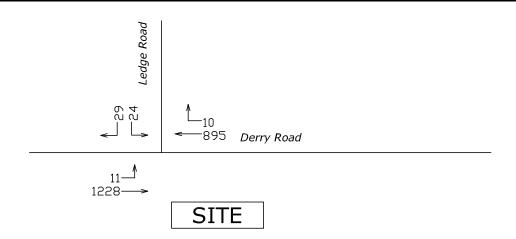
- 28,667 weekday-daily vehicles
- 2,157 vehicles during the weekday AM street-peak hour, predominantly southbound
- 2,290 vehicles during the weekday PM street-peak hour, predominantly northbound

VEHICLE SPEEDS

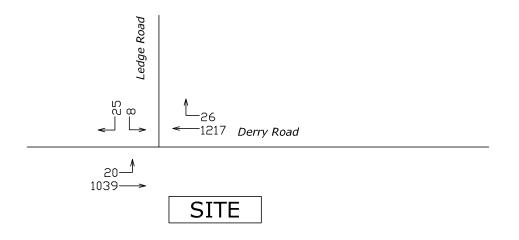
The ATR collected vehicle speeds:

- on Derry Road along the site frontage
- from Wednesday, June 2, to Thursday, June 3, 2021

b K = hour volume as a percent of daily volume.



Weekday AM-Street-Peak Hour



Not to Scale

Weekday PM-Street-Peak Hour

Figure 2. 2021 existing traffic volumes.

The data are in Appendix D and are summarized in Table 2.

Table 2 indicates that on Derry Road:

Table 2. Vehicle spee	ds.					
		Speeds (mp	h)			
Location and Direction	Speed Limit	Meana	85 th Percentile ^a			
Derry Road along Site Frontage						
Northbound	30	35.3	39.0			
Southbound	30	33.6	37.2			

^a From ATR conducted from Wednesday, June 2, to Thursday, June 3, 2021.

- the posted speed limit was 30 mph
- the northbound the mean speed was 35.3 mph and the 85th percentile speed was 39.0 mph
- for southbound the mean speed was 33.6 mph and the 85th percentile speed was 37.2 mph

SIGHT DISTANCES

The American Association of State Highway and Transportation Officials (AASHTO) has established authoritative policy for sight distances at unsignalized intersections¹ in terms of:

- stopping sight distance (SSD)
- optional intersection sight distance (ISD)

SSD: 2

- provides for safety
- enables a driver, on the major road, to perceive and react accordingly to a vehicle entering the major road from a minor road
- is conservative because it encompasses a wide range of brake-reaction times and deceleration rates

¹ AASHTO, A Policy on Geometric Design of Highways and Streets, 6th Edition (Washington, DC, 2011), pages 9-28 to 9-29.

² AASHTO, pages 3-2 to 3-6.



Optional ISD:³

- is ordinarily greater than SSD and may enhance traffic operations
- is not required for safety

Table 3 shows relevant available sight distances that are at least 400 ft, per NHDOT practice, and are adequate.

Table 3. Sight distances.						
Intersection, Movements, and View	Available Sight Distance (ft) ^a	Speeds (miles per hour)				
		Limit	SSD Provides For	ISD Provides For		
Portland Street/Proposed Road for Proposed Road Movements						
Portland Street to/from South	400	30	45+	36+		
Portland Street to/from North	400	30	45+	36+		

^a With appropriate roadside and vegetation maintenance.

-

³ AASHTO, pages 9-22 to 9-55.

FUTURE CONDITIONS

INTRODUCTION

Future conditions include:

- planned road improvements independent of the proposed redevelopment
- future no-build traffic volumes, with background-traffic growth and without the proposed redevelopment
- future build traffic volumes, with background-traffic growth and with the proposed redevelopment

PLANNED ROAD IMPROVEMENTS

TEPP LLC identified no significant planned road improvement in the study area independent of the project.

BACKGROUND-TRAFFIC GROWTH

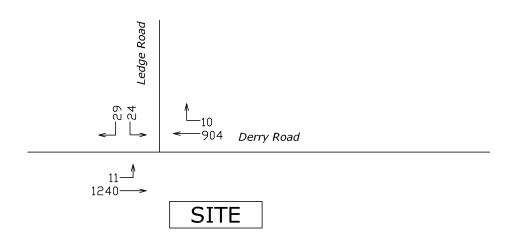
Background-traffic growth:

- is independent of the proposed redevelopment
- is related to land development in the immediate area, population and economic development in the region, and changes in travel patterns in the region
- typically considers two factors: a general traffic-growth rate and specific planned land developments in the immediate area

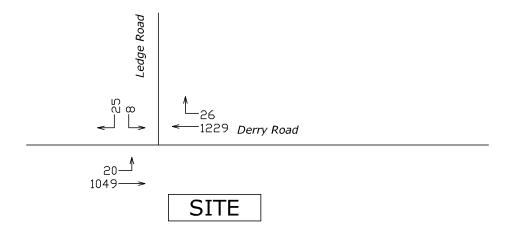
This TIAS uses a 1.0-percent annual growth rate. This yields about 11.6-percent growth between 2021 and 2032.

NO-BUILD TRAFFIC VOLUMES

The background-traffic growth described above was applied to 2021 existing traffic volumes. Figures 3 and 4 show 2022 and 2032 no-build traffic volumes.



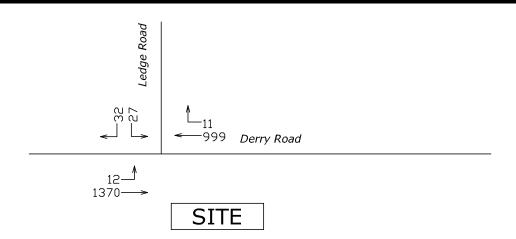
Weekday AM-Street-Peak Hour



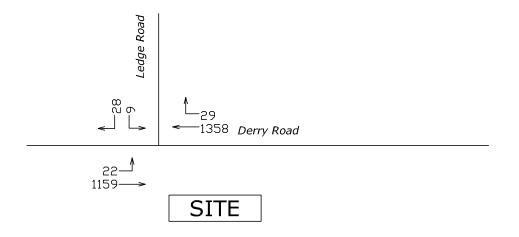
Not to Scale

Weekday PM-Street-Peak Hour

Figure 3. 2022 no-build traffic volumes.



Weekday AM-Street-Peak Hour



Not to Scale

Weekday PM-Street-Peak Hour

Figure 4. 2032 no-build traffic volumes.



TRIP GENERATION

BASIC TRIP GENERATION

The Institute of Transportation Engineers (ITE) compiles and publishes trip-generation information for a variety of land uses in *Trip Generation Manual*.⁴ This guide for estimating site traffic includes coffee/donut shop with drive-through window and no indoor seating, land use 938, based on floor area.⁵ However, this information is based on sites with floor areas of 90 square feet (sf) and is not applicable to the proposed land use, with a floor area of about 900 sf.

Stephen G. Pernaw & Company, Inc. has published appropriate and applicable trip-generation information specific to this land use, which estimates trip generation based on traffic volumes passing the site.⁶ Basic trip generation is based on this information.

TRIP TYPES

Total trips appear on site driveways but not all are added to roads near the site. Accordingly, ITE compiles information on three trip types, based on empirical data for many land uses, in the authoritative Hooper, *Trip Generation Handbook*.⁷ These three trip types are:

- primary trips that are added to the area and are primarily for visiting the site
- diverted trips that not added to the general area; these trips are from existing traffic on roads near the site
- pass-by trips that are not added to the general area; these trips are from existing traffic passing the site⁸

RESULTS

Table 4 shows calculated weekday vehicle-trip generation for the site.

⁴ ITE, *Trip Generation Manual*, 10th edition (Washington DC, September 2017).

⁵ ITE, *Trip Generation Manual*, V Volume 2, Data, Services (Land Uses 900-999), pages 250 and 251, pages 249 to 254.

⁶ Stephen G. Pernaw & Company, Inc., *Traffic Impact Assessment, Proposed Drive-Thru Coffee Shop, Northwood, New Hampshire* (Concord, New Hampshire, October 2019), page 10 and Appendix E.

⁷ Kevin G. Hooper, P.E., Principal Editor, *Trip Generation Handbook*, 3rd edition (Washington DC: Institute of Transportation Engineers, September 2017).

⁸ Definitions of primary trips, diverted trips, and pass-by trips are in Hooper, page 93. Relevant data on primary trips, diverted trips and pass-by trips are in Hooper, 3rd edition, page 216.

		AM-	Street-Peak 1	Hour	PM-Street-Peak Hour					
	Daily ^a	Total ^b	In	Out	Total ^c	In	Out			
2022 Vehicle-Tr	rips									
Primary	69	12	6	6	4	2	2			
Pass-By ^d	<u>560</u>	<u>94</u>	<u>47</u>	<u>47</u>	<u>36</u>	<u>18</u>	<u>18</u>			
Total	629	106	53	53	40	20	20			
2032 Vehicle-Tr	rips									
Primary	78	13	6	7	6	3	3			
Pass-By ^d	<u>616</u>	<u>104</u>	<u>52</u>	<u>52</u>	<u>38</u>	<u>19</u>	<u>19</u>			
Total	694	117	58	59	44	22	22			

^a Estimated total weekday daily trips are 5.93 times weekday AM-street-peak hour trips, based on ITE, *Trip Generation Manual*, Volume 2, Data, Services (Land Uses 900-999), pages 250 and 251.

Total trips appear on the site driveway but not all are added to Derry Road near the site. 2022 total vehicle-trips are:

- weekday daily, 629 (total of in and out)
- weekday AM-street-peak hour, 106 (53 in and 53 out)
- weekday PM-street-peak hour, 40 (20 in and 20 out)

2032 total vehicle-trips are:

- weekday daily, 694 (total of in and out)
- weekday AM-street-peak hour, 117 (58 in and 539 out)
- weekday PM-street-peak hour, 44 (22 in and 22 out)

Primary trips are added to Derry Road near the site. 2022 primary vehicle-trips are:

- weekday daily, 69 (total of in and out)
- weekday AM-street-peak hour, 12 (6 in and 6 out)

^b Total weekday AM-street-peak hour trips are 0.0488 times 2021 no-build weekday AM-street-peak hour volume on Derry Road along the site frontage. Stephen G. Pernaw & Company, Inc., Appendix E.

^c Total weekday PM-street-peak hour trips are 0.0172 times 2021 no-build weekday PM-street-peak hour volume on Derry Road along the site frontage. Stephen G. Pernaw & Company, Inc., Appendix E.

d Pass-by trip percentage is 89. Based on Hooper, *Trip Generation Handbook*, 3rd edition, page 216, coffee/donut shop with drive-through window and no indoor seating, land use 938.

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• weekday PM-street-peak hour, 4 (2 in and 2 out)

2032 primary vehicle-trips are:

- weekday daily, 78 (total of in and out)
- weekday AM-street-peak hour, 13 (6 in and 7 out)
- weekday PM-street-peak hour, 6 (3 in and 3 out)

TRIP DISTRIBUTION AND NETWORK ASSIGNMENT

Trip distribution and network assignment of vehicle-trips to and from the site may consider such factors as existing site distribution, travel patterns, population, regional land development, and site access. Trip distribution and network assignment for this TIAS considered the 2021 existing volumes.

Table 5 shows trip distribution and network assignment for primary trips. Pass-by trips were assigned reflecting peak-hour directional distributions on Derry Road: 58-percent southbound for the weekday AM-street-peak hour and 54-percent northbound for the weekday PM-street-peak hour. Figures 5 and 6 show site traffic volumes.

Table 5. Trip distribution and netw	vork assignment.
Road and Direction (To/From)	Approximate Percent
Derry Road to/from South	45
Derry Road to/from South	<u>55</u>
Total	100

BUILD TRAFFIC VOLUMES

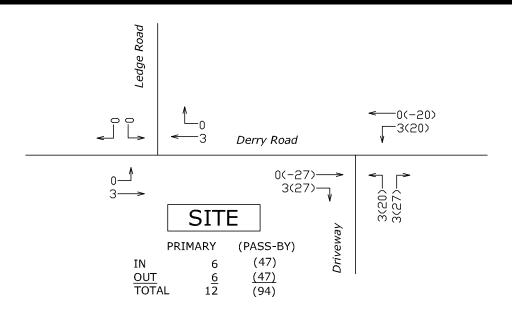
Site traffic volumes were superimposed on the no-build traffic volumes to estimate build traffic volumes. Figures 7 and 8 show the resulting 2022 and 2032 build traffic volumes.

TRAFFIC-VOLUME CHANGES

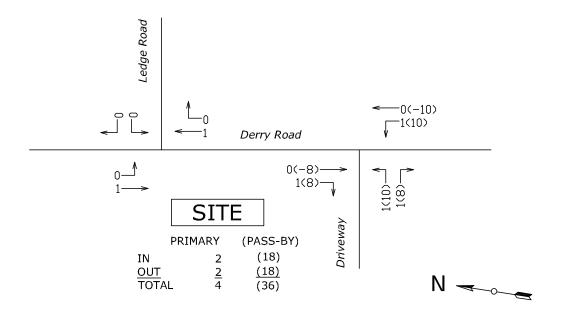
Table 6 presents calculated traffic-volume changes due to the proposed redevelopment for the:

- weekday AM-street-peak hour
- weekday PM-street-peak hour

Not to Scale

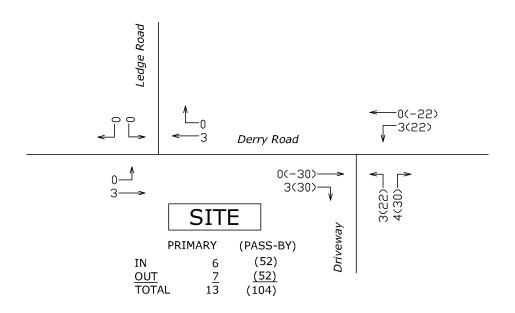


Weekday AM-Street-Peak Hour

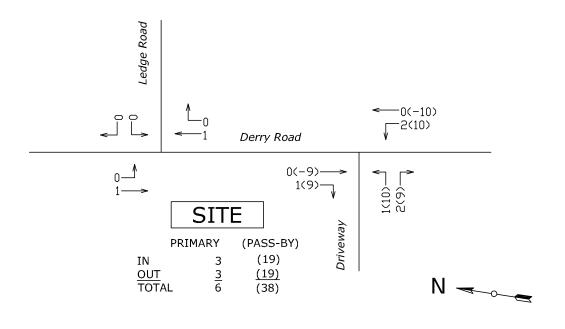


Weekday PM-Street-Peak Hour

Figure 5. 2022 site traffic volumes.



Weekday AM-Street-Peak Hour

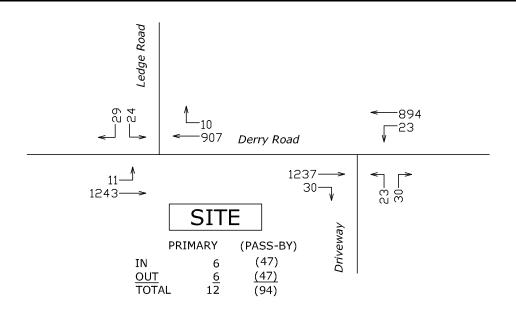


Not to Scale

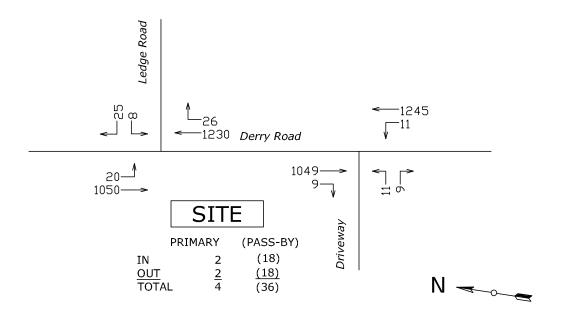
Weekday PM-Street-Peak Hour

Figure 6. 2032 site traffic volumes.

Not to Scale

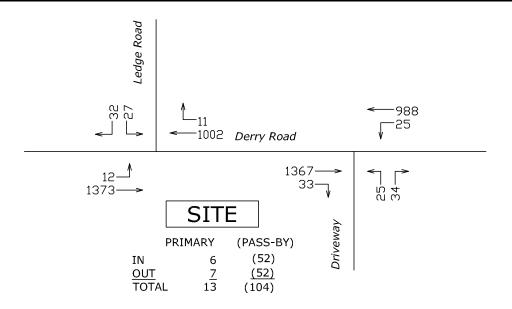


Weekday AM-Street-Peak Hour

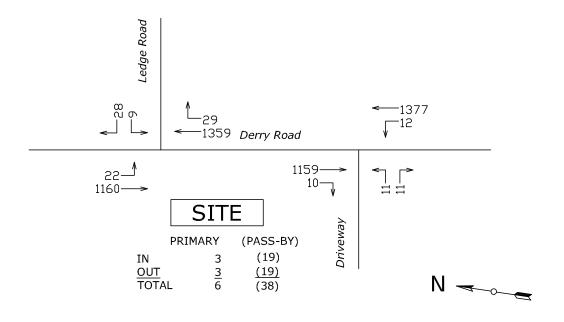


Weekday PM-Street-Peak Hour

Figure 7. 2022 build traffic volumes.



Weekday AM-Street-Peak Hour



Not to Scale

Weekday PM-Street-Peak Hour

Figure 8. 2032 build traffic volumes.

Table 6. Traffic-volume char	iges.							
	2022 Traf	fic Volume	es (vph)a	2032 Traffic Volumes (vph)				
Location and Time Period	No-Build	Build	Change	No-Build	Build	Change		
Derry Road North of Driveway								
Weekday AM-Street-Peak Hour	2,178	2,184	6	2,407	2.413	6		
Weekday PM-Street-Peak Hour	2,312	2,314	2	2,555	2,557	2		
Derry Road South of Driveway								
Weekday AM-Street-Peak Hour	2,178	2,184	6	2,407	2,414	7		
Weekday PM-Street-Peak Hour	2,312	2,314	2	2,555	2,559	4		

^a Two-way total volumes.

Table 6 shows peak-hour-traffic-volume increases:

- of 2 to 7 vehicle-trips
- constituting averages about one vehicle-trip per 8 to 30 minutes
- that are further split by northbound and southbound direction on Derry Road



CAPACITY ANALYSIS

INTRODUCTION

This TIAS has quantified existing, future-no-build and future-build traffic volumes. Capacity analysis models the quality of traffic operations. Comparing build conditions to the no-build conditions indicates impacts of the proposed redevelopment on quality of traffic operations.

METHODS

Capacity analysis calculates LOS for transportation facilities. LOS indicates the quality of traffic operations based on delay and other measures. The six LOS are designated A to F. LOS A represents the best or highest operating conditions. LOS F is the lowest, but does not necessarily connote failure.

LOS is a function of traffic volumes and traffic control. Because these volumes can vary, LOS of a transportation facility can differ by time of day, day of the week, or month. For example, a transportation facility with a low LOS during peak hours may have a high LOS during other hours. The operational analysis methods of the Transportation Research Board (TRB)⁹ models LOS for intersections based on calculated delay per vehicle, as shown in Table 7. Synchro analysis software was used.

Method inputs include:

- intersection geometry
- traffic control, such as YIELD sign, two-way STOP sign, all-way STOP sign, roundabout, or signal (including phasing, timing, and progression)
- traffic volumes
- vehicle composition, such as passenger cars and trucks

The methods are all approximate. In particular, the method for two-way STOP-sign control can be conservative, with observed delays and queuing shorter than those modeled.

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⁹ TRB, *Highway Capacity Manual 2000* (Washington DC 2000) and *Highway Capacity Manual 2010* (Washington DC, 2010).

Table 7. Level-of-service criteria for intersections.

	Control Delay (see	conds/vehicle)
Level of Service	Unsignalized Intersections ^a	Signalized Intersections
A	≤10.0	≤10.0
В	>10.0 and ≤ 15.0	$>$ 10.0 and \le 20.0
C	>15.0 and ≤ 25.0	$>$ 20.0 and \le 35.0
D	>25.0 and ≤ 35.0	$>$ 35.0 and \leq 55.0
E	$>$ 35.0 and \leq 50.0	>55.0 and ≤ 80.0
F	>50	>80

From Transportation Research Board, Highway Capacity Manual 2010 (Washington D.C., 2010).

RESULTS

Table 8 shows computed LOS, delays, and queues at study-area intersections for the:

- weekday AM-street-peak hour
- weekday PM-street-peak hour

The analysis is under the following conditions, as applicable:

- 2021 existing
- 2022 and 2032 no build
- 2022 and 2032 build

Capacity-analysis worksheets that give detail and explanation are in Appendix E.

Table 8 shows, for the Derry Road/Ledge Road intersection

- low delays for left turns from Derry Road
- moderate delays or delayed operations for movements from Ledge Road
- insignificant project impacts

Table 8 shows, for the Derry Road/driveway intersection:

^a For YIELD sign, two-way STOP sign or all-way STOP sign, control delay defines LOS. For roundabout approaches and overall intersection, control delay defines LOS. For roundabout lanes with volume/capacity ratio ≤1.0, control delay defines LOS. For roundabout lanes with volume/capacity ratio > 1.0, LOS is F regardless of control delay.

TEPP

- low delays for left turns from Derry Road
- moderate delays or delayed operations for movements from the driveway

Delayed operations on minor-street approaches to high-volume arterials are typical and acceptable.

Table 8. Capacity-a	analysis sı	ummary.																		
Interspection Control		2021 E	xisting		2022 No Build				2032 No Build				2022	Build			2032 N	No Build		
Intersection, Control, Hour and Movement	LOSa	Delayb	V/C ^c	Queued	LOS	Delay	V/C	Queue	LOS	Delay	V/C	Queue	LOS	Delay	V/C	Queue	LOS	Delay	V/C	Queue
Derry Road/Ledge Road	Intersection	, Unsignalize	d, Weekda	y AM-Street-	Peak Hour															
Derry Road SB L	В	10.7	0.019	0.1	В	10.9	0.020	0.1	В	11.6	0.024	0.1	В	11.0	0.020	0.1	В	11.6	0.024	0.1
Ledge Road WB LR	D	33.3	0.370	1.6	D	31.8	0.307	1.2	E	41.2	0.402	1.8	E	35.2	0.333	1.4	E	41.6	0.405	1.8
Derry Road/Ledge Road	Intersection	, Unsignalize	d, Weekda	y PM-Street-I	Peak Hour															
Derry Road SB L	В	11.7	0.037	0.1	В	12.6	0.045	0.1	В	13.7	0.056	0.2	В	12.6	0.045	0.1	В	13.7	0.056	0.2
Ledge Road WB LR	D	29.6	0.250	1.0	D	33.4	0.225	0.8	E	42.9	0.305	1.2	D	33.6	0.226	0.8	E	43.3	0.307	1.2
Derry Road/Driveway Int	ersection, U	Jnsignalized,	Weekday A	AM-Street-Pe	ak Hour															
Derry Road NB L													В	12.9	0.053	0.2	В	14.1	0.2	0.066
Driveway EB L													E	37.5	0.188	0.7	E	46.9	0.246	0.9
Driveway EB R													D	30.5	0.192	0.7	E	39.3	0.266	1.0
Derry Road/Driveway Int	tersection, U	Jnsignalized,	Weekday l	PM-Street-Pea	ak Hour															
Derry Road NB L													В	11.2	0.021	0.1	В	11.9	0.025	0.1
Driveway EB L													E	36.1	0.095	0.3	E	42.5	0.113	0.4
Driveway EB R													C	21.0	0.043	0.1	C	24.3	0.061	0.2

a LOS = level of service.

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b Delay = average delay in seconds per vehicle.

^c V/C = volume/capacity ratio.

d 95th percentile queue in vehicles.

EB = eastbound, WB = westbound, SB = southbound, NB = northbound, L = left, T = through, R = right.

CONCLUSION

PROJECT DESCRIPTION

The proposed redevelopment will:

- be at 56 Derry Road
- provide one drive-through coffee shop
- have one driveway to the west side of Derry Road, with a one-lane entrance and a two-lane exit

TRIP GENERATION

Total trips appear on the site driveway but not all are added to Derry Road near the site. 2022 total vehicle-trips are:

- weekday daily, 629 (total of in and out)
- weekday AM-street-peak hour, 106 (53 in and 53 out)
- weekday PM-street-peak hour, 40 (20 in and 20 out)

2032 total vehicle-trips are:

- weekday daily, 694 (total of in and out)
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Primary trips are added to Derry Road near the site. 2022 primary vehicle-trips are:

- weekday daily, 69 (total of in and out)
- weekday AM-street-peak hour, 12 (6 in and 6 out)
- weekday PM-street-peak hour, 4 (2 in and 2 out)

2032 primary vehicle-trips are:

- weekday daily, 78 (total of in and out)
- weekday AM-street-peak hour, 13 (6 in and 7 out)



• weekday PM-street-peak hour, 6 (3 in and 3 out)

CAPACITY ANALYSIS

Capacity analysis shows, for the Derry Road/Ledge Road intersection

- low delays for left turns from Derry Road
- moderate delays or delayed operations for movements from Ledge Road
- insignificant project impacts

Capacity analysis shows, for the Derry Road/driveway intersection:

- low delays for left turns from Derry Road
- moderate delays or delayed operations for movements from the driveway

Delayed operations on minor-street approaches to high-volume arterials are typical and acceptable.

TRAFFIC IMPACTS

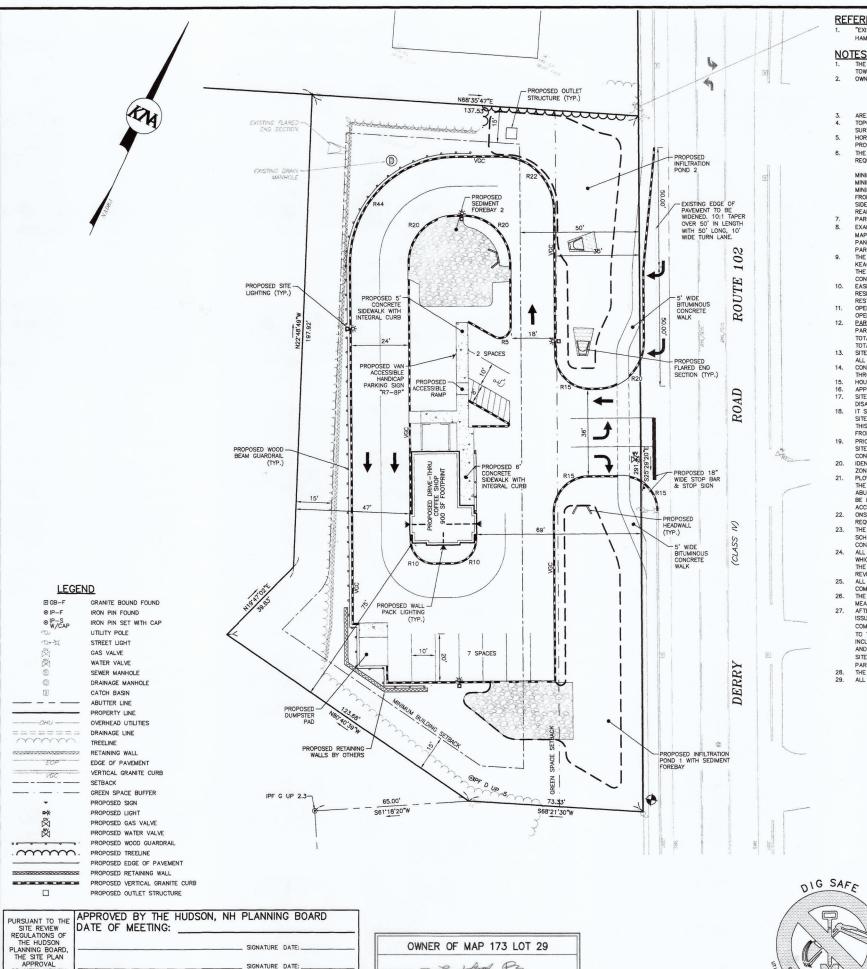
Analysis indicates no significant area impact due to the proposed redevelopment.



APPENDIX



Appendix A: Project Plan



"EXISTING CONDITIONS PLAN, AROMA JOE"S, MAP 173 LOT 29, 56 DERRY STREET, HUDSON, NEW HAMPSHIRE, DATED MAY 7, 2021," PREPARED BY KEACH—NORDSTROM ASSOCIATES, INC.

NOTES:

1. THE PURPOSE OF THIS PLAN IS TO DEPICT THE PROPOSED SITE LAYOUT ON MAP 173 LOT 29 IN THE TOWN OF HUDSON, NEW HAMPSHIRE AS SHOWN HEREON.

TOWN OF HUDSON, NEW HAMPSHIRE AS SHOWN HEREON.

OWNER OF RECORD:

STEVE S. & HSIANG HWA W. PAN

13 KING HENRY DRIVE.

LONDONDERRY, N.H. 03053

BK. 6281 PG, 776

AREA OF SUBJECT PARCEL = 40,793 SF, OR 0.935 ACRES

TOPOGRAPHIC AND BOUNDARY INFORMATION SHOWN HEREON ARE BASED ON AN ACTUAL FIELD

SURVEY MADE BY THIS OFFICE IN APRIL OF 2021.

HORIZONTAL DATUM IS NADB3. VERTICAL DATUM IS NOVD29 FROM GPS SURVEY METHODS POST

PROCESSED THROUGH NOAM-OPUS.

THE SUBJECT PARCEL IS JOCATED WITHIN THE BILISHESS (B) ZONING DISTRICT. DIMENSIONAL

PROCESSED THROUGH NOAA-OPUS.
THE SUBJECT PARCEL IS LOCATED WITHIN THE BUSINESS (B) ZONING DISTRICT. DIMENSIONAL REQUIREMENTS ARE AS FOLLOWS FOR LOTS SERVICED WITH MUNICIPAL SEWER AND WATER:

REQUIRED. PROPOSED.
MINIMUM LOT AREA 30,000 SF 40,793 SF MINIMUM LOT FRONTAGE 150 FT 291.47 FT MINIMUM LOT AREA MINIMUM LOT FRONTAGE MINIMUM BUILDING SETBACKS:

PARCEL WILL BE SERVICED BY MUNICIPAL WATER & SEWER.

PARCEL WILL BE SERVICED BY MUNICIPAL WAILER & SEWER.

EXAMINATION OF THE FEDERAL EMERGENCY MANAGEMENT AGENCY (FEMA) FLOOD INSURANCE RATE

MAP (FIRM) FOR THE TOWN OF MERRIMACK N.H., HILLSBOROUGH COUNTY, MAP NUMBER 33011C0514E,

PANEL 501 OF 701, EFFECTIVE DATE: APRIL 18, 2011 INDICATES THAT NO PORTION OF THE SUBJECT

PARCEL IS LOCATED WITHIN A DESIGNATED FLOOD HAZARD AREA.

THE LOCATION OF ANY UNDERGROUND UTILITY INFORMATION SHOWN HEREON IS APPROXIMATE.

KEACH-NORDSTROM ASSOCIATES, INC. MAKES NO CLAIM TO THE ACCURACY OR COMPLETENESS OF

THE UTILITIES SHOWN, PRIOR TO ANY EXCAVATION ON SITE THE CONTRACTOR OR OWNER SHALL

CONTACT DIG-SAFE AT 811.

EASEMENTS, RIGHTS AND RESTRICTIONS SHOWN OR IDENTIFIED HEREON ARE THOSE FOUND DURING EASEMENTS, RIGHTS AND RESTRICTIONS SHOWN OR IDENTIFIED HEREON ARE THOSE FOUND DURING RESEARCH AT THE HILLSBOROUGH COUNTY REGISTRY OF DEEDS. OTHER EASEMENTS, RIGHTS AND RESTRICTIONS MAY EXIST WHICH A TITLE EXAMINATION OF THE SUBJECT PREMISES MAY DETERMINE. OPEN SPACE REQUIRED = 40% OPEN SPACE PROPOSED = 58.6% PARKING CACULATIONS.

PARKING ACQUIATIONS.

PARKING FROURED = 1 SPACE 7100 SF X 900 SF = 9 SPACES

TOTAL PARKING REQUIRED = 9 SPACES TOTAL PARKING PROVIDED = 9 SPACES

TOTAL PARKING PROVIDED = 9 SPACES
SITE LIGHTING SHALL BE AS SHOWN ON THE PLAN, DIRECTED ONTO SITE, AND SHALL CONFORM WITH
ALL APPLICABLE TOWN OF HUDSON ZONING REGULATIONS.
CONSTRUCTION ACTIVITIES SHALL BE LIMITED TO THE HOURS OF 7:00 AM & 7:00 PM, MONDAY
THROUGH SATURDAY. NO EXTERIOR CONSTRUCTION ACTIVITIES SHALL OCCUR ON SUNDAY.
HOURS OF OPERATION: 6:00 AM TO 6:00 PM, MONDAY THROUGH SATURDAY.
APPROVAL OF THIS PLAN SHALL BE SUBJECT TO FINAL ENGINEERING REVIEW.
SITE IMPROVEMENTS DEPICTED ON THE PLAN SHALL CONFORM WITH TITLE III OF THE AMERICANS WITH
DISABILITIES ACT WITH REGARD TO DIMENSION AND GRADE.
IT SHALL BE UNLAWFUL TO MODIFY, CHANGE, OR ALTER ANY STRUCTURE OR USE SHOWN ON THIS
SITE PLAN IN ANYWAY WHATSOCYER, OR CONVERT OR ALTER ANY STRUCTURE OR USE SHOWN ON
THIS SITE PLAN, OR CHANGE THE ABOVE USE INDICATED ON THE PLAN WITHOUT RECRIVING APPROVAL
FROM THE TOWN OF HUDSON PLANNING BOARD.
PRIOR TO THE ISSUANCE OF A FINAL CERTIFICATE OF OCCUPANCY, AN LLLS. CERTIFIED "AS-PRIJII T"

FROM THE TOWN OF HUDSON PLANNING BOARD.
PRIOR TO THE ISSUANCE OF A FINAL CERTIFICATE OF OCCUPANCY, AN LLLS. CERTIFIED "AS-BUILT"
SITE PLAN SHALL BE PROVIDED TO THE TOWN OF HUDSON COMMUNITY DEVELOPMENT DEPARTMENT
CONFIRMING THAT THE SITE CONFORMS WITH THE PLANNING BOARD APPROVED SITE PLAN.
IDENTRICATION SIGNAGE SHALL NOT BE ERECTED UNTIL APPROVED BY THE BUILDING INSPECTOR AND
ZONING ADMINISTRATOR.
PLOWED SOWD FROM THE FACILITIES, DRIVEWAY, PARKING LOTS AND SUBEWALK SHALL BE STORED IN
THE DESIGNATED AREAS SHOWN IN THIS PLAN SET. NO SNOW MAY BE PLOWED OR STORED ON THE
ABUTTING PARCELS. WHEN THE SNOW STORAGE AREAS ARE AT CAPACITY, SUBSEQUENT SNOW SHALL
BE HAULED OFF-SITE AND DISPOSED OF IN AN ENVIRONMENTALLY SOUND FASHION AND IN
ACCORDANCE WITH ALL LOCAL, STATE AND FEDERAL REGULATIONS.

ONSITE DRAINAGE SYSTEM SHALL BE CONSTRUCTED AND MAINTAINED IN COMPLIANCE WITH NHDES REQUIREMENTS FOR SUCH SYSTEMS.

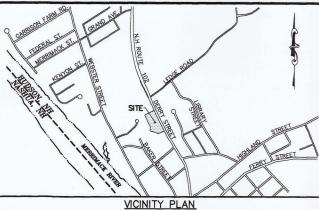
THE APPLICANT'S ENGINEER AND OR CONTRACTOR SHALL CONTACT THE TOWN OF HUDSON TO CHEDULE A PRE-CONSTRUCTION MEETING, WHICH WILL BE HELD WITH STAFF PRIOR TO STARTING

SCHEDULE A PRE-CONSTRUCTION MEETING, WHICH WILL BE HELD WITH STAFF PRIOR TO STARTING CONSTRUCTION.

ALL STIPULATIONS OF APPROVAL SHALL BE INCORPORATED INTO THE DEVELOPMENT AGREEMENT, WHICH SHALL BE RECORDED AT THE HILLSBOROUGH COUNTY REGISTRY OF DEEDS, TOGETHER WITH THE SITE PLAN-OF-RECORD AND ALL AGREED UPON EASEMENT DEEDS, WHICH SHALL BE FAVORABLY REVIEWED BY TOWN COUNSEL PRIOR TO PLANNING BOARD ENDORSEMENT OF PLAN.

ALL IMPROVEMENTS SHOWN ON THE SITE PLAN-OF-RECORD, INCLUDING NOTES 1-29, SHALL BE COMPLETED IN THEIR ENTIRETY AND AT THE EXPENSE OF THE APPLICANT OR HIS ASSIGNS. THE TOWN OF HUDSON SHALL RESERVE THE RIGHT TO REQUIRE ADDITIONAL EROSION CONTROL MEASURES DURING CONSTRUCTION.

MEASURES DURING CONSTRUCTION.
AFTER ISSUANCE OF THE FOUNDATION PERMIT FOR THE PROPOSED BUILDING, AND PRIOR TO THE ISSUANCE OF THE FRAMING PERMIT THEREOF, THE APPLICANT SHALL SUBMIT TO THE HUDSON COMMUNITY DEVELOPMENT DEPARTMENT A FOUNDATION "AS-BUILT" PLAN ON A TRANSPARENCY AND TO THE SAME SCALE AS THE APPROVED SITE PLAN. THE FOUNDATION "AS-BUILT" PLAN SHALL INCLUDE ALL STRUCTURAL DIMENSIONS AND LOT LINE SETBACK MEASUREMENTS TO THE FOUNDATION AND BE STAMPED BY A LICENSED LAND SURVEYOR. ANY DISCREPANCY BETWEEN THE APPROVED AND BE STAMPLED BY A DICEMBED LAND SOMETON. ANY DISCREMENT BETWEEN THE APPRICANT AND BE SITE PLAN AND FOUNDATION "AS-BUILT" PLANS SHALL BE DOCUMENTED BY THE APPLICANT AND BE PART OF THE FOUNDATION "AS-BUILT" SUBMISSION. THE PROPOSED PROJECT HAS BEEN DESIGNED TO MEET 2018 MS4 REQUIREMENTS. ALL SIGNS ARE SUBJECT TO APPROVAL BY THE HUDSON PLANNING BOARD PRIOR TO INSTALLATION.



NON RESIDENTIAL SITE PLAN

AROMA JOE'S

MAP 173 LOT 29 56 DERRY STREET HUDSON, NEW HAMPSHIRE HILLSBOROUGH COUNTY

OWNER OF RECORD:

STEVE S. & HSIANG HWA W. PAN 13 KING HENRY DRIVE LONDONDERRY, N.H. 03053 BK. 6281 PG. 776

APPLICANT: SCOTT ZIEFELDER 169 CANAAN BACK ROAD BARRINGTON, NH 03825



KEACH-NORDSTROM ASSOCIATES, INC.

Civil Engineering Land Surveying Landscape Architecture 10 Commerce Park North, Suite 3B, Bedford, NH 03110 Phone (603) 627-2881



No.	DATE		DESCRIPTION	B
DATE: JUNE 22, 2021 PROJECT NO: 21-0311-1		SCALE: 1"=20'		
		SHEET 2 OF 12		

LOAM & SEED ALL DISTURBED AREAS (TYP.)

GRAPHIC SCALE 1 inch = 20 ft.

SURVEYOR'S CERTIFICATION:

LOSURE OF ONE PART IN TEN

LICENSED LAND SURVEYOR

I HEREBY CERTIFY THAT THIS PLAN WAS PREPARED BY ME OR THOSE UNDER MY DIRECT SUPERVISION. FURTHER, THAT THIS PLAN IS BASED ON AN ACTUAL FIELD SURVEY MADE BY THIS OFFICE DURING APRIL OF -2021. SAID SURVEY HAS A RELATIVE ERROR OF

SITE PLANS ARE VALID FOR TWO YEARS FROM THE DATE OF PLANNING BOARD MEETING FINAL APPROVAL. FINAL APPROVAL COMMENCES AT THE PLANNING BOARD MEETING DATE AT WHICH THE PLAN RECEIVES FINAL APPROVAL.

GRANTED HEREIN

OF APPROVAL

DATE: 6-16-2021

SIGNATURE: 28 JANA Pa



Appendix B: Traffic Counts

15530001

Accurate Counts 978-664-2565

Location: Derry Road Location: South of Ledge Road City/State: Hudson, NH

5/31/2021	Mond		Tueso		Wedne		Thurse		Frida		Saturd	ay	Sunda		Week Av	
Time	SB,	NB,	SB,	NB,	SB,	NB,	SB,	NB,	SB,	NB,	SB,	NB,	SB,	NB,	SB,	NB,
12:00 AM	*	*	*	*	33	41	44	51	*	*	*	*	*	*	38	46
1:00	*	*	*	*	27	22	28	27	*	*	*	*	*	*	28	24
2:00	*	*	*	*	18	19	36	23	*	*	*	*	*	*	27	21
3:00	*	*	*	*	46	40	49	23	*	*	*	*	*	*	48	32
4:00	*	*	*	*	155	75	138	91	*	*	*	*	*	*	146	83
5:00	*	*	*	*	428	241	421	264	*	*	*	*	*	*	424	252
6:00	*	*	*	*	759	544	744	490	*	*	*	*	*	*	752	517
7:00	*	*	*	*	1048	842	1026	871	*	*	*	*	*	*	1037	856
8:00	*	*	*	*	956	735	945	774	*	*	*	*	*	*	950	754
9:00	*	*	*	*	767	654	721	664	*	*	*	*	*	*	744	659
10:00	*	*	*	*	813	640	748	673	*	*	*	*	*	*	780	656
11:00	*	*	*	*	738	755	810	690	*	*	*	*	*	*	774	722
12:00 PM	*	*	*	*	840	799	819	814	*	*	*	*	*	*	830	806
1:00	*	*	*	*	798	808	825	877	*	*	*	*	*	*	812	842
2:00	*	*	*	*	1013	921	1008	986	*	*	*	*	*	*	1010	954
3:00	*	*	*	*	981	1065	979	1072	*	*	*	*	*	*	980	1068
4:00	*	*	*	*	927	1163	1009	1120	*	*	*	*	*	*	968	1142
5:00	*	*	*	*	943	1106	950	1098	*	*	*	*	*	*	946	1102
6:00	*	*	*	*	783	883	700	841	*	*	*	*	*	*	742	862
7:00	*	*	*	*	605	642	528	638	*	*	*	*	*	*	566	640
8:00	*	*	*	*	448	507	396	447	*	*	*	*	*	*	422	477
9:00	*	*	*	*	267	329	264	304	*	*	*	*	*	*	266	316
10:00	*	*	*	*	154	165	164	171	*	*	*	*	*	*	159	168
11:00	*	*	*	*	72	99	78	104	*	*	*	*	*	*	75	102
Total	0	0	0	0	13619	13095	13430	13113	0	0	0	0	0	0	13524	13101
Day	0		0		2671		2654		0		0		0		2662	
AM Peak					7:00	7:00	7:00	7:00							7:00	7:00
Volume					1048	842	1026	871							1037	856
PM Peak					2:00	4:00	4:00	4:00							2:00	4:00
Volume					1013	1163	1009	1120							1010	1142
Comb Total	0		0		2671	14	2654	13	0		0		0		2662	25
ADT	AD	T: 26,628	AAD	T: 26,628												

N/S Street : Derry Road E/W Street : Ledge Road City/State : Hudson, NH Weather : Clear

File Name : 15530001 Site Code : 15530001 Start Date : 6/2/2021

Page No : 1

Groups Printed- Cars - Trucks

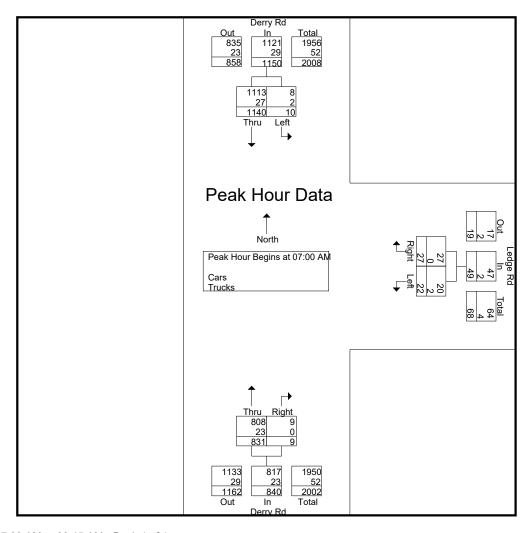
	Derry Rd		Ledge	e Rd	Derry	/ Rd	
	From Nort	h	From	East	From S		
Start Time	Left	Thru	Left	Right	Thru	Right	Int. Total
07:00 AM	3	246	7	10	211	3	480
07:15 AM	3	297	7	5	225	0	537
07:30 AM	1	319	5	4	191	3	523
07:45 AM	3	278	3	8	204	3	499
Total	10	1140	22	27	831	9	2039
08:00 AM	2	251	3	3	162	4	425
08:15 AM	2	250	2	4	153	3	414
08:30 AM	2	288	7	2	175	2	476
08:45 AM	2	240	3	5	212	6	468
Total	8	1029	15	14	702	15	1783
Grand Total	18	2169	37	41	1533	24	3822
Apprch %	0.8	99.2	47.4	52.6	98.5	1.5	
Total %	0.5	56.8	1	1.1	40.1	0.6	
Cars	16	2092	34	40	1492	23	3697
% Cars	88.9	96.4	91.9	97.6	97.3	95.8	96.7
Trucks	2	77	3	1	41	1	125
% Trucks	11.1	3.6	8.1	2.4	2.7	4.2	3.3

		Derry Rd From North			Ledge Rd From East					
Start Time	Left	Thru	App. Total	Left	Right	App. Total	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From	07:00 AM to	08:45 AM - P	eak 1 of 1		_			_		
Peak Hour for Entire Inter	section Begir	ns at 07:00 AN	Л							
07:00 AM	3	246	249	7	10	17	211	3	214	480
07:15 AM	3	297	300	7	5	12	225	0	225	537
07:30 AM	1	319	320	5	4	9	191	3	194	523
07:45 AM	3	278	281	3	8	11	204	3	207	499
Total Volume	10	1140	1150	22	27	49	831	9	840	2039
% App. Total	0.9	99.1		44.9	55.1		98.9	1.1		
PHF	.833	.893	.898	.786	.675	.721	.923	.750	.933	.949
Cars	8	1113	1121	20	27	47	808	9	817	1985
% Cars	80.0	97.6	97.5	90.9	100	95.9	97.2	100	97.3	97.4
Trucks	2	27	29	2	0	2	23	0	23	54
% Trucks	20.0	2.4	2.5	9.1	0	4.1	2.8	0	2.7	2.6

N/S Street : Derry Road E/W Street : Ledge Road City/State : Hudson, NH Weather : Clear

File Name: 15530001 Site Code : 15530001 Start Date : 6/2/2021

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Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1

Peak Hour for Each Appr	<u>oach Begins a</u>	t:							
	07:15 AM			07:00 AM			07:00 AM		
+0 mins.	3	297	300	7	10	17	211	3	214
+15 mins.	1	319	320	7	5	12	225	0	225
+30 mins.	3	278	281	5	4	9	191	3	194
+45 mins.	2	251	253	3	8	11	204	3	207
Total Volume	9	1145	1154	22	27	49	831	9	840
% App. Total	0.8	99.2		44.9	55.1		98.9	1.1	
PHF	.750	.897	.902	.786	.675	.721	.923	.750	.933
Cars	8	1120	1128	20	27	47	808	9	817
% Cars	88.9	97.8	97.7	90.9	100	95.9	97.2	100	97.3
Trucks	1	25	26	2	0	2	23	0	23
% Trucks	11.1	2.2	2.3	9.1	0	4.1	2.8	0	2.7

N/S Street : Derry Road E/W Street : Ledge Road City/State : Hudson, NH Weather : Clear

File Name: 15530001 Site Code : 15530001 Start Date : 6/2/2021 Page No : 10

Groups Printed- Bikes Peds

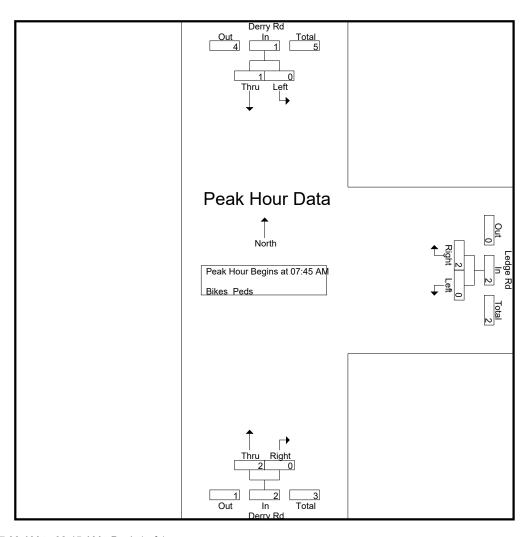
	Г	Jorny Dd		1	odgo Pd			Dorny Dd				
		Derry Rd			_edge Rd			Derry Rd				
	Fr	om North		F	rom East		Fr	rom South				
Start Time	Left	Thru	Peds	Left	Right	Peds	Thru	Right	Peds	Exclu. Total	Inclu. Total	Int. Total
07:00 AM	0	0	0	0	0	0	0	0	0	0	0	0
07:15 AM	0	0	0	0	0	0	0	0	0	0	0	0
07:30 AM	0	0	0	0	0	0	0	0	0	0	0	0
07:45 AM	0	0	0	0	0	0	2	0	0	0	2	2
Total	0	0	0	0	0	0	2	0	0	0	2	2
			i									
MA 00:80	0	0	0	0	0	0	0	0	0	0	0	0
08:15 AM	0	0	0	0	0	0	0	0	0	0	0	0
08:30 AM	0	1	0	0	2	1	0	0	0	1	3	4
08:45 AM	0	0	0	0	0	0	0	0	0	0	0	0_
Total	0	1	0	0	2	1	0	0	0	1	3	4
Grand Total	0	1	0	0	2	1	2	0	0	1	5	6
Apprch %	0	100		0	100	.	100	Ô	· ·	•	Ū	Ü
Total %	0	20		0	40		40	0		16.7	83.3	

		Derry Rd From North			Ledge Rd From East					
Start Time	Left	Thru	App. Total	Left	Right	App. Total	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From	07:00 AM to 0	8:45 AM - Pe	eak 1 of 1							
Peak Hour for Entire Inter	section Begins	at 07:45 AM								
07:45 AM	0	0	0	0	0	0	2	0	2	2
08:00 AM	0	0	0	0	0	0	0	0	0	0
08:15 AM	0	0	0	0	0	0	0	0	0	0
08:30 AM	0	1	1	0	2	2	0	0	0	3
Total Volume	0	1	1	0	2	2	2	0	2	5
% App. Total	0	100		0	100		100	0		
PHF	.000	.250	.250	.000	.250	.250	.250	.000	.250	.417

N/S Street : Derry Road E/W Street : Ledge Road City/State : Hudson, NH Weather : Clear

File Name: 15530001 Site Code : 15530001 Start Date : 6/2/2021

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Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1 Peak Hour for Each Approach Begins at:

Peak Hour for Each Appr	oacii begiiis a	al.							
	07:45 AM			07:45 AM			07:00 AM		
+0 mins.	0	0	0	0	0	0	0	0	0
+15 mins.	0	0	0	0	0	0	0	0	0
+30 mins.	0	0	0	0	0	0	0	0	0
+45 mins.	0	1	1	0	2	2	2	0	2
Total Volume	0	1	1	0	2	2	2	0	2
% App. Total	0	100		0	100		100	0	
PHF	.000	.250	.250	.000	.250	.250	.250	.000	.250

N/S Street : Derry Road E/W Street : Ledge Road City/State : Hudson, NH Weather : Clear

File Name: 15530001 Site Code : 15530001 Start Date : 6/2/2021 Page No : 1

Groups Printed- Cars - Trucks

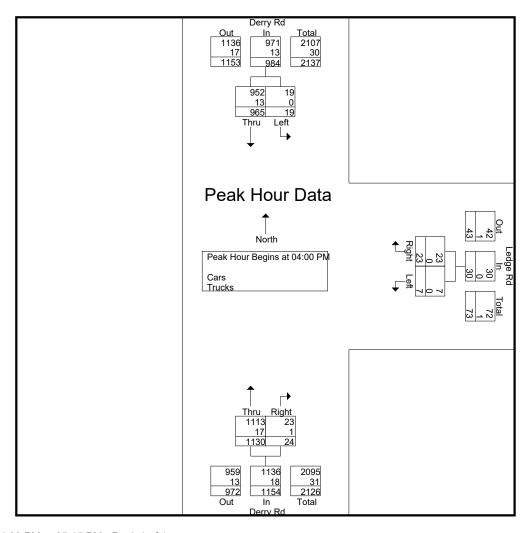
	Derry Rd		Ledge	Rd	Derry	/ Rd	
	From Nort	h	From	East	From S	South	
Start Time	Left	Thru	Left	Right	Thru	Right	Int. Total
04:00 PM	2	251	2	6	284	8	553
04:15 PM	2	239	1	6	277	6	531
04:30 PM	6	227	1	3	287	5	529
04:45 PM	9	248	3	8	282	5	555
Total	19	965	7	23	1130	24	2168
05:00 PM	3	237	1	6	258	6	511
05:15 PM	6	269	4	5	282	6	572
05:30 PM	8	220	2	8	261	6	505
05:45 PM	3	244	1	5	277	4	534
Total	20	970	8	24	1078	22	2122
Grand Total	39	1935	15	47	2208	46	4290
Apprch %	2	98	24.2	75.8	98	2	
Total %	0.9	45.1	0.3	1.1	51.5	1.1	
Cars	39	1913	15	47	2188	45	4247
% Cars	100	98.9	100	100	99.1	97.8	99
Trucks	0	22	0	0	20	1	43
% Trucks	0	1.1	0	0	0.9	2.2	1

		Derry Rd From North			Ledge Rd From East		Derry Rd From South			
Start Time	Left	Thru	App. Total	Left	Right	App. Total	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From	04:00 PM to	05:45 PM - Pe	eak 1 of 1							
Peak Hour for Entire Inter	section Begin	ıs at 04:00 PM	1							
04:00 PM	2	251	253	2	6	8	284	8	292	553
04:15 PM	2	239	241	1	6	7	277	6	283	531
04:30 PM	6	227	233	1	3	4	287	5	292	529
04:45 PM	9	248	257	3	8	11	282	5	287	555
Total Volume	19	965	984	7	23	30	1130	24	1154	2168
% App. Total	1.9	98.1		23.3	76.7		97.9	2.1		
PHF	.528	.961	.957	.583	.719	.682	.984	.750	.988	.977
Cars	19	952	971	7	23	30	1113	23	1136	2137
% Cars	100	98.7	98.7	100	100	100	98.5	95.8	98.4	98.6
Trucks	0	13	13	0	0	0	17	1	18	31
% Trucks	0	1.3	1.3	0	0	0	1.5	4.2	1.6	1.4

N/S Street : Derry Road E/W Street : Ledge Road City/State : Hudson, NH Weather : Clear

File Name: 15530001 Site Code : 15530001 Start Date : 6/2/2021

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Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1

Peak Hour for Each Appr	<u>oach Begins a</u>	<u>t:</u>							
	04:30 PM			04:45 PM			04:00 PM		
+0 mins.	6	227	233	3	8	11	284	8	292
+15 mins.	9	248	257	1	6	7	277	6	283
+30 mins.	3	237	240	4	5	9	287	5	292
+45 mins.	6	269	275	2	8	10	282	5	287
Total Volume	24	981	1005	10	27	37	1130	24	1154
% App. Total	2.4	97.6		27	73		97.9	2.1	
PHF	.667	.912	.914	.625	.844	.841	.984	.750	.988
Cars	24	975	999	10	27	37	1113	23	1136
% Cars	100	99.4	99.4	100	100	100	98.5	95.8	98.4
Trucks	0	6	6	0	0	0	17	1	18
% Trucks	0	0.6	0.6	0	0	0	1.5	4.2	1.6

N/S Street : Derry Road E/W Street : Ledge Road City/State : Hudson, NH Weather : Clear

File Name: 15530001 Site Code : 15530001 Start Date : 6/2/2021 Page No : 10

Groups Printed- Bikes Peds

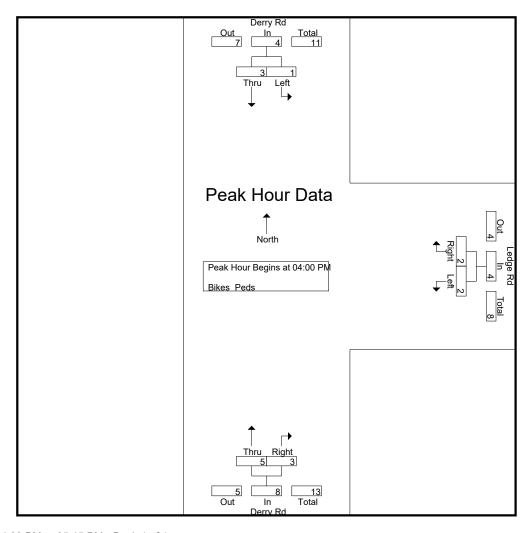
		Derry Rd		L	edge Rd			Derry Rd				
	Fr	om North		F	rom East		F	rom South				
Start Time	Left	Thru	Peds	Left	Right	Peds	Thru	Right	Peds	Exclu. Total	Inclu. Total	Int. Total
04:00 PM	0	0	0	0	0	0	1	0	1	1	1	2
04:15 PM	0	2	1	0	1	0	1	1	0	1	5	6
04:30 PM	1	1	0	0	0	0	3	2	0	0	7	7
04:45 PM	0	0	0	2	1	0	0	0	0	0	3	3_
Total	1	3	1	2	2	0	5	3	1	2	16	18
05:00 PM	0	0	0	0	0	0	0	1	0	0	1	1
05:15 PM	0	1	0	0	0	0	0	0	0	0	1	1
05:30 PM	0	1	0	0	0	0	0	0	0	0	1	1
05:45 PM	0	0	0	4	0	2	0	0	0	2	4	6
Total	0	2	0	4	0	2	0	1	0	2	7	9
Grand Total	1	5	1	6	2	2	5	4	1	4	23	27
Apprch %	16.7	83.3		75	25		55.6	44.4				
Total %	4.3	21.7		26.1	8.7		21.7	17.4		14.8	85.2	

		Derry Rd From North								
Start Time	Left	Thru	App. Total	Left	Right	App. Total	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From	04:00 PM to 0	05:45 PM - Pe	eak 1 of 1							
Peak Hour for Entire Inter	section Begins	s at 04:00 PM	1							
04:00 PM	0	0	0	0	0	0	1	0	1	1
04:15 PM	0	2	2	0	1	1	1	1	2	5
04:30 PM	1	1	2	0	0	0	3	2	5	7
04:45 PM	0	0	0	2	1	3	0	0	0	3
Total Volume	1	3	4	2	2	4	5	3	8	16
% App. Total	25	75		50	50		62.5	37.5		
PHF	.250	.375	.500	.250	.500	.333	.417	.375	.400	.571

N/S Street : Derry Road E/W Street : Ledge Road City/State : Hudson, NH Weather : Clear

File Name: 15530001 Site Code : 15530001 Start Date : 6/2/2021

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Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1 Peak Hour for Each Approach Begins at:

Peak Hour for Each Appr	oacii begiiis a	11.							
	04:00 PM			04:00 PM			04:00 PM		
+0 mins.	0	0	0	0	0	0	1	0	1
+15 mins.	0	2	2	0	1	1	1	1	2
+30 mins.	1	1	2	0	0	0	3	2	5
+45 mins.	0	0	0	2	1	3	0	0	0
Total Volume	1	3	4	2	2	4	5	3	8
% App. Total	25	75		50	50		62.5	37.5	
PHF	.250	.375	.500	.250	.500	.333	.417	.375	.400



Appendix C: Monthly Traffic Volumes

Year 2019 Monthly Data

Group 4 Averages: Urban Highways

		Adjustment	Adjustment
<u>Month</u>	ADT	to Average	to Peak
January	11,431	1.12	1.23
February	11,848	1.08	1.18
March	12,141	1.06	1.15
April	12,860	1.00	1.09
May	13,551	0.95	1.03
June	13,785	0.93	1.02
July	13,942	0.92	1.01
August	14,016	0.92	1.00
September	13,379	0.96	1.05
October	13,339	0.96	1.05
November	12,265	1.05	1.14
December	11,496	1.12	1.22

Average ADT: 12,838 Peak ADT: 14,016



Appendix D: Vehicle Speeds

Location: Derry Road Location: South of Ledge Road City/State: Hudson, NH Direction: SB, 15530001

6/2/202	1 0 - 15	> 15 -	> 20 -	> 25 -	> 30 -	> 35 -	> 40 -	> 45 -	> 50 -	> 55 -	> 60 -	> 65 -	> 70	
0/2/202 Tim											65 MPH		MPH	Total
12:00 AN				3		9	3	1	1	0	0	0	0	33
1:0		0	0	2	13	8	4	0	0	0	0	0	0	27
2:0		0	0	4	8	6	0	0	0	0	0	0	0	18
3:0	0 0	0	0	4	17	15	8	1	0	1	0	0	0	46
4:0	0 0	1	2	8	37	73	31	3	0	0	0	0	0	155
5:0	0 0	2	1	9	108	237	59	12	0	0	0	0	0	428
6:0	0 0	0	17	69	257	350	63	3	0	0	0	0	0	759
7:0	0 0	2	3	154	618	244	25	2	0	0	0	0	0	1048
8:0	0 0	8	24	255	435	206	26	2	0	0	0	0	0	956
9:0	0 2	. 7	8	133	363	227	27	0	0	0	0	0	0	767
10:0	0 0	8	5	101	400	264	33	1	1	0	0	0	0	813
11:0	0 0	6	10	104	364	219	33	2	0	0	0	0	0	738
12:00 PM	/I C	5	15	134	397	255	31	3	0	0	0	0	0	840
1:0	0 1	9	14	121	427	197	25	4	0	0	0	0	0	798
2:0	0 0	10	27	182	566	215	12	1	0	0	0	0	0	1013
3:0	0 1	3	29	222	475	228	20	3	0	0	0	0	0	981
4:0	0 2	4	11	119	498	253	37	3	0	0	0	0	0	927
5:0	0 1	4	38	102	438	313	45	2	0	0	0	0	0	943
6:0	0 0	5	8	77	334	298	59	2	0	0	0	0	0	783
7:0	0 0	3	2	38	298	221	39	3	1	0	0	0	0	605
8:0	0 0	2	1	27	206	180	30	2	0	0	0	0	0	448
9:0	0 0	2	1	21	129	89	21	3	1	0	0	0	0	267
10:0	0 0	0	0	15	69	54	14	2	0	0	0	0	0	154
11:0		2		7	26	30	7	0	0	0	0	0	0	72
Tota	al 7	83	216	1911	6499	4191	652	55	4	1	0	0	0	13619

Percentile 15th 50th 85th 95th Speed 29.7 33.5 37.8 40.3

Mean Speed (Average) 10 MPH Pace Speed 33.7 30-39 Number in Pace 10644 Percent in Pace 78.2% Number > 30 MPH 11402 Percent > 30 MPH 83.7%

Location: Derry Road Location: South of Ledge Road City/State: Hudson, NH Direction: SB, 15530001

6/3/2021	0 - 15	> 15 -	> 20 -	> 25 -	> 30 -	> 35 -	> 40 -	> 45 -	> 50 -	> 55 -	> 60 -	> 65 -	> 70	
Time	MPH	20 MPH	25 MPH	30 MPH	35 MPH	40 MPH	45 MPH	50 MPH	55 MPH	60 MPH	65 MPH	70 MPH	MPH	Total
12:00 AM	0	0	0	8	10	21	4	1	0	0	0	0	0	44
1:00	0	1	0	8	10	4	3	1	1	0	0	0	0	28
2:00	0	0	2	5	13	13	2	1	0	0	0	0	0	36
3:00	0	0	0	1	19	19	8	1	1	0	0	0	0	49
4:00	0	0	2	4	38	71	22	1	0	0	0	0	0	138
5:00	0	1	1	15	154	193	50	7	0	0	0	0	0	421
6:00	0	2	0	61	308	335	36	2	0	0	0	0	0	744
7:00	1	12	20	195	534	248	10	5	1	0	0	0	0	1026
8:00	0	3	23	252	463	180	23	1	0	0	0	0	0	945
9:00	0	3	8	87	352	241	28	2	0	0	0	0	0	721
10:00	0	7	11	107	401	204	16	1	0	0	1	0	0	748
11:00	0	8	22	163	428	169	15	5	0	0	0	0	0	810
12:00 PM	0	6	20	91	438	235	29	0	0	0	0	0	0	819
1:00	0	6	11	132	434	216	26	0	0	0	0	0	0	825
2:00	0	8	42	233	498	194	32	1	0	0	0	0	0	1008
3:00	1	6	18	251	476	200	25	1	1	0	0	0	0	979
4:00	10	17	56	222	400	265	37	1	0	1	0	0	0	1009
5:00	0	3	9	97	480	327	32	2		0	0	0	0	950
6:00	0	1	2	30	297	306	58	5	1	0	0	0	0	700
7:00	0	0	1	44	226	210	42	5		0	0	0	0	528
8:00	1	3	5	37	141	166	39	2	0	2	0	0	0	396
9:00	1	1	0	13	85	129	30	5	0	0	0	0	0	264
10:00	1	2	0	10	62	58	25	6		0	0	0	0	164
11:00	0		0	10	28	30	7	2		0	0	0	0	78
Total	15		253	2076	6295	4034	599	58	5	3	1	0	0	13430
		ı	Percentile		50th	85th	95th							
			Speed	29.7	33.5	37.2	39.7							
		an Speed		33.5										
	10) MPH Pa	ce Speed	30-39										

Number in Pace 10290 Percent in Pace 76.6%

Number > 30 MPH 10995

	P	ercent > 3	0 MPH	81.9%										
Grand Total	22	174	469	3987	12794	8225	1251	113	9	4	1	0	0	27049
Stats		Pe	rcentile	15th	50th	85th	95th							

37.2

33.5

39.7

29.7 Speed Mean Speed (Average) 33.6 10 MPH Pace Speed 30-39 Number in Pace 20934 Percent in Pace 77.4% Number > 30 MPH 22397 Percent > 30 MPH 82.8%

Location: Derry Road Location: South of Ledge Road City/State: Hudson, NH Direction: NB, 15530001

_ =	,														
_	6/2/2021	0 - 15	> 15 -	> 20 -	> 25 -	> 30 -	> 35 -	> 40 -	> 45 -	> 50 -	> 55 -	> 60 -	> 65 -	> 70	
	Time	MPH	20 MPH	25 MPH	30 MPH	35 MPH	40 MPH	45 MPH	50 MPH	55 MPH	60 MPH	65 MPH	70 MPH	MPH	Total
_	12:00 AM	0	0	1	1	7	23	7	2	0	0	0	0	0	41
	1:00	0	0	0	0	10	10	2	0	0	0	0	0	0	22
	2:00	0	0	0	2	4	6	5	2	0	0	0	0	0	19
	3:00	0	0	0	0	7	18	7	7	0	0	0	0	1	40
	4:00	0	0	2	0	11	29	27	4	0	0	0	0	2	75
	5:00	0	0	2	3	38	102	77	17	0	1	0	0	1	241
	6:00	0	5	21	61	133	217	94	11	2	0	0	0	0	544
	7:00	3	1	17	87	235	404	86	7	1	0	0	0	1	842
	8:00	3	1	24	63	221	352	68	1	0	0	0	0	2	735
	9:00	1	2	12	34	204	323	70	5	0	0	0	1	2	654
	10:00	3	6	8	30	238	274	72	7	0	0	0	0	2	640
	11:00	2	6	ū	63	275	312	80	6	0	0	0	1	2	755
	12:00 PM	2	7	16	39	296	362	72	5	0	0	0	0	0	799
	1:00	6	2	17	90	312	317	60	3	1	0	0	0	0	808
	2:00	3	2	4	75	349	411	70	5	0	0	0	1	1	921
	3:00	4	4	20	86	423	431	94	3	0	0	0	0	0	1065
	4:00	6	8	24	204	498	389	32	1	0	0	0	0	1	1163
	5:00	0	3	6	105	408	493	86	5	0	0	0	0	0	1106
	6:00	3	5	3	45	293	418	104	11	0	0	0	0	1	883
	7:00	0	3	3	28	179	348	70	9	2	0	0	0	0	642
	8:00	0	1	5	41	189	222	42	7	0	0	0	0	0	507
	9:00	1	0	0	13	103	159	50	3	0	0	0	0	0	329
	10:00	0	0	2	10	44	81	25	2	0	1	0	0	0	165
_	11:00	0	0	3	3	23	51	15	4	0	0	0	0	0	99
	Total	37	56	198	1083	4500	5752	1315	127	6	2	0	3	16	13095

Percentile 15th 50th 85th 95th Speed 31 35.3 39 41.5

Mean Speed (Average) 10 MPH Pace Speed 35.4 30-39 Number in Pace 10159 Percent in Pace 77.6% Number > 30 MPH 11721 Percent > 30 MPH 89.5%

Location: Derry Road Location: South of Ledge Road City/State: Hudson, NH Direction: NB, 15530001

Direction: NB,														
6/3/2021	0 - 15	> 15 -	> 20 -	> 25 -	> 30 -	> 35 -	> 40 -	> 45 -	> 50 -	> 55 -	> 60 -	> 65 -	> 70	
Time	MPH	20 MPH	25 MPH	30 MPH	35 MPH		45 MPH	50 MPH	55 MPH	60 MPH	65 MPH	70 MPH	MPH	Total
12:00 AM	0	1	0	5	9	27	7	0	0	0	1	1	0	51
1:00	0	0	0	2	7	8			1	0	0	0	0	27
2:00	0	0	1	1	4	7	7	3	0	0	0	0	0	23
3:00	0	0	0	1	5	9	7	0	1	0	0	0	0	23
4:00	0	0	0	4	11	45	25		0	0	0	0	0	91
5:00	0	0	5	9	52	108	68	21	0	0	0	0	1	264
6:00	1			66	104	199			1	1	0	0	2	490
7:00	4			103	315	359			1	0	0	2	1	871
8:00	7	2	20	56	261	326			1	0	0	0	9	774
9:00	1	1	3	48	218	316	65	9	1	0	0	0	2	664
10:00	1			51	199	318		7	0	0	0	0	0	673
11:00	2	5	9	71	232	270	95	1	0	0	0	3	2	690
12:00 PM	3	3	7	50	310	358	75	8	0	0	0	0	0	814
1:00	2	4	16	74	373	349	56	3	0	0	0	0	0	877
2:00	5	7	15	129	431	347	47	2	0	0	1	2	0	986
3:00	5	2	17	180	447	366	53	1	1	0	0	0	0	1072
4:00	15	16	39	121	396	460	73	0	0	0	0	0	0	1120
5:00	0	5	12	68	433	471	104	5	0	0	0	0	0	1098
6:00	0	0	2	46	317	359	103	13	1	0	0	0	0	841
7:00	3	0	6	38	195	301	89	6	0	0	0	0	0	638
8:00	0	0	1	24	160	215	44	2	1	0	0	0	0	447
9:00	0	3	4	16	99	119	62	1	0	0	0	0	0	304
10:00	0	0	1	13	38	81	30	7	1	0	0	0	0	171
11:00	0			2	22	53	25	1	0	0	0	0	0	104
Total	49	57	196	1178	4638	5471	1365		10	1	2	8	17	13113
			Percentile	15th	50th	85th	95th							
			Speed	31	35.3	39	41.5							
		an Speed		35.3										
	10	0 MPH Pa	ice Speed	30-39										
		Numbe	er in Pace	10023										
		Percei	nt in Pace	76.4%										
		Number >	> 30 MPH	11633										
		Percent >	> 30 MPH	88.7%										
Grand Total	86			2261	9138	11223			16	3	2	11	33	26208
Stats			Percentile	15th	50th	85th	95th							
			Speed	31	35.3	39	41.5							
	Mea	an Speed	(Average)	35.3										
	10	0 MPH Pa	ice Speed	30-39										
		Numbe	er in Pace	20182										
		Percei	nt in Pace	77.0%										
		AL I.	OO MADU	00054										

Number > 30 MPH 23354 Percent > 30 MPH 89.1%



Appendix E: Capacity-Analysis Worksheets

Intersection						
Int Delay, s/veh	1.1					
		MDD	NET	NES	051	ODT
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W		₽		- 1	
Traffic Vol, veh/h	24	29	895	10	11	1228
Future Vol, veh/h	24	29	895	10	11	1228
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	0	-
Veh in Median Storage	e, # 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	72	72	93	93	90	90
Heavy Vehicles, %	9	0	3	0	20	2
Mvmt Flow	33	40	962	11	12	1364
WWW.CT TOW	00	10	702			1001
	Minor1		/lajor1		Major2	
Conflicting Flow All	2356	968	0	0	973	0
Stage 1	968	-	-	-	-	-
Stage 2	1388	-	-	-	-	-
Critical Hdwy	6.49	6.2	-	-	4.3	-
Critical Hdwy Stg 1	5.49	-	-	-	-	-
Critical Hdwy Stg 2	5.49	-	-	-	-	-
Follow-up Hdwy	3.581	3.3	-	-	2.38	-
Pot Cap-1 Maneuver	37	311	-	-	642	-
Stage 1	358	-	_	_	-	_
Stage 2	223	_	_	_	_	_
Platoon blocked, %	220		_	_		
Mov Cap-1 Maneuver	36	311			642	-
	139	311	-		042	
Mov Cap-2 Maneuver		-	-	-	-	-
Stage 1	358	-	-	-	-	-
Stage 2	219	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s	33.3		0		0.1	
HCM LOS	D				J.1	
Minor Lane/Major Mvn	nt	NBT	NBRV	VBLn1	SBL	SBT
Capacity (veh/h)		-	-	199	642	-
HCM Lane V/C Ratio		-	-	0.37	0.019	-
HCM Control Delay (s))	-	-	33.3	10.7	-
HCM Lane LOS		-	-	D	В	-
HCM 95th %tile Q(veh	١	-	-		0.1	_

Intersection						
Int Delay, s/veh	0.7					
		WDD	NDT	NDD	CDI	CDT
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W.		f)		ሻ	↑
Traffic Vol, veh/h	8	25	1217	26	20	1039
Future Vol, veh/h	8	25	1217	26	20	1039
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	0	-
Veh in Median Storage	e, # 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	68	68	99	99	96	96
Heavy Vehicles, %	0	0	2	4	0	1
Mvmt Flow	12	37	1229	26	21	1082
				_		
	Minor1		/lajor1		Major2	
Conflicting Flow All	2366	1242	0	0	1255	0
Stage 1	1242	-	-	-	-	-
Stage 2	1124	-	-	-	-	-
Critical Hdwy	6.4	6.2	-	-	4.1	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	-	-	2.2	-
Pot Cap-1 Maneuver	39	215	-	-	561	-
Stage 1	275	-	-	-	-	-
Stage 2	313	-	-	-	-	-
Platoon blocked, %	310		_	_		_
Mov Cap-1 Maneuver	38	215	_	_	561	_
Mov Cap-1 Maneuver	148	213	_		JU 1 -	
Stage 1	275	-	-	-	-	-
	301	-	-	-	-	•
Stage 2	301	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s	29.6		0		0.2	
HCM LOS	D					
, = = =						
Minor Lane/Major Mvn	nt	NBT	NBRV	VBLn1	SBL	SBT
Capacity (veh/h)		-	-	194	561	-
HCM Lane V/C Ratio		-	-	0.25	0.037	-
HCM Control Delay (s))	-	-	29.6	11.7	-
HCM Lane LOS		-	-	D	В	-
HCM 95th %tile Q(veh	1)	-	-		0.1	-

Intersection						
Int Delay, s/veh	0.8					
		MDD	NET	NDD	051	ODT
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	¥		\$		ች	1010
Traffic Vol, veh/h	24	29	904	10	11	1240
Future Vol, veh/h	24	29	904	10	11	1240
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	0	-
Veh in Median Storage	e, # 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	9	0	3	0	20	2
Mvmt Flow	27	32	1004	11	12	1378
N.A. i. a. u/N.A.i. a. u	N /!4		1-1-1		1-1-2	
	Minor1		/lajor1		/lajor2	
Conflicting Flow All	2412	1010	0	0	1015	0
Stage 1	1010	-	-	-	-	-
Stage 2	1402	-	-	-	-	-
Critical Hdwy	6.49	6.2	-	-	4.3	-
Critical Hdwy Stg 1	5.49	-	-	-	-	-
Critical Hdwy Stg 2	5.49	-	-	-	-	-
Follow-up Hdwy	3.581	3.3	-	-	2.38	-
Pot Cap-1 Maneuver	34	294	-	-	618	-
Stage 1	342	-	-	-	-	-
Stage 2	220	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	33	294	-	-	618	-
Mov Cap-2 Maneuver	135		_	-	-	_
Stage 1	342	-	_	-	-	-
Stage 2	216	_	_	_	_	_
Jugo 2						
Approach	WB		NB		SB	
HCM Control Delay, s	31.8		0		0.1	
HCM LOS	D					
Minor Lanc/Major Mun	ot	NBT	NDDV	VBLn1	SBL	SBT
Minor Lane/Major Mvn	TIC .	INDI				SDI
Capacity (veh/h)		-	-	192	618	-
HCM Lane V/C Ratio	_	-		0.307	0.02	-
HCM Control Delay (s))	-	-	31.8	10.9	-
HCM Lane LOS HCM 95th %tile Q(veh		-	-	D 1.2	В	-
			-	4.0	0.1	_

Intersection						
Int Delay, s/veh	0.6					
					25/	
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	¥		. ∱			
Traffic Vol, veh/h	8	25	1229	26	20	1049
Future Vol, veh/h	8	25	1229	26	20	1049
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	0	-
Veh in Median Storage	e, # 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	0	0	2	4	0	1
Mvmt Flow	9	28	1366	29	22	1166
	Minor1		/lajor1		Major2	
Conflicting Flow All	2591	1381	0	0	1395	0
Stage 1	1381	-	-	-	-	-
Stage 2	1210	-	-	-	-	-
Critical Hdwy	6.4	6.2	-	-	4.1	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	-	-	2.2	-
Pot Cap-1 Maneuver	28	178	-	-	497	-
Stage 1	236	-	-	-	-	-
Stage 2	285	-	_	_	-	-
Platoon blocked, %			_	_		_
Mov Cap-1 Maneuver	27	178	_		497	_
Mov Cap-1 Maneuver	128	-	_	_	- 7/1	_
Stage 1	236			_	_	
Stage 2	272	-	_	_	-	-
Staye 2	212	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s	33.4		0		0.2	
HCM LOS	D					
N. A		NDT	NDD	NDL 4	CDI	CDT
Minor Lane/Major Mvm	I	NBT	NRKA	VBLn1	SBL	SBT
Capacity (veh/h)		-	-	163	497	-
HCM Lane V/C Ratio		-	-	0.225		-
		_	_	33.4	12.6	-
HCM Control Delay (s)		-				
HCM Control Delay (s) HCM Lane LOS HCM 95th %tile Q(veh)		-	-	D 0.8	B 0.1	-

Intersection						
Int Delay, s/veh	1					
	WDI	WDD	NDT	NDD	CDI	CDT
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	\	าา	\$	11	ነ	1270
Traffic Vol, veh/h	27	32	999	11	12	1370
Future Vol, veh/h	27	32	999	11	12	1370
Conflicting Peds, #/hr	0 Stop		0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	0	-
Veh in Median Storag		-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	9	0	3	0	20	2
Mvmt Flow	30	36	1110	12	13	1522
Major/Minor	Minor1	N	Major1	P	Major2	
Conflicting Flow All	2664	1116	0	0	1122	0
Stage 1	1116	-	-	-	-	-
Stage 2	1548	-	-	-	-	-
Critical Hdwy	6.49	6.2	-	-	4.3	-
Critical Hdwy Stg 1	5.49	-	-	-	-	-
Critical Hdwy Stg 2	5.49	-	-	-	-	-
Follow-up Hdwy	3.581	3.3	-	-	2.38	-
Pot Cap-1 Maneuver	~ 24	255	-	-	561	-
Stage 1	303	-	-	-	-	-
Stage 2	186	-	-	-	-	-
Platoon blocked, %			-	_		-
Mov Cap-1 Maneuver	~ 23	255	-	-	561	_
Mov Cap-2 Maneuver	114	-	_	_	-	_
Stage 1	303	-	_	_	-	_
Stage 2	182	-	_	-	_	_
olago 2						
	14/5		ND		0.0	
Approach	WB		NB		SB	
HCM Control Delay, s			0		0.1	
HCM LOS	E					
Minor Lane/Major Mvr	nt	NBT	NBRV	VBLn1	SBL	SBT
Capacity (veh/h)		-	-	4.0	561	-
HCM Lane V/C Ratio		_		0.402		_
HCM Control Delay (s)	-	-	44.0	11.6	-
HCM Lane LOS	1	-	-	41.Z E	В	-
HCM 95th %tile Q(ver	1)		_	4.0	0.1	
·	'/			1.0	0.1	
Notes						
~: Volume exceeds ca	pacity	\$: De	elay exc	ceeds 30	00s	+: Comp

Intersection						
Int Delay, s/veh	0.7					
		WDD	NDT	NDD	CDI	CDT
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W		ĵ»			↑
Traffic Vol, veh/h	9	28	1358	29	22	1159
Future Vol, veh/h	9	28	1358	29	22	1159
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	0	-
Veh in Median Storage		-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	0	0	2	4	0	1
Mvmt Flow	10	31	1509	32	24	1288
Major/Minor	Minor1	Λ	Major1	N	Major2	
						^
Conflicting Flow All	2861 1525	1525	0	0	1541	0
Stage 1		-	-	-	-	-
Stage 2	1336		-	-		
Critical Hdwy	6.4	6.2	-	-	4.1	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	-	-	2.2	-
Pot Cap-1 Maneuver	19	147	-	-	437	-
Stage 1	200	-	-	-	-	-
Stage 2	248	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	18	147	-	-	437	-
Mov Cap-2 Maneuver	107	-	-	-	-	-
Stage 1	200	-	-	-	-	-
Stage 2	234	-	-	-	-	-
Annroach	WB		NB		SB	
Approach						
HCM Control Delay, s	42.9		0		0.3	
HCM LOS	Е					
Minor Lane/Major Mvm	nt	NBT	NBRV	VBLn1	SBL	SBT
Capacity (veh/h)			_		437	
HCM Lane V/C Ratio		_		0.305		_
HCM Control Delay (s)		_	-		13.7	_
HCM Lane LOS		_	_	E	В	_
HCM 95th %tile Q(veh)	_	-	1.2	0.2	_
HOW 75th 70th Q(Ven	1			1.2	0.2	_

Intersection						
Int Delay, s/veh	0.9					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W		ĵ.		ነ	↑
Traffic Vol, veh/h	29	24	907	10	11	1243
Future Vol, veh/h	29	24	907	10	11	1243
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	_	None	_	None
Storage Length	0	-	-	-	0	-
Veh in Median Storage		_	0	-	-	0
Grade, %	0	_	0	_	_	0
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	90	0	3	0	20	2
Mvmt Flow	32	27	1008	11	12	1381
IVIVIIIL FIOW	32	21	1008	- 11	12	1301
Major/Minor	Minor1	N	Najor1	N	/lajor2	
Conflicting Flow All	2419	1014	0	0	1019	0
Stage 1	1014	_	-	_	-	_
Stage 2	1405	_	_	_	_	_
Critical Hdwy	6.49	6.2	_	_	4.3	_
Critical Hdwy Stg 1	5.49	- 0.2	_	_	٦.٥	_
	5.49	-		-	-	-
Critical Hdwy Stg 2			-	-		-
Follow-up Hdwy	3.581	3.3	-	-	2.38	-
Pot Cap-1 Maneuver	34	292	-	-	616	-
Stage 1	340	-	-	-	-	-
Stage 2	219	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	33	292	-	-	616	-
Mov Cap-2 Maneuver	134	-	-	-	-	-
Stage 1	340	-	-	-	-	-
Stage 2	215	-	-	-	-	-
J						
Approach	WB		NB		SB	
HCM Control Delay, s	35.2		0		0.1	
HCM LOS	35.2 E		U		U. I	
HOW LUS	E					
Minor Lane/Major Mvn	nt	NBT	NBRV	VBLn1	SBL	SBT
Capacity (veh/h)		_		177	616	
HCM Lane V/C Ratio		-	_	0.333	0.02	-
HCM Control Delay (s)	_	-	35.2	11	_
HCM Lane LOS		_	_	55.2 E	В	_
HCM 95th %tile Q(veh	1)	_		1.4	0.1	_
HOW FOUT WILL WIVE	1)	_	-	1.4	U. I	_

Intersection						
Int Delay, s/veh	0.9					
		EDD	NIDI	NDT	CDT	CDD
Movement Lang Configurations	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	ነ	70	\	004	1227	20
Traffic Vol, veh/h	23	30	23	894	1237	30
Future Vol, veh/h	23	30	23	894	1237	30
Conflicting Peds, #/hr	0	0	_ 0	_ 0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	0	0	-	-	-
Veh in Median Storag		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	2	2	3	3	3	3
Mvmt Flow	26	33	26	993	1374	33
N A = ' =/N A' =	N.4'		14-!1		M-!0	
Major/Minor	Minor2		Major1		Major2	
Conflicting Flow All	2436	1391	1407	0	-	0
Stage 1	1391	-	-	-	-	-
Stage 2	1045	-	-	-	-	-
Critical Hdwy	6.42	6.22	4.13	-	-	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	2.227	-	-	-
Pot Cap-1 Maneuver	35	174	482	-	-	-
Stage 1	231	-	-	-	-	-
Stage 2	339	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	33	174	482	-	-	-
Mov Cap-2 Maneuver		-	-	_	_	_
Stage 1	219		_	_	_	-
Stage 2	339	_	_	_	_	_
Stage 2	337					
Approach	EB		NB		SB	
HCM Control Delay, s	33.5		0.3		0	
HCM LOS	D					
N 4: /N 4-: N 4:		NIDI	NDT	EDI 1 I	ΓDI Δ	CDT
Minor Lane/Major Mvi	nı	NBL	INRI	EBLn1 I		SBT
Capacity (veh/h)		482	-	136	174	-
HCM Lane V/C Ratio		0.053	-	0.188		-
HCM Control Delay (s	5)	12.9	-	37.5	30.5	-
HCM Lane LOS		В	-	Ε	D	-
HCM 95th %tile Q(vel	1)	0.2	-	0.7	0.7	-

Intersection						
Int Delay, s/veh	0.6					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	¥		₽			
Traffic Vol, veh/h	8	25	1230	26	20	1050
Future Vol, veh/h	8	25	1230	26	20	1050
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	0	-
Veh in Median Storage	e, # 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	0	0	2	4	0	1
Mvmt Flow	9	28	1367	29	22	1167
Major/Minor	\/lipor1	N	Notor1		10ior2	
	Minor1		Major1		Major2	
Conflicting Flow All	2593	1382	0	0	1396	0
Stage 1	1382	-	-	-	-	-
Stage 2	1211	-	-	-	-	-
Critical Hdwy	6.4	6.2	-	-	4.1	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	-	-	2.2	-
Pot Cap-1 Maneuver	28	178	-	-	496	-
Stage 1	235	-	-	-	-	-
Stage 2	285	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	27	178	-	-	496	-
Mov Cap-2 Maneuver	127	-	-	-	-	-
Stage 1	235	-	-	-	-	-
Stage 2	272	-	-	-	-	-
Annraach	MD		ND		CD	
Approach	WB		NB		SB	
HCM Control Delay, s	33.6		0		0.2	
HCM LOS	D					
Minor Lane/Major Mvm	nt	NBT	NBRV	VBLn1	SBL	SBT
Capacity (veh/h)				162	496	
HCM Lane V/C Ratio		-		0.226		-
HCM Control Delay (s)				33.6	12.6	
HCM Lane LOS		_	_	55.0 D	12.0 B	_
HCM 95th %tile Q(veh))	_		0.8	0.1	
How four four Q(veri)		-	0.0	0.1	

Intersection						
Int Delay, s/veh	0.3					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
						SDK
Lane Configurations	<u>ነ</u>		<u>ነ</u>	1245	1040	0
Traffic Vol, veh/h	11	9	11	1245	1049	9
Future Vol, veh/h	11	9	11	1245	1049	9
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	0	0	-	-	-
Veh in Median Storage		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	12	10	12	1383	1166	10
Major/Minor	Minor2		Major1	1	Major2	
Conflicting Flow All	2578	1171	1176	0	- viajoi z	0
Stage 1	1171		1170		-	-
	1407	-	-	-	-	-
Stage 2			112	-		-
Critical Hdwy	6.42	6.22	4.12	-	-	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	2 210	-	-	-
Follow-up Hdwy	3.518	3.318	2.218	-	-	-
Pot Cap-1 Maneuver	28	235	594	-	-	-
Stage 1	295	-	-	-	-	-
Stage 2	226	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	27	235	594	-	-	-
Mov Cap-2 Maneuver	128	-	-	-	-	-
Stage 1	289	-	-	-	-	-
Stage 2	226	-	-	-	-	-
Annroach	EB		NB		SB	
Approach						
HCM Control Delay, s	29.3		0.1		0	
HCM LOS	D					
Minor Lane/Major Mvn	nt	NBL	NBT	EBLn1 l	EBLn2	SBT
Capacity (veh/h)		594		128	235	
HCM Lane V/C Ratio		0.021		0.095		_
HCM Control Delay (s)	1	11.2			21	
HCM Lane LOS		11.2 B		30.1	C	_
HCM 95th %tile Q(veh)	0.1	-	0.3	0.1	-
	<i>'</i>	U. I	-	0.3	U. I	-

Intersection						
Int Delay, s/veh	1.1					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	WDL	WOR	Tabi	אטוז	3DL Š	<u>301</u>
Traffic Vol, veh/h	'T' 27	32	1002	11	12	T 1373
Future Vol, veh/h	27	32	1002	11	12	1373
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	_	-	0	_
Veh in Median Storage		-	0	-	-	0
Grade, %	0	-	0	_	-	0
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	9	0	3	0	20	2
Mvmt Flow	30	36	1113	12	13	1526
Major/Minor	Minor1	N	Najor1	N	Majora	
	2671	1119	Major1		Major2 1125	0
Conflicting Flow All Stage 1	1119	1119	0	0	1125	0
Stage 2	1552	-	-	-	-	-
Critical Hdwy	6.49	6.2	-	-	4.3	-
Critical Hdwy Stg 1	5.49	- 0.2	_	_	4.5	_
Critical Hdwy Stg 2	5.49	_			_	
Follow-up Hdwy	3.581	3.3	_	_	2.38	_
Pot Cap-1 Maneuver	~ 23	254	_	_	559	_
Stage 1	302	- 254	_	_	-	_
Stage 2	185	_	_	_	-	_
Platoon blocked, %	100		_	_		_
Mov Cap-1 Maneuver	~ 22	254	_	_	559	_
Mov Cap-2 Maneuver	113	-	_	_	-	_
Stage 1	302	_	_	-	-	-
Stage 2	181	_	_	_	_	_
Jugo Z	101					
	14/5		ND		0.5	
Approach	WB		NB		SB	
HCM Control Delay, s	41.6		0		0.1	
HCM LOS	E					
Minor Lane/Major Mvn	nt	NBT	NBRV	VBLn1	SBL	SBT
Capacity (veh/h)		-	-	162	559	-
HCM Lane V/C Ratio		-	-	0.405		-
HCM Control Delay (s))	-	-		11.6	-
HCM Lane LOS		-	-	Е	В	-
HCM 95th %tile Q(veh	ı)	-	-	1.8	0.1	-
Notes						
	nacity	¢. Do	lay ove	oods 2	ΩΩς	L. Comi
~: Volume exceeds ca	pacity	\$: De	elay exc	eeds 30	UUS	+: Com

Intersection								
Int Delay, s/veh	1.1							
Movement	EBL	EBR	NBL	NBT	SBT	SBR		
Lane Configurations	ኘ	7	ሻ	†	1	02.1		
Traffic Vol, veh/h	25	34	25	988	1367	33		
Future Vol, veh/h	25	34	25	988	1367	33		
Conflicting Peds, #/hr	0	0	0	0	0	0		
Sign Control	Stop	Stop	Free	Free	Free	Free		
RT Channelized	- -		-	None	-	None		
Storage Length	0	0	0	-	_	-		
/eh in Median Storage		-	-	0	0	_		
Grade, %	0	_	_	0	0	_		
Peak Hour Factor	90	90	90	90	90	90		
Heavy Vehicles, %	2	2	3	3	3	3		
Nymt Flow	28	38	28	1098	1519	37		
VIVIIIL FIOW	20	30	20	1090	1319	31		
Major/Minor	Minor2	1	Major1	N	Major2			
Conflicting Flow All	2692	1538	1556	0	-	0		
Stage 1	1538	1000	1000	-	-	-		
Stage 2	1154	-		-	-	-		
Stage 2 Critical Hdwy	6.42	6.22	4.13	-	-	-		
ritical Hdwy Stg 1	5.42	0.22	4.13	-	-	-		
	5.42		-	-		-		
ritical Hdwy Stg 2		2 210	2 227	-	-	-		
ollow-up Hdwy	3.518 ~ 24	3.318	2.227	-	-	-		
ot Cap-1 Maneuver			422	-	-	-		
Stage 1	195	-	-	-	-	-		
Stage 2	300	-	-	-	-	-		
Platoon blocked, %	-00	1.40	400	-	-	-		
Mov Cap-1 Maneuver	~ 22	142	422	-	-	-		
Mov Cap-2 Maneuver	113	-	-	-	-	-		
Stage 1	182	-	-	-	-	-		
Stage 2	300	-	-	-	-	-		
			r i c		0.5			
pproach	EB		NB		SB			
HCM Control Delay, s	42.5		0.3		0			
HCM LOS	E							
/linor Lane/Major Mvm	nt	NBL	NBT	EBLn1 l	EBLn2	SBT	SBR	
Capacity (veh/h)		422	-	113	142	-	-	
ICM Lane V/C Ratio		0.066	-	0.246	0.266	-	-	
ICM Control Delay (s)		14.1	-	46.9	39.3	-	-	
ICM Lane LOS		В	-	Е	Е	-	-	
HCM 95th %tile Q(veh)	0.2	-	0.9	1	-	-	
Votes								
: Volume exceeds ca	nacity	\$· De	elav exc	ceeds 3	00s	+: Comi	outation Not Defined	*: All major volume in platoon
. Volumo ondecas ca	paorty	ψ. Δ(nay one	,50 4 5 5	003	50111	odiation Not Defined	. 7 iii major voidine iii pidtoon

Intersection						
Int Delay, s/veh	0.8					
		WDD	NDT	NDD	CDI	CDT
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	¥	20	1250	20	ነ	11/0
Traffic Vol, veh/h	9	28	1359	29	22	1160
Future Vol, veh/h	9	28	1359	29	22	1160
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	0	-
Veh in Median Storage		-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	0	0	2	4	0	1
Mvmt Flow	10	31	1510	32	24	1289
Major/Minor N	Minor1	N	/lajor1	N	Major2	
Conflicting Flow All	2863	1526	0	0	1542	0
Stage 1	1526	-	-	-	-	-
Stage 2	1337	_	_	_	_	_
Critical Hdwy	6.4	6.2	_	_	4.1	_
Critical Hdwy Stg 1	5.4	-	_	_	-	_
Critical Hdwy Stg 2	5.4	_	_	_	_	_
Follow-up Hdwy	3.5	3.3	_	_	2.2	_
Pot Cap-1 Maneuver	19	146	_	_	436	_
Stage 1	200	-	_	_	-	_
Stage 2	247	_	_	_	_	_
Platoon blocked, %	271		_	_		_
Mov Cap-1 Maneuver	18	146	_	_	436	_
Mov Cap-2 Maneuver	106	-	_	_	-	_
Stage 1	200	-	-	-	-	-
	233	-	-	-	-	-
Stage 2	233	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s	43.3		0		0.3	
HCM LOS	Ε					
Minor Lane/Major Mvm	t	NBT	NIDD\/	VBLn1	SBL	SBT
	ll .					
Capacity (veh/h)		-	-	101	436	-
HCM Control Polov (c)		-		0.307		-
HCM Control Delay (s)		-	-		13.7	-
		_	-	Ε	В	-
HCM Lane LOS HCM 95th %tile Q(veh)			_	1.2	0.2	_

Intersection							
Int Delay, s/veh	0.3						
Movement	EBL	EBR	NBL	NBT	SBT	SBR	
Lane Configurations	ሻ	7	<u>ነ</u>	↑	₽		
Traffic Vol, veh/h	11	11	12	1377	1159	10	
Future Vol, veh/h	11	11	12	1377	1159	10	
Conflicting Peds, #/hr	0	0	0	0	0	0	
Sign Control	Stop	Stop	Free	Free	Free	Free	
RT Channelized	-	None	-	None	-	None	
Storage Length	0	0	0	-	-	_	
Veh in Median Storag		-	-	0	0	-	
Grade, %	0	_	_	0	0	_	
Peak Hour Factor	90	90	90	90	90	90	
Heavy Vehicles, %	2	2	2	2	2	2	
Mymt Flow	12	12	13	1530	1288	11	
IVIVIIIL FIOW	12	12	13	1330	1200	11	
Major/Minor	Minor2	1	Major1	1	Major2		
Conflicting Flow All	2850	1294	1299	0		0	
Stage 1	1294	-	-	_	_	-	
Stage 2	1556	_	_	_	_	_	
Critical Hdwy	6.42	6.22	4.12	_	_	_	
Critical Hdwy Stg 1	5.42	0.22	7.12	_	_	_	
Critical Hdwy Stg 2	5.42				_		
Follow-up Hdwy	3.518		2.218	-	-	-	
				-		-	
Pot Cap-1 Maneuver	19	199	533	-	-	-	
Stage 1	257	-	-	-	-	-	
Stage 2	191	-	-	-	-	-	
Platoon blocked, %				-	-	-	
Mov Cap-1 Maneuver		199	533	-	-	-	
Mov Cap-2 Maneuver		-	-	-	-	-	
Stage 1	251	-	-	-	-	-	
Stage 2	191	-	-	-	-	-	
Approach	EB		NB		SB		
HCM Control Delay, s	33.4		0.1		0		
HCM LOS	D						
Minor Lane/Major Mvr	nt	NBL	NBT	EBLn1 I	EBLn2	SBT	
Capacity (veh/h)		533	1101	108	199	- 051	
HCM Lane V/C Ratio		0.025	-	0.113		-	
HCM Control Delay (s	1		-			-	
	1	11.9	-	42.5	24.3	-	
	/	D			^		
HCM Lane LOS HCM 95th %tile Q(veh		B 0.1	-	E 0.4	0.2	-	



TOWN OF HUDSON



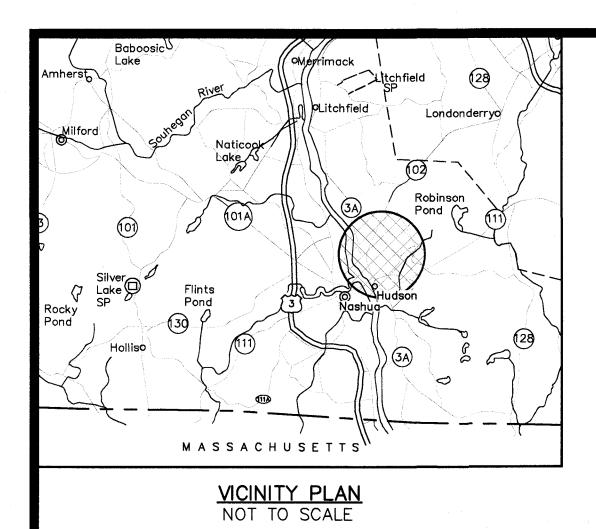
Planning Board

Timothy Malley, Chairman

12 School Street • Hudson, New Hampshire 03051 • Tel: 603-886-6008 • Fax: 603-594-1142

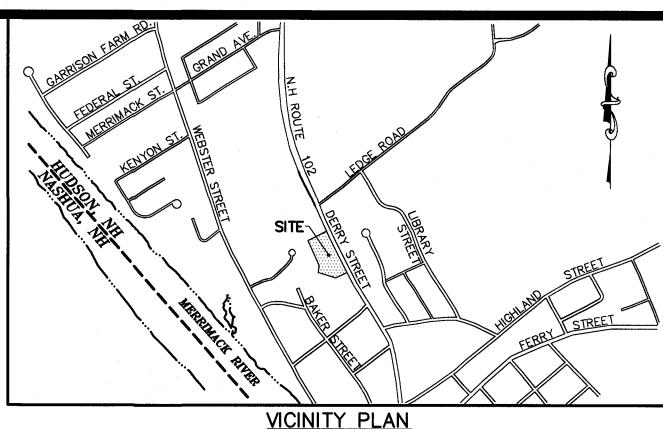
CAP FEE WORKSHEET - 2021

Date <u>: 07</u>	<u>'-21-21</u> Zor	ne #1_	Map/Lot: _	173/029-000
	ne: A			56 Derry Street
Proposed IT	ΓE Use #1:	Commercia	l – Coffee Shop	
New Daily T	Trips (15% of T	Гotal):	104	trips.
CAP FEES:	(ONE CHEC	K NEEDED)		
1.	(Bank 09) 2070-701	Coffee/Dor (104 trips (\$ 20,696.00	
		Total CAP	Fee	\$_20,696.00
Check should	d be made payal	ole to the <u>Tow</u>	n of Hudson.	
Thank you,	- •			
Brian Groth				
Town Planner	r			



NON RESIDENTIAL SITE PLAN AROMA JOE'S MAP 173; LOTS 29 56 DERRY STREET

HUDSON, NEW HAMPSHIRE



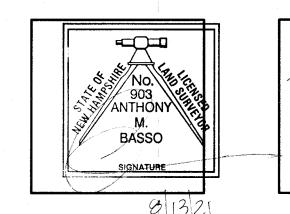
OWNER:

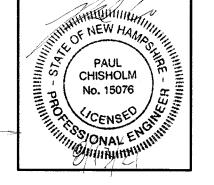
STEVE S. & HSIANG HWA W. PAN 13 KING HENRY DRIVE LONDONDERRY, NH 03053

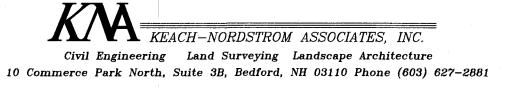
OWNER/APPLICANT: SCOTT ZIEFELDER 169 CANAAN BACK ROAD BARINGTON, NH 03825

PREPARED BY: KEACH-NORDSTROM ASSOCIATES, INC. 10 COMMERCE PARK NORTH, SUITE 3B BEDFORD, NEW HAMPSHIRE 03110 (603) 627-2881



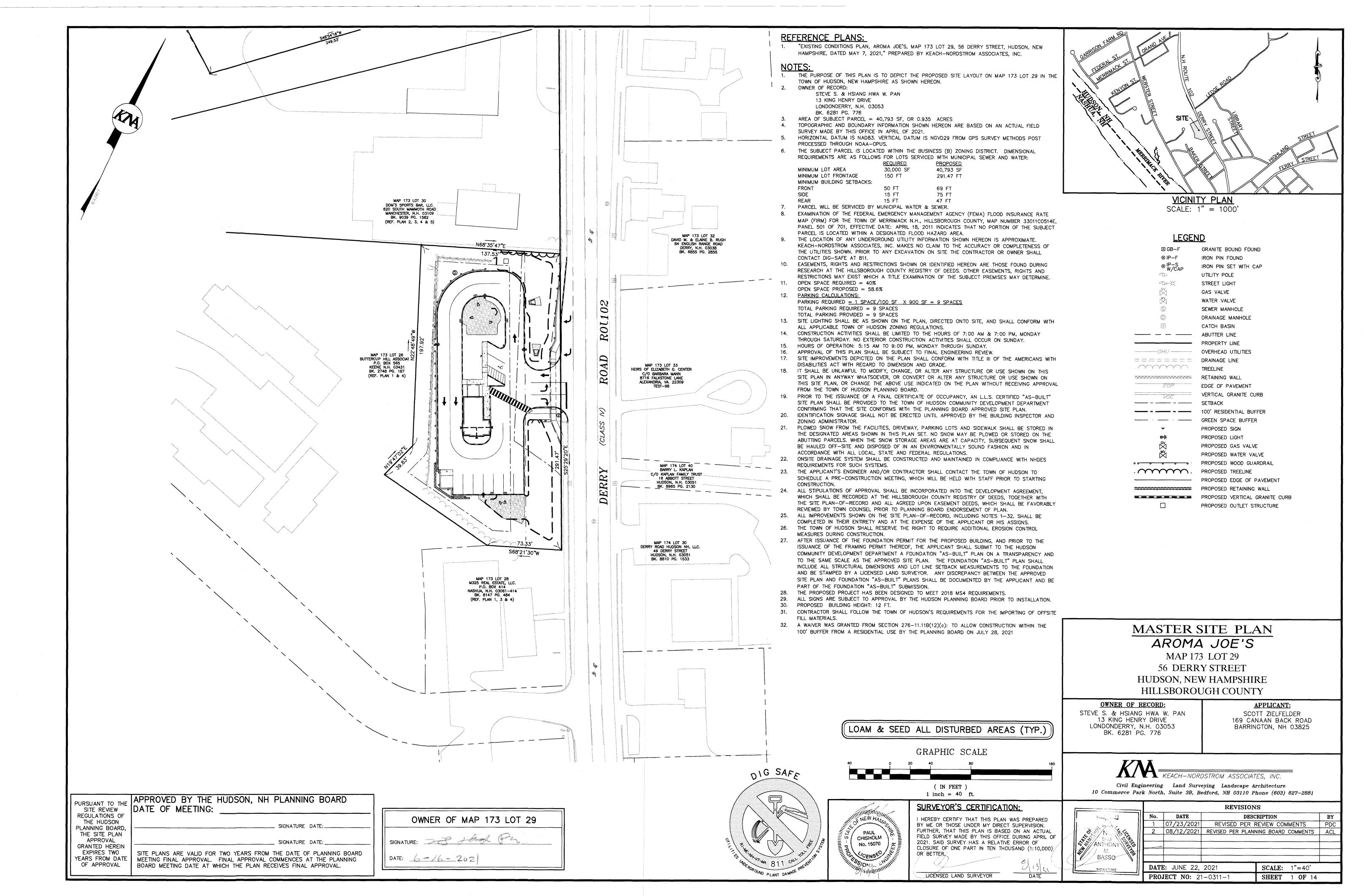


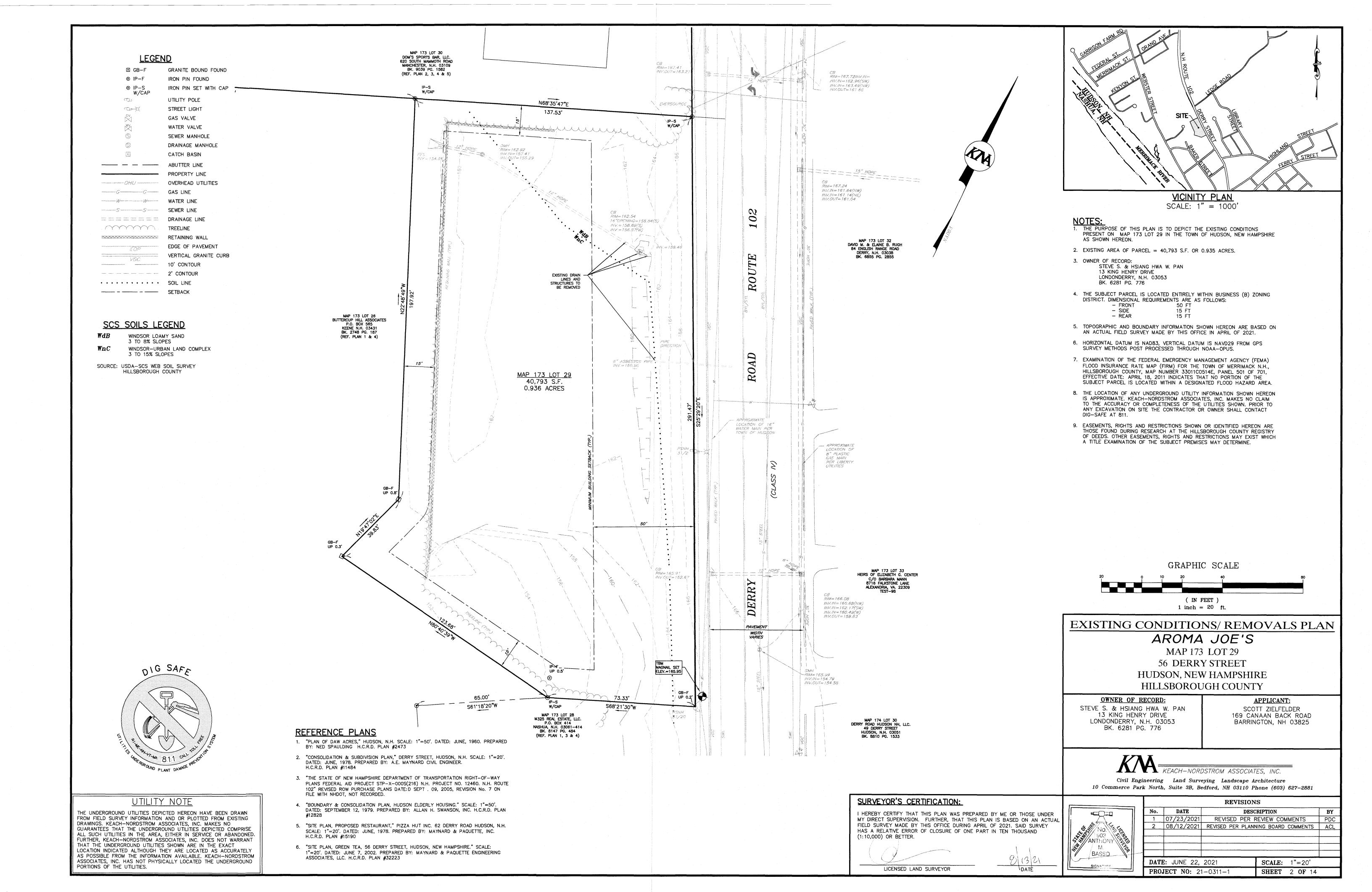


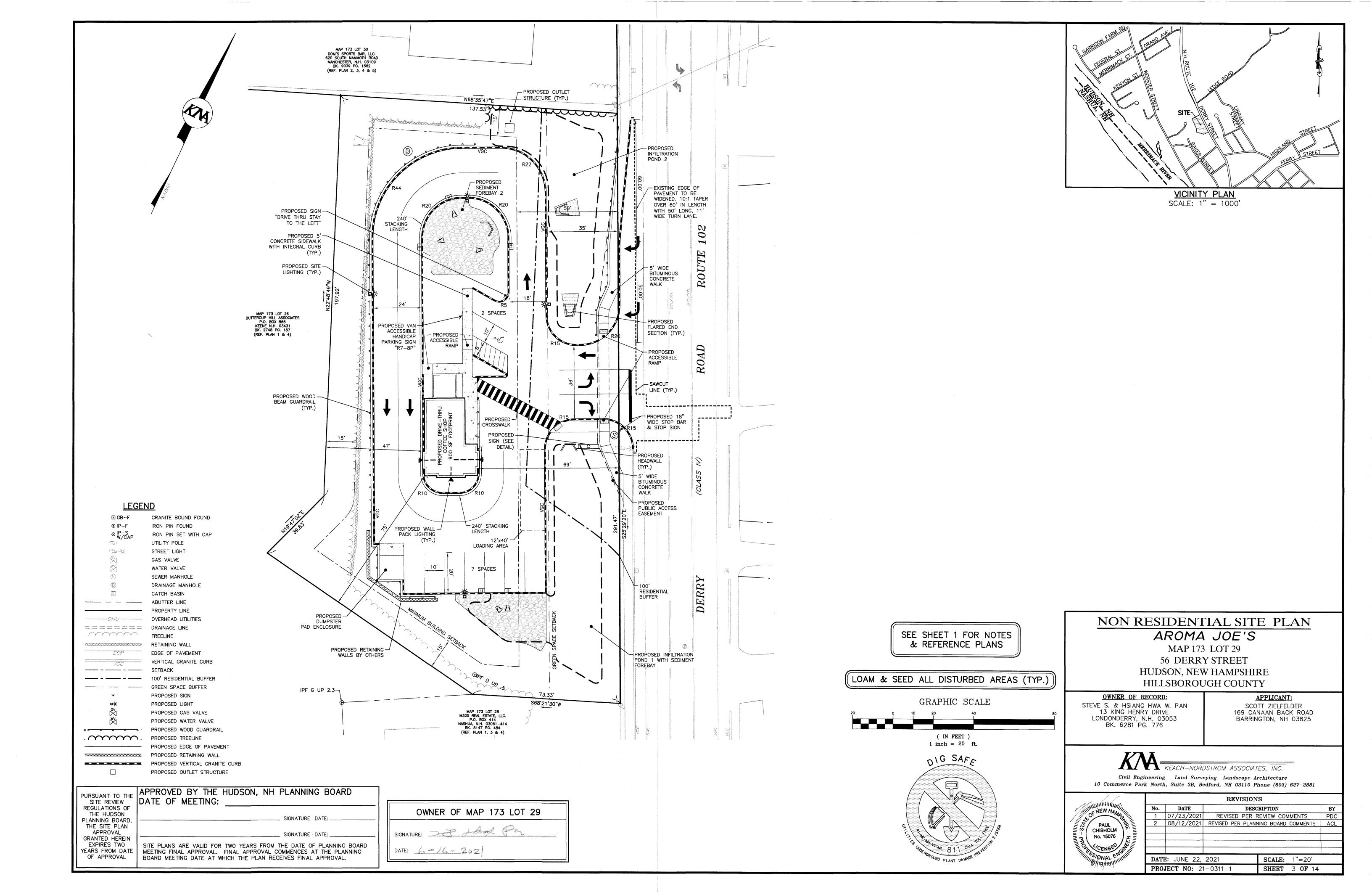


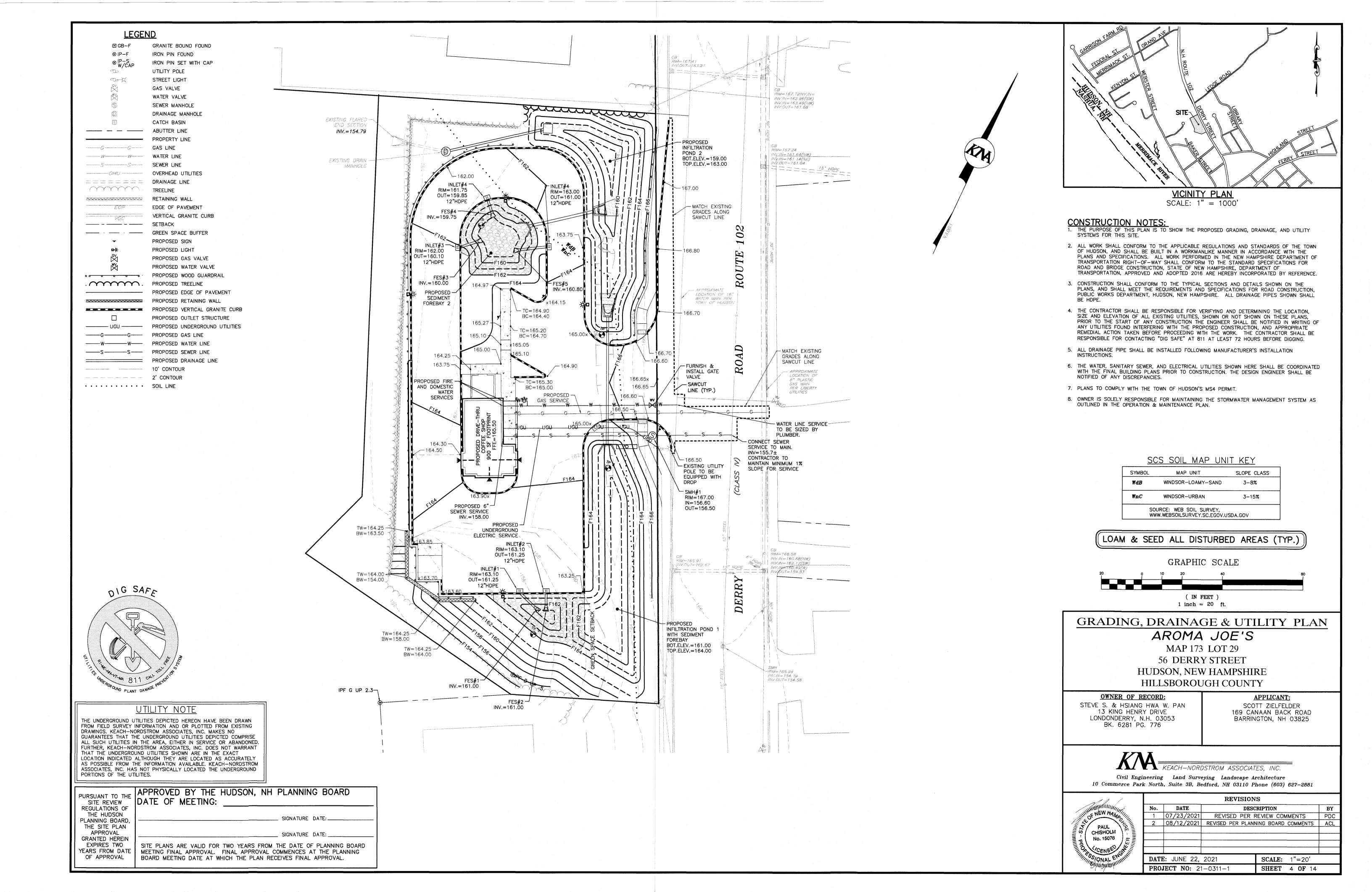
JUNE 22, 2021 LAST REVISED: AUGUST 12, 2021 PROJECT NO. 21-0311-1

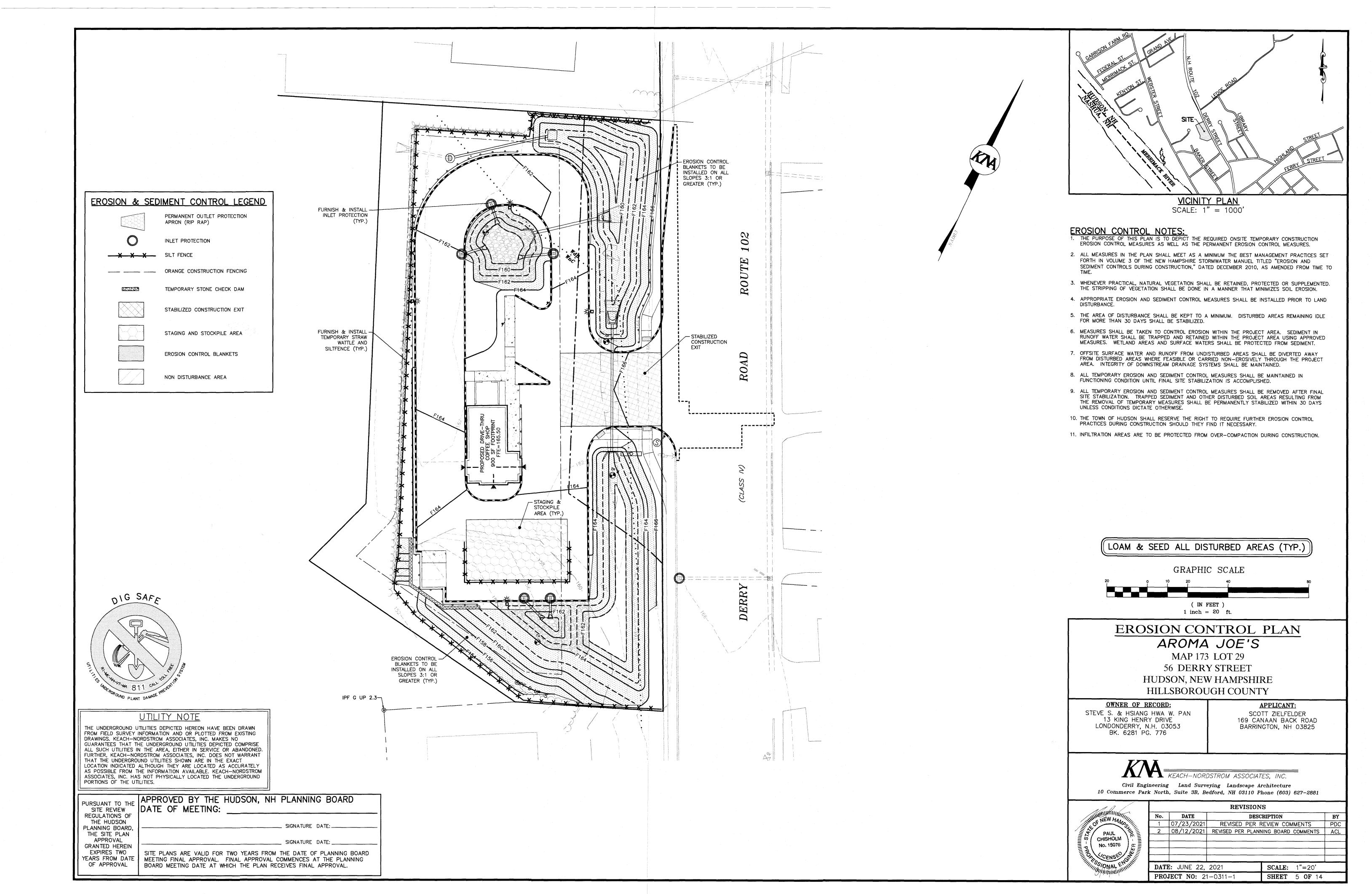
SHEET TITLE	SHEET No.
MASTER PLAN	1
EXISTING CONDITIONS/REMOVALS PLAN	2
NON-RESIDENTIAL SITE PLAN	3
GRADING, DRAINAGE, AND UTILITY PLAN	4
EROSION CONTROL PLAN	5
LANDSCAPE PLAN	6
LIGHTING PLAN	7
SIGHT DISTANCE PLAN	8
CONSTRUCTION DETAILS	9-14
EASEMENT PLAN	E1

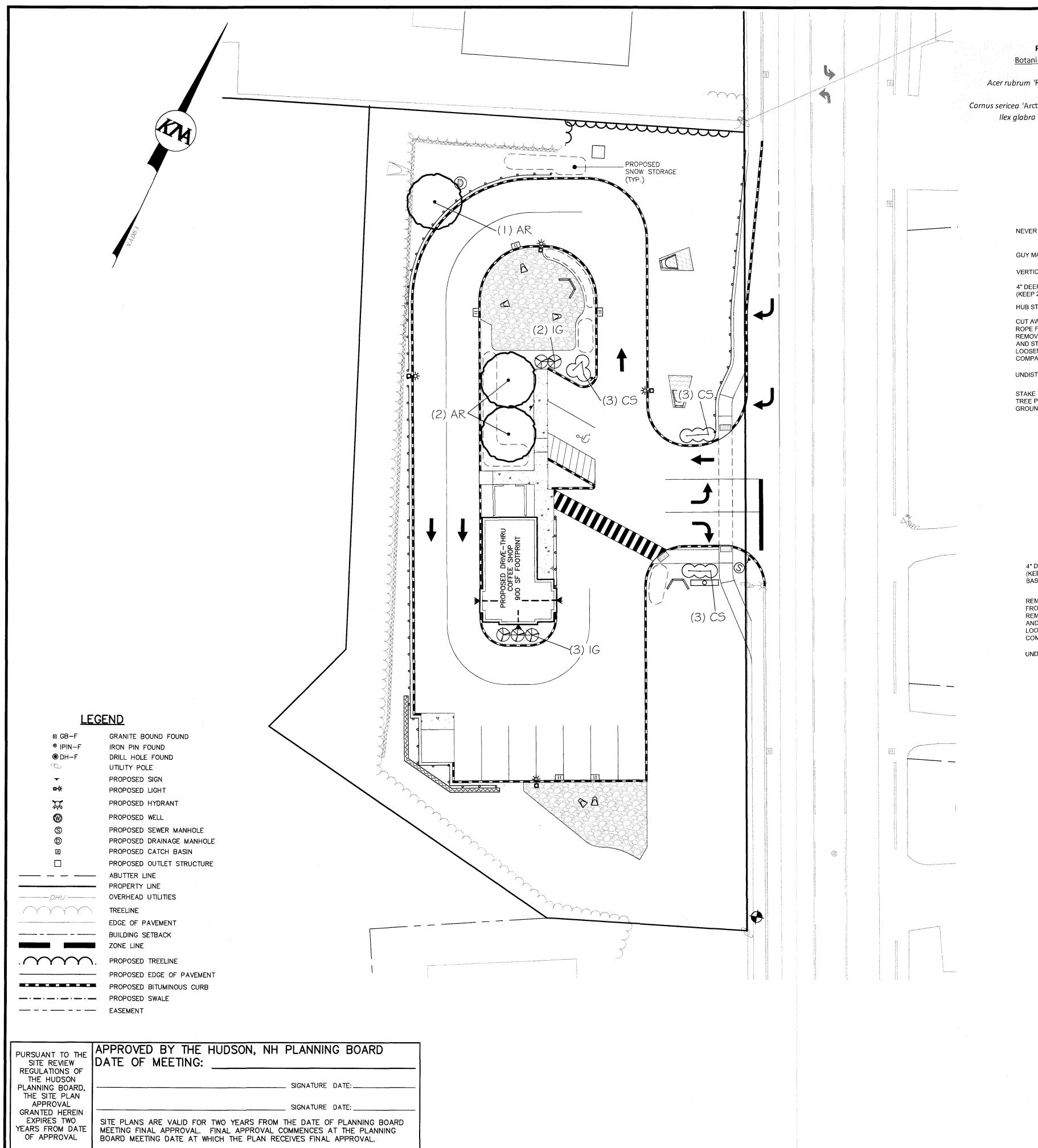


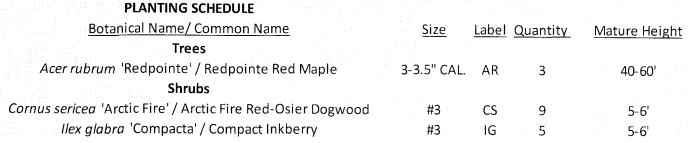


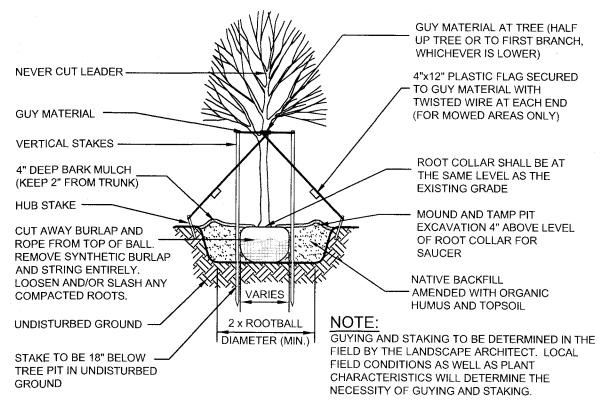






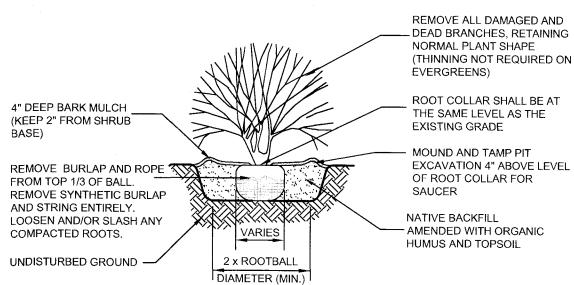






DECIDUOUS TREE PLANTING DETAIL NOT TO SCALE

(JANUARY 2012)



BALLED & BURLAP SHRUB PLANTING DETAIL

DIG SAFE

NOT TO SCALE (JANUARY 2012)

LANDSCAPE NOTES:

1. THE PURPOSE OF THIS PLAN IS TO SHOW THE PROPOSED SITE LANDSCAPE WHICH PROVIDES CLIMATIC RELIEF AND AESTHETIC APPEAL.

- 2. ALL PLANT MATERIALS USED SHALL BE NURSERY STOCK AND SHALL BE GUARANTEED FOR A PERIOD OF ONE (1) YEAR FROM DATE OF INSTALLATION. ANY MATERIAL WHICH DIES OR DOES NOT SHOWN HEALTHY APPEARANCE WITHIN THIS TIME SHALL BE REPLACED AT THE CONTRACTOR'S EXPENSE; WITH SAME WARRANTY REQUIREMENTS AS THE ORIGINAL. WARRANTIES TYPICALLY DO NOT COVER LOSS DUE TO INSECT INFESTATION OR MECHANICAL DAMAGE (I.E. SNOW STORAGE).
- 3. IF THE SOIL CONDITIONS ARE EXTREMELY SANDY, ALL TREES SHALL HAVE A 6" LAYER OF COMPACTED TOPSOIL PLACED IN THE BASE OF THE PLANT PIT AS A MOISTURE RETENTION LAYER. THE PLANT PIT SIDEWALLS SHALL BE OVER EXCAVATED BY AN ADDITIONAL 12" BEYOND THE NORMAL OUTSIDE RADIUS OF THE HOLE. A TOPSOIL MIXTURE SHALL BE USED TO BACKFILL THE HOLE AS FOLLOWED; ORGANIC TOPSOIL, AMENDED WITH 10% WOOD ASH, 10% MANURE, 30% PEATMOSS AND A GRANULAR HYDROGEL TO ABSORB AND RETAIN WATER.
- 4. PLANTING BEDS AND SAUCERS SHALL RECEIVE A 4" MINIMUM THICKNESS OF PINE/HEMLOCK BARK MULCH OVER A 5oz. POLYPROPYLENE WEED CONTROL FABRIC.
- 5. PAVEMENT AND ROAD BASE MATERIAL ENCOUNTERED IN ANY LAWN OR PLANTING BED SHALL BE REMOVED AND DISPOSED OF BY THE CONTRACTOR AND SUITABLE AMENDED SOIL INSTALLED AS SPECIFIED IN THE TURF ESTABLISHMENT SCHEDULE.
- 6. PLANT TYPES SHOWN ARE SUBJECT TO AVAILABILITY. SUBSTITUTE MATERIALS CAN BE IMPLEMENTED WITH APPROVAL FROM KEACH NORDSTROM ASSOCIATES PRIOR TO

LANDSCAPE CALCULATIONS REQUIRED PARKING LOT INTERIOR LANDSCAPE AREA PROPOSED PARKING AREA PAVED: 5,151 10% REQUIRED LANDSCAPE AREA:

PROVIDED LANDSCAPE AREA: 1,496 SF REQUIRED PARKING LOT SHADE TREES AND SHRUBS
PROPOSED PAVED AREA: 5,151 SF

3 TREES REQUIRED SHADE TREES REQUIRED (5,151/1,600): (OR 1 TREE/5 PROP. PARKING SPACES) 2 TREES REQUIRED SHADE TREES PROVIDED: SHRUBS REQUIRED (5,151/200): 26 SHRUBS, OR (OR 1.6 x 9 PROP. PARKING SPACES) 14 SHRUBS REQUIRED SHRUBS PROVIDED:

14 SHRUBS PROPOSED

LANDSCAPE PLAN

AROMA JOE'S

MAP 173 LOT 29

56 DERRY STREET HUDSON, NEW HAMPSHIRE HILLSBOROUGH COUNTY

OWNER OF RECORD:

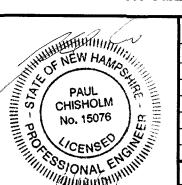
STEVE S. & HSIANG HWA W. PAN 13 KING HENRY DRIVE LONDONDERRY, N.H. 03053 BK. 6281 PG. 776

APPLICANT: SCOTT ZIELFELDER 169 CANAAN BACK ROAD BARRINGTON, NH 03825



KEACH-NORDSTROM ASSOCIATES, INC.

Civil Engineering Land Surveying Landscape Architecture 10 Commerce Park North, Suite 3B, Bedford, NH 03110 Phone (603) 627-2881

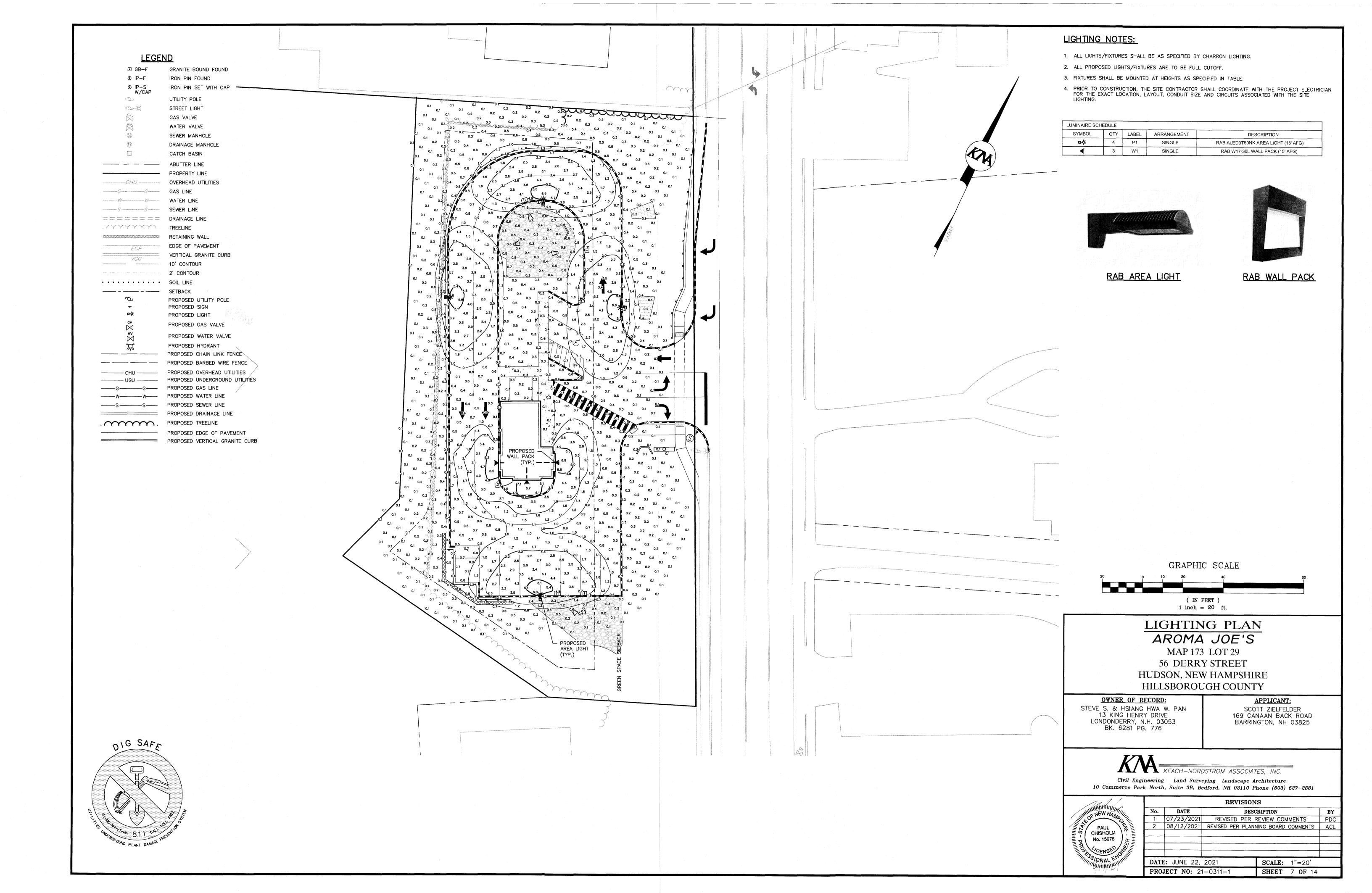


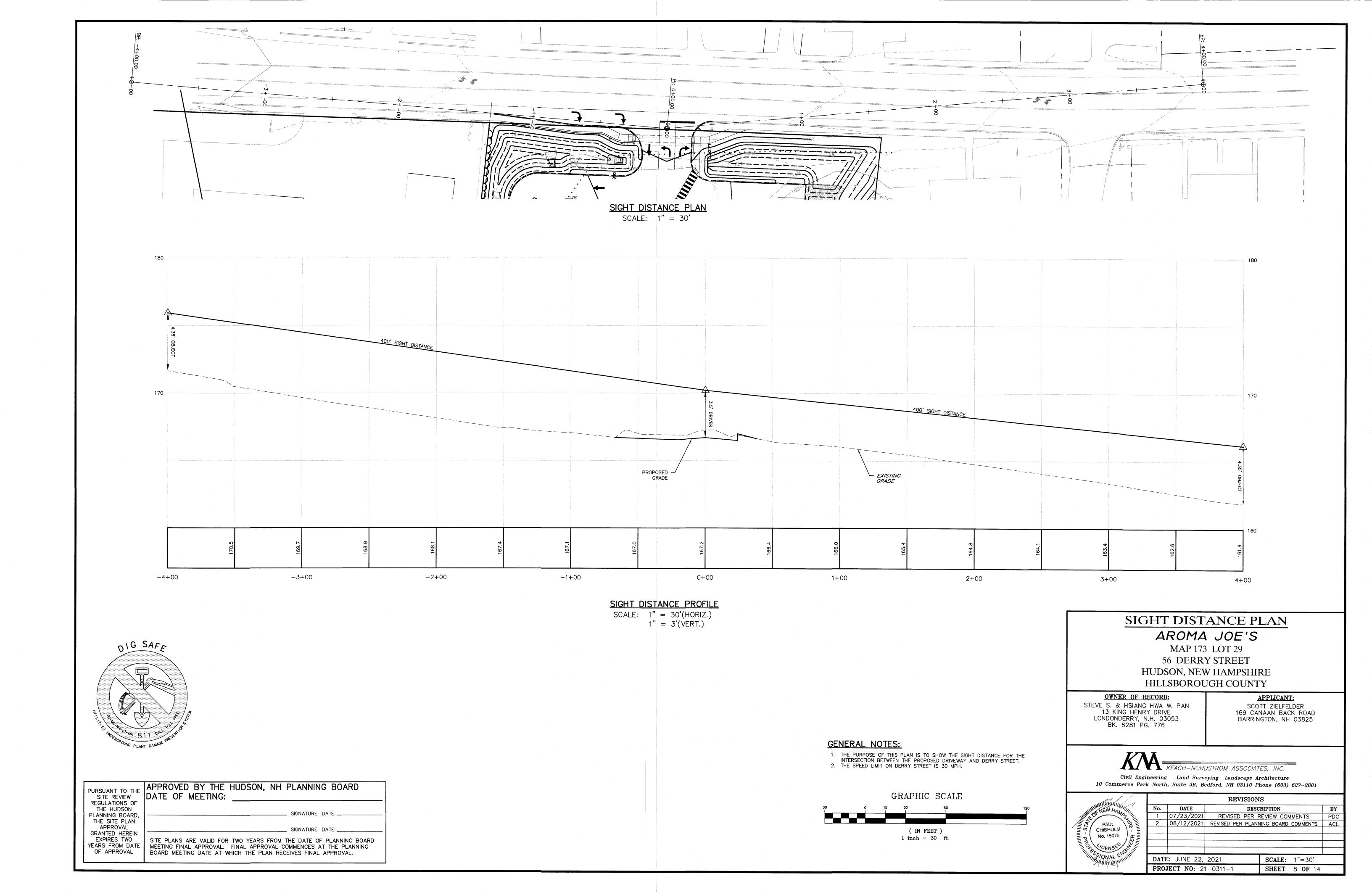
		REVIS	IONS	
No.	DATE		DESCRIPTION	BY
1	07/23/2021	REVISED P	ER REVIEW COMMENTS	PDC
2	08/12/2021	REVISED PER P	PLANNING BOARD COMMENTS	ACL
DAT	E: JUNE 22,	2021	SCALE: 1"=20'	
PRO	JECT NO: 2	1-0311-1	SHEET 6 OF 14	

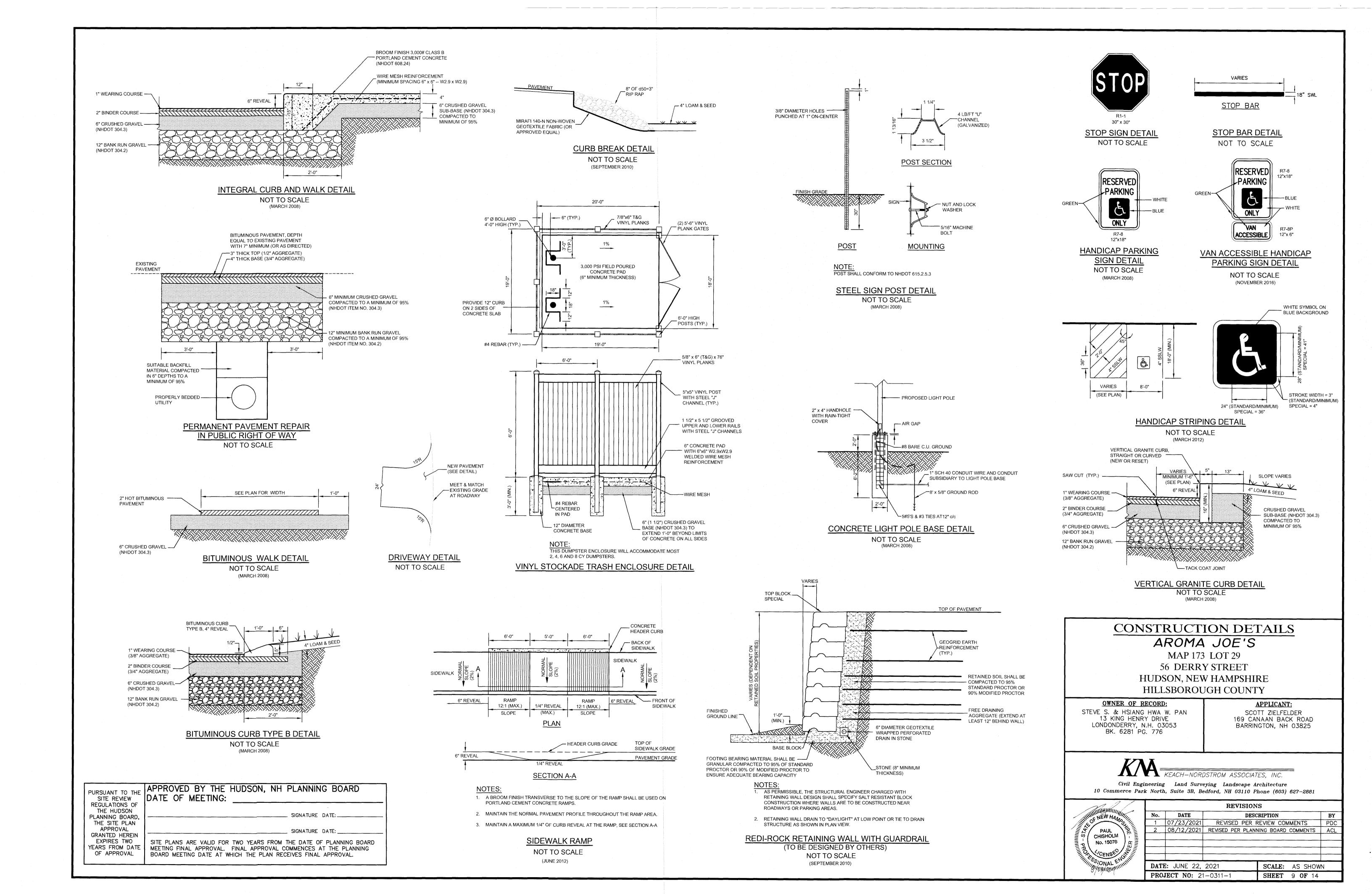
LOAM & SEED ALL DISTURBED AREAS (TYP.)

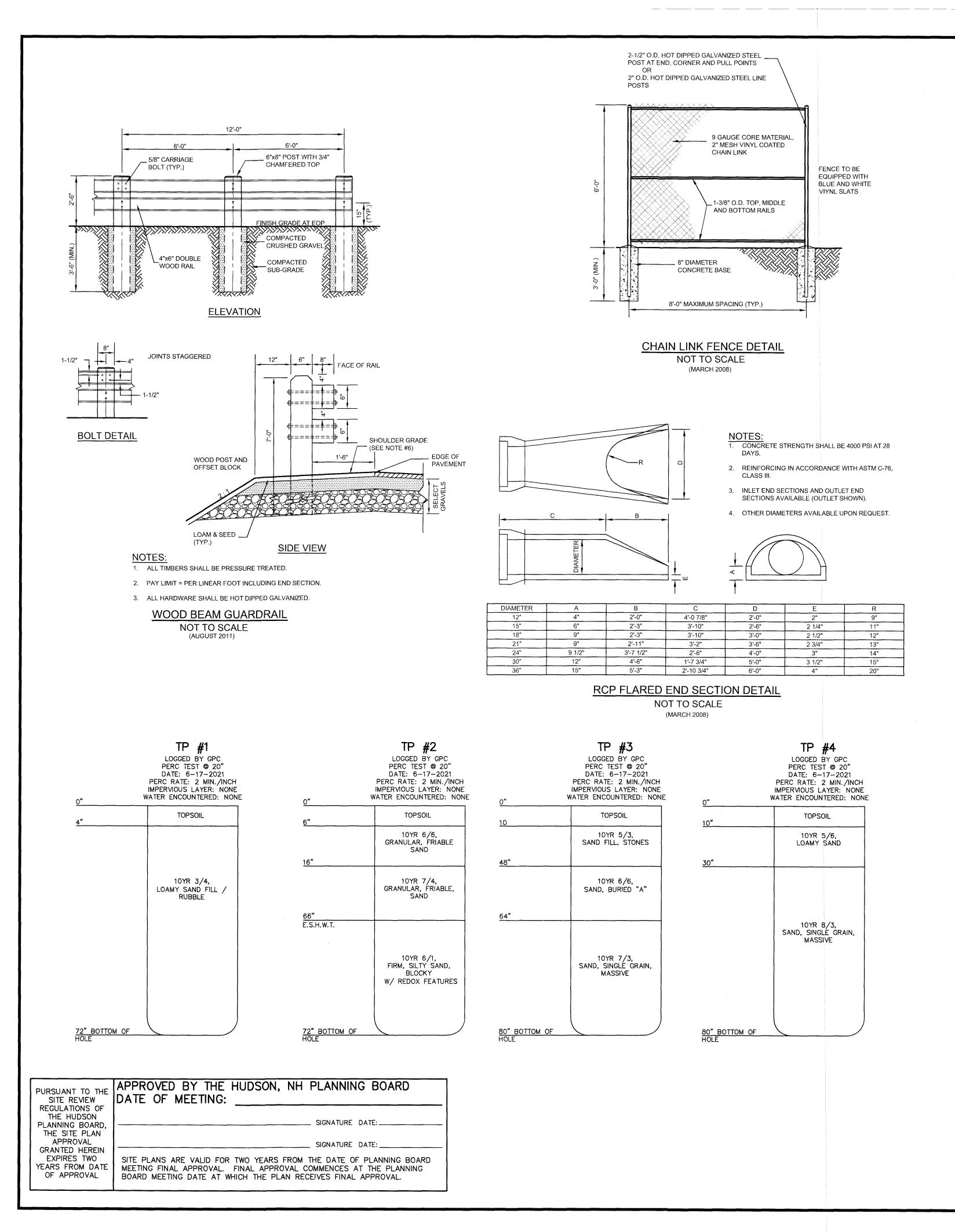
GRAPHIC SCALE

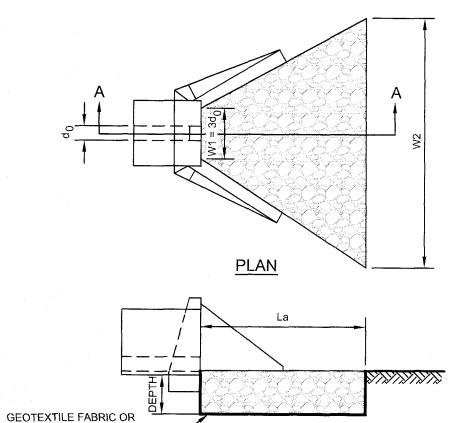
(IN FEET) 1 inch = 20 ft.











PERCENT OF WEIGHT SMALLER THAN THE GIVEN SIZE	SIZE OF STONE
100%	1.5 TO 2.0 d50
85%	1.3 TO 1.8 d50
50%	1.0 TO 1.5 d50
15%	0.3 TO 0.5 d50

CONSTRUCTION SPECIFICATIONS:

- THE SUBGRADE FOR THE FILTER MATERIAL, GEOTEXTILE FABRIC AND RIP RAP SHALL BE PREPARED TO THE LINES AND GRADES SHOWN ON THE PLANS.
- 2. THE ROCK OR GRAVEL USED FOR FILTER OR RIP RAP SHALL CONFORM TO THE SPECIFIED GRADATION.
- 3. GEOTEXTILE FABRICS SHALL BE PROTECTED FROM PUNCTURE OR TEARING DURING THE PLACEMENT OF THE ROCK RIP RAP. DAMAGED AREAS IN THE FABRIC SHALL BE REPAIRED BY PLACING A PIECE OF FABRIC OVER THE DAMAGED AREA OR BY COMPLETE REPLACEMENT OF THE FABRIC. ALL OVERLAPS REQUIRED FOR REPAIRS OR JOINING TWO PIECES OF FABRIC SHALL BE A MINIMUM OF 12 INCHES.
- 4. STONE FOR THE RIP RAP MAY BE PLACED BY EQUIPMENT AND SHALL BE CONSTRUCTED TO THE FULL LAYER THICKNESS IN ONE OPERATION AND IN SUCH A MANNER AS TO PREVENT SEGREGATION OF THE STONE SIZES.

MAINTENANCE:

THE OUTLET PROTECTION SHOULD BE CHECKED AT LEAST ANNUALLY AND AFTER EVERY MAJOR RAIN EVENT. IF THE RIP RAP HAS BEEN DISPLACED, UNDERMINED, OR DAMAGED, IT SHOULD BE REPAIRED IMMEDIATELY. THE CHANNEL IMMEDIATELY BELOW THE OUTLET SHOULD BE CHECKED TO SEE THAT EROSION IS NOT OCCURRING. THE DOWNSTREAM CHANNEL SHOULD BE KEPT CLEAR OF OBSTRUCTIONS SUCH AS FALLEN TREES, DEBRIS, AND SEDIMENT THAT COULD CHANGE FLOW PATTERNS AND/OR TAILWATER DEPTHS ON THE PIPES. REPAIRS MUST BE CARRIED OUT IMMEDIATELY TO AVOID ADDITIONAL DAMAGE TO THE OUTLET PROTECTION APRON.

PIPE OUTLET TO FLAT AREA WITH NO DEFINED CHANNEL NOT TO SCALE

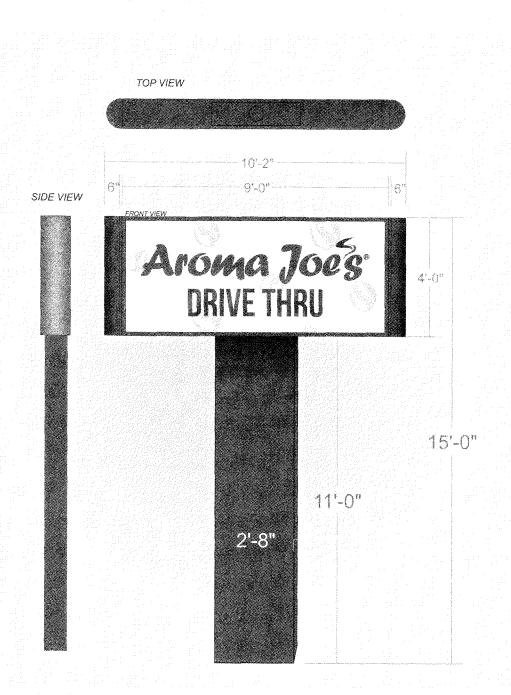
SECTION A-A

FILTER MATERIAL TO BE

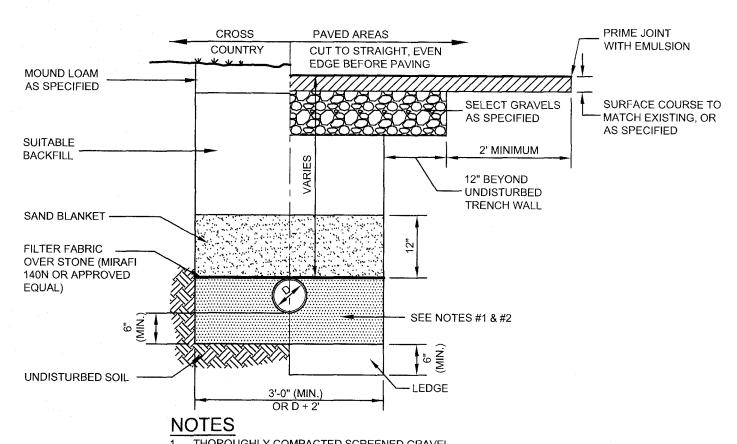
PLACED BETWEEN SOIL

AND RIP RAP (TYP.)

LOCATION	La	W1	W2	d50	DEPTH
N. PIPE	30'	9'	21'	4"	10"
S, PIPE	35'	9'	23'	4"	10"
HW#31	15'	10'	10'	4"	10"
HW#14	30'	10'	10'	4"	10"
LI\N/#16	31'	Λ'	Δ'	A"	10"



FREESTANDING SIGN DETAIL
NOT TO SCALE



THOROUGHLY COMPACTED SCREENED GRAVEL
 FOR RCP PIPE. SCREENED GRAVEL TO EXTEND
 TO SELECT FILL LINE.

2. FOR HDPE OR PVC PIPE, BEDDING SHALL BE 3/4" STONE TO THE TOP OF THE PIPE.

STORM DRAINAGE TRENCH DETAIL
NOT TO SCALE
(MARCH 2008)

CONSTRUCTION DETAILS AROMA JOE'S

MAP 173 LOT 29 56 DERRY STREET HUDSON, NEW HAMPSHIRE

HILLSBOROUGH COUNTY

OWNER OF RECORD:

STEVE S. & HSIANG HWA W. PAN

13 KING HENRY DRIVE

APP

SCOTT

169 CANAA

APPLICANT:
SCOTT ZIELFELDER
169 CANAAN BACK ROAD
BARRINGTON, NH 03825

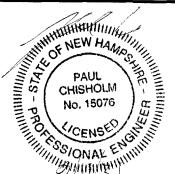


LONDONDERRY, N.H. 03053

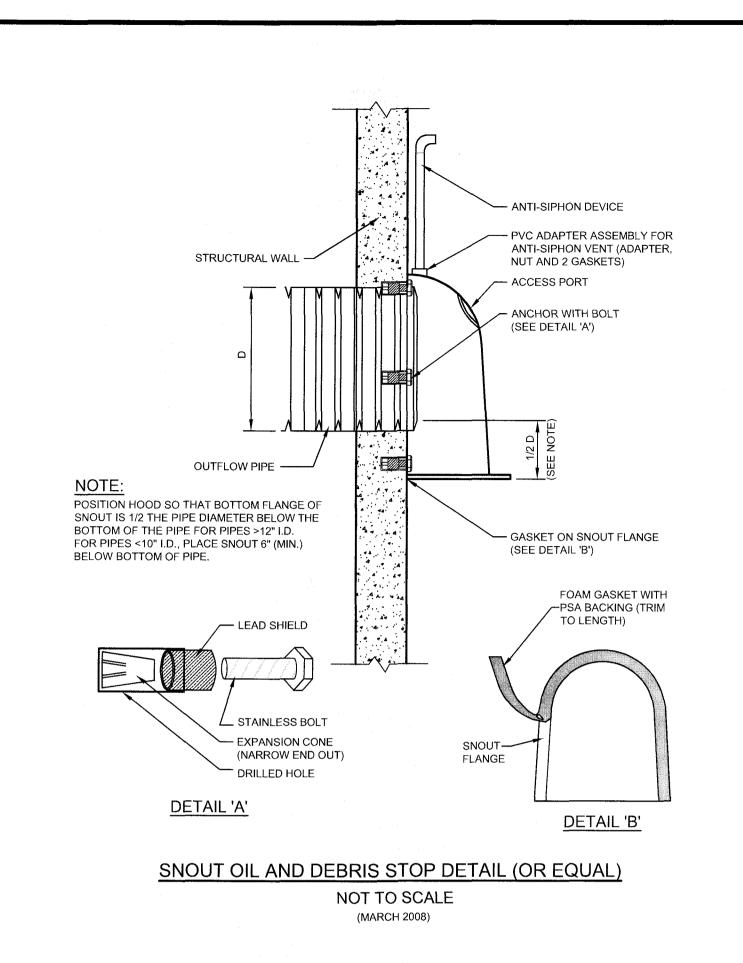
BK. 6281 PG. 776

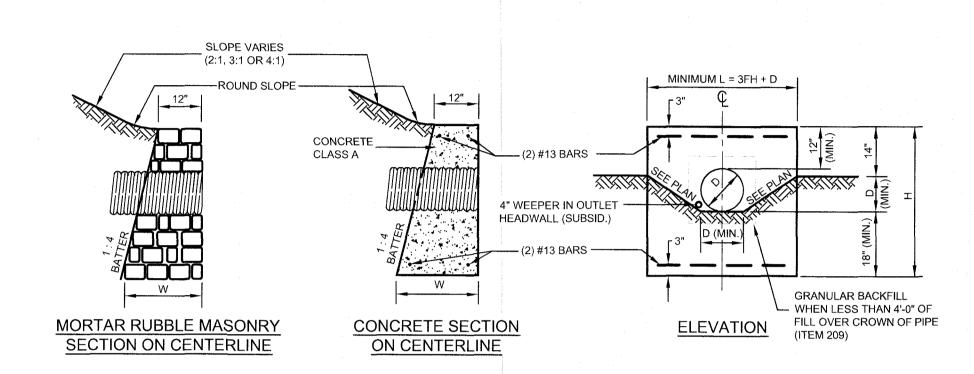
L \perp KEACH-NORDSTROM ASSOCIATES, INC.

Civil Engineering Land Surveying Landscape Architecture
10 Commerce Park North, Suite 3B, Bedford, NH 03110 Phone (603) 627-2881



		REVISIONS	
No.	DATE	DESCRIPTION	BY
1	07/23/2021	REVISED PER REVIEW COMMENTS	PDC
2	08/12/2021	REVISED PER PLANNING BOARD COMMENTS	ACL
			ļ
	<u> </u>		<u> </u>
DAT	E: JUNE 22,	2021 SCALE: AS SHOW	٧N
PRO	JECT NO: 2°	1-0311-1 SHEET 10 OF 14	





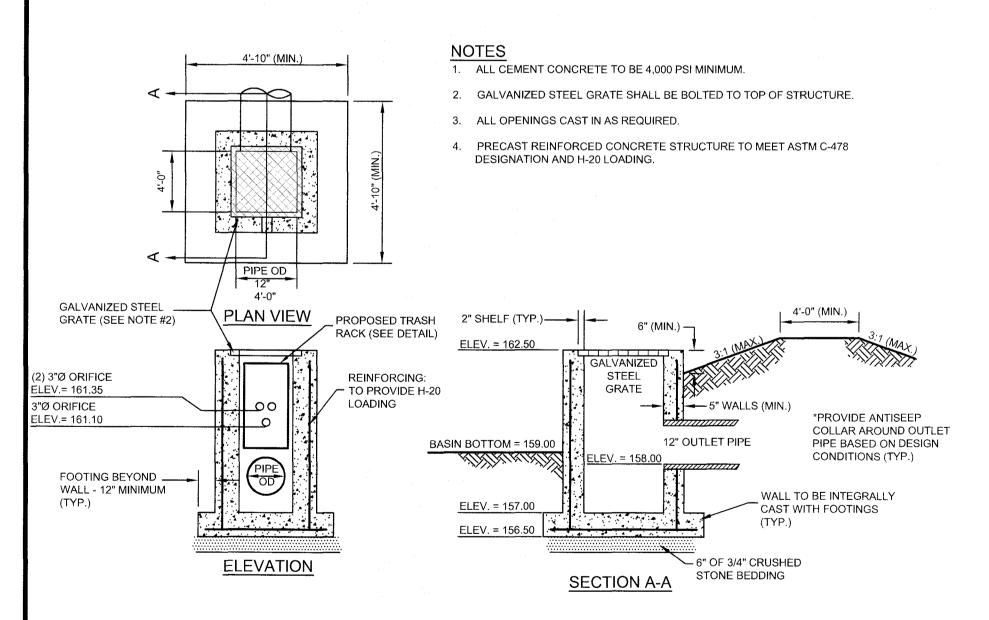
	PPED OUTSIDE F PING FACES.	AYMENT LINES C)N											
						i							"L" HE	ADWALL
AREA OF PIPE (SF)	MASONRY PER FOOT OF WALL (CU. YD.)	MASONRY PER HOLE (CU. FT.)	MASONRY PER STANDARD HEADER (CU. YD.)	STEEL PER STANDARD HEADER (LB)	LENGTH OF BARS	PIPE EXC. 1' DEPTH 1' LENGTH (CU. YD.)	HEADER EXC. PER HEADER 1' DEPTH (CU. YD.)	ITEM 209 PER LINEAR FOOT	HEADER LENGTH L	HEADER HEIGHT H	FILL HEIGHT FH	WIDTH AT BOTTOM OF HEADER W	MASONRY IN CORNER FRUSTRUM (CU. YD)	HEADER EXC. PER HEADER 1' DEPTH (CU. YD.)
1.23	0.202	1.73	0.85	11	3'-10"	0.120	0.947	0.35	4'-6"	3'-9"	1'-1"	1'-11 1/4"	0.31	1.232
1.77	0.222	2.52	1.13	14	5'-2"	0.130	1.111	0.39	5'-6"	4'-0"	1'-4"	2'-0"	0.35	1.406

NOTE: STEEL QUANTITIES ARE FOR CONCRETE HEADWALLS ONLY.

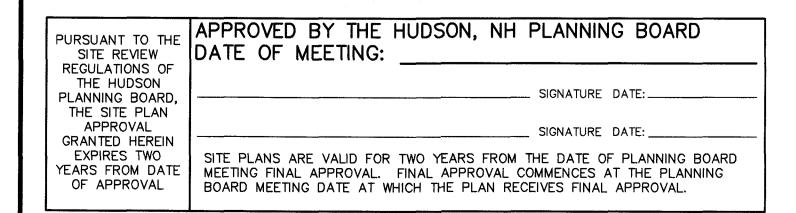
DIMENSIONS SHOWN ARE TO PAYMENT

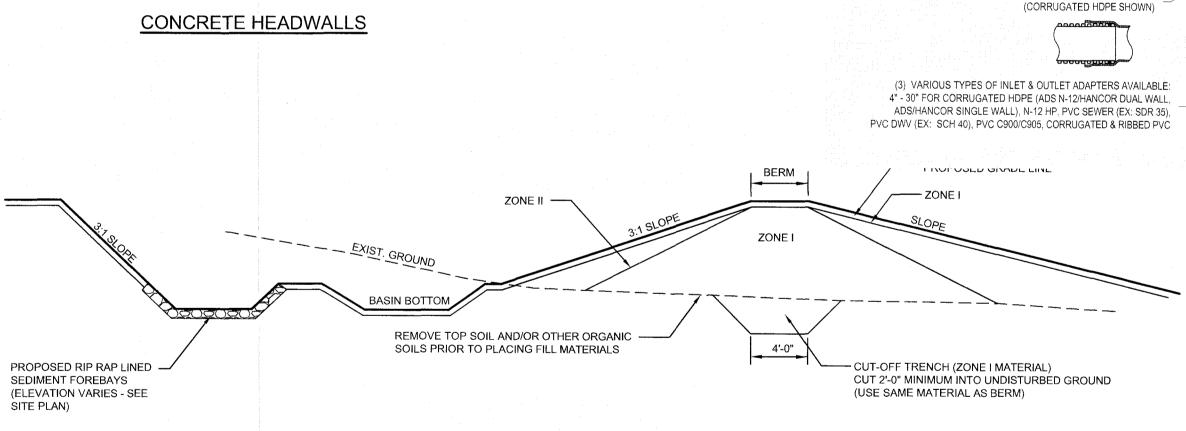
D (INCH)

LINES. MORTAR RUBBLE MASONRY TO BE



OUTLET STRUCTURE #1 NOT TO SCALE





STORMWATER PONDS CONSTRUCTION SEQUENCE

- 1. CONTRACTOR TO NOTIFY DIG-SAFE 72 HOURS PRIOR TO COMMENCEMENT OF CONSTRUCTION. 2. CUT AND CLEAR TREES AND BRUSH FROM CONSTRUCTION AREAS TO THE EXTENT NECESSARY. ALL BRANCHES, TOPS AND BRUSH TO BE PROPERLY DISPOSED OF BY CONTRACTOR.
- 3. PRIOR TO GRUBBING OF CLEARED AREAS, ALL SILTATION BARRIERS DESIGNED FOR USE AS TEMPORARY EROSION CONTROL MEASURES SHALL BE INSTALLED AS CALLED FOR ON PROJECT PLANS.
- 4. COMPLETE GRUBBING OPERATIONS. ALL STUMPS AND SIMILAR DEBRIS SHALL BE PROPERLY DISPOSED OF BY CONTRACTOR. ORGANIC MATERIAL SUITABLE FOR USE AS TOPSOIL SHALL BE STOCKPILED IN UPLAND AREAS. ALL STOCKPILES SHALL BE SEEDED WITH WINTER RYE AND, IF NECESSARY, SURROUNDED WITH HAY BALES IN ORDER TO PREVENT LOSS DUE TO EROSION.
- 5. CONSTRUCT TEMPORARY CULVERTS AS NECESSARY TO FACILITATE CONSTRUCTION ACTIVITIES. ALL SUCH CROSSINGS SHALL BE PROTECTED WITH HAY BALE BARRIERS TO LIMIT EROSION.
- 6. CONSTRUCT CUT-OFF TRENCH (PART OF ZONE I). 7. CONSTRUCT BROAD CRESTED WEIR, ANTI SEEP COLLARS, HEADWALL, AND RIP RAP OUTLET PROTECTION AS SHOWN
- ON PLANS.
- 8. CONSTRUCT ZONE I PORTION OF EARTH EMBANKMENT.
- 9. CONSTRUCT ZONE II PORTION OF EARTH EMBANKMENT.
- 10. APPLY TOPSOIL TO SLOPES AND OTHER AREAS DISTURBED BY CONSTRUCTION. TOPSOIL USED MAY BE NATIVE ORGANIC MATERIAL SCREENED SO AS TO BE FREE OF ROOTS, BRANCHES, STONES, AND OTHER DELETERIOUS MATERIALS. TOPSOIL SHALL BE APPLIED SO AS TO PROVIDE A MINIMUM OF A 4-INCH COMPACTED THICKNESS. UPON COMPLETION OF TOPSOILING, FINISHED SECTIONS ARE TO BE LIMED, SEEDED AND MULCHED. CONSTRUCTION PERSONNEL SHALL INSPECT COMPLETED SECTIONS OF WORK ON A REGULAR BASIS AND REMEDY ANY PROBLEM
- AREAS UNTIL A HEALTHY STAND OF GRASS HAS BECOME ESTABLISHED. 11. MAINTAIN, REPAIR, AND REPLACE AS NECESSARY TEMPORARY EROSION CONTROL MEASURES UNTIL SUCH TIME AS THE ENTIRE CONSTRUCTION AREA HAS BEEN STABILIZED (A MINIMUM OF ONE WINTER SHALL HAVE PASSED).
- 12. AFTER STABILIZATION, REMOVE AND SUITABLY DISPOSE OF TEMPORARY EROSION CONTROL MEASURES. 13. MONITOR CONSTRUCTION ACTIVITIES TO INSURE CONSTRUCTION ACTIVITIES ARE BEING PERFORMED IN SUCH A WAY AS NOT TO ENDANGER THE INTEGRITY OF EARTH EMBANKMENTS, STORMWATER CONTROL STRUCTURE, CULVERT AND RIP RAP OUTLET PROTECTION.
- 14. THE CONTRACTOR SHALL STAKE OUT AND PROTECT THE PROPOSED INFILTRATION AREA DURING ALL CONSTRUCTION ACTIVITIES. NO SOIL COMPACTION OF ANY KIND SHALL TAKE PLACE WITHIN THE INFILTRATION AREA IN ORDER TO MAINTAIN CURRENT SOIL INFILTRATION QUALITIES.

INFILTRATION POND DETAIL NOT TO SCALE

MATERIAL TYPE/SPECIFICATIONS

WELL GRADED MIXTURE OF GRAVEL, SAND, SILT OR CLAY WITH MAX. 6-INCH

SIZE STONE AND GEADATION AS INDICATED BELOW. PLACE IN MAX. 12-INCH THICK LIFTS TO 95% OF MAX. DRY DENSITY IN ACCORDANCE WITH ASTM D1557.

SCARIFY SURFACE PRIOR TO PLACING SUBSEQUENT LIFT. IN ADDITION, REMOVE ORGANIC SOILS.

SIEVE SIZE PERCENT BY WEIGHT PASSING 6-INCH

NO. 40

NO. 200

50 TO 100

30 TO 70

20 TO 40

DRAINAGE LAYER: PLACE IN MAX. 12-INCH THICK LIFTS TO 95% OF MAX. DRY

DENSITY IN ACCORDANCE WITH ASTM D1557. SIEVE SIZE PERCENT BY WEIGHT PASSING 1-INCH

NO. 4 70-100 NO. 200 0-12 (IN SAND PORTION ONLY)

AROMA JOE'S MAP 173 LOT 29

3099CGRDF & 3099CGRFH

APPROX. GRATE DRAIN AREA = 232.87 SQ IN *APPROX. WEIGHT WITH FRAME & HOOD = 344.00 LBS

24.25 27.25

8.73 HIGHEST HOOD SETTING

4.73 LOWEST HOOD SETTING

*WEIGHT DOES NOT INCLUDE

18BASER: APPROX. 133.50 LBS 24BASER: APPROX. 122.00 LBS

30BASER: APPROX. 92.50 LBS

DUCTILE IRON BASE PLATE

18" MIN WIDTH GUIDELINE

DESIGN FACTORS.

(1) INTEGRATED DUCTILE IRON

BASE PLATE TO MATCH BASIN O.D.

4" MIN ON 18" & 24" 6" MIN ON 30"

- 8" MIN THICKNESS GUIDELINE

TRAFFIC LOADS: CONCRETE SLAB DIMENSIONS ARE FOR GUIDELINE PURPOSES ONLY. ACTUAL CONCRETE SLAB

MUST BE DESIGNED TAKING INTO CONSIDERATION LOCAL

SOIL CONDITIONS, TRAFFIC LOADING, & OTHER APPLICABLE

(2) VARIABLE SUMP DEPTH

ACCORDING TO PLANS

(6" MIN ON 18" & 24", 10" MIN ON 30"

BASED ON MANUFACTURING REQ.)

THE BACKFILL MATERIAL SHALL BE CRUSHED STONE OR OTHER

GRANULAR MATERIAL MEETING THE REQUIREMENTS OF CLASS I,

PLACED & COMPACTED UNIFORMLY IN ACCORDANCE WITH ASTM D232:

NOT TO SCALE

CLASS II, OR CLASS III MATERIAL AS DEFINED IN ASTM D2321. BEDDING & BACKFILL FOR SURFACE DRAINAGE INLETS SHALL BE

PRAINS TO WATERWAYS CNYLORLAST

ADJUSTMENT SLOTS

TOP OF BASE PLATE TO TOP OF DRAIN BASIN 18BASER: 2.83

24BASER: 2.83

30BASER: .44

2' X 3' CURB INLET DIAGONAL GRATE 3099CGRDF

> 2' X 3' CURB INLET FRAME & HOOD

NYLOPLAST 2

CONCRETE CURB & GUTTER

MINIMUM PIPE BURIAL

DEPTH PER PIPE

MANUFACTURER RECOMMENDATION

(MIN. MANUFACTURING

REQ. SAME AS MIN. SUMP)

WATERTIGHT JOINT

(1) DUCTILE IRON

FRAME, GRATE, & HOOD

3099CGRFH

34.75

- 16 X SLOT Ø 1.00 THRU

(2) VARIABLE INVERT HEIGHTS

AVAILABLE (ACCORDING TO

PLANS/TAKE OFF)

56 DERRY STREET HUDSON, NEW HAMPSHIRE HILLSBOROUGH COUNTY

CONSTRUCTION DETAILS

OWNER OF RECORD: STEVE S. & HSIANG HWA W. PAN 13 KING HENRY DRIVE LONDONDERRY, N.H. 03053 BK. 6281 PG. 776

NYLOPLAST TYPICAL DETAILS

SCOTT ZIELFELDER 169 CANAAN BACK ROAD BARRINGTON, NH 03825

APPLICANT:

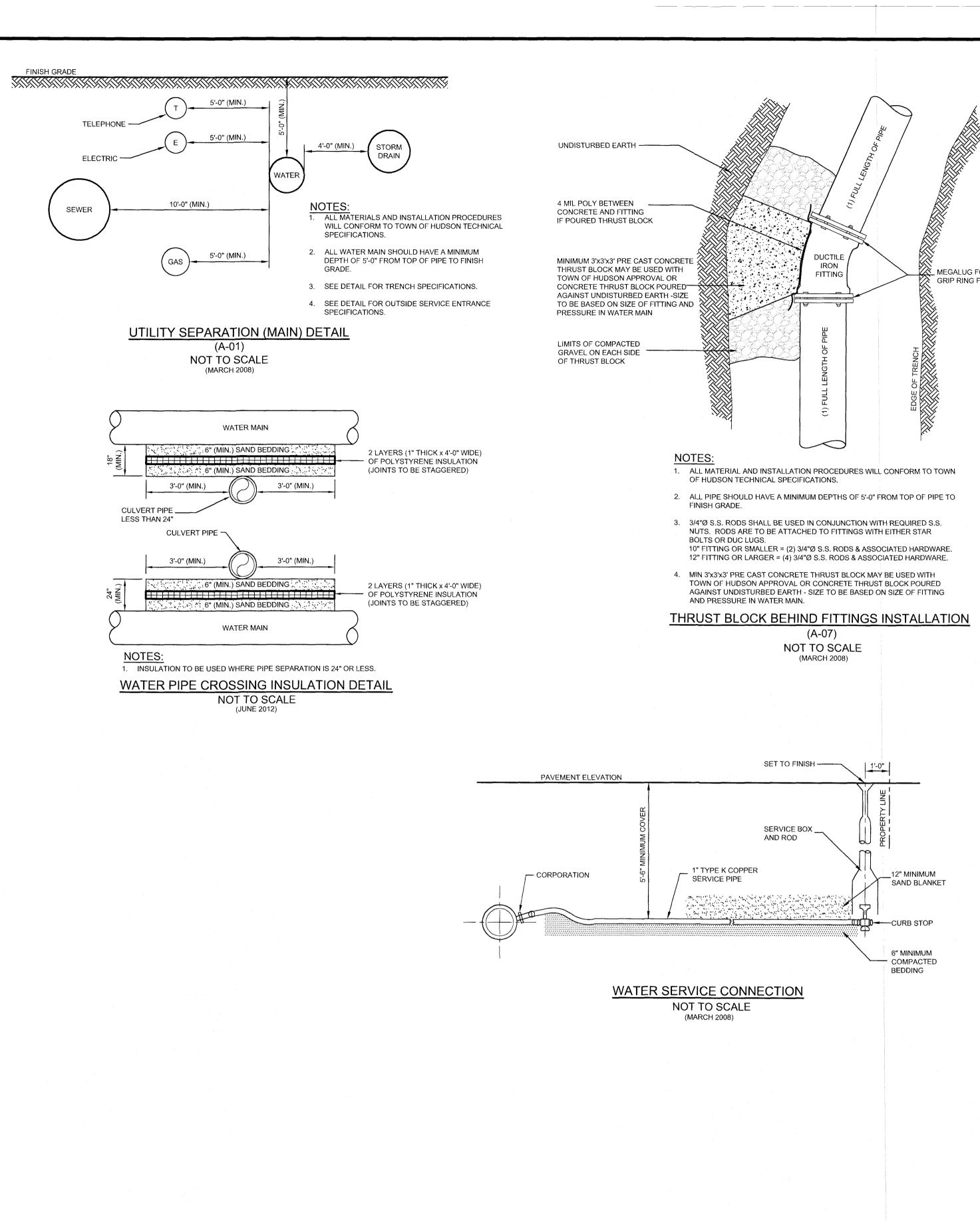


** KEACH-NORDSTROM ASSOCIATES, INC.

Civil Engineering Land Surveying Landscape Architecture 10 Commerce Park North, Suite 3B, Bedford, NH 03110 Phone (603) 627-2881

PAUL CHISHOLM No. 15076 CENSED ONAL ENGINEERS	
/ / / C	

REVISIONS							
No.	DATE	DESC	DESCRIPTION				
1	07/23/2021	REVISED PER F	REVIEW CON	MENTS	PDC		
2	08/12/2021	REVISED PER PLANT	NING BOARD	COMMENTS	ACL		
DATE	DATE: JUNE 22, 2021 SCALE: AS SHOWN						
PRO.	JECT NO: 2	1-0311-1	SHEET	11 OF 14			



APPROVED BY THE HUDSON, NH PLANNING BOARD

SITE PLANS ARE VALID FOR TWO YEARS FROM THE DATE OF PLANNING BOARD

BOARD MEETING DATE AT WHICH THE PLAN RECEIVES FINAL APPROVAL.

EARS FROM DATE | MEETING FINAL APPROVAL. FINAL APPROVAL COMMENCES AT THE PLANNING

SIGNATURE DATE: ___

SIGNATURE DATE: ___

DATE OF MEETING:

PURSUANT TO THE

SITE REVIEW

REGULATIONS OF

THE HUDSON

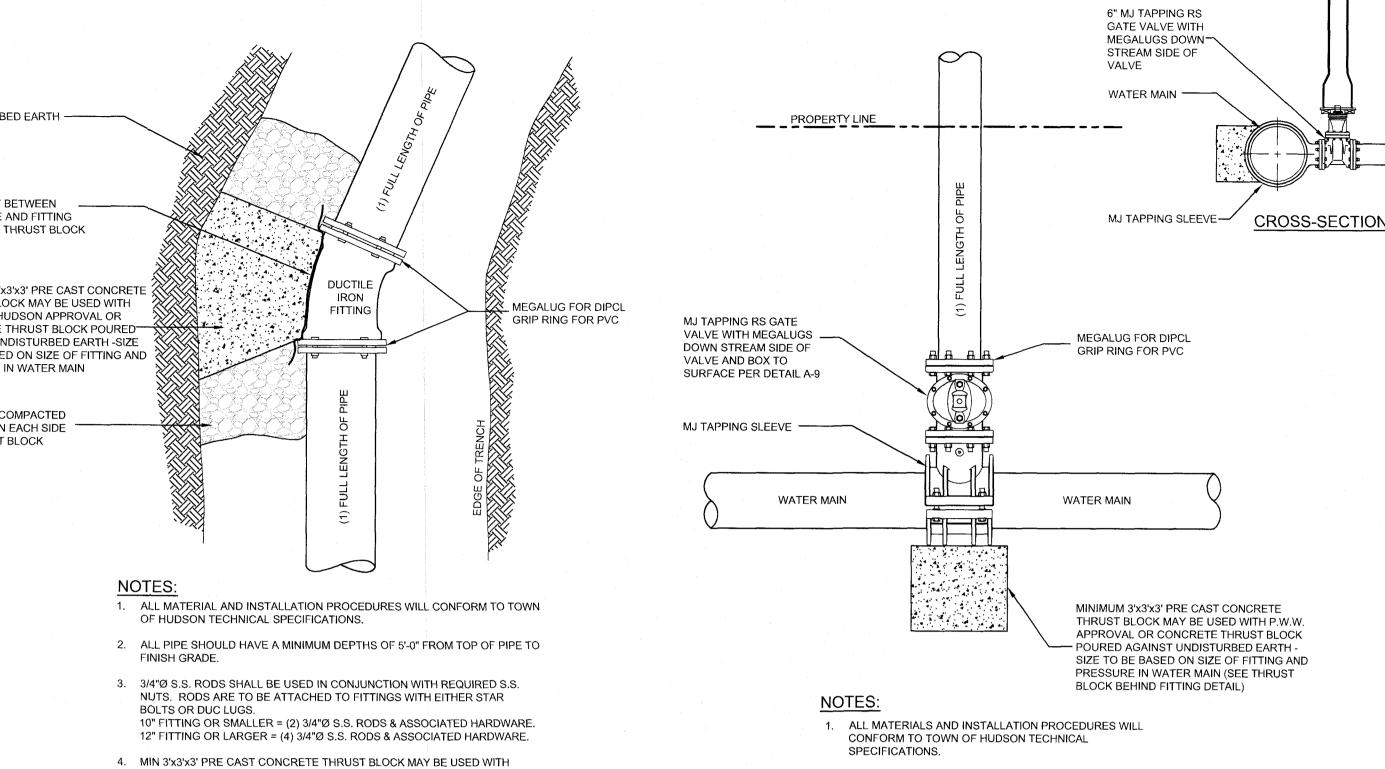
PLANNING BOARD

THE SITE PLAN

APPROVAL

GRANTED HEREIN EXPIRES TWO

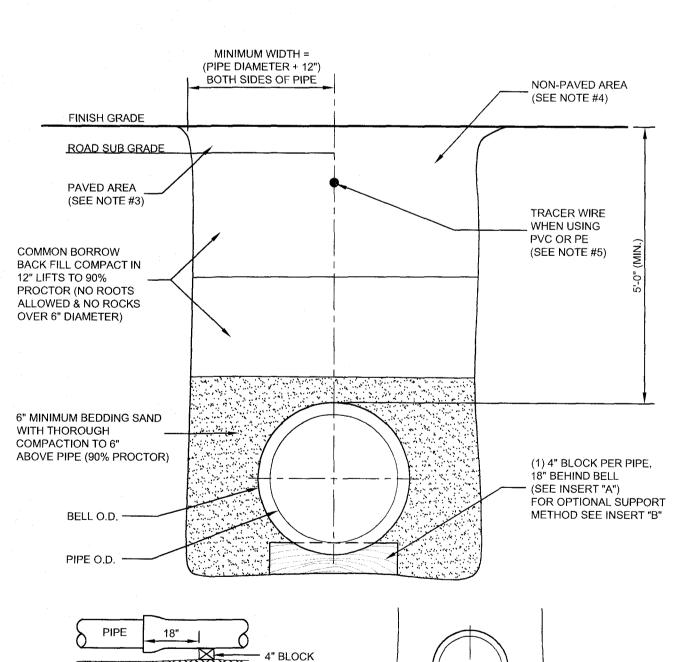
OF APPROVAL



2. ALL PIPE SHOULD HAVE A MINIMUM DEPTH OF 5'-0" FROM TOP OF PIPE TO FINISH GRADE.

LARGE SERVICE AND/OR TAPPING SLEEVE DETAIL NOT TO SCALE

(MARCH 2008)



1. ALL MATERIALS AND INSTALLATION PROCEDURES WILL CONFORM TO TOWN OF HUDSON TECHNICAL SPECIFICATIONS.

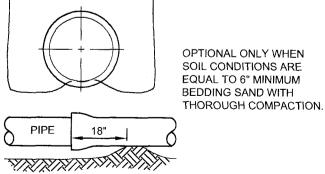
INSERT "A"

2. ALL PIPE SHOULD HAVE A MINIMUM DEPTH OF 5'-0" FROM TOP OF PIPE TO FINISH GRADE.

3. REQUIREMENTS FOR SUBBASE AND BASE MATERIAL TYPE ARE

TO BE IN ACCORDANCE WITH LOCAL AUTHORITY HAVING LOCAL

- JURISDICTION IN PAVED AREAS. 4. REQUIREMENTS FOR GRAVEL, LOAM AND/OR SEED ARE TO BE
- IN ACCORDANCE WITH LOCAL AUTHORITY HAVING LOCAL JURISDICTION IN NON-PAVED AREAS.
- 5. 10 GAUGE TRACER WIRE AS MANUFACTURED BY BMS, DIVISION OF ALBESTAR CORP., AVON, MA OR EQUIVALENT.



TRENCH DETAIL (A-02)NOT TO SCALE

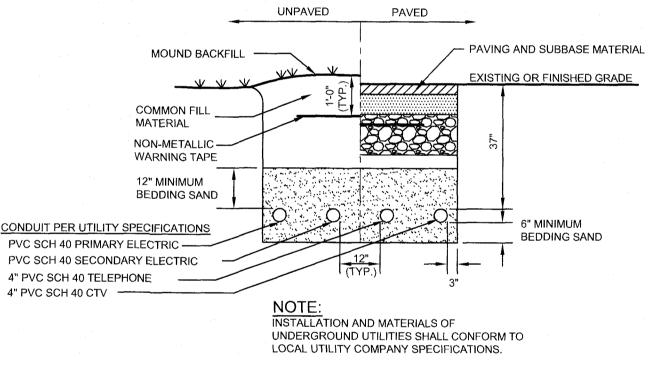
(MARCH 2008)

INSERT "B"

CONSTRUCTION NOTES:

D.I. PIPE

- THE CONTRACTOR SHALL VERIFY THE LOCATION AND ELEVATION OF ALL EXISTING UTILITIES PRIOR TO COMMENCING CONSTRUCTION. IN THE EVENT THAT A DISCREPANCY FROM THE INFORMATION SHOWN ON THESE PLANS IS FOUND, THE CONTRACTOR SHALL NOTIFY THE ENGINEER TO RESOLVE THE SITUATION.
- 2. PRIOR TO CONSTRUCTION, CONTACT DIG SAFE CENTER, TOLL FREE 811. NEW HAMPSHIRE STATE LAW REQUIRES NOTIFICATION AT LEAST THREE BUSINESS DAYS BEFORE DIGGING OPERATIONS START. IF AN EMERGENCY, CALL IMMEDIATELY.
- ALL WORKMANSHIP AND MATERIALS INCORPORATED INTO THE CONSTRUCTION OF ROADS AND DRAINAGE SHOWN HEREON SHALL CONFORM WITH THE STANDARDS OF THE TOWN OF HUDSON ENGINEERING DEPARTMENT AND THE "STANDARD SPECIFICATIONS FOR ROAD AND BRIDGE CONSTRUCTION" AS PUBLISHED AND AMENDED BY THE N.H.D.O.T.
- THE CONTRACTOR SHALL BE RESPONSIBLE FOR MAINTAINING ALL EROSION AND SEDIMENT CONTROL DEVICES, AS SHOWN IN THESE PLANS, THROUGHOUT THE DURATION OF THE PROJECT IN ACCORDANCE WITH APPLICABLE NHDES STANDARDS. THE EROSION CONTROL MEASURES PROVIDED SERVE AS A GUIDE ONLY AND SHALL BE CONSIDERED A MINIMUM STANDARD. CONTRACTOR SHALL IMPLEMENT ANY AND ALL ADDITIONAL MEASURES AS FIELD CONDITIONS DICTATE OR AS REQUIRED BY LOCAL OR STATE GUIDELINES. ALL EROSION CONTROL SHALL BE MAINTAINED AND/OR REPLACED IF
- 5. THE CONTRACTOR SHALL STABILIZE ALL DITCHES, SWALES AND PONDS PRIOR TO
- DIRECTING STORM WATER RUN-OFF TO THEM. 6. SILT SOXX TO BE CONSTRUCTED AS IDENTIFIED ON THIS PLAN. SILT SOXX BARRIERS TO BE CHECKED WEEKLY AND/OR AFTER LARGE STORM EVENTS. ALL SILT MATERIAL
- TO BE REMOVED. ANY DAMAGED SILT SOXX SHALL BE REINSTALLED IMMEDIATELY. 7. SLOPE STABILIZATION METHODS SHALL BE PROVIDED IN ACCORDANCE WITH THE NHDES SITE SPECIFIC PERMIT OR AS FIELD CONDITIONS DICTATE. SLOPE STABILIZATION SHALL BE PROVIDED ON ALL SLOPES 3:1 AND GREATER AS A MINIMUM STANDARD. PROVIDE SLOPE STABILIZATION ON SLOPES LESS THAN 3:1 IF WARRANTED, AS FIELD CONDITIONS
- 8. THE DRAINAGE DESIGN AND EROSION CONTROL MEASURES AS PROPOSED MEET ALL TOWN AND STATE REQUIREMENTS AND BEST MANAGEMENT PRACTICES AS PROMULGATED
- 9. ALL EXCAVATIONS SHALL BE THOROUGHLY SECURED ON A DAILY BASIS BY THE CONTRACTOR AT THE COMPLETION OF CONSTRICTION OPERATIONS IN THE IMMEDIATE
- 10. TOPSOIL SHALL BE REMOVED FOR ITS TOTAL DEPTH WITHIN THE LIMITS OF GRADING. UNLESS OTHERWISE DIRECTED. THE TOPSOIL SHALL BE STOCKPILED AND USED IN ITS ENTIRETY. STOCKPILE SOIL IN ACCORDANCE WITH NHDOT SPECIFICATIONS FOR ROAD AND BRIDGE CONSTRUCTION (CURRENT EDITION).
- 11. UNSUITABLE MATERIAL, ROOTS, AND STUMPS WITHIN THE LIMITS OF WORK SHALL BE REMOVED FROM THE SITE AS ORDERED. DISPOSAL OF SUCH MATERIALS SHALL BE IN ACCORDANCE WITH LOCAL, STATE, AND FEDERAL REGULATIONS AND GUIDELINES AS
- APPLICABLE OR AS DIRECTED. 12. THE SUBGRADE SHALL BE SCARIFIED TO ASSURE THAT ALL BOULDERS AND COBBLES
- OVER 6" ARE REMOVED PRIOR TO SHAPE, GRADE, AND COMPACTION ACTIVITIES. 13. ALL WORKMANSHIP AND MATERIALS INCORPORATED INTO THE CONSTRUCTION OF ELECTRIC LINES SHALL CONFORM WITH THE STANDARDS OF THE OWNING UTILITY
- 14. ALL WORKMANSHIP AND MATERIALS INCORPORATED INTO THE CONSTRUCTION OF
- TELEPHONE LINES SHALL CONFORM WITH THE STANDARDS OF THE OWNING TELEPHONE COMPANY 15. THE CONTRACTOR SHALL CONTACT ALL UTILITY COMPANIES OWNING UTILITIES, EITHER OVERHEAD OR UNDERGROUND WITHIN THE CONSTRUCTION AREA. THE PROTECTION OR
- RELOCATION OF UTILITIES IS ULTIMATELY THE RESPONSIBILITY OF THE CONTRACTOR. 16. THE CONTRACTOR SHALL COORDINATE MATERIALS AND INSTALLATION SPECIFICATIONS WITH THE INDIVIDUAL UTILITY AGENCIES/COMPANIES AND ARRANGE FOR ALL
- 17. UTILITY STUB CONNECTIONS SHOWN TO PROPOSED LOTS ARE APPROXIMATE AND INTENDED TO ILLUSTRATE FEASIBILITY OF CONSTRUCTING CONNECTIONS. THE CONTRACTOR SHALL PROVIDE SERVICE STUBS AS DIRECTED BY THE OWNING UTILITY COMPANY AND LOCAL AUTHORITY.
- 18. CONTRACTOR SHALL COORDINATE WITH PHONE, CABLE, AND ELECTRIC COMPANIES FOR POTENTIAL NEED OF INDIVIDUAL SECTORS AT EACH LOT SERVICE CONNECTION. 19. THE CONTRACTOR SHALL MAINTAIN EMERGENCY ACCESS TO ALL AREAS AFFECTED BY HIS WORK AT ALL TIMES.
- 20. THE CONTRACTOR SHALL VERIFY TBM ELEVATIONS PRIOR TO CONSTRUCTION.



UTILITY TRENCH DETAIL NOT TO SCALE (MARCH 2008)

CONSTRUCTION DETAILS

AROMA JOE'S MAP 173 LOT 29

56 DERRY STREET HUDSON, NEW HAMPSHIRE HILLSBOROUGH COUNTY

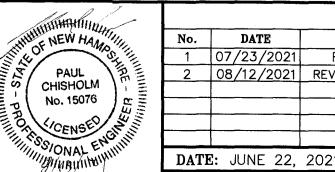
OWNER OF RECORD: STEVE S. & HSIANG HWA W. PAN 13 KING HENRY DRIVE LONDONDERRY, N.H. 03053

BK. 6281 PG. 776

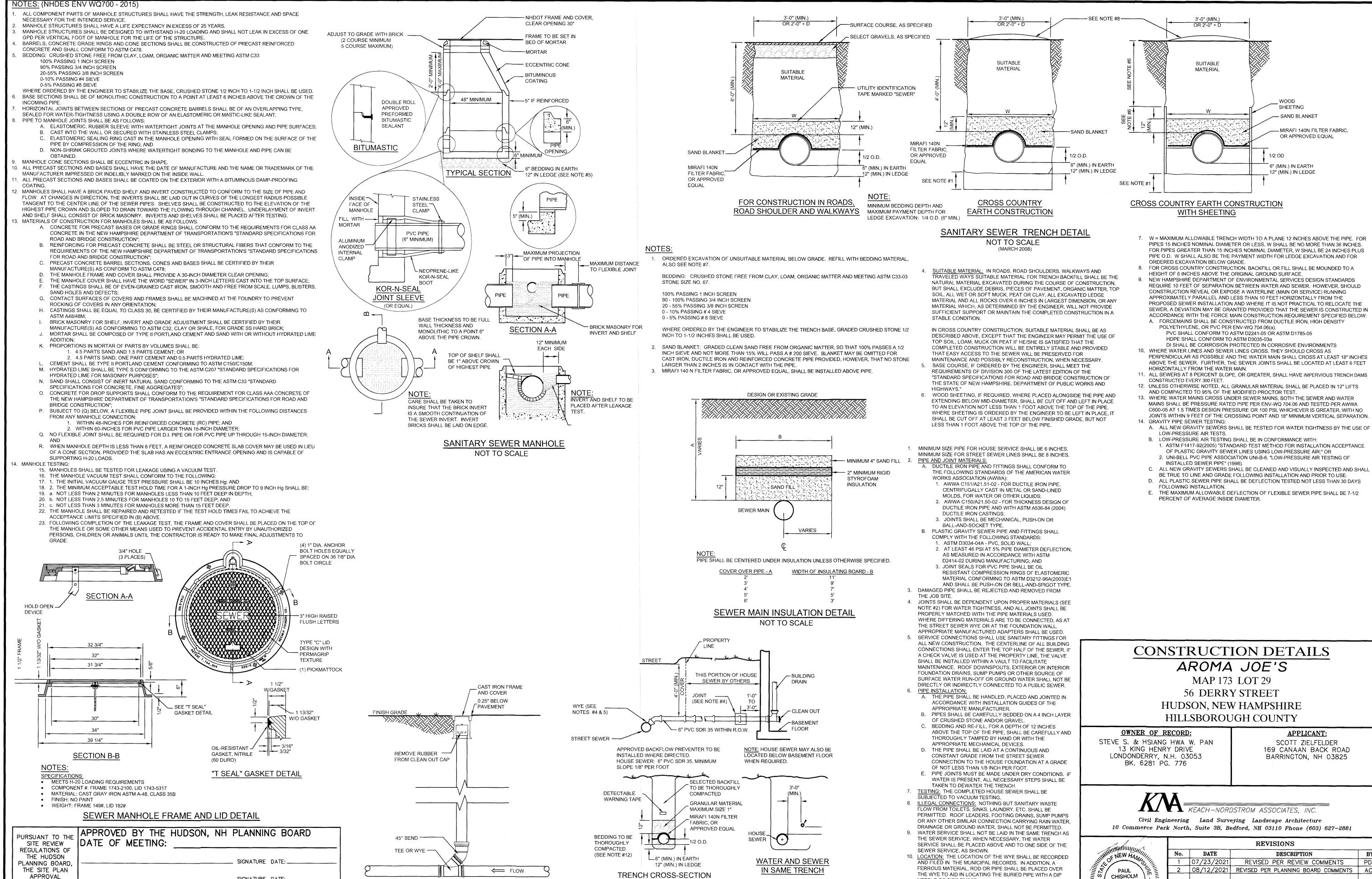
APPLICANT: SCOTT ZIELFELDER 169 CANAAN BACK ROAD BARRINGTON, NH 03825



Civil Engineering Land Surveying Landscape Architecture 10 Commerce Park North, Suite 3B, Bedford, NH 03110 Phone (603) 627-2881



		REVISION	NS .	
No.	DATE	DES	CRIPTION	BY
1	07/23/2021	REVISED PER	REVIEW COMMENTS	PDC
2	08/12/2021	REVISED PER PLAN	NING BOARD COMMENTS	ACL
DAT	E: JUNE 22,	2021	SCALE: AS SHOW	N
PRO	JECT NO: 21	1-0311-1	SHEET 12 OF 14	



SANITARY SEWER SERVICE DETAIL

NOT TO SCALE

SIGNATURE DATE: _

SITE PLANS ARE VALID FOR TWO YEARS FROM THE DATE OF PLANNING BOARD

MEETING FINAL APPROVAL. FINAL APPROVAL COMMENCES AT THE PLANNING

BOARD MEETING DATE AT WHICH THE PLAN RECEIVES FINAL APPROVAL.

CLEANOUTS ON SERVICE LATERALS (4" OR 6")

NOT TO SCALE

GRANTED HEREIN

EXPIRES TWO

OF APPROVAL

YEARS FROM DATE |

DATE: JUNE 22, 2021 SCALE: AS SHOWN **PROJECT NO:** 21-0311-1 **SHEET** 13 **OF** 14

No. 15076

(CENSE)

MONAL

NEEDLE OR PIPE FINDER.

THE MODIFIED PROCTOR TEST DENSITY.

11. UNLESS OTHERWISE NOTED, ALL GRANULAR MATERIAL SHALL

BE PLACED IN 12" MAXIMUM LIFTS AND COMPACTED TO 95% OF

TURF ESTABLISHMENT SCHEDULE

TO ESTABLISH AND MAINTAIN PERMANENT AND TEMPORARY TURF AREAS, RESTORE GROWTH TO EXISTING TURF AREAS DISTURBED DURING CONSTRUCTION AND CONTROL SOIL EROSION.

PREPARATION AND EXECUTION:

- RAKE THE SUBGRADE OF ALL AREAS TO BE LOAMED AND SEEDED TO REMOVE RUBBISH, STICKS, ROOTS AND STONES LARGER THAN 1 INCH.
- PLACE LOAM OVER AREAS TO BE SEEDED AND SPREAD
- FINE GRADE SURFACE AND SUPPLEMENT WITH SUITABLE LOAM WHERE NEEDED TO CREATE A UNIFORM SURFACE ACCORDING TO THE FINISH GRADES INDICATED; TOP AND BOTTOM OF SLOPES SHALL BE ROUNDED. NO LOAM SHALL BE SPREAD IF THE SUBGRADE IS EXCESSIVELY WET OR
- 4. IF THE pH OF THE SOIL NEEDS TO BE RAISED, APPLY LIME EVENLY OVER LOAM SURFACE AND THOROUGHLY INCORPORATE LIME INTO THE LOAM BY HEAVY RAKING TO AT LEAST ONE-HALF THE DEPTH OF THE LOAM.
- APPLY FERTILIZER AND MIX WITH THE UPPER 2 INCHES OF LOAM.
- DETERMINE APPROPRIATE MIXTURE FOR AREA TO BE SEEDED BASED ON EXAMINATION OF PROJECT PLANS. UNIFORMLY SPREAD THE SEED BY BROADCASTING OR HYDROSEEDING. IF BROADCASTING. LIGHTLY RAKE INTO THE PREPARED SURFACE AND ROLL. IF, HYDROSEEDING, USE 4 TIMES THE RECOMMENDED RATE OF INOCULANT. AFTER SEED IS SPREAD, WATER THOROUGHLY WITH A FINE
- SEEDING AND INITIAL FERTILIZING SHALL BE DONE BETWEEN APRIL 1 AND JUNE 1 OR BETWEEN AUGUST 15 AND OCTOBER 14, OR AS PERMITTED. SEEDING SHALL NOT BE DONE DURING WINDY WEATHER OR WHEN THE GROUND IS FROZEN, EXCESSIVELY WET OR OTHERWISE UNTILLABLE.
- 8. WITHIN 24 HOURS AFTER SEEDING OPERATION, UNIFORMLY MULCH THE AREA WITH STRAW. ANCHOR MULCH ON ALL SLOPES EXCEEDING 3: 1 USING MULCH NETTING INSTALLED IN ACCORDANCE WITH THE MANUFACTURER.
- 9. PROTECT AND PREVENT AGAINST WASHOUTS, ANY WASHOUTS WHICH OCCUR SHALL BE PROMPTLY REGRADED AND RESERVED.
- 10. WHEN IT IS IMPRACTICAL TO ESTABLISH PERMANENT GROWTH ON DISTURBED EARTH BY OCTOBER 14, A TEMPORARY SEED MIXTURE SHALL BE USED. WHEN TEMPORARY SEEDING CANNOT ESTABLISH VISIBLE GROWTH, THE DISTURBED AREA SHALL BE COVERED WITH SIX INCHES OF MULCH FOR THE

MAINTENANCE

ALL SEEDED AREAS SHALL BE KEPT WATERED AND IN GOOD CONDITION. RESEED AS NECESSARY TO ESTABLISH HEALTHY UNIFORM GROWTH OVER THE ENTIRE SEEDED AREA. MAINTAIN SEEDED AREAS IN AN APPROVED CONDITION UNTIL FINAL ACCEPTANCE. MAINTENANCE SHALL INCLUDE REPAIRS FOR DAMAGE CAUSED BY EROSION

LOAM SHALL BE APPLIED AT A MINIMUM COMPACTED THICKNESS OF 4 INCHES. 2. LIME SHALL BE USED WHEN NECESSARY TO RAISE THE pH OF THE SOIL AND APPLIED AT ONE OF THE

EXISTING SOIL Ph	TONS/ACRE	POUNDS/CUBIC YARD
4.0 - 4.4	3	1.2
4.5 - 4.9	2	0.8
5.0 - 5.4	1	0.4

3. FERTILIZER SHALL BE APPLIED AT THE FOLLOWING RATE:

INITIAL APPLICATION	POUNDS/1,000 SF	MEASUREMENT FACTOR
10-10-10	20.0	1.0
15-15-15	13.4	1.5
19-19-19	10.5	1.9
REFERTILIZATION	POUNDS/1,000 SF	MEASUREMENT FACTOR
10-3-6	20.0	1.0
12-2-8	16.7	1.2

4. MULCH SHALL BE APPLIED AT A RATE OF 13 CUBIC YARDS PER 1,000 S.F. OF LANDSCAPE BED.

- LOAM SHALL CONSIST OF LOOSE, FRIABLE TOPSOIL WITH NO ADMIXTURE OF REFUSE OR MATERIAL TOXIC TO PLANT GROWTH. LOAM SHALL BE FREE OF VIABLE PARTS OF PROHIBITED INVASIVE PLANTS AND BE GENERALLY FREE OF STONES, LUMPS, STUMPS AND SIMILAR OBJECTS LARGER THAN 2 INCHES IN GREATEST DIAMETER, SUBSOIL, ROOTS AND WEEDS. THE MINIMUM AND MAXIMUM
- pH VALUE SHALL BE FROM 5.5 TO 7.6. LIME SHALL BE A CALCIC OR DOLOMITIC GROUND AGRICULTURAL LIMESTONE CONTAINING NOT LESS THAN 95% OF EITHER CALCIUM OR MAGNESIUM CARBONATE, OR BOTH. IT SHALL CONFORM TO THE STANDARDS OF THE ASSOCIATION OF OFFICIAL AGRICULTURAL CHEMISTS AND SHALL COMPLY WITH ALL STATE AND FEDERAL RULES AND REGULATIONS.
- FERTILIZER SHALL BE STANDARD COMMERCIAL GRADE FERTILIZER CONFORMING TO ALL STATE AND FEDERAL RULES AND REGULATIONS AND TO THE STANDARDS OF THE ASSOCIATION OF OFFICIAL AGRICULTURAL CHEMISTS. EXCEPT AS PERMITTED. THE ANALYSIS RATIO SHALL BE 1:1:1 FOR INITIAL APPLICATION AND 3:1:2 FOR REFERTILIZATION APPLICATION
- GRASS SEED SHALL MEET THE REQUIREMENTS OF THE NEW HAMPSHIRE AGRICULTURAL AND VEGETABLE SEED LAWS AND SHALL INCLUDE NO "PRIMARY NOXIOUS WEED SEEDS."

5. SEED MIXTURE FOR LAWN AREAS SHALL CONSIST OF THE FOLLOWING:							
KIND OF SEED	MINIMUM PURITY (%)	MINIMUM GERMANATION (%)	POUNDS/ACRE (TOTAL 120 POUNDS)				
CREEPING RED FESCUE	96	85	40				
PERENNIAL RYEGRASS	98	90	50				

6. SEED MIXTURE FOR SLOPE AREAS SHALL CONSIST OF THE FOLLOWING:

WINTER CONSTRUCTION NOTES

APPROPRIATE FOR THE DESIGN FLOW CONDITIONS.

KENTUCKY BLUEGRASS

REDTOP

SITE REVIEW

REGULATIONS OF

THE HUDSON

PLANNING BOARD,

THE SITE PLAN

APPROVAL

GRANTED HEREIN

EXPIRES TWO

OF APPROVAL

YEARS FROM DATE

KIND OF SEED	MINIMUM PURITY (%)	MINIMUM GERMANATION (%)	POUNDS/ACRE (TOTAL 95 POUNDS)
CREEPING RED FESCUE	96	85	35
PERENNIAL RYEGRASS	98	90	30
REDTOP	95	80	5
ALSIKE CLOVER	97	90	5
BIRDSFOOT TREFOIL	98	80	5
LANCE-LEAVED COREOPSIS	95	80	4
OXEYE DAISY	95	80	3
BLACKEYED SUSAN	95	80	4
WILD LUPINE	95	80	4

ALL PROPOSED POST-DEVELOPMENT VEGETATED AREAS WHICH DO NOT EXHIBIT A MINIMUM OF 85% VEGETATIVE

TONS OF MULCH PER ACRE, SECURED WITH ANCHORED NETTING, ELSEWHERE. THE PLACEMENT OF EROSION

CONTROL BLANKETS OR MULCH AND NETTING SHALL NOT OCCUR OVER ACCUMULATED SNOW OR ON FROZEN

2. ALL DITCHES OR SWALES WHICH DO NOT EXHIBIT A MINIMUM OF 85% VEGETATIVE GROWTH BY OCTOBER 15TH, OR

AFTER OCTOBER 15TH, INCOMPLETE ROAD OR PARKING SURFACES SHALL BE PROTECTED WITH A MINIMUM OF 3

GROUND AND SHALL BE COMPLETED IN ADVANCE OF THAW OR SPRING MELT EVENTS.

SEASON, BE CLEARED OF ANY ACCUMULATED SNOW AFTER EACH STORM EVENT

4. AN AREA SHALL BE CONSIDERED STABLE IF ONE OF THE FOLLOWING HAS OCCURRED:

A. BASE COURSE GRAVELS ARE INSTALLED IN AREAS TO BE PAVED;

D. EROSION CONTROL BLANKETS HAVE BEEN PROPERLY INSTALLED.

IDATE OF MEETING:

. A MINIMUM OF 85% VEGETATED GROWTH HAS BEEN ESTABLISHED:

GROWTH BY OCTOBER 15TH, OR WHICH ARE DISTURBED AFTER OCTOBER 15TH, SHALL BE STABILIZED BY SEEDING

AND INSTALLING EROSION CONTROL BLANKETS ON SLOPES GREATER THAN 4:1, AND SEEDING AND PLACING 3 TO 4

WHICH ARE DISTURBED AFTER OCTOBER 15TH, SHALL BE STABILIZED WITH STONE OR EROSION CONTROL BLANKETS

INCHES OF CRUSHED GRAVEL PER NHDOT ITEM 304.3 OR, IF CONSTRUCTION IS TO CONTINUE THROUGH THE WINTER

C. A MINIMUM OF 3" OF NON-EROSIVE MATERIAL SUCH AS STONE OR RIP RAP HAS BEEN INSTALLED; OR

APPROVED BY THE HUDSON, NH PLANNING BOARD

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MEETING FINAL APPROVAL. FINAL APPROVAL COMMENCES AT THE PLANNING

BOARD MEETING DATE AT WHICH THE PLAN RECEIVES FINAL APPROVAL.

SIGNATURE DATE:.

SIGNATURE DATE: _

TEMPORARY SEEDING MIXTURE SHALL BE APPLIED AT A RATE OF 2 POUNDS PER 1,000 SF AND SHALL BE AN APPROVED CONSERVATION MIX OR

EROSION CONTROL BLANKETS - SLOPE INSTALLATION

(AUGUST 2011)

- CONSIST OF THE FOLLOWING 15% BLACKWELL OR SHELTER SWITCHGRASS
- 30% NIAGRA OR KAW BIG BLUESTEM

MATS/BLANKETS SHOULD BE INSTALLED VERTICALLY

DOWNSLOPE AND SHALL BE

APPROVED EQUAL

MINIMUM 4

(100mm)

OVERLAP

BLANKETS

DOUBLE NET STRAW BLANKETS

BY NORTH AMERICAN GREEN OR

TAMP SOIL OVER MAT/BLANKET

0.14 STAPLES

4'-0" (1.2m)

30% CAMPER OR BLAZE LITTLESTEM

ISOMETRIC VIEW

1. SLOPE SURFACE SHALL BE FREE OF ROCKS,

SHALL HAVE GOOD SOIL CONTACT.

CLODS, STICKS AND GRASS. MATS/ BLANKETS

2. APPLY PERMANENT SEEDING BEFORE PLACING

3. LAY BLANKETS LOOSELY AND STAKE OR STAPLE

TO MAINTAIN DIRECT CONTACT WITH THE SOIL.

- 15% NE-27 OR BLAZE SAND LOVEGRASS 10% VIKING BIRDSFOOT TREFOIL
- INOCULUM SPECIFIC TO BIRDSFOOT TREFOIL MUST BE USED WITH THIS MIXTURE. IF SEEDING BY HAND, A STICKING AGENT SHALL BE USED. IF SEEDING WITH A HYDROSEEDER, USE FOUR TIMES THE RECOMMENDED AMOUNT OF INOCULUM.
- SEED MIXTURE FOR STORMWATER MANAGEMENT AREAS, INCLUDING DETENTION BASINS AND VEGETATED TREATMENT SWALES, SHALL BE APPLIED AT A RATE OF 70 POUNDS PER ACRE OR 1.6 POUNDS PER 1,000 SF AND SHALL CONSIST OF THE FOLLOWING:
 - 25% CREEPING RED FESCUE
 - 15% SWITCH GRASS 15% FOX SEDGE
 - 15% CREEPING BENTGRASS 10% FLATPEA
- 20% WILDFLOWER VARIETY STRAW USED FOR MULCH SHALL CONSIST OF MOWED AND PROPERLY CURED GRASS OR LEGUME MOWINGS, FREE FROM WEEDS, TWIGS, DEBRIS. INVASIVE SPECIES OR OTHER DELETERIOUS MATERIAL AND ROT OR MOLD.

SOD SPECIFICATIONS:

- SOD SHALL BE PROVIDED WITH A STRONG ROOT SYSTEM, NOT LESS THAN TWO YEARS OLD AND SHALL BE FREE OF ANY UNDESIRABLE NATIVE GRASSES OR WEEDS
- SOD SHALL BE MACHINE CUT TO A THICKNESS NOT LESS THAN 3/4", EXCLUDING THATCH, AND SHALL BE CAPABLE OF VIGOROUS GROWTH WHEN PLANTED
- SOD PADS SHALL BE OF UNIFORM SIZE AND COMPOSED OF AT LEAST TWO LOCAL GRASS VARIETIES.
- LAY SOD TO FORM A SOLID MASS WITH TIGHTLY FITTED JOINTS, DO NOT OVERLAP. STAGGER STRIPS TO OFFSET JOINTS IN ADJACENT COURSES.
- TAMP SOD TO ENSURE CONTACT WITH WITH SOIL WATER WITHIN ONE HOUR OF PLANTING WITH A FINE SPRAY

50'-0" (MIN.) 3" MINUS STONE BERM WHERE GRADE EXCEEDS 2% (3" TO 6" HIGH) EXISTING **≻**FILTER 3" MINUS STONE BERM CLOTH EXISTING **PROFILE** (WHERE REQUIRED) GROUND 50'-0" (MIN.) 6" (MIN.) THICKNESS OF 3" COARSE AGGREGATE

PLAN VIEW

(APRIL 2018)

STABILIZED CONSTRUCTION EXIT DETAIL NOT TO SCALE

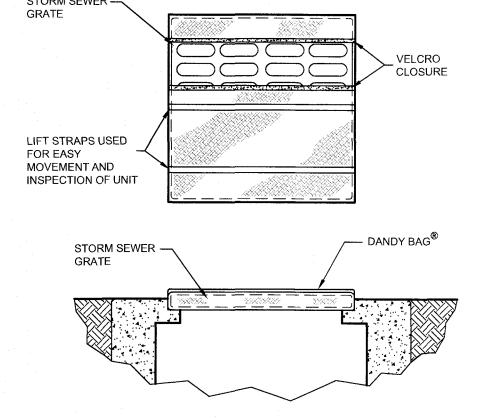
MAINTENANCE

MUD AND SOIL PARTICLES WILL EVENTUALLY CLOG THE VOIDS IN THE CRUSHED STONE AND THE EFFECTIVENESS OF THE CRUSHED STONE PAD WILL NOT BE SATISFACTORY. WHEN THIS OCCURS, THE PAD SHOULD BE TOPDRESSED WITH NEW CRUSHED STONE OR COMPLETE REPLACEMENT OF THE PAD MAY BE NECESSARY WHEN THE PAD BECOMES COMPLETELY

IF WASHING FACILITIES ARE USED, THE SEDIMENT TRAPS SHOULD BE CLEANED OUT AS OFTEN AS NECESSARY TO ASSURE THAT ADEQUATE TRAPPING EFFICIENCY AND STORAGE VOLUME IS AVAILABLE. VEGETATIVE FILTER STRIPS SHOULD BE MAINTAINED TO INSURE A VIGOROUS STAND OF VEGETATION AT ALL TIMES.

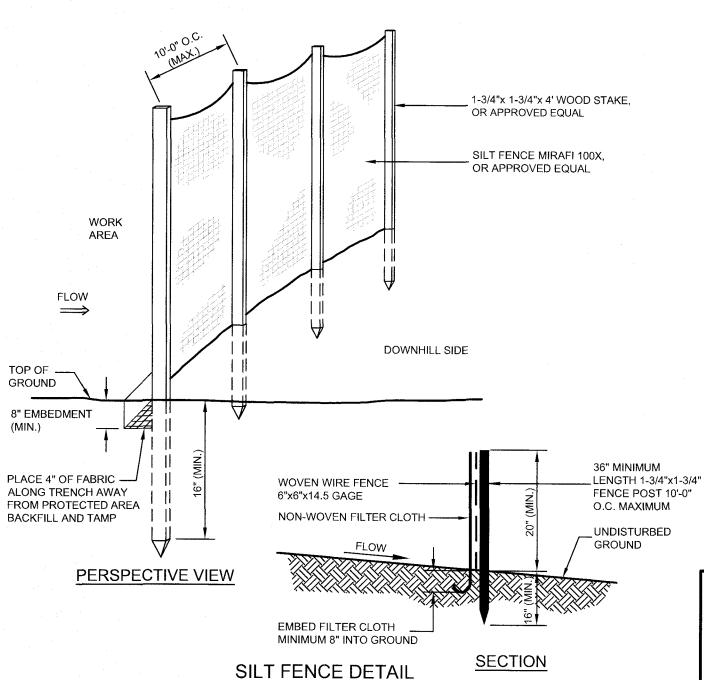
CONSTRUCTION SPECIFICATIONS:

- 1. STONE FOR A STABILIZED CONSTRUCTION EXIT SHALL BE 1 TO 2 INCH STONE, RECLAIMED STONE OR RECYCLED CONCRETE EQUIVALENT.
- THE LENGTH OF THE STABILIZED EXIT SHALL NOT BE LESS THAN 50 FEET, EXCEPT FOR A SINGLE RESIDENTIAL LOT WHERE A 30 FOOT MINIMUM LENGTH WOULD APPLY.
- 3. THE THICKNESS OF THE STONE FOR THE STABILIZED EXIT SHALL NOT BE LESS THAN 6 INCHES.
- 4. THE WIDTH OF THE EXIT SHALL NOT BE LESS THAN THE FULL WIDTH OF THE AREA WHERE INGRESS OR EGRESS OCCURS OR 10 FEET, WHICHEVER IS GREATER.
- 5. GEOTEXTILE FILTER CLOTH SHALL BE PLACED OVER THE ENTIRE AREA PRIOR TO PLACING THE STONE. FILTER CLOTH IS NOT REQUIRED FOR A SINGLE FAMILY LOT OR DUPLEX.
- 6. ALL SURFACE WATER THAT IS FLOWING TO OR DIVERTED TOWARD THE CONSTRUCTION EXIT SHALL BE PIPED BENEATH THE EXIT. IF PIPING IS IMPRACTICAL, A BERM WITH 5:1 SLOPES THAT CAN BE CROSSED BY VEHICLES MAY BE SUBSTITUTED FOR THE PIPE.
- THE EXIT SHALL BE MAINTAINED IN A CONDITION THAT WILL PREVENT TRACKING OR FLOWING OF SEDIMENT ONTO PUBLIC RIGHTS-OF-WAY. THIS MAY REQUIRE PERIODIC TOPDRESSING WITH ADDITIONAL STONE AS CONDITIONS. DEMAND AND REPAIR AND/OR CLEANOUT OF ANY MEASURES USED TO TRAP SEDIMENT. ALL SEDIMENT SPILLED, WASHED OR TRACKED ONTO PUBLIC RIGHT-OF-WAY MUST BE REMOVED PROMPTLY.
- WHEELS SHALL BE CLEANED TO REMOVE MUD PRIOR TO ENTRANCE ONTO PUBLIC RIGHTS-OF-WAY, WHEN WASHING IS REQUIRED, IT SHALL BE DONE ON AN AREA STABILIZED WITH STONE WHICH DRAINS INTO AN APPROVED SEDIMENT TRAPPING DEVICE.



HI-FLOW DANDY BAG® (SAFETY ORANGE

MECHANICAL PROPERTIES	TEST METHOD	UNITS	MARV
GRAB TENSILE STRENGTH	ASTM D 4632	kN (lbs)	1.62 (365) x 0.89 (200)
GRAB TENSILE ELONGATION	ASTM D 4632	%	24 x 10
PUNCTURE STRENGTH	ASTM D 4833	kN (lbs)	0.40 (90)
MULLEN BURST STRENGTH	ASTM D 3786	kPa (psi)	3097 (450)
TRAPEZOID TEAR STRENGTH	ASTM D 4533	kN (lbs)	0.51 (115) x 0.33 (75)
UV RESISTANCE	ASTM D 4355	%	90
APPARENT OPENING SIZE	ASTM D 4751	Mm (US Std Sieve)	0.425 (40)
FLOW RATE	ASTM D 4491	1/min/m ² (gal/min/ft ²)	5907 (145)
PERMITTIVITY	ASTM D 4491	Sec ⁻¹	2.1



CONSTRUCTION SPECIFICATIONS

THE GEOTEXTILE FABRIC SHALL MEET THE DESIGN CRITERIA FOR SILT FENCES.

- THE FABRIC SHALL BE EMBEDDED A MINIMUM OF 8 INCHES INTO THE GROUND AND THE SOIL COMPACTED OVER THE
- 3. WOVEN WIRE FENCE SHALL BE FASTENED SECURELY TO THE FENCE POSTS WITH WIRE TIE OR STAPLES WHERE NOTED OR AS DIRECTED BY DESIGN ENGINEER.
- 4. FILTER CLOTH SHALL BE FASTENED SECURELY TO THE WOVEN WIRE FENCE WITH TIES SPACED EVERY 24 INCHES AT 5. WHEN TWO SECTIONS OF FILTER CLOTH ADJOIN EACH OTHER, THEY SHALL BE OVERLAPPED BY 6 INCHES, FOLDED
- AND STAPLED
- 6. FENCE POSTS SHALL BE A MINIMUM OF 36 INCHES LONG AND DRIVEN A MINIMUM OF 16 INCHES INTO THE GROUND. WOOD POSTS SHALL BE OF SOUND QUALITY HARDWOOD AND SHALL HAVE A MINIMUM CROSS SECTIONAL AREA OF 3.0 SQUARE INCHES.
- 7. MAINTENANCE SHALL BE PERFORMED AS NEEDED AND MATERIAL REMOVED WHEN "BULGES" DEVELOP IN THE SILT

MAINTENANCE

- SILT FENCES SHALL BE INSPECTED IMMEDIATELY AFTER EACH RAINFALL AND AT LEAST DAILY DURING PROLONGED RAINFALL. ANY REPAIRS THAT ARE REQUIRED SHALL BE MADE IMMEDIATELY
- 2. IF THE FABRIC ON A SILT FENCE SHOULD DECOMPOSE OR BECOME INEFFECTIVE DURING THE EXPECTED LIFE OF THE FENCE, THE FABRIC SHALL BE REPLACED PROMPTLY. 3. SEDIMENT DEPOSITS SHOULD BE INSPECTED AFTER EVERY STORM EVENT. THE DEPOSITS SHOULD BE REMOVED
- WHEN THEY REACH APPROXIMATELY ONE-HALF THE HEIGHT OF THE BARRIER. SEDIMENT DEPOSITS THAT ARE REMOVED OR LEFT IN PLACE AFTER THE FABRIC HAS BEEN REMOVED SHALL BE

GRADED TO CONFORM WITH THE EXISTING TOPOGRAPHY AND VEGETATED.

- **CONSTRUCTION SEQUENCE**
- CONTRACTOR TO NOTIFY DIG-SAFE 72-HOURS PRIOR TO COMMENCEMENT OF CONSTRUCTION. PRIOR TO GRUBBING OF CLEARED AREAS, ALL SILTATION BARRIERS DESIGNED FOR USE AS TEMPORARY
- EROSION CONTROL MEASURES SHALL BE INSTALLED AS CALLED FOR ON PROJECT PLANS. INSTALL STABILIZED CONSTRUCTION EXIT AT LOCATION OF CONSTRUCTION ACCESS AT LOCATION OF INTERSECTION WITH EXISTING
- 3. CUT AND CLEAR TREES AND BRUSH FROM CONSTRUCTION AREAS TO THE EXTENT NECESSARY. ALL
- BRANCHES, TOPS AND BRUSH TO BE PROPERLY DISPOSED OF BY CONTRACTOR. THIS PROJECT IS MANAGED TO MEET THE REQUIREMENTS AND INTENT OF RSA 430:53 AND AGR 3800 RELATIVE TO INVASIVE SPECIES. COMPLETE GRUBBING OPERATIONS UNDER THE ROADWAY AND SLOPE SECTIONS. ALL STUMPS AND SIMILAR DEBRIS SHALL BE PROPERLY DISPOSED OF BY CONTRACTOR. ORGANIC MATERIAL SUITABLE FOR USE AS
- TOPSOIL SHALL BE STOCKPILED IN UPLAND AREAS. ALL STOCKPILES SHALL BE SEEDED WITH WINTER RYE AND. IF NECESSARY, SURROUNDED WITH HAY BALES IN ORDER TO PREVENT LOSS DUE TO EROSION. 5. CONSTRUCT TEMPORARY CULVERTS AS NECESSARY TO FACILITATE CONSTRUCTION ACTIVITIES. ALL SUCH
- CROSSINGS SHALL BE PROTECTED WITH HAY BALE BARRIERS TO LIMIT EROSION, RUNOFF MUST BE DIRECTED TO TEMPORARY PRACTICES UNTIL STORMWATER BEST MANAGEMENT PRACTICES ARE STABILIZED.
- DO NOT DIRECT RUNOFF TO TREATMENT SWALES UNTIL THE SWALES AND ALL CONTRIBUTING AREAS HAVE
- BEEN FULLY STABILIZED. STABILIZE ALL DITCHLINES PRIOR TO DIRECTING FLOW INTO THEM, CONSTRUCT DRAINAGE SYSTEM, SEWER
- AND OTHER SUBSURFACE UTILITIES. PONDS/SWALES MUST BE INSTALLED BEFORE ROUGH GRADING OF THE SITE.
- COMMENCE CONSTRUCTION OF ROADWAY, PERFORM EXCAVATION ACTIVITIES REQUIRED TO ACHIEVE SUBGRADE ELEVATION. ALL EXCAVATED EMBANKMENTS, DITCHES, SWALES AND ROADWAY CROSS CULVERTS SHALL BE INSTALLED AND STABILIZED. ALL SWALES AND DITCHLINES SHALL BE PROTECTED FROM EROSION BY IMPLEMENTATION OF HAY BALE SILTATION FENCES AS SHOWN ON PROJECT PLANS. DIVERT STORMWATER RUNOFF THROUGH THE USE OF TEMPORARY CULVERTS, OR OTHER MEANS NECESSARY PRIOR TO THE COMPLETIONS OF A FUNCTIONAL STORM DRAINAGE SYSTEM. SLOPES AND EMBANKMENTS SHALL BE
- STABILIZED BY TRACKING AND TEMPORARY SEEDING WITH WINTER RYE PRIOR TO TURF ESTABLISHMENT. ALL DITCHES AND SWALES SHALL BE STABILIZED PRIOR TO HAVING RUNOFF DIRECTED TO THEM. 10. COMPLETE CONSTRUCTION OF ROADWAY EMBANKMENTS BY ADDING APPROPRIATE BASE MATERIALS GRADED TO PROPER ELEVATION.
- 11. APPLY TOPSOIL TO ROADWAY SLOPES AND OTHER AREAS DISTURBED BY CONSTRUCTION. TOPSOIL USED MAY BE NATIVE ORGANIC MATERIAL SCREENED SO AS TO BE FREE OF ROOTS, BRANCHES, STONES AND OTHER DELETERIOUS MATERIALS. TOPSOIL SHALL BE APPLIED SO AS TO PROVIDE A MINIMUM OF A 4-INCH COMPACTED THICKNESS. UPON COMPLETION OF TOPSOILING, FINISHED SECTIONS ARE TO BE LIMED, SEEDED AND MULCHED. CONSTRUCTION PERSONNEL SHALL INSPECT COMPLETED SECTIONS OF WORK ON A REGULAR BASIS AND REMEDY ANY PROBLEM AREAS UNTIL A HEALTHY STAND OF GRASS HAS BECOME ESTABLISHED.
- 12. PERFORM FINE GRADING OF ROADWAY BASE MATERIALS. 13. MAINTAIN, REPAIR AND REPLACE AS NECESSARY TEMPORARY EROSION CONTROL MEASURES UNTIL SUCH TIME AS THE ENTIRE CONSTRUCTION AREA HAS BEEN STABILIZED (A MINIMUM OF ONE WINTER SHALL HAVE PASSED).
- 14. AFTER STABILIZATION .REMOVE AND SUITABLY DISPOSE OF TEMPORARY EROSION CONTROL MEASURES. 15. MONITOR CONSTRUCTION ACTIVITIES ON INDIVIDUAL LOTS TO INSURE CONSTRUCTION ACTIVITIES ARE BEING PERFORMED IN SUCH A WAY AS NOT TO ENDANGER THE INTEGRITY OF ROADWAY EMBANKMENTS.
- STORMWATER SYSTEMS AND UTILITIES. ALL DRIVEWAYS ACROSS DITCHLINES SHALL HAVE CULVERTS INSTALLED IN ACCORDANCE WITH LOCAL REQUIREMENTS. 16. LOT DISTURBANCE, OTHER THAN THAT SHOWN ON THE APPROVED PLANS, SHALL NOT COMMENCE UNTIL AFTER
- THE ROADWAY HAS THE BASE COURSE TO DESIGN ELEVATION AND THE ASSOCIATED DRAINAGE IS COMPLETE 17. PRIOR TO CONSTRUCTION A STORMWATER PROTECTION PLAN SHALL BE PREPARED PER FEDERAL
- REGULATRIONS. 18. SINCE THIS SITE WILL DISTURB MORE THAN 5 ACRES AT ONE TIME WEEKLY INSPECTION SHALL OCCUR, AS WELL AS DURING ANY RAIN EVENT IN WHICH 0.5 INCH OF PRECIPITATION OR MORE FALLS WITHIN A 24 HOUR PERIOD. PROVIDED THAT IF THE MONITOR IS UNABLE TO BE PRESENT DURING SUCH A STORM, THE MONITOR SHALL
- 19. A REPORT FOR EACH INSPECTION SHALL BE STAMPED BY A QUALIFIED ENGINEER OR A CPESC SPECIALIST AND SUBMITTED WITHIN 24 HOURS OF EACH INSPECTION.

EROSION CONTROL NOTES:

INSPECT THE SITE WITHIN 24 HOURS OF THE RAIN EVENT.

- EXPOSED EARTHWORK SHALL BE CONFINED TO AS LIMITED AN AREA AS IS PRACTICAL AT ANY GIVEN TIME THROUGHOUT THE CONSTRUCTION SEQUENCE. AT NO TIME SHALL MORE THAN FIVE (5) ACRES OF SITE AREA BE IN AN UNSTABLE CONDITION. NO GIVEN AREA OF THE SITE SHALL BE LEFT IN AN UNSTABILIZED CONDITION FOR A PERIOD OF TIME EXCEEDING THIRTY (30) CALENDAR DAYS.
- 2. TEMPORARY EROSION CONTROL MEASURES SHALL BE INSTALLED IN STRICT ACCORDANCE WITH PROJECT PLANS, I ADDITION, SIMILAR MEASURES SHALL BE INSTALLED WHERE AND WHEN THE FIELD CONDITION, OR FIELD OPERATION OF THE INDIVIDUAL SITE CONTRACTOR, MAY WARRANT. ALL TEMPORARY EROSION CONTROL MEASURES USED SHALL BE INSPECTED WEEKLY AND WITHIN 24 HOURS AFTER 0.25" OF RAINFALL OR MORE. THEY SHALL BE CLEANED AND MAINTAINED AND OTHERWISE KEPT IN AN EFFECTIVE OPERATING MANNER THROUGHOUT THE CONSTRUCTION
- 3. ALL DISTURBED AREAS DESIGNATED TO BE TURF, SHALL RECEIVE A MINIMUM APPLICATION OF 4 INCHES OF LOAM (COMPACTED THICKNESS), PRIOR TO FINAL SEEDING AND MULCHING.
- ALL SWALES AND DITCHLINES SHALL BE PERIODICALLY CLEANED OF DEPOSITED SEDIMENT SO AS TO MAINTAIN AN EFFECTIVE GRADE AND CROSS SECTION. ALL SWALES AND DITCHLINES SHALL BE FULLY STABILIZED PRIOR TO HAVING STORMWATER DIRECTED TOWARDS THEM.
- 5. IN THE EVENT THAT, DURING CONSTRUCTION OF ANY PORTION OF THIS PROJECT, A WINTER SHUTDOWN IS NECESSARY, THE CONTRACTOR SHALL STABILIZE ALL INCOMPLETE WORK AND PROVIDE FOR SUITABLE METHODS OF DIVERTING RUNOFF IN ORDER TO ELIMINATE SHEET FLOW ACROSS FROZEN SURFACES.
- 6. AN AREA SHALL BE CONSIDERED STABLE IF ONE OF THE FOLLOWING HAS OCCURRED: A. BASE COURSE GRAVELS ARE INSTALLED IN AREAS TO BE PAVED;
 - B. A MINIMUM OF 85% VEGETATED GROWTH HAS BEEN ESTABLISHED: C. A MINIMUM OF 3" OF NON-EROSIVE MATERIAL SUCH AS STONE OR RIP RAP HAS BEEN INSTALLED; OR
- D. EROSION CONTROL BLANKETS HAVE BEEN PROPERLY INSTALLED. DUST SHALL BE CONTROLLED BY THE USE OF WATER AS NECESSARY THROUGHOUT THE CONSTRUCTION PERIOD. ACCORDANCE WITH ENV-A 1000 8. IN NO WAY ARE THOSE TEMPORARY EROSION CONTROL MEASURES INDICATED ON THESE PLANS TO BE CONSIDERE
- ALL INCLUSIVE. THE CONTRACTOR SHALL USE JUDGEMENT IN INSTALLING SUPPLEMENTARY EROSION CONTROL MEASURES WHERE AND WHEN SPECIFIC SITE CONDITIONS AND/OR CONSTRUCTION METHODOLOGIES MAY AREAS HAVING FINISH GRADE SLOPES OF 3: 1 OR STEEPER, SHALL BE STABILIZED WITH JUTE MATTING WHEN AND
- FIELD CONDITIONS WARRANT, OR IF SO ORDERED. JUTE MATTING INSTALLED TO CONFORM WITH THE RECOMMENDED BEST MANAGEMENT PRACTICE OUTLINED IN VOLUME 3 OF THE NEW HAMPSHIRE STORMWATER MANUAL "EROSION AND SEDIMENT CONTROLS DURING CONSTRUCTION."
- 10. ALL DETENTION PONDS AND TREATMENT SWALES SHALL BE CONSTRUCTED PRIOR TO ANY EARTH MOVING ACTIVITIES THAT WILL INFLUENCE STORMWATER RUNOFF.
- 11. ALL ROADWAYS AND PARKING AREAS SHALL BE STABILIZED WITHIN 72 HOURS OF ACHIEVING FINISHED GRADE. ENGTH 1-3/4"x1-3/4" 12. ALL CUT AND FILL SLOPES SHALL BE STABILIZED WITHIN 72 HOURS OF ACHIEVING FINISHED GRADE.

CONSTRUCTION DETAILS

AROMA JOE'S MAP 173 LOT 29

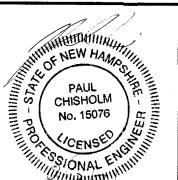
56 DERRY STREET HUDSON, NEW HAMPSHIRE HILLSBOROUGH COUNTY

OWNER OF RECORD: STEVE S. & HSIANG HWA W. PAN 13 KING HENRY DRIVE LONDONDERRY, N.H. 03053 BK. 6281 PG. 776

APPLICANT: SCOTT ZIELFELDER 169 CANAAN BACK ROAD BARRINGTON, NH 03825



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REVISIONS DATE DESCRIPTION REVISED PER REVIEW COMMENTS 08/12/2021 REVISED PER PLANNING BOARD COMMENTS **DATE:** JUNE 22, 2021 SCALE: AS SHOWN **PROJECT NO:** 21-0311-1 SHEET 14 OF 14

