

PUBLIC MEETING - SEPTEMBER 8, 2021

The Town of Hudson Planning Board will hold a regularly scheduled meeting on Wednesday, September 8, 2021 at 7:00 p.m. in the "Buxton Community Development Conference Room" at Town Hall. The following items will be on the agenda:

- I. CALL TO ORDER BY CHAIRPERSON AT 7:00 P.M.
- II. PLEDGE OF ALLEGIANCE
- III. ROLL CALL
- IV. SEATING OF ALTERNATES
- V. MINUTES OF PREVIOUS MEETING(S)
 - 25 August 2021 Meeting Minutes Decisions

VI. OLD BUSINESS

- A. S.L. Chasse Steel Site Plan 201 Robinson Road SP# 03-21 Map 105/Lot 017-002
 Purpose of Plan: to show a proposed industrial building totaling 22,500 SF and associated parking.
- B. S.L. Chasse Steel Site Plan
 SP# 04-21
 Purpose of plan: to show three (3) proposed industrial buildings totaling 50,400 SF and associated parking.
- C. S.L. Chasse Steel Conditional Use Permit CUP# 07-21 Purpose of Plan: to show public water connection from the existing stub approximately 900 feet north of the site to lots 17-2 & 17-3. Application acceptance & hearing.
- D. Aroma Joe's Site Plan 56 Derry Street SP# 08-21 Map 173/Lot 29 Purpose of Plan: to propose a drive-thru coffee shop with associated parking and drives. Application acceptance & hearing.

VII. OTHER BUSINESS A. Discussion on zoning amendment workshops for Fall 2021.

VIII. ADJOURNMENT

Comments may be submitted in writing until 10:00 a.m. on the Tuesday prior to the day of the meeting.

Brian Groth Town Planner

POSTED: Town Hall, Library, Post Office & Web - 8/27/21

5	TOWN OF	HUDSON	NUEW TRANSSHURE
$\int $	Planning	Board	THORPORATED TH
	Timothy Malley, Chairman	Marilyn McGrath, Selectmen Liaison	1

12 School Street • Hudson, New Hampshire 03051 • Tel: 603-886-6008 • Fax: 603-594-1142

MINUTES/DECISIONS OF THE PLANNING BOARD MEETING DATE: AUGUST 25, 2021

In attendance = X	Alternate Seated = S	Partial Attendance = P	Excused Absence = E
Tim Malley	Ed Van der Veen	Elliott Veloso	Jordan Ulery
ChairX	Vice-Chair <u>X</u>	SecretaryX	MemberE
Dillon Dumont	William Collins	Victor Oates	Leo Fauvel
MemberX	MemberX	AlternateE	AlternateX
David Morin	Marilyn McGrath	Brian Groth	
Select. Rep. <u>X</u>	Alt. Select Rep. <u>E</u>	Town RepX	

- I. CALL TO ORDER BY CHAIRPERSON AT 7:00 P.M.
- II. PLEDGE OF ALLEGIANCE
- III. ROLL CALL
- IV. SEATING OF ALTERNATES

Mr. Fauvel seated for Mr. Ulery.

- V. MINUTES OF PREVIOUS MEETING(S)
 - 28 July 21 Meeting Minutes Decisions

Mr. Dumont moved to accept 28 July 21 Meeting Minutes (as written/amended).

Motion seconded by Mr. Collins. All in favor – motion carried 6/0/0.

VI. NEW BUSINESS

A. Plante Driveway Conditional Use Permit
CUP# 09-21Wason Road
Map 206/Lot 001-002

Purpose of Plan: to show a proposed driveway crossing in support of a state wetland permit where a conditional use permit is also required from the town due to impacting the 50-foot wetlands buffer.

Mr. Dumont recused himself from this application.

Mr. Van der Veen moved to accept the conditional use permit application for the Plante Driveway Application, CUP# 09-21, on an unnumbered lot on Wason Road, Map 206/Lot 001-002.

Motion seconded by Mr. Collins. All in favor – motion carried 6/0/0.

Public input opened @ 7:06 p.m.

Caitlin Hamm, 8 Pasture Drive – Concerns about the vernal stream that runs through her yard onto the Plante parcel that since the construction of the driveway, no longer flows and is backing up into her yard causing an increase in mosquitos and a wet/muddy yard.

Tracy Bloom, 18 Pasture Drive – Not opposed to the new home being built, but concerned about the right-of-way (paper street) being utilized next to her property line as the access.

Susan Proulx, 20 Pasture Drive – Does not support any driveway being placed in the right-of-way. Feels it would be disruptive with construction and effect their quality of life.

Doug Gagnon, 19 Pasture Drive – Opposed to the road going in on the right-of-way. Lights from the cars would shine right on his house. The road is already in despair.

Andrew Bernard, 131 Wason Road – Is there going to be a "fan" at the end of the driveway? Concerned about the driveway being at the bottom of the hill and people go down that hill at 40 - 50 MPH. Concerned for the owner pulling out onto Wason.

Bob St. Cyr, 8 Wende Drive – Concerned about the driveway going through the wetland buffer. Looking for mitigation on his daughters property @ 8 Pasture Drive.

Mark & Susan Chase, 6 Pasture Drive (Not in attendance – written letter to the board) – Wants to ensure pipes are situated at current elevation to not disrupt the current natural flow of water to ensure there is no backup of water flow to abutting properties. Close proximity to property line and removal of nature buffer (tree line). To help ease the disturbance of construction activity, we would suggest preserving existing trees or planting trees along stone wall near abutter property line.

Public input closed @ 7:21 p.m.

Mr. Collins moved to approve the conditional use permit for the Plante Driveway Application on an unnumbered lot on Wason Road, Map 206/Lot 001-002 consisting of the plan entitled: Tax Map 206 Lot 1-2 Wetland Permit Plan; prepared by S&H Land Services, LLC, 141 Londonderry Tpke., Hooksett, NH; prepared for Marco Plante, Wason Road, Hudson, New Hampshire; consisting of a single sheet with general notes 1-6 dated July 8, 2021, last revised August 5, 2021; subject to, and revised per, the following stipulations:

- 1. All stipulations of approval shall be incorporated into the Notice of Decision, which shall be recorded at the HCRD, together with the Plan.
- 2. A cost allocation procedure (CAP) amount of \$5,880 per single-family residential unit, or \$5,133 per residential unit within a duplex (or two-family structure) shall be paid prior to the issuance of a Certificate of Occupancy for the new house lot.
- 3. Prior to the issuance of a final certificate of occupancy, an L.L.S. Certified "as-built" site plan shall be provided to the Town of Hudson Land Use Development, confirming that the driveway conforms to the Plan approved by the Planning Board.
- 4. Prior to the Planning Board endorsement of the Plan, it shall be subject to final administrative review by Town Planner and Town Engineer.
- 5. Construction activities involving the subject lot shall be limited to the hours between 7:00 A.M. and 7:00 P.M. No exterior construction activities shall be allowed on Sundays.

Stipulations proposed by Conservation Commission:

- 6. Construction and restoration shall comply with Best Management Practices set forth in New Hampshire Storm Water Manual Volume 3: Erosion and Sediment Control
- 7. During construction and restoration erosion control barriers shall be installed and maintained to the satisfaction of the Town Engineer.
- 8. The Town Engineer or his representative shall be allowed to inspect the boundaries of the wetland and wetland buffer areas during construction and report any finding to the applicant and the Conservation Commission for remediation.
- 9. The applicant will file a Dredge and Fill permit application with the NHDES per Title L, Water Management and Protection Chapter 482-A.
- 10. The wetland buffer boundary shall be identified and marked prior to the start of construction per Hudson Zoning Ordinance, Article IX §334-35 (E.).
- 11. "No Cut/No Disturb" signage shall be installed along the wetland buffer boundary prior to issuing Certificates of Occupancy per Hudson Zoning Ordinance, Article IX §334-35 (E.).
- 12. Stockpiling of construction materials is not allowed in the wetland buffer areas during construction.
- 13. Shoulder reduction along impact area shall be reduced to one-foot on either side of the driveway along the wetlands, wetland buffer area, and wetland crossing.

Motion seconded by Mr. Veloso. All in favor – motion carried 6/0/0.

VII. OLD BUSINESS

A. S.L. Chasse Steel Conditional Use Permit CUP# 07-21 201 & 199 Robinson Road Map 105/Lots 17-2 & 17-3

Purpose of Plan: to show public water connection from the existing stub approximately 900 feet north of the site to lots 17-2 & 17-3. Application acceptance & hearing.

Mr. Collins moved to continue the public hearing for the Conditional Use Permit for S.L. Chasse Steel at 199 & 201 Robinson Road, Map 105/Lots 017-002 & 017-003 to date certain, September 8, 2021.

Motion seconded by Mr. Dumont. All in favor – motion carried 6/0/0.

В.	S.L. Chasse Steel Site Plan	201 Robinson Road
	SP# 03-21	Map 105/Lot 017-002

Purpose of Plan: to show a proposed industrial building totaling 22,500 SF and associated parking.

Mr. Collins moved to continue the public hearing for the Site Plan Applications for S.L. Chasse Steel at 199 & 201 Robinson Road, Map 105/Lots 017-002 & 017-003 to date certain, September 8, 2021. All in favor – motion carried 6/0/0.

Motion seconded by Mr. Dumont. All in favor – motion carried 6/0/0.

C. S.L. Chasse Steel Site Plan SP# 04-21 199 Robinson Road Map 105/Lot 017-003

Purpose of plan: to show three (3) proposed industrial buildings totaling 50,400 SF and associated parking.

Mr. Collins moved to continue the public hearing for Site Plan Applications for S.L. Chasse Steel at 199 & 201 Robinson Road, Map 105/Lots 017-002 & 017-003 to date certain, September 8, 2021. All in favor – motion carried 6/0/0.

Motion seconded by Mr. Dumont. All in favor – motion carried 6/0/0.

D.	Aroma Joe's Site Plan	56 Derry Street
	SP# 08-21	Map 173/Lot 29

Purpose of Plan: to propose a drive-thru coffee shop with associated parking and drives. Application acceptance & hearing.

Mr. Veloso moved to continue the public hearing for the site plan application for Aroma Joe's at 56 Derry Street; Map 173/Lot 029-000, to date certain, September 8, 2021.

Motion seconded by Mr. Dumont. All in favor – motion carried 6/0/0.

Hudson Planning Board Minutes/Decisions August 25, 2021 Page 4

VIII. OTHER BUSINESS

A. Master Plan – Historic Resources and Community Facilities Chapters.

Mr. Groth introduced the draft chapters for Community Facilities and Historic Resources and requested that Board members email him their feedback and suggestions over the coming weeks. Once Board members feels they have had adequate time to review these drafts, they will be reviewed and potentially adopted at a future meeting.

B. Request to establish a bond for Granite Heights Subdivision by Elvis Dhima, Town Engineer.

Mr. Dumont moved to establish a performance surety in the amount of \$463,086.60 for the Granite Heights Subdivision, Map 161/Lot 029, in its entirety, and in accordance with the written recommendation of the Town Engineer, Elvis Dhima's Interoffice Memo on file, dated August 20, 2021, together with the Road Guarantee Estimate Form. Note: said surety shall be established in the form of a Hampton-style letter of credit or cash deposit held by the Town.

Motion seconded by Mr. Van der Veen. All in favor – motion carried 6/0/0.

C. Planning Board Applications -Proposed language change to revision submission deadline.

The following information is required to filed with the Planning Department *no later than 10:00 A.M., Tuesday ONE WEEK prior to the scheduled Planning meeting. The purpose of these materials is hardcopy distribution to Planning Board members, not review. Any plan revisions that require staff review must be submitted no later than 10:00A.M., Tuesday TWO WEEKS prior to the scheduled Planning meeting. Depending on the complexity of changes, more time may be required for review. Please contact the Town Planner if you have any questions on this matter.*

Mr. Dumont moved to approve the revised application language as presented concerning submittal deadlines.

Motion seconded by Mr. Van der Veen. All in favor – motion carried 6/0/0.

IX. ADJOURNMENT

Motion to adjourn by Mr. Dumont. Seconded by Mr. Veloso. All in favor – motion carried 6/0/0.

Meeting adjourned at 8:23 p.m.

Elliott Veloso Secretary

These minutes are in draft form and have not yet been approved by the Planning Board.

Note: Planning Board minutes are not a transcript. For full details on public input comments, please view the meeting on HCTV (Hudson Community Television).

Hudson Planning Board Minutes/Decisions August 25, 2021 Page 5

S.L. CHASSE STEEL – 201 ROBINSON ROAD

SITE PLAN APPLICATION #03-21

STAFF REPORT #4

September 8, 2021

SITE: 201 Robinson Road; Map 105 Lot 17-2

ZONING: General-One (G-1)

PURPOSE OF PLANS: To show one proposed industrial building totaling 22,500 SF (including 300 SF office space) and associated parking on Robinson Road.

PLANS UNDER REVIEW: Non-residential Site Plan, S.L. Chasse Steel, Map 105 Lot 17-2, Robinson Road, Hudson, New Hampshire; prepared by Keach-Nordstrom Associates, Inc., 10 Commerce Park North, Suite 3, Bedford, New Hampshire 03110; prepared for Steel Properties, LLC, 8 Christine Drive, Hudson, New Hampshire 03051; consisting of 13 sheets plus a cover page, with general notes 1-38 on Sheet 1; dated April 6, 2021, last revised August 30, 2021.

Note: Included in this packet is revised Sheet 5 showing fire suppression storage tanks.

ATTACHMENTS:

- A. Third Round of Peer Review of SP #03-21 by Fuss & O'Neill, dated August 30, 2021
- B. Third Round of Peer Review of Drainage for both SP #03-21 and #04-21, dated August 30, 2021.
- C. CAP Fee worksheet.
- D. Revised Traffic Study, prepared by TEPP, received August 30, 2021.

APPLICATION TRACKING:

- April 7, 2021 Site Plan applications received.
- May 28, 2021 Conditional Use Permit application received.
- June 1, 2021 Revised plans received.
- June 9, 2021 Public hearing scheduled, SP applications accepted, continued to June 23, 2021.
- June 23, 2021 Public hearing scheduled, applicant requested continuance to July 28, 2021.
- July 28, 2021 Public hearing scheduled, applicant requested continuance to August 25, 2021.
- August 17, 2021 Revised plan sets received.
- August 25, 2021 Public hearing scheduled.

WAIVER REQUESTS

The Applicant is seeking relief from two land use regulations:

- §276-11.1.B(12) requires a 200-foot buffer between residential and industrial uses. The revised plan set shows a limited amount of parking area within approximately 50-feet of the buffer (~150 away from residential property line).
- §276-11.1.B(25) This regulation permits the Planning Board to allow access ways across side lot lines. This enables the application to connect to the Owner's neighboring Lot 17-3 which is the subject of SP #04-21.

COMMENTS:

FIRE SUPPRESSION SUPPLY

Upon conferring with the Fire Department and Engineering Department, we have found the following:

- 1. The total fire suppression need to cover both sites is 60,000 gallons/hour for 3 hours, or 180,000 gallons
- 2. Our consultant's conservative estimate is that 500 gallons/minute are available, equating to 90,000 gallons over 3 hours.
- 3. This leaves a gap of 90,000 gallons.
- 4. 3x 30,000 gallon tanks would cover the supply gap for both sites.
- 5. Engineering requests an offsite improvement, a surge valve, in order to ensure the 500 gallons/minute is available from town supply. The Town would handle installation.

Additionally, during final design of the sprinkler systems for the respective buildings, if a "quick-response" system is designed and built; further forgiveness on the fire flow may become available. This has the potential to reduce the needs described above. Accordingly, this may mean starting the building permit review prior to recording the final Mylar in order to determine the final tank design.

A revised plan sheet has been provided showing proposed tank locations, if necessary.

(Draft Motions on the follow pages)

DRAFT MOTIONS

To GRANT a waiver:

I move to grant a waiver from §276-11.1.B(12), to reduce the residential buffer to 100-feet, based on the Board's discussion, the testimony of the Applicant's representative, and in accordance with the language included in the submitted Waiver Request Form for said waiver.

Motion by: ______Second: _____Carried/Failed: ______

To GRANT a waiver:

I move to grant a waiver from §276-11.1.B(25), to allow access across the side lot line between lot 17-2 and Lot 17-4, based on the Board's discussion, the testimony of the Applicant's representative, and in accordance with the language included in the submitted Waiver Request Form for said waiver.

Motion by: _____ Second: _____ Carried/Failed: _____

<u>CONTINUE</u> the public hearing to a date certain:

I move to continue the public hearing for the conditional use permit application and site plan applications for S.L. Chasse Steel at 199 and 201 Robinson Road; Map 105 Lot 17-3 and Lot 17-2 to date certain, _____, 2021.

Motion by: ______Second: _____Carried/Failed: _____

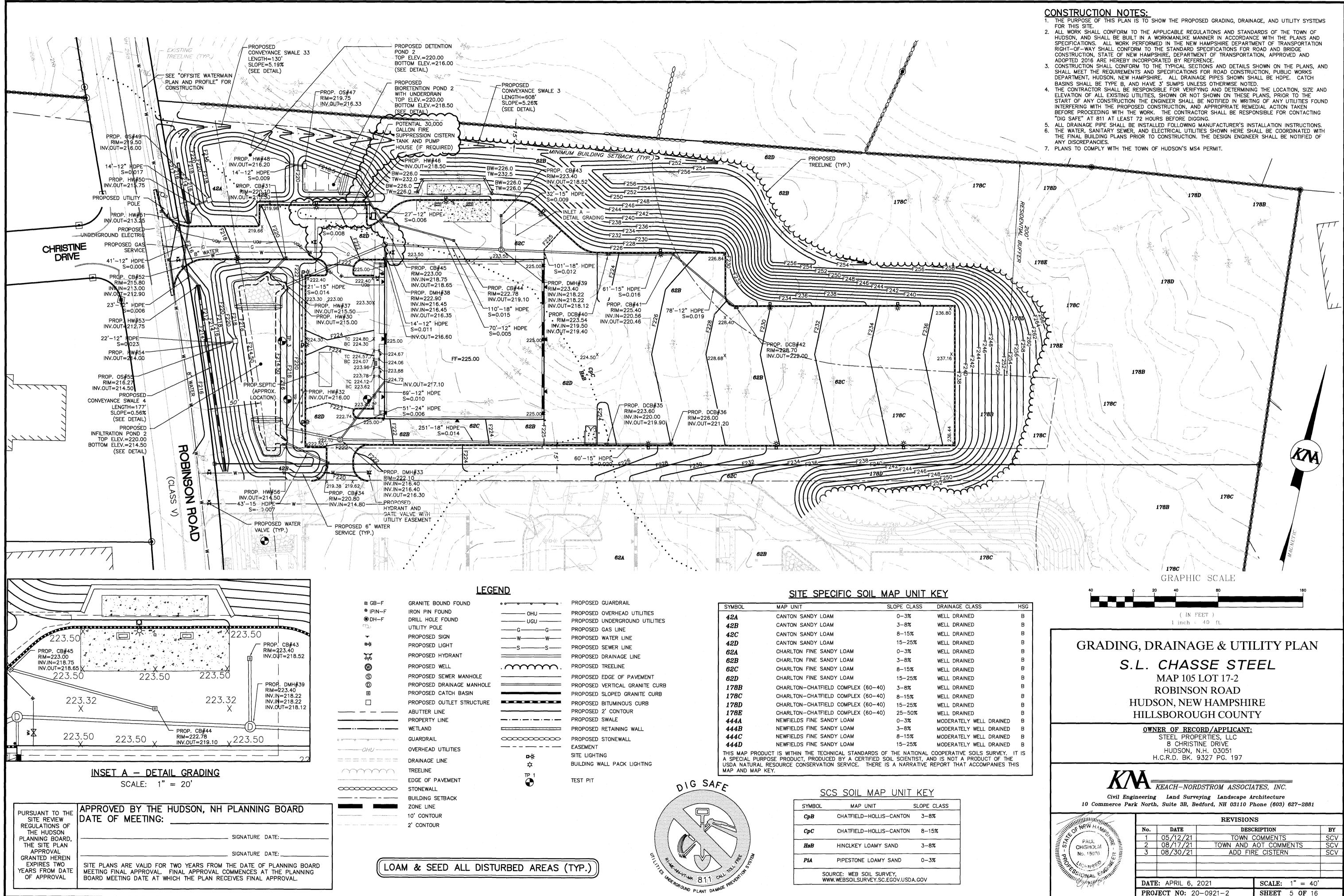
<u>APPROVE</u> the site plan application:

I move to approve the site plan for Non-residential Site Plan, S.L. Chasse Steel, Map 105 Lot 17-2, Robinson Road, Hudson, New Hampshire; prepared by Keach-Nordstrom Associates, Inc., 10 Commerce Park North, Suite 3, Bedford, New Hampshire 03110; prepared for Steel Properties, LLC, 8 Christine Drive, Hudson, New Hampshire 03051; consisting of 13 sheets plus a cover page, with general notes 1-38 on Sheet 1; dated April 6, 2021, last revised September 2, 2021.; subject to, and revised per, the following stipulations:

- 1. All stipulations of approval shall be incorporated into the Notice of Decision and the Development Agreement, which shall be recorded at the HCRD, together with the Plan.
- 2. All improvements shown on the Plan shall be completed in their entirety and at the expense of the applicant or the applicant's assigns.
- 3. A cost allocation procedure (CAP) amount of \$31,050 shall be paid prior to the issuance of a Certificate of Occupancy.

- 4. An offsite improvement, a Surge Valve for the Route 102 Booster Station, is necessitated by this application in tandem with SP #04-21. This shall be coordinated with the Engineering Department.
- 5. Prior to the issuance of a Certificate of Occupancy, an L.L.S. Certified "as-built" site plan shall be provided to the Planning Department, confirming that the site conforms to the Plan approved by the Planning Board.
- 6. The final design and size of the fire suppression water supply tanks shall be subject to the Fire Department's determination. The final Plan will reflect the needs identified during the building permit review process, which may begin prior to recording of the Plan. A building permit will not be issued until the Plan is recorded.
- 7. Prior to the Planning Board endorsement of the Plan, it shall be subject to final administrative review by Town Planner and Town Engineer.
- 8. The applicant shall schedule a pre-construction meeting with the Town Engineer prior to applying for a building permit.
- 9. Construction activities involving the subject lot shall be limited to the hours between 7:00 A.M. and 7:00 P.M. No exterior construction activities shall be allowed on Sundays.
- Blasting or ramming activities shall be limited to the hours between 9:00 A.M and 5:00 P.M, Monday through Friday. Blasting activities are prohibited on Saturday and Sunday.

Motion by: _____Second: _____Carried/Failed: _____



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	OHU	PROPOSED OVERHEAD UTILITIES		42A	CANTON SANDY LOAM	0-3%	WELL DRAINED
	UGU	PROPOSED UNDERGROUND UTILITIES		42B	CANTON SANDY LOAM	3-8%	WELL DRAINED
	GG	PROPOSED GAS LINE		42C	CANTON SANDY LOAM	8-15%	WELL DRAINED
	W W	PROPOSED WATER LINE		42D	CANTON SANDY LOAM	15-25%	WELL DRAINED
	SS	PROPOSED SEWER LINE		62A	CHARLTON FINE SANDY LOAM	0-3%	WELL DRAINED
		PROPOSED DRAINAGE LINE		62B	CHARLTON FINE SANDY LOAM	3-8%	WELL DRAINED
		PROPOSED TREELINE		62C	CHARLTON FINE SANDY LOAM	8-15%	WELL DRAINED
		PROPOSED EDGE OF PAVEMENT		62D	CHARLTON FINE SANDY LOAM	15-25%	WELL DRAINED
OLE		PROPOSED VERTICAL GRANITE CURB		178B	CHARLTON-CHATFIELD COMPLEX (60-40) 3–8%	WELL DRAINED
		PROPOSED SLOPED GRANITE CURB		178C	CHARLTON-CHATFIELD COMPLEX (60-40	•	WELL DRAINED
JRE		PROPOSED BITUMINOUS CURB		178D	CHARLTON-CHATFIELD COMPLEX (60-40		WELL DRAINED
	<u> </u>	PROPOSED 2' CONTOUR		178E	CHARLTON-CHATFIELD COMPLEX (60-40) 25-50%	WELL DRAINED
		PROPOSED SWALE		444A	NEWFIELDS FINE SANDY LOAM	0-3%	MODERATELY WELL DRA
	<u> </u>	PROPOSED RETAINING WALL		444B	NEWFIELDS FINE SANDY LOAM	3-8%	MODERATELY WELL DRA
	∞	PROPOSED STONEWALL		444C	NEWFIELDS FINE SANDY LOAM	8-15%	MODERATELY WELL DRA
		EASEMENT		444D	NEWFIELDS FINE SANDY LOAM	15-25%	MODERATELY WELL DRA
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	\$	BUILDING WALL PACK LIGHTING			IRPOSE PRODUCT, PRODUCED BY A CERTIFIED S AL RESOURCE CONSERVATION SERVICE. THERE		
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2	SCS SOIL MAP UNIT KEY
SYMBOL	MAP UNIT SLOPE CLASS
СрВ	CHATFIELD-HOLLIS-CANTON 3-8%
СрС	CHATFIELD-HOLLIS-CANTON 8-15%
HsB	HINCLKEY LOAMY SAND 3-8%
PiA	PIPESTONE LOAMY SAND 0-3%
	SOURCE: WEB SOIL SURVEY, WWW.WEBSOILSURVEY.SC.EGOV.USDA.GOV



August 30, 2021

Mr. Brian Groth Town Planner Town of Hudson 12 School Street Hudson, NH 03051

Re: Town of Hudson Planning Board Review SL Chasse Steel Site Plan, Robinson Road Tax Map 105 Lot 17-2; Acct. #1350-532 Reference No. 20030249.2020

Dear Mr. Groth:

Fuss & O'Neill (F&O) has reviewed the third submission of the materials received on August 17, 2021, related to the above-referenced project. A list of items reviewed is enclosed. The scope of our review is based on the Site Plan Review Codes, Stormwater Codes, Driveway Review Codes, Sewer Use Ordinance 77, Zoning Regulations, and criteria outlined in the CLD Consulting Engineers Proposal approved September 16, 2003, revised September 20, 2004, June 4, 2007, September 3, 2008, and October 2015.

Please note that comments related to the proposed development at lot 17-3 will be forwarded with a separate letter. Also, the stormwater design documents provided as part of the review package incorporate both lots, so our drainage related comments have been provided separately.

Previous review comments that did not require further action or input have been removed from this letter for brevity/clarity.

The following items have outstanding issues:

3. Driveway Review Codes (HR 275-8.B. (34)/Chapter 193)

a. Former/Current Fuss & O'Neill Comment: HR 193.10.E. The applicant has shown sight distances of 400 feet for the proposed driveway on the plan set. We note that the site line is very close to the existing grade at Station 1+00 and that it does not take into account snow or vegetation. The applicant should review the need to modify this area to better account for seasonal obstructions.

5. Utility Design/Conflicts

b. Former Fuss & O'Neill Comment: HR 275-9.E. The applicant has not shown the existing well or septic system on the plans. / The applicant has stated that they have not been located and will be decommissioned. The applicant should provide the information or review the need for a waiver.

Current Fuss & O'Neill Comment: The applicant has shown the existing well on the plan and noted it is to be decommissioned. The applicant should review the need for a waiver from showing the existing septic on the plan.

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Mr. Brian Groth August 30, 2021 Page 2 of 4

c. Former Fuss & O'Neill Comment: The applicant did not provide any information about the fuel pumping area or the underground tanks typically associated with the pumps. / The applicant has stated that they are in the process of design for the tanks.

Current Fuss & O'Neill Comment: The applicant has stated that the fuel pump area has been revised and is in the process of being designed and permitted by others.

e. Former Fuss & O'Neill Comment: The applicant should review with the Hudson Fire Department whether hydrants should be installed along the proposed water main on Robinson Road. If required, hydrants shall meet the requirements of Hudson Engineering Technical Guidelines section 825.4.10.

Current Fuss & O'Neill Comment: The applicant has stated they are working with the Town on fire protection. We note that a hydrant detail has been provided on the plan set.

10. State and Local Permits (HR 275-9.G.)

b. Former Fuss & O'Neill Comment: HR 275-9.G. The applicant has not provided any information or details related to sizing or containment design for the fuel pumps, nor provided any information about state and local permitting for these structures.

Current Fuss & O'Neill Comment: The applicant has noted that the fuel pumps are in the process of being designed.

The following items require Town evaluation or input:

1. Site Plan Review Codes (HR 275)

a. Former Fuss & O'Neill Comment: Hudson Regulation (HR) 275-6.I. The scope of this review does not include the adequacy of any fire protection provisions for the proposed buildings. Fuss & O'Neill defers to the Hudson Fire Department for review of proposed fire protection for this facility. We note that the site is proposed to be serviced by a private well. The Town should review the need for an onsite cistern depending on the well capacity.

Current Fuss & O'Neill Comment: The applicant has stated that they are coordinating with the Town and the Fire Department.

2. Administrative Review Codes (HR 276)

b. Former Fuss & O'Neill Comment: HR 276-11.1.B.(6) The owner's signature is not shown on the plan set.

Current Fuss & O'Neill Comment: The applicant noted that the owner will sign the final plan.

c. Former Fuss & O'Neill Comment: HR 276-11.1.B.(12).(a). The applicant has shown a 100 foot residential buffer setback from adjacent residential lots, but based on the proposed industrial use for the site a 200 foot setback would be required for all buildings, parking or display areas.

Current Fuss & O'Neill Comment: The applicant has requested a waiver to reduce the buffer to 100 feet.

7. Zoning (ZO 334)

f. Former Fuss & O'Neill Comment: ZO 334-36.C.(2). The applicant is proposing installation of a water main along Robinson Road that impacts a wetlands buffer. The applicant should review with the Town to



Mr. Brian Groth August 30, 2021 Page 3 of 4

> determine if the proposed water main requires a Conditional Use Permit in accordance with the Ordinance. Current Fuss & O'Neill Comment: The applicant has stated that a Conditional Use Permit had been submitted. No further Fuss & O'Neill comment.

The following items are resolved or have no further Fuss & O'Neill input:

1. Site Plan Review Codes (HR 275)

g. Former Fuss & O'Neill Comment: HR 275-9.F. The applicant did not provide copies of any easements or deeds as part of the package received for review. / The applicant has provided a copy of the deed. We note no easements were provided, however, the deed references a State of New Hampshire easement.

Current Fuss & O'Neill Comment: The applicant has stated that the easement noted affected the lot before the 2020 subdivision and does not affect this parcel. No further Fuss & O'Neill comment.

2. Administrative Review Codes (HR 276)

f. Former Fuss & O'Neill Comment: HR 276-11.1.B.(17). We were unable to locate any benchmarks within the Site plan.

Current Fuss & O'Neill Comment: The applicant has added a benchmark to the plan set. No further Fuss & O'Neill comment.

4. Traffic

a. Former Fuss & O'Neill Comment: HR 275-9.B. The applicant has not provided any traffic information as part of the review package. / The applicant has indicated that the Town has not requested any traffic information, and their understanding is that it is not required to this point. We note that between lots 2 and 3 there are over 130 parking spaces proposed, which may provide traffic impacts that warrant further review.

Current Fuss & O'Neill Comment: The applicant has submitted a traffic study and our review comments were provided to the Town on August 6, 2021 for the report dated July 23, 2021. Further traffic reviews will be responded to separately.

5. Utility Design/Conflicts

a. Former Fuss & O'Neill Comment: HR 275-9.E and 276-13. The applicant has provided a typical septic system detail and shown the approximate septic location. We note that no water or well details were provided. / The applicant has stated that they are currently working on a water connection to the site, and an off-site water main extension plan was provided. We note that water lines are now shown on the site but details, size and materials are not provided.

Current Fuss & O'Neill Comment:

d. Former Fuss & O'Neill Comment: The applicant has provided a water main profile that shows a minimum of 4 feet of cover in some sections. The minimum cover required by the Town of Hudson is 5 feet.

Current Fuss & O'Neill Comment: The applicant has revised the plan to provide the minimum 5 feet of cover. No further Fuss & O'Neill comment.



Mr. Brian Groth August 30, 2021 Page 4 of 4

6. Drainage Design/Stormwater Management (HR 275-9.A./Chapter 290)

The review of the drainage design and Alteration of Terrain report was provided under separate letters from Fuss & O'Neill dated April 30, 2021, June 14, 2021, and August 30, 2021.

Steven W. Reichert, PE Bate 2021.08.30 14:10:50-04/00

gitally signed by Steven W. Reicher

Please feel free to call if you have any questions.

Very truly yours,

Steven W. Reichert, P.E.

SWR:

Enclosure

Town of Hudson Engineering Division - File cc: Keach-Nordstrom Associates, Inc. - svando@keachnordstrom.com



August 30, 2021

Mr. Brian Groth Town Planner Town of Hudson 12 School Street Hudson, NH 03051

Re: Town of Hudson Planning Board Review - Stormwater Design Review SL Chasse Steel Site Plan, Robinson Road Tax Map 105 Lot 17-2 & 17-3; Acct. #1350-532 Reference No. 20030249.2020

Dear Mr. Groth:

Fuss & O'Neill (F&O) has reviewed the third submission of the materials received on August 17, 2021, related to the above-referenced project. The scope of this review letter is related to stormwater aspects of the project design only. Site plan, subdivision, and other review elements are provided under separate cover.

This review is based on the recently adopted Stormwater Regulations (Chapter 290), Subdivision Regulations (Chapter 289), Site Plan Review Regulations (Chapter 275), Hudson's Engineering Technical Guidelines and Typical Details, and general engineering practices. Due to the combined Alteration of Terrain Application for both lots 17-2 and 17-3, we have included comments for both of those lots as part of the overall stormwater design, and issued these separately from our Site Plan review comments.

Previous review comments that did not require further action or input have been removed from this letter for brevity/clarity.

The following items have outstanding issues:

6. Drainage Design/Stormwater Management (HR 275-9.A./Chapter 290)

i. New Fuss & O'Neill Comment: The applicant provided revised drainage calculations that only contained the NHDES required 10-year storm calculations. The applicant should provide a copy of (at a minimum) the HydroCAD node listing of the 25-Year and 50-Year revised drainage calculations for Town records.

The following items require Town evaluation or input:

6. Drainage Design/Stormwater Management (HR 275-9.A./Chapter 290)

c. Former Fuss & O'Neill Comment: HR 290-5.A.(5). The applicant has proposed a decrease in stormwater rates from the pre to post conditions in all storms analyzed, however, we do note an increase in stormwater volume in all storms analyze at Analysis Point A. The applicant should provide additional information and

F:\Proj2003\030249 Hudson\Site\2020 SL Chasse Lots 2 & 3\2020 SL Chasse Drainage Letter3 08xx21.Docx © 2021 Fuss & O'Neill,

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Mr. Brian Groth August 30, 2021 Page 2 of 2

> review this volumetric increase with the Town. / As previously noted, the design proposes a decrease in runoff rate, but the design does not meet the Regulation for runoff volumes at all storms analyzed for Analysis Point A. The most recent drainage calculations provided for review (during the first submittal-stamped April 6, 2021), illustrate the following increases in volume at Analysis Point A:

Year Storm	Pre-Development	Post Development
Analyzed	Volume (cfs)	Volume (cfs)
10 year	75,737	94,931
25 year	135,577	161,520
50 year	201,728	230,049

The applicant shall provide additional information and review this volumetric increase with the Town.

Current Fuss & O'Neill Comment: The applicant has offered reasoning for the increased volume from Pre to Post Development at Analysis Point A, and states this meets NHDES AoT regulations. We note the volume of the latest round of drainage calculations provided with this review is slightly greater in volume than the last round of calculations provided. The data provided does not currently meet Town of Hudson Regulations, the applicant shall review this volumetric increase with the Town, to ensure this increase is allowed and whether a waiver is necessary.

The following items are resolved or have no further Fuss & O'Neill input:

6. Drainage Design/Stormwater Management (HR 275-9.A./Chapter 290)

f. Former Fuss & O'Neill Comment: HR 290-7.B.(13). The applicant should provide the certified soil scientist's stamp on the Existing Conditions Plan for each plan set with the final plan submissions. / The applicant has stated that the final plan will be stamped. We recommend the Town make this a Condition of Approval.

Current Fuss & O'Neill Comment: The applicant has provided the certified soil scientist's stamp on the plan set. No further Fuss & O'Neill comment.

Please feel free to call if you have any questions.

Very truly yours,

Steven W. Bichert, Policially signed by Steven W. Reichert, Reichert, PE De: cr=Steven W. Reichert, PE, c=US, c=Fus & AONeil, Inc. email=sreichert@fand.com Date: 22/108/30 11:4:09-04000

Steven W. Reichert, P.E.

SWR:

Enclosure

cc: Town of Hudson Engineering Division – File Keach- Nordstrom Associates, Inc. - alewis@keachnordstrom.com



TOWN OF HUDSON

Planning Board



Timothy Malley, Chairman

12 School Street • Hudson, New Hampshire 03051 • Tel: 603-886-6008 • Fax: 603-594-1142

CAP FEE WORKSHEET - 2021

Date <u>:</u>	09-01-21	Zone #	1	Map/Lot:	<u>105/017-002</u> 201 Robinson	DJ
Project	t Name:	S.L. Ch	asse Steel		201 KODIIISOII	Ku.
Propos	ed ITE Use #1	:Indus	trial			
Propos	ed Building A	rea (square	footage):_	22,5	500	<u> </u>
CAP F	EES: (ONE C	HECK NEE	DED)			

1.	(Bank 09) 2070-701	Light Industrical (22,500 s.f @ \$1.38 per s.f)	<u>\$31,050.00</u>
		Total CAP Fee	<u>\$_31,050.00</u>

Check should be made payable to the <u>Town of Hudson</u>.

Thank you,

Brooke Dubowik

Planning Administrative Aide

TRAFFIC IMPACT AND ACCESS STUDY

ROBINSON ROAD Hudson, New Hampshire

Revised August 25, 2021

Prepared for Keach-Nordstrom Associates, Inc.



TRANSPORTATION ENGINEERING, PLANNING AND POLICY

TRAFFIC-IMPACT AND ACCESS STUDY

ROBINSON ROAD Hudson, New Hampshire

Revised August 25, 2021



Prepared for Keach-Nordstrom Associates, Inc.

TEPP LLC

TRANSPORTATION ENGINEERING, PLANNING AND POLICY

93 Stiles Road, Suite 201, Salem, New Hampshire 03079 USA 800 Turnpike Street, Suite 300, North Andover, Massachusetts 01845 USA Phone (603) 212-9133 and Fax (603) 226-4108 Email tepp@teppllc.com and Web www.teppllc.com

CONTENTS

SUMMARY1
Project Description1
Study Scope1
Trip Generation1
Capacity Analysis2
Traffic Impacts2
INTRODUCTION
Project Description
Study Approach
EXISTING CONDITIONS
Introduction5
Physical Conditions5
Traffic Volumes
Vehicle Speeds7
Sight Distances
FUTURE CONDITIONS11
Introduction11
Planned Road Improvements11
Background-Traffic Growth11
No-Build Traffic Volumes11
Trip Generation14
Trip Distribution and Network Assignment14
Build Traffic Volumes15
Traffic-Volume Changes15
CAPACITY ANALYSIS
Introduction
Methods20

TEPP

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Results		21
CONCLUSION		25
Project Descrip	otion	25
Trip Generatio	n	25
Capacity Analy	ysis	25
Traffic Impacts	S	25
APPENDIX		1
Appendix A:	Project Plan	
Appendix B:	Traffic Counts	
Appendix C:	Monthly Traffic Volumes	
Appendix D:	Vehicle Speeds	
Appendix E:	Capacity-Analysis Worksheets	
Appendix F:	Comments and Responses	



TABLES

Table 1.	2021 existing traffic volumes	7
Table 2.	Vehicle speeds.	
Table 3.	Sight distances.	
Table 4.	Calculated weekday trip generation	
Table 5.	Trip distribution and network assignment.	
Table 6.	Traffic-volume changes.	
Table 7.	Level-of-service criteria for intersections	
Table 8.	Capacity-analysis summary.	



FIGURES

Figure 1.	Site location.	4
Figure 2.	2021 existing traffic volumes	8
Figure 3.	2022 no-build traffic volumes	12
Figure 4.	2032 no-build traffic volumes	13
Figure 5.	Site traffic volumes added by redevelopment	16
Figure 6.	2022 build traffic volumes.	17
Figure 7.	2032 build traffic volumes.	18



SUMMARY

PROJECT DESCRIPTION

Keach-Nordstrom Associates, Inc. (KNA) has retained TEPP LLC to prepare this traffic impact and access study (TIAS) for a proposed commercial redevelopment in the Town of Hudson, New Hampshire.

The proposed redevelopment will:

- be at on Robinson Road opposite Christine Drive
- provide about 72,900 square feet (sf) of light-industrial floor area
- have two driveways along the east side of Robinson Road, with the north driveway opposite Christine Drive and with the south driveway about 250 feet (ft) to the south

STUDY SCOPE

The TIAS study area includes the following unsignalized intersections:

- Robinson Road/Christine Drive/north driveway
- Robinson Road/south driveway

This TIAS analyzes the following conditions as applicable:

- 2021 existing
- 2022 and 2032 no-build, with background-traffic growth
- 2022 and 2032 build, with background-traffic growth and the proposed redevelopment

This TIAS analyzes traffic operations for the following hours as applicable:

- weekday AM-street-peak hour
- weekday PM-street-peak hour

TRIP GENERATION

Calculated changes in weekday vehicle-trips for the redeveloped site are:



- daily, 329 (total of in and out)
- AM-street-peak hour, 35 (31 in and 4 out)
- PM-street-peak hour, 25 (2 in and 23 out)

CAPACITY ANALYSIS

Capacity analysis shows low delays throughout the study area.

TRAFFIC IMPACTS

Analysis indicates no significant area impact due to the proposed redevelopment.

INTRODUCTION

PROJECT DESCRIPTION

KNA has retained TEPP LLC to prepare this TIAS for a proposed commercial redevelopment in the Town of Hudson, New Hampshire.

The proposed redevelopment will:

- be at on Robinson Road opposite Christine Drive
- provide about 72,900 sf of light-industrial floor area
- have two driveways along the east side of Robinson Road, with the north driveway opposite Christine Drive and with the south driveway about 250 ft to the south

Figure 1 shows site location. The project plan is in Appendix A.

STUDY APPROACH

This TIAS assesses traffic impacts and access for the proposed redevelopment.

The TIAS study area includes the following unsignalized intersections:

- Robinson Road/Christine Drive/north driveway
- Robinson Road/south driveway

This TIAS analyzes the following conditions as applicable:

- 2021 existing
- 2022 and 2032 no-build, with background-traffic growth
- 2022 and 2032 build, with background-traffic growth and the proposed redevelopment

This TIAS analyzes traffic operations for the following hours as applicable:

- weekday AM-street-peak hour
- weekday PM-street-peak hour

Differences in traffic operations between the no-build and build conditions approximate traffic impacts of the proposed redevelopment.



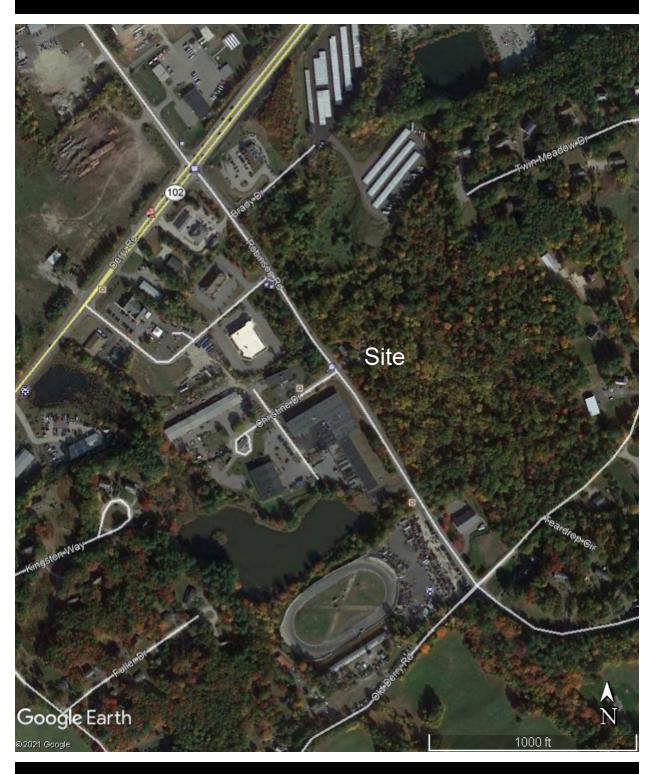


Figure 1. Site location.



EXISTING CONDITIONS

INTRODUCTION

Existing conditions include:

- physical conditions of the transportation network, roads, and intersections
- traffic volumes
- other relevant information

PHYSICAL CONDITIONS

INTRODUCTION

Figure 1 shows the transportation network.

The TIAS study area includes the following existing unsignalized intersection: Robinson Road/Christine Drive.

Description of the TIAS study area follows.

ROBINSON ROAD

Robinson Road:

- is oriented approximately north-south
- functions as a collector street
- to the north, connects with NH 102 (Derry Road), an arterial highway that leads to the Towns of Londonderry and Derry
- near the site, has a horizontal alignment that includes minor curvature
- near the site, has a vertical alignment that includes minor grades
- has a two-lane cross-section, with one travel lane per direction and paved shoulders
- has asphaltic-cement concrete pavement in overall good condition
- near the site, includes utility poles along the east side
- has nearby commercial development



• is under the jurisdiction of the Town

ROBINSON ROAD/CHRISTINE DRIVE INTERSECTION

The intersection:

- is four legged
- has Robinson Road as the major north-south street
- has Christine Drive as the minor west leg
- has the north driveway as the minor east leg
- has one-lane approaches
- operates as unposted STOP control on the minor-leg approaches
- has commercial development nearby

TRAFFIC VOLUMES

TRAFFIC COUNTS

TEPP LLC obtained an automatic traffic counter (ATR) count:

- on Robinson Road south of Christine Drive
- from Tuesday, June 29, to Wednesday, June 30, 2021

The ATR data are in Appendix B.

TEPP LLC obtained turning-movement counts:

- including vehicle classifications, bicycles, and pedestrians
- at the Robinson Road/Christine Drive/north driveway intersection
- on Tuesday, June 29, 2021, from 7:00 to 9:00 AM and from 4:00 to 6:00 PM

ADJUSTMENTS

The June 2021 traffic counts were adjusted to reflect peak-month and non-pandemic conditions.

The increase to peak month was 2.0 percent, based on based on NHDOT 2019 monthly volumes for Group 4 (Urban Highways) averages in Appendix C,

The increase to pre-pandemic was 5.6 percent. NHDOT continuous count station 82229031, on Daniel Webster Highway north of Hilton Drive, in the Town of Merrimack showed May 2021 two-way average-daily traffic (ADT) of 15,404 vehicles. The station showed May 2019 pre-pandemic two-way ADT of 16,260 vehicles, which is 5.6 percent greater.

The combined increase was 7.7 percent.

RESULTS

Table 1 and Figure 2 show 2021 existing traffic volumes.

Table 1. 2021 existing traffic volumes.					
Location and Time Period	Vehicles ^a	K-factor ^b	Percent Direction		
Robinson Road South of Christine Drivee					
Weekday Daily	4,133				
Weekday AM-Street-Peak Hour	303	7.3	61 Northbound		
Weekday PM-Street-Peak Hour	375	9.1	51 Southbound		

^a Two-way-total volumes.

^b K = hour volume as a percent of daily volume.

Robinson Road near the site frontage showed about:

- 4,133 weekday-daily vehicles
- 303 vehicles during the weekday AM street-peak hour, predominantly northbound
- 375 vehicles during the weekday PM street-peak hour, slightly southbound

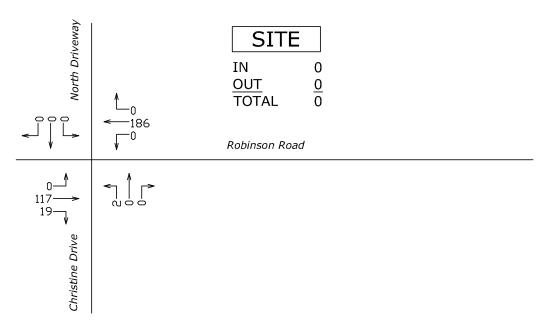
VEHICLE SPEEDS

The ATR collected vehicle speeds:

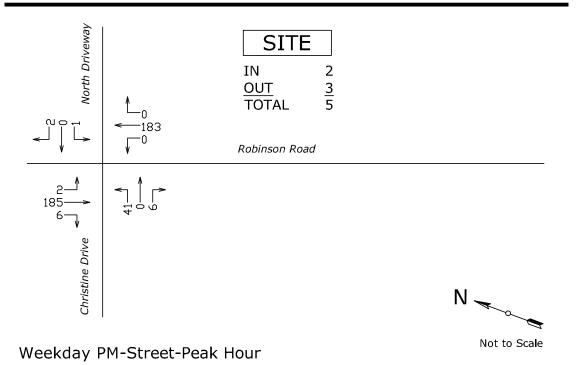
- on Robinson Road south of Christine Drive
- from Tuesday, June 29, to Wednesday, June 30, 2021

The data are in Appendix D and are summarized in Table 2.





Weekday AM-Street-Peak Hour



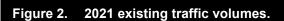


Table 2. Vehicle speeds.									
	Speeds (mph)								
Location and Direction	Speed Limit	Mean ^a	85 th Percentile ^a						
Robinson Road South of Christine Drive									
Northbound		39.7	44.0						
Southbound		38.6	42.8						

^a From ATR conducted from Tuesday, June 29, to Wednesday, June 30, 2021.

Table 2 indicates that on Robinson Road:

- speed limit was not posted
- the northbound the mean speed was 39.7 mph and the 85th percentile speed was 44.0 mph
- for southbound the mean speed was 38.6 mph and the 85th percentile speed was 42.8 mph

SIGHT DISTANCES

The American Association of State Highway and Transportation Officials (AASHTO) has established authoritative policy for sight distances at unsignalized intersections in terms of:

- stopping sight distance (SSD)
- optional intersection sight distance (ISD)¹

SSD:²

- provides for safety
- enables a driver, on the major road, to perceive and react accordingly to a vehicle entering the major road from a minor road
- is conservative because it encompasses a wide range of brake-reaction times and deceleration rates

Optional ISD:³

¹ AASHTO, *A Policy on Geometric Design of Highways and Streets*, 7th Edition (Washington, DC, 2018), pages 9-35 to 9-36.

 $^{^{2}}$ AASHTO, pages 3-2 to 3-6.

³ AASHTO, pages 9-35 to 9-59.



- is ordinarily greater than SSD and may enhance traffic operations
- is not required for safety

Table 3 shows relevant available sight distances that are at least 400 ft, per NHDOT practice, and are adequate. Appendix A includes sight-distance plans and profiles.

Table 3. Sight distances.									
Intersection, Movements, and View	Available Sight		Speeds (miles per hour)						
	Distance (ft) ^a	Limit	SSD Provides For	ISD Provides For					
Robinson Road/Christine Drive/North Driveway, for North Driveway Movements									
Robinson Road to/from North	400	30	45+	36+					
Robinson Road to/from South	400	30	45+	36+					
Robinson Road/Christine Drive/South Driveway, for South Driveway Movements									
Robinson Road to/from North	400	30	45+	36+					
Robinson Road to/from South	400	30	45+	36+					

^a With appropriate roadside and vegetation maintenance.



FUTURE CONDITIONS

INTRODUCTION

Future conditions include:

- planned road improvements independent of the proposed redevelopment
- future no-build traffic volumes, with background-traffic growth and without the proposed redevelopment
- future build traffic volumes, with background-traffic growth and with the proposed redevelopment

PLANNED ROAD IMPROVEMENTS

TEPP LLC identified no significant planned road improvement in the study area independent of the project.

BACKGROUND-TRAFFIC GROWTH

Background-traffic growth:

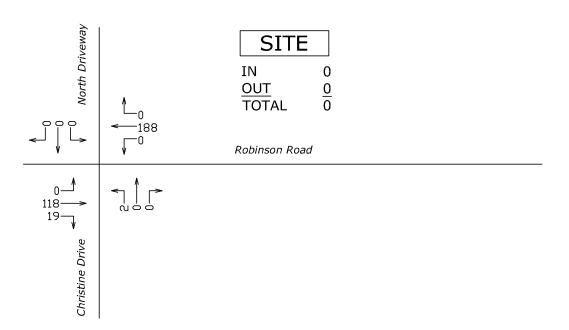
- is independent of the proposed redevelopment
- is related to land development in the immediate area, population and economic development in the region, and changes in travel patterns in the region
- typically considers two factors: a general traffic-growth rate and specific planned land developments in the immediate area

This TIAS uses a 1.0-percent annual growth rate. This yields about 11.6-percent growth between 2021 and 2032.

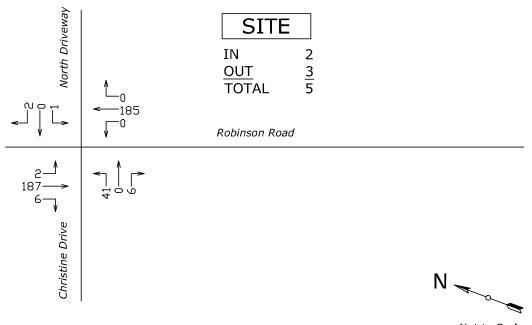
NO-BUILD TRAFFIC VOLUMES

The background-traffic growth described above was applied to 2021 existing traffic volumes. Figures 3 and 4 show 2022 and 2032 no-build traffic volumes.





Weekday AM-Street-Peak Hour

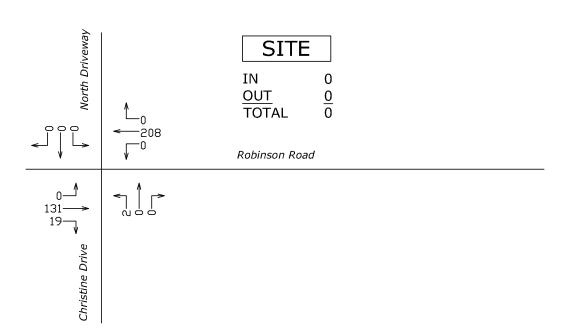


Weekday PM-Street-Peak Hour

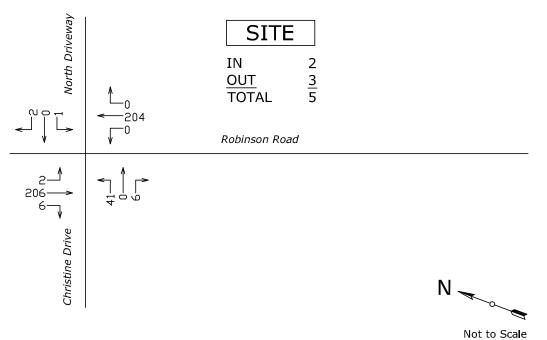


Figure 3. 2022 no-build traffic volumes.

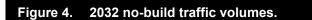




Weekday AM-Street-Peak Hour



Weekday PM-Street-Peak Hour





TRIP GENERATION

The Institute of Transportation Engineers (ITE) publishes trip-generation information in the authoritative *Trip Generation Manual.*⁴ This information is based on empirical data for a variety of land uses including general light industrial, land use 110, based on floor area.⁵

Table 4 shows calculated changes in weekday vehicle-trips for the redeveloped site as:

Table 4. Calculated weekday trip generation.							
		Vehicle-Trips					
Time Period and Direction	Existing ^a	Future ^b	Difference				
Daily	5	334	329				
AM-Street-Peak Hour							
In	0	31	31				
Out	<u>0</u>	<u>4</u>	<u>4</u>				
Total	0	35	35				
PM-Street-Peak Hour							
In	2	4	2				
Out	<u>3</u>	<u>26</u>	<u>23</u>				
Total	5	30	25				

^a Based on 2021 existing traffic volumes.

^b Based on ITE, general light industrial, land use 110, 72,900-sf floor area.

- daily, 329 (total of in and out)
- AM-street-peak hour, 35 (31 in and 4 out)
- PM-street-peak hour, 25 (2 in and 23 out)

TRIP DISTRIBUTION AND NETWORK ASSIGNMENT

Trip distribution and network assignment of vehicle-trips to and from the site may consider such factors as existing site distribution, travel patterns, population, regional land development, and

⁴ ITE, *Trip Generation Manual*, 10th Edition (Washington DC, September 2017).

⁵ ITE, *Trip Generation Manual*, Volume 2, Data, Industrial (Land Uses 100-199), pages 1 to 19.

TEPP

site access. Trip distribution and network assignment for this TIAS considered the 2021 existing volumes entering and exiting Christine Drive.

Table 5. Trip distribution and	network assignment.
Road and Direction (To/From)	Approximate Percent
Robinson Road to/from North	87
Robinson Road to/from South	<u>13</u>
Total	100

Table 5 and Figure 5 shows trip distribution and network assignment for site trips.

BUILD TRAFFIC VOLUMES

Site traffic volumes were superimposed on the no-build traffic volumes to estimate build traffic volumes. Figures 6 and 7 show the resulting 2022 and 2032 build traffic volumes.

TRAFFIC-VOLUME CHANGES

Table 6 presents calculated traffic-volume changes due to the proposed redevelopment for the:

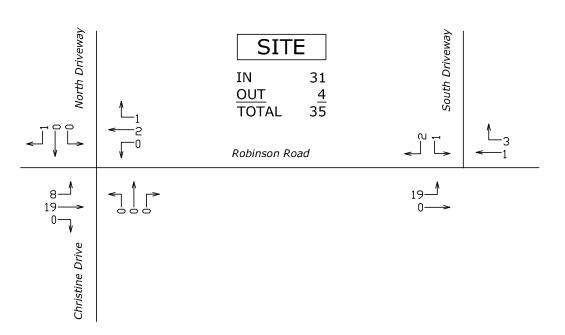
- weekday AM-street-peak hour
- weekday PM-street-peak hour

Table 6 shows peak-hour-traffic-volume increases:

- of 3 to 30 vehicle-trips
- constituting averages about one vehicle-trip per 2 to 20 minutes
- that are further split by northbound and southbound direction on Robinson Road

The Derry Road/Robinson Road/West Road signalized intersection is about 1,200 ft north of the Robinson Road/Christine Drive intersection. Traffic-volume increases at the former intersection due to the proposed redevelopment are up to:





Weekday AM-Street-Peak Hour

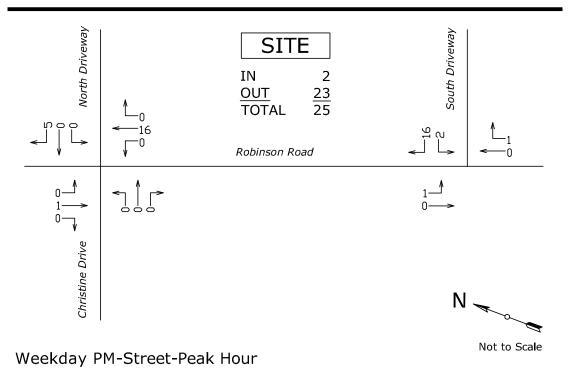
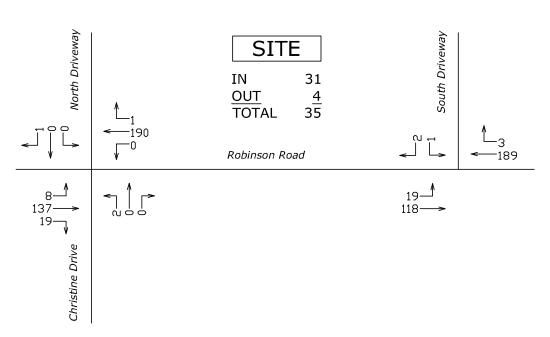
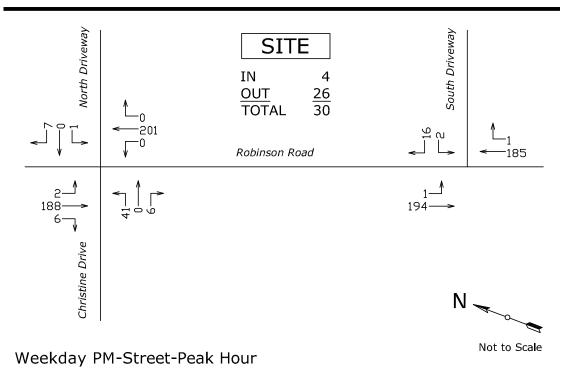


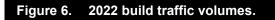
Figure 5. Site traffic volumes added by redevelopment.



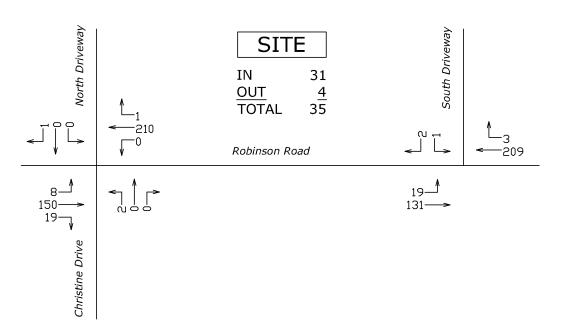


Weekday AM-Street-Peak Hour

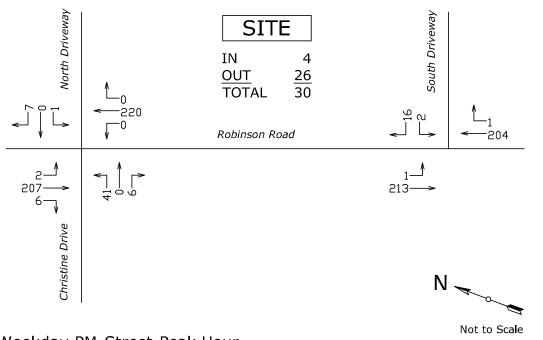








Weekday AM-Street-Peak Hour



Weekday PM-Street-Peak Hour





Table 6. Traffic-volume changes.									
	2022 Traf	fic Volume	es (vph) ^a	2032 Traffic Volumes (vph)					
Location and Time Period	No-Build	Build	Change	No-Build	Build	Change			
Robinson Road North of Site									
Weekday AM-Street-Peak Hour	327	357	30	360	390	30			
Weekday PM-Street-Peak Hour	423	445	22	461	483	22			
Robinson Road South of Site									
Weekday AM-Street-Peak Hour	306	311	5	339	344	5			
Weekday PM-Street-Peak Hour	379	382	3	417	420	3			

^a Two-way total volumes.

- for the weekday AM-street-peak hour, up to 27 vehicle-trips entering for the site
- for the weekday PM-street-peak hour, up to 21 vehicle-trips leaving the site

This approximates less than one vehicle-trip per signal cycle and constitutes no significant impact. Therefore, no further analysis of the Derry Road/Robinson Road/West Road intersection is warranted.

CAPACITY ANALYSIS

INTRODUCTION

This TIAS has quantified existing, future-no-build and future-build traffic volumes. Capacity analysis models the quality of traffic operations. Comparing build conditions to the no-build conditions indicates impacts of the proposed redevelopment on quality of traffic operations.

METHODS

Capacity analysis calculates LOS for transportation facilities. LOS indicates the quality of traffic operations based on delay and other measures. The six LOS are designated A to F. LOS A represents the best or highest operating conditions. LOS F is the lowest but does not necessarily connote failure.

LOS is a function of traffic volumes and traffic control. Because these volumes can vary, LOS of a transportation facility can differ by time of day, day of the week, or month. For example, a transportation facility with a low LOS during peak hours may have a high LOS during other hours. The operational analysis methods of the Transportation Research Board (TRB)⁶ models LOS for intersections based on calculated delay per vehicle, as shown in Table 7. Synchro analysis software was used.

Method inputs include:

- intersection geometry
- traffic control, such as YIELD sign, two-way STOP sign, all-way STOP sign, roundabout, or signal (including phasing, timing, and progression)
- traffic volumes
- vehicle composition, such as passenger cars and trucks

The methods are all approximate. In particular, the method for two-way STOP-sign control can be conservative, with observed delays and queuing shorter than those modeled.

⁶ TRB, *Highway Capacity Manual 2000* (Washington DC 2000) and *Highway Capacity Manual 2010* (Washington DC, 2010).

Table 7. Level-of-service criteria for intersections.								
	Control Delay (seconds/vehicle)							
Level of Service	Unsignalized Intersections ^a	Signalized Intersections						
А	≤10.0	≤10.0						
В	>10.0 and ≤15.0	$>10.0 \text{ and } \le 20.0$						
С	>15.0 and ≤25.0	>20.0 and ≤35.0						
D	>25.0 and ≤35.0	>35.0 and ≤ 55.0						
Е	>35.0 and ≤ 50.0	>55.0 and ≤ 80.0						
F	>50	>80						

From Transportation Research Board, Highway Capacity Manual 2010 (Washington D.C., 2010).

^a For YIELD sign, two-way STOP sign or all-way STOP sign, control delay defines LOS. For roundabout approaches and overall intersection, control delay defines LOS. For roundabout lanes with volume/capacity ratio ≤1.0, control delay defines LOS. For roundabout lanes with volume/capacity ratio > 1.0, LOS is F regardless of control delay.

RESULTS

Table 8 shows computed LOS, delays, and queues at study-area intersections for the:

- weekday AM-street-peak hour
- weekday PM-street-peak hour

The analysis is under the following conditions, as applicable:

- 2021 existing
- 2022 and 2032 no-build
- 2022 and 2032 build

Capacity-analysis worksheets that give detail and explanation are in Appendix E.

Table 8 shows low delays throughout the study area.

Intersection Control Hour	2021 Existing			2022 No Build			2032 No Build			2022 Build				2032 Build						
	Intersection, Control, Hour and Movement	LOS ^a Delay ^b V/C ^c Queue	Queued	LOS	Delay	V/C	Queue	LOS	Delay	V/C	Queue	LOS	Delay	V/C	Queue	LOS	Delay	V/C	Queue	
Robinson Road/Christine Drive	e/North Driv	eway Inters	ection, Uns	ignalized, We	eekday AN	1-Street-Pea	k Hour													
Robinson Road NB L	А	0.0	0.000	0.0	А	0.0	0.000	0.0	А	0.0	0.000	0.0	А	0.0	0.000	0.0	А	0.0	0.000	0.0
Robinson Road SB L	А	0.0	0.000	0.0	А	0.0	0.000	0.0	А	0.0	0.000	0.0	А	7.7	0.007	0.0	А	7.7	0.007	0.0
Christine Road EB LTR	В	13.1	0.009	0.0	В	13.1	0.009	0.0	В	13.6	0.010	0.0	В	13.7	0.010	0.0	В	14.3	0.010	0.0
North Driveway WB LTR	А	0.0	0.000	0.0	А	0.0	0.000	0.0	А	0.0	0.000	0.0	В	11.5	0.002	0.0	В	11.9	0.002	0.0
Robinson Road/Christine Drive	e/North Driv	eway Inters	ection, Uns	ignalized, We	eekday PM	-Street-Peal	k Hour													
Robinson Road NB L	А	0.0	0.000	0.0	А	0.0	0.000	0.0	А	0.0	0.000	0.0	А	0.0	0.000	0.0	А	0.0	0.000	0.0
Robinson Road SB L	А	7.6	0.002	0.0	А	7.6	0.002	0.0	А	7.6	0.002	0.0	А	7.7	0.002	0.0	А	7.7	0.002	0.0
Christine Road EB LTR	В	12.9	0.144	0.5	В	12.9	0.145	0.5	В	13.6	0.155	0.5	В	13.4	0.153	0.5	В	14.1	0.163	0.6
North Driveway WB LTR	В	10.3	0.012	0.0	В	10.3	0.012	0.0	В	10.6	0.012	0.0	А	9.9	0.028	0.1	В	10.1	0.029	0.1
Robinson Road/South Drivewa	y, Unsignal	ized, Weekd	ay AM-Stro	eet-Peak Hou	r															
Robinson Road SB L													А	7.7	0.016	0.0	А	7.8	0.016	0.0
North Driveway WB LR													А	9.9	0.005	0.0	В	10.1	0.005	0.0
Robinson Road/South Drivewa	y, Unsignal	ized, Weekd	ay PM-Stre	et-Peak Hour	r															
Robinson Road SB L													А	7.6	0.001	0.0	А	7.7	0.001	0.0
North Driveway WB LR													А	9.6	0.025	0.1	А	9.8	0.026	0.1

^a LOS = level of service.

^b Delay = average delay in seconds per vehicle.

^c V/C = volume/capacity ratio.

^d 95th percentile queue in vehicles.

EB = eastbound, WB = westbound, SB = southbound, NB = northbound, L = left, T = through, R = right.





CONCLUSION

PROJECT DESCRIPTION

The proposed redevelopment will:

- be at on Robinson Road opposite Christine Drive
- provide about 72,900 sf of light-industrial floor area
- have two driveways along the east side of Robinson Road, with the north driveway opposite Christine Drive and with the south driveway about 250 ft to the south

TRIP GENERATION

Calculated changes in weekday vehicle-trips for the redeveloped site are:

- daily, 329 (total of in and out)
- AM-street-peak hour, 35 (31 in and 4 out)
- PM-street-peak hour, 25 (2 in and 23 out)

CAPACITY ANALYSIS

Capacity analysis shows low delays throughout the study area.

TRAFFIC IMPACTS

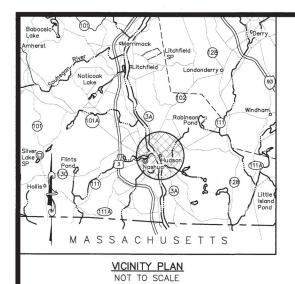
Analysis indicates no significant area impact due to the proposed redevelopment.



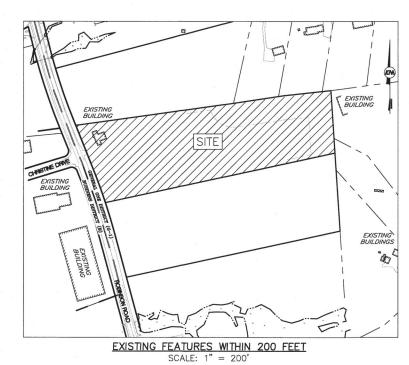
APPENDIX



Appendix A: Project Plan



NON-RESIDENTIAL SITE PLAN S.L. CHASSE STEEL MAP 105 LOT 17-2 ROBINSON ROAD HUDSON, NEW HAMPSHIRE



LEGEND WETLAND EDGE OF PAVEMENT ZONE BOUNDARY

> PROPERTY LINE PROPERTY LINE

OWNER OF RECORD/APPLICANT: STEEL PROPERTIES, LLC 8 CHRISTINE DRIVE HUDSON, NEW HAMPSHIRE 03051

PREPARED BY:

KEACH-NORDSTROM ASSOCIATES, INC. 10 COMMERCE PARK NORTH, SUITE 3 BEDFORD, NEW HAMPSHIRE 03110 (603) 627-2881



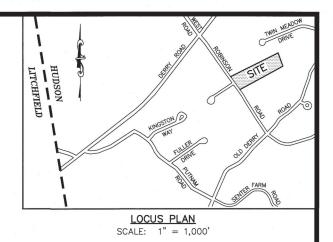
KEACH-NORDSTROM ASSOCIATES, INC. Civil Engineering Land Surveying Landscape Architecture nerce Park North, Suite 3B, Bedford, NH 03110 Phone (603) 627-2881

APRIL 6, 2021 REVISED MAY 12, 2021 PROJECT NO. 20-0921-2

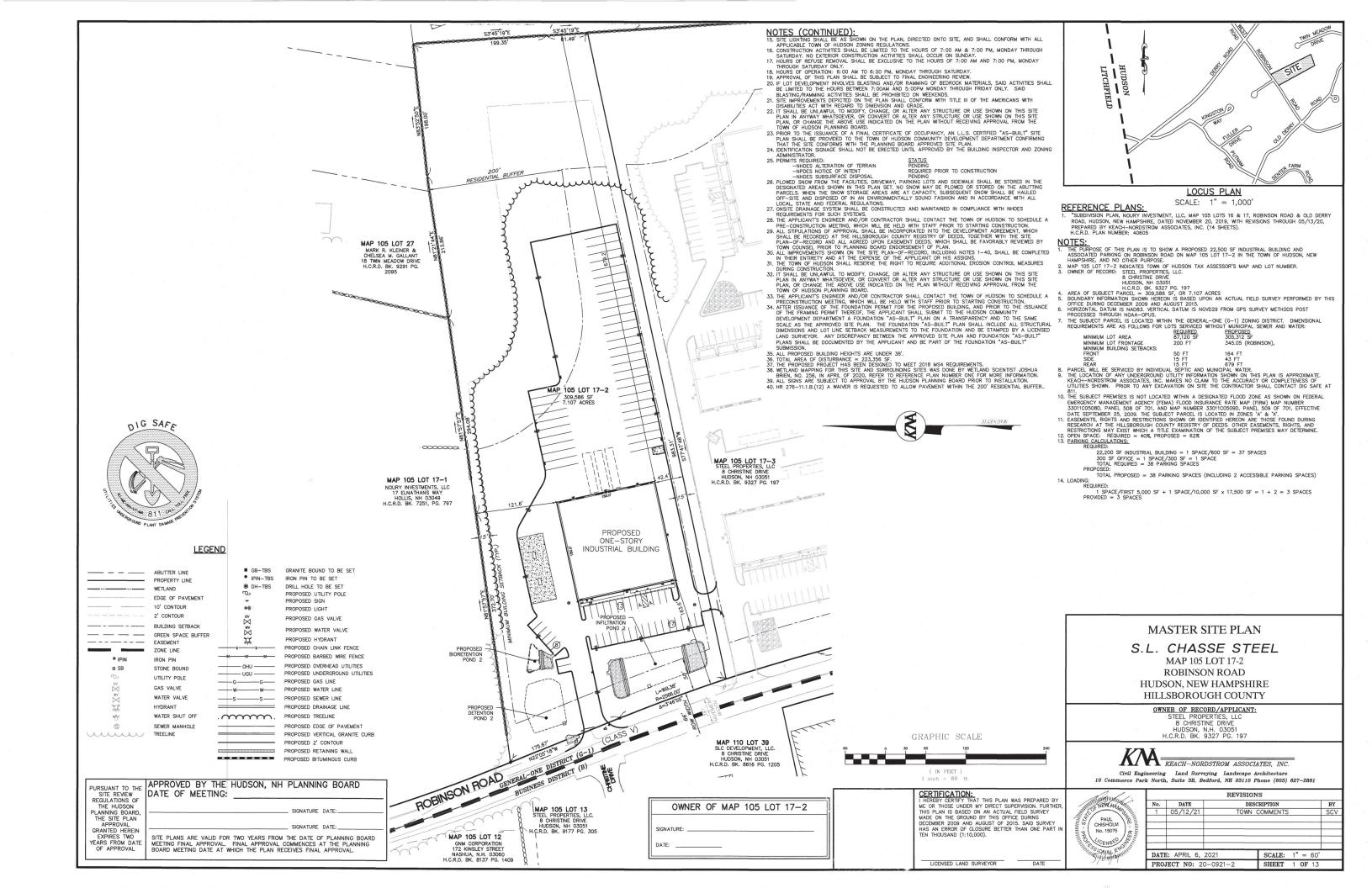
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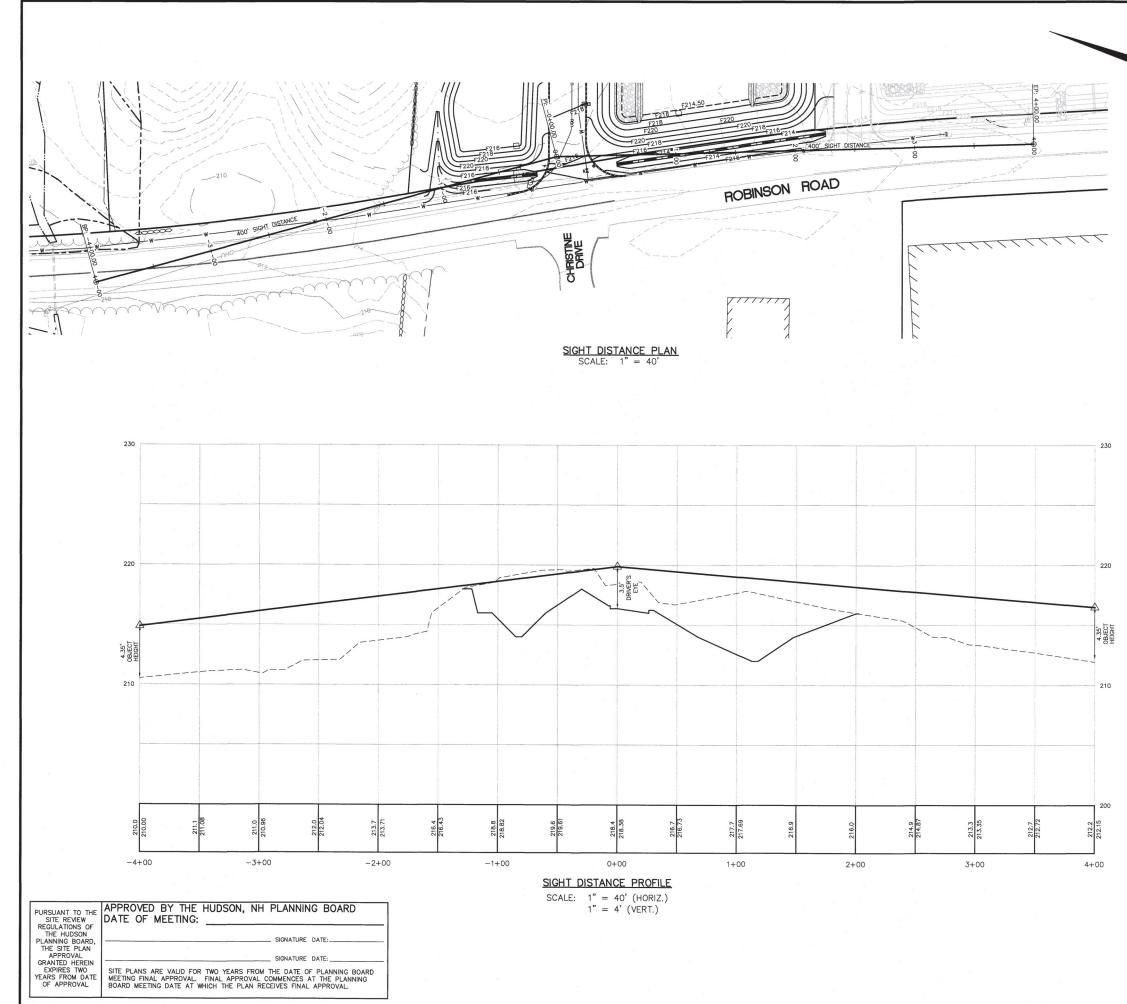
MASTER SITE EXISTING CON REMOVALS PLA NON-RESIDEN GRADING, DRA EROSION CONT LANDSCAPE PI LIGHTING PLA SIGHT DISTAN CONSTRUCTION

PURSUANT TO THE SITE REVIEW	APPROVED BY THE HUDSON, NH PLANNING BOARD DATE OF MEETING:
REGULATIONS OF THE HUDSON PLANNING BOARD, THE SITE PLAN APPROVAL	SIGNATURE DATE:
GRANTED HEREIN EXPIRES TWO YEARS FROM DATE OF APPROVAL	SITE PLANS ARE VALID FOR TWO YEARS FROM THE DATE OF PLANNING BOARD MEETING FINAL APPROVAL. FINAL APPROVAL COMMENCES AT THE PLANNING BOARD MEETING DATE AT WHICH THE PLAN RECEIVES FINAL APPROVAL.



<u>'LE</u>	SHEET No.
PLAN	1
NDITIONS PLAN	2
AN	3
TIAL SITE LAYOUT PLAN	4
AINAGE & UTILITY PLAN	5
TROL PLAN	6
LAN	7
AN .	8
ICE PLAN & PROFILE	9
N DETAILS	10-13





	UTILITY POLE ABUTTER LINE PROPERTY LINE OVERHEAD UTILITIES TREELINE EDGE OF PAVEMENT VERTICAL GRANITE CURB BITUMINOUS CURB 10' CONTOUR
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HUDSON, NEW HAMPSHIRE HILLSBOROUGH COUNTY							
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K	OWNER OF RECO STEEL PROP 8 CHRISTI HUDSON, N H.C.R.D. BK. S	DRD/APPLICANT: ERTIES, LLC NE DRIVE I.H. 03051 J327 PG. 197	4				
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DATE: APRIL 6, 2021

PROJECT NO: 20-0921-2

SCALE: 1'' = 40'

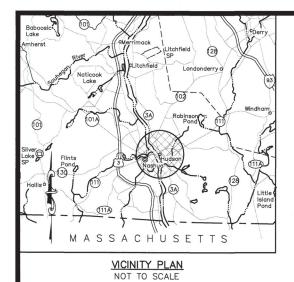
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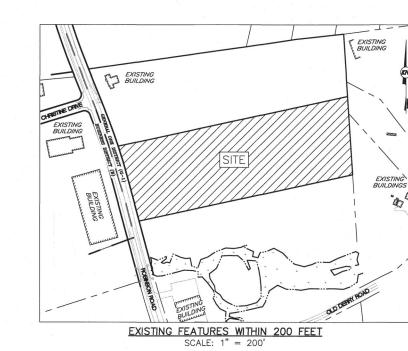
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NON-RESIDENTIAL SITE PLAN S.L. CHASSE CONTRACTOR BUILDINGS MAP 105 LOT 17-3 ROBINSON ROAD HUDSON, NEW HAMPSHIRE



LEGEND WETLAND EDGE OF PAVEMENT ZONE BOUNDARY PROPERTY LINE PROPERTY LINE

OWNER OF RECORD/APPLICANT: SLC DEVELOPMENT, LLC 8 CHRISTINE DRIVE HUDSON, NEW HAMPSHIRE 03051

PREPARED BY:

KEACH-NORDSTROM ASSOCIATES, INC. 10 COMMERCE PARK NORTH, SUITE 3 BEDFORD, NEW HAMPSHIRE 03110 (603) 627-2881

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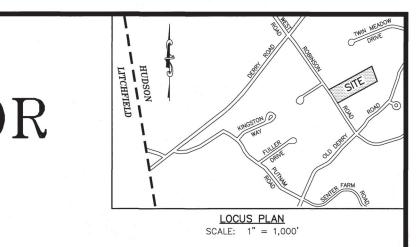
Civil Engineering Land Surveying Landscape Architecture 10 Commerce Park North, Suite 3B, Bedford, NH 03110 Phone (603) 627-2881

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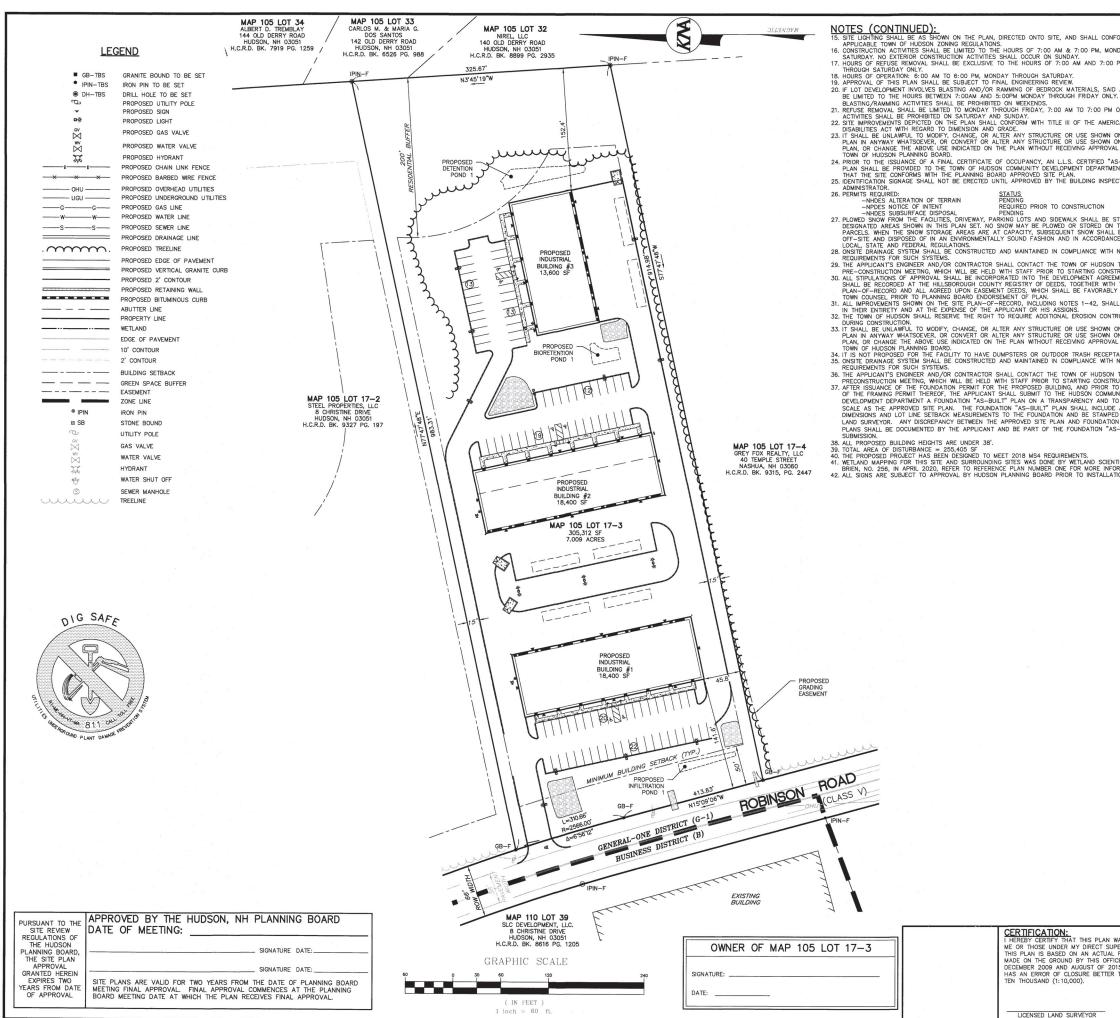
APRIL 6, 2021 REVISED MAY 12, 2021 PROJECT NO. 20-0921-2

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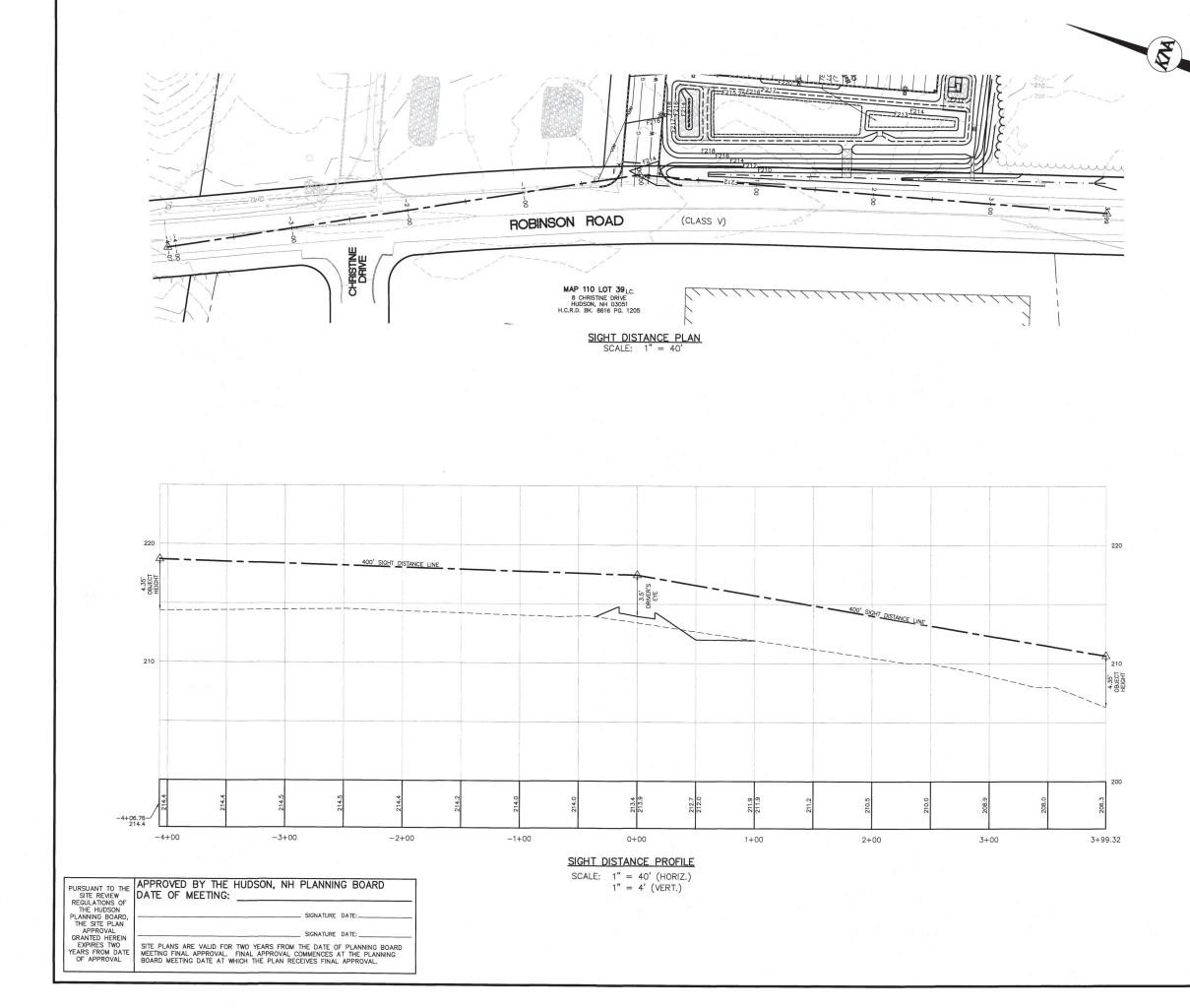
PURSUANT TO THE	APPROVED BY THE HUDSON, NH PLANNING BOARD
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PLANNING BOARD,	SIGNATURE DATE:
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ITH NHDES SON TO SCHEDULE A	ROAD, HUDSON, NEW HAMPSH PREPARED BY KEACH-NORDS H.C.R.D. PLAN NUMBER: 4060	IRE, DATED NOVEMBER	20, 2019, WITH REVI:	SIONS THROUGH 05/13/2	20,
NSTRUCTION. REEMENT, WHICH WITH THE SITE	NOTES: 1. THE PURPOSE OF THIS PLAN AND ASSOCIATED PARKING O HAMPSHIRE, AND NO OTHER	IS TO SHOW THREE PF N ROBINSON ROAD ON	ROPOSED INDUSTRIAL MAP 105 LOT 17-3 I	BUILDINGS TOTALING 50, IN THE TOWN OF HUDSON	400 SF N, NEW
	 MAP 105 LOT 17 INDICATES OWNER OF RECORD: STEEL I 	TOWN OF HUDSON TAX PROPERTIES, LLC.	ASSESSOR'S MAP AN	D LOT NUMBER.	
	HUDSON H.C.R.D. 4. AREA OF SUBJECT PARCEL =	STINE DRIVE N, N.H. 03051 . BK. 9327 PG. 197 = 305,312 SF, OR 7.00	9 ACRES		
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ON TO SCHEDULE A	7. THE SUBJECT PARCEL IS LOC REQUIREMENTS ARE AS FOLL MINIMUM LOT AREA	CATED WITHIN THE GENE OWS FOR LOTS SERVICE REQUIRED	PROPOSED		NAL
R TO THE ISSUANCE MMUNITY D TO THE SAME	MINIMUM LOT FRONTAG MINIMUM BUILDING SETI FRONT	BACKS: 50 FT	322.83 (RC 142 FT		
UDE ALL STRUCTURAL MPED BY A LICENSED ATION "AS-BUILT" "AS-BUILT"	SIDE REAR 8. PARCEL WILL BE SERVICED B 9. THE LOCATION OF ANY UNDE	15 FT 15 FT Y INDIVIDUAL SEPTIC AI RGROUND UTILITY INFOR	46 FT 152 FT ND WELL. RMATION SHOWN ON T	THIS PLAN IS APPROXIMA	TF.
	KEACH-NORDSTROM ASSOCIA UTILITIES SHOWN. PRIOR TO 811.				
CIENTIST JOSHUA NFORMATION.	 THE SUBJECT PREMISES IS N EMERGENCY MANAGEMENT AG 33011C0508D, PANEL 508 OF DATE SEPTEMBER 25, 2009. 	ENCY (FEMA) FLOOD IN 7 701, AND MAP NUMBE	ISURANCE RATE MAP ER 33011C0509D, PAN	(FIRM) MAP NUMBER NEL 509 OF 701, EFFECT	IVE
	 EASEMENTS, RIGHTS AND RES RESEARCH AT THE HILLSBORG RESTRICTIONS MAY EXIST WHI 	STRICTIONS SHOWN OR OUGH COUNTY REGISTR' ICH A TITLE EXAMINATIO 40%, PROPOSED = 48%	IDENTIFIED HEREON AN Y OF DEEDS. OTHER E ON OF THE SUBJECT I	RE THOSE FOUND DURING EASEMENTS, RIGHTS, AND PREMISES MAY DETERMIN	G) IE.
	13. <u>PARKING CALCULATIONS:</u> REQUIRED: INDUSTRIAL ≈ 1 SPACE	E/600 SF			
	BUILDING 2: 1 SP BUILDING 3: 1 SP	ACE/600 SF X 17,480 ACE/600 SF X 17,480 ACE/600 SF X 12,920	SF = 29.13 SPACES		~
	BUILDING 2: 1 SP	0 SF ACE/300 SF X 920 SF ACE/300 SF X 920 SF ACE/300 SF X 680 SF	= 3.07 SPACES		
	TOTAL BUILDING 1: 29.13 BUILDING 2: 29.13	3 SPACES + 3.07 SPAC 3 SPACES + 3.07 SPAC 3 SPACES + 2.27 SPAC	CES = 33 SPACES CES = 33 SPACES		
	TOTAL COMBINED	SPACES REQUIRED = 9	O SPACES	s	×
0 2	14. LOADING:	PACES + 2 HANDICAP PACES + 2 HANDICAP PACES + 2 HANDICAP = 44 SPACES + 31 S	SPACES = 31 SPACE SPACES = 26 SPACE PACES + 26 SPACES	S = 101 TOTAL SPACES	
	REQUIRED: 1 SPACE/FIRST 5, BUILDING 1: 1 + 1.34 BUILDING 2: 1 + 1.34	,000 SF + 1 SPACE/10 = 3 LOADING SPACES = 3 LOADING SPACES	0,000 SF x 45,400 SF	7 = 1 + 4.54 = 6 SPAC	ES
	BUILDING 3: 1 + 0 = PROPOSED:	1 LOADING SPACE			
	BUILDING 1: 3 SPACES BUILDING 2: 3 SPACES BUILDING 3: 1 SPACE TOTAL PROPOSED: 7 S				
		MASTER	SITE PLAN	N	
	S.	L. CHAS	SSE STE		
	×	MAP 105 ROBINSC	LOT 17-3 DN ROAD		
		IUDSON, NEV HILLSBOROU			
		OWNER OF REC	ORD/APPLICANT:		
	3	8 CHRIST	PERTIES, LLC INE DRIVE NH 03051 9327 PG. 197		
		п.с.к.р. вк.	3021 FG. 19/		
			DSTROM ASSOCIA		
				Phone (603) 627-2881	
N WAS PREPARED BY SUPERVISION. FURTHER, JAL FIELD SURVEY	WHITTIN NEW HARD	No. DATE		VS CRIPTION COMMENTS	BY
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DATE	SIONAL CONTRACTOR	DATE: APRIL 6, PROJECT NO: 2		SCALE: $1" = 60$ SHEET 1 OF 12	



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GRAPHIC SCALE

(IN FEET) 1 inch = 40 ft. SIGHT DISTANCE PLAN & PROFILE S.L. CHASSE STEEL MAP 105 LOT 17-3 ROBINSON ROAD HUDSON, NEW HAMPSHIRE HILLSBOROUGH COUNTY

OWNER OF RECORD/APPLICANT: STEEL PROPERTIES, LLC 8 CHRISTINE DRIVE HUDSON, NH 03051 H.C.R.D. BK. 9327 PG. 197												
	neering	Land Surv	DSTROM ASSOCIA eying Landscape A edford, NH 03110 P.									
111			REVISION	S								
WHAT HIM IN MILLER	No.	DATE	DESC	RIPTION	BY							
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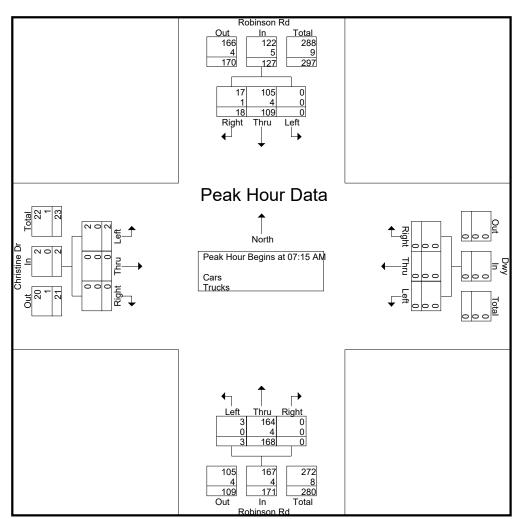
Appendix B: Traffic Counts

6/28/2021	Monday	/	Tuesda		Wednes		Thursd		Frida		Saturda		Sunda	ау	Week Ave	
Time	SB,	NB,	SB,	NB,	SB,	NB,	SB,	NB,	SB,	NB,	SB,	NB,	SB,	NB,	SB,	NB,
12:00 AM	*	*	3	4	7	2	*	*	*	*	*	*	*	*	5	3
1:00	*	*	7	7	2	2	*	*	*	*	*	*	*	*	4	4
2:00	*	*	3	6	3	4	*	*	*	*	*	*	*	*	3	5
3:00	*	*	2	5	2	6	*	*	*	*	*	*	*	*	2	6
4:00	*	*	10	26	9	28	*	*	*	*	*	*	*	*	10	27
5:00	*	*	48	79	39	75	*	*	*	*	*	*	*	*	44	77
6:00	*	*	83	103	80	117	*	*	*	*	*	*	*	*	82	110
7:00	*	*	91	179	77	173	*	*	*	*	*	*	*	*	84	176
8:00	*	*	101	138	114	167	*	*	*	*	*	*	*	*	108	152
9:00	*	*	102	133	104	127	*	*	*	*	*	*	*	*	103	130
10:00	*	*	87	113	98	122	*	*	*	*	*	*	*	*	92	118
11:00	*	*	134	115	134	113	*	*	*	*	*	*	*	*	134	114
12:00 PM	*	*	130	134	130	120	*	*	*	*	*	*	*	*	130	127
1:00	*	*	99	105	112	122	*	*	*	*	*	*	*	*	106	114
2:00	*	*	128	126	126	109	*	*	*	*	*	*	*	*	127	118
3:00	*	*	145	156	134	157	*	*	*	*	*	*	*	*	140	156
4:00	*	*	152	161	178	155	*	*	*	*	*	*	*	*	165	158
5:00	*	*	178	145	173	108	*	*	*	*	*	*	*	*	176	126
6:00	*	*	105	106	92	92	*	*	*	*	*	*	*	*	98	99
7:00	*	*	94	77	56	54	*	*	*	*	*	*	*	*	75	66
8:00	*	*	76	61	69	48	*	*	*	*	*	*	*	*	72	54
9:00	*	*	56	28	41	27	*	*	*	*	*	*	*	*	48	28
10:00	*	*	26	16	22	18	*	*	*	*	*	*	*	*	24	17
11:00	*	*	13	9	13	7	*	*	*	*	*	*	*	*	13	8
Total	0	0	1873	2032	1815	1953	0	0	0	0	0	0	0	0	1845	1993
Day	0	•	3905		3768	•	0	•	0		0		0		3838	
AM Peak			11:00	7:00	11:00	7:00									11:00	7:00
Volume			134	179	134	173									134	176
PM Peak			5:00	4:00	4:00	3:00									5:00	4:00
Volume			178	161	178	157									176	158
Comb Total	0		3905	!	3768		0		0		0		0		3838	
ADT	ADT	: 3,836	AAD	T: 3,836												

					Groups P	rinted- Ca	ars - Trucks						
	Rol	binson Rd			Dwy		Rc	obinson Rd		Cł	hristine Dr		
	Fro	om North		Fr	rom East		<u> </u>	rom South		<u> </u>	rom West		
Start Time	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Int. Total
07:00 AM	0	16	1	0	0	0	0	32	0	0	0	0	49
07:15 AM	0	21	3	0	0	0	0	43	0	1	0	0	68
07:30 AM	0	28	4	0	0	0	0	43	0	0	0	0	75
07:45 AM	0	31	5	0	0	0	2	47	0	1	0	0	86
Total	0	96	13	0	0	0	2	165	0	2	0	0	278
08:00 AM	0	29	6	0	0	0	1	35	0	0	0	0	71
08:15 AM	0	16	5	0	0	0	0	34	1	1	0	0	57
08:30 AM	0	29	4	0	0	0	0	35	0	3	0	1	72
08:45 AM	0	23	4	1	0	0	2	28	0	3	0	0	61
Total	0	97	19	1	0	0	3	132	1	7	0	1	261
Grand Total	0	193	32	1	0	0	5	297	1	9	0	1	539
Apprch %	0	85.8	14.2	100	0	0	1.7	98	0.3	90	0	10	
Total %	0	35.8	5.9	0.2	0	0	0.9	55.1	0.2	1.7	0	0.2	
Cars	0	186	30	1	0	0	5	289	1	8	0	1	521
% Cars	0	96.4	93.8	100	0	0	100	97.3	100	88.9	0	100	96.7
Trucks	0	7	2	0	0	0	0	8	0	1	0	0	18
% Trucks	0	3.6	6.2	0	0	0	0	2.7	0	11.1	0	0	3.3

			son Rd				wy				son Rd				tine Dr		
		From	North			From	n East			From	South			From	n West		
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analy	sis From	n 07:00 /	AM to 08	8:45 AM -	Peak 1 c	of 1											
Peak Hour for Er	ntire Inte	rsection	Begins	at 07:15 A	M												
07:15 AM	0	21	3	24	0	0	0	0	0	43	0	43	1	0	0	1	68
07:30 AM	0	28	4	32	0	0	0	0	0	43	0	43	0	0	0	0	75
07:45 AM	0	31	5	36	0	0	0	0	2	47	0	49	1	0	0	1	86
08:00 AM	0	29	6	35	0	0	0	0	1	35	0	36	0	0	0	0	71
Total Volume	0	109	18	127	0	0	0	0	3	168	0	171	2	0	0	2	300
% App. Total	0	85.8	14.2		0	0	0		1.8	98.2	0		100	0	0		
PHF	.000	.879	.750	.882	.000	.000	.000	.000	.375	.894	.000	.872	.500	.000	.000	.500	.872
Cars	0	105	17	122	0	0	0	0	3	164	0	167	2	0	0	2	291
% Cars	0	96.3	94.4	96.1	0	0	0	0	100	97.6	0	97.7	100	0	0	100	97.0
Trucks	0	4	1	5	0	0	0	0	0	4	0	4	0	0	0	0	9
% Trucks	0	3.7	5.6	3.9	0	0	0	0	0	2.4	0	2.3	0	0	0	0	3.0

Accurate Counts 978-664-2565



Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1 Peak Hour for Each Approach Begins at:

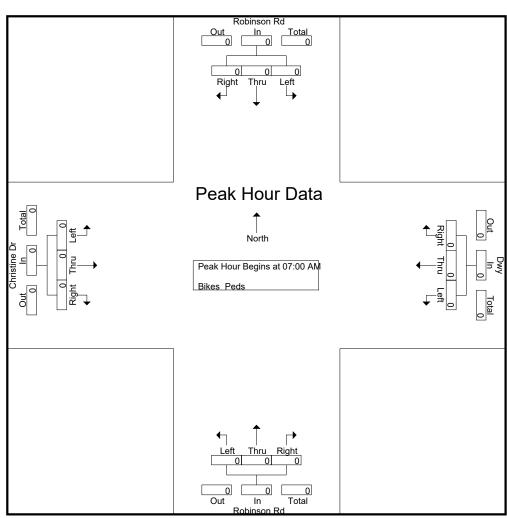
		04011 20	gine an													
	07:15 AM	I			08:00 AN	1			07:15 AN	1			08:00 AN	1		
+0 mins.	0	21	3	24	0	0	0	0	0	43	0	43	0	0	0	0
+15 mins.	0	28	4	32	0	0	0	0	0	43	0	43	1	0	0	1
+30 mins.	0	31	5	36	0	0	0	0	2	47	0	49	3	0	1	4
+45 mins.	0	29	6	35	1	0	0	1	1	35	0	36	3	0	0	3
Total Volume	0	109	18	127	1	0	0	1	3	168	0	171	7	0	1	8
% App. Total	0	85.8	14.2		100	0	0		1.8	98.2	0		87.5	0	12.5	
PHF	.000	.879	.750	.882	.250	.000	.000	.250	.375	.894	.000	.872	.583	.000	.250	.500
Cars	0	105	17	122	1	0	0	1	3	164	0	167	6	0	1	7
% Cars	0	96.3	94.4	96.1	100	0	0	100	100	97.6	0	97.7	85.7	0	100	87.5
Trucks	0	4	1	5	0	0	0	0	0	4	0	4	1	0	0	1
% Trucks	0	3.7	5.6	3.9	0	0	0	0	0	2.4	0	2.3	14.3	0	0	12.5

Accurate Counts 978-664-2565

								Groups	s Printec	I- Bikes	Peds								
		Robins	son Rd			Dv	vy			Robins	on Rd			Christi	ne Dr				
		From I	North			From	East			From	South			From	West				
Start Time	Left	Thru	Right	Peds	Left	Thru	Right	Peds	Left	Thru	Right	Peds	Left	Thru	Right	Peds	Exclu. Total	Inclu. Total	Int. Total
07:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
07:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
07:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
07:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
08:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
08:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
08:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
08:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Grand Total	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Apprch %	0	0	0		0	0	0		0	0	0		0	0	0				
Total %					i												0	0	

		Robin	son Rd			D	wy			Robin	son Rd						
		From	North			From	n East			From	South						
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analy	sis From	07:00	AM to 08	3:45 AM -	Peak 1 o	f 1											
Peak Hour for Er	ntire Inter	section	Begins	at 07:00 A	M												
07:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
07:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
07:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
07:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total Volume	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
% App. Total	0	0	0		0	0	0		0	0	0		0	0	0		
PHF	.000	.000	.000	.000	.000	.000	.000	.000	.000	.000	.000	.000	.000	.000	.000	.000	.000

Accurate Counts 978-664-2565



Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1 Peak Hour for Each Approach Begins at:

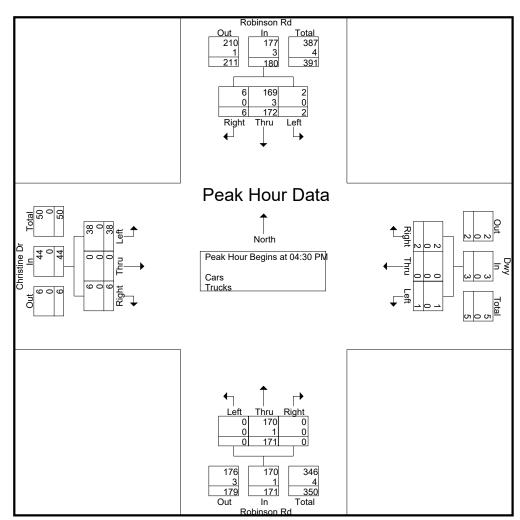
	07:00 AN	1	•		07:00 AN				07:00 AN	1			07:00 AN	1		
+0 mins.	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
+15 mins.	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
+30 mins.	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
+45 mins.	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total Volume	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
% App. Total	0	0	0		0	0	0		0	0	0		0	0	0	
PHF	.000	.000	.000	.000	.000	.000	.000	.000	.000	.000	.000	.000	.000	.000	.000	.000

Groups Printed- Cars - Trucks													
		binson Rd		-	Dwy			binson Rd		Christine Dr			
	From North			<u>F</u> r	rom East		<u> </u>	rom South		From West			
Start Time	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Int. Total
04:00 PM	1	41	2	0	0	0	1	38	0	5	0	1	89
04:15 PM	0	38	5	0	0	0	1	25	0	5	0	0	74
04:30 PM	0	31	1	0	0	0	0	49	0	16	0	2	99
04:45 PM	1	39	1	0	0	1	0	41	0	9	0	2	94
Total	2	149	9	0	0	1	2	153	0	35	0	5	356
05:00 PM	0	55	3	0	0	0	0	34	0	4	0	1	97
05:15 PM	1	47	1	1	0	1	0	47	0	9	0	1	108
05:30 PM	0	47	0	0	0	0	0	39	0	0	0	1	87
05:45 PM	0	32	0	0	0	0	0	30	0	3	0	1	66
Total	1	181	4	1	0	1	0	150	0	16	0	4	358
1						1							
Grand Total	3	330	13	1	0	2	2	303	0	51	0	9	714
Apprch %	0.9	95.4	3.8	33.3	0	66.7	0.7	99.3	0	85	0	15	
Total %	0.4	46.2	1.8	0.1	0	0.3	0.3	42.4	0	7.1	0	1.3	
Cars	3	326	13	1	0	2	2	300	0	51	0	9	707
% Cars	100	98.8	100	100	0	100	100	99	0	100	0	100	99
Trucks	0	4	0	0	0	0	0	3	0	0	0	0	7
% Trucks	0	1.2	0	0	0	0	0	1	0	0	0	0	1

		Robin	son Rd			D	wy			Robin	ison Rd						
		From	North			From	n East			From	South						
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 04:30 F					PM												
04:30 PM	0	31	1	32	0	0	0	0	0	49	0	49	16	0	2	18	99
04:45 PM	1	39	1	41	0	0	1	1	0	41	0	41	9	0	2	11	94
05:00 PM	0	55	3	58	0	0	0	0	0	34	0	34	4	0	1	5	97
05:15 PM	1	47	1	49	1	0	1	2	0	47	0	47	9	0	1	10	108
Total Volume	2	172	6	180	1	0	2	3	0	171	0	171	38	0	6	44	398
% App. Total	1.1	95.6	3.3		33.3	0	66.7		0	100	0		86.4	0	13.6		
PHF	.500	.782	.500	.776	.250	.000	.500	.375	.000	.872	.000	.872	.594	.000	.750	.611	.921
Cars	2	169	6	177	1	0	2	3	0	170	0	170	38	0	6	44	394
% Cars	100	98.3	100	98.3	100	0	100	100	0	99.4	0	99.4	100	0	100	100	99.0
Trucks	0	3	0	3	0	0	0	0	0	1	0	1	0	0	0	0	4
% Trucks	0	1.7	0	1.7	0	0	0	0	0	0.6	0	0.6	0	0	0	0	1.0

Accurate Counts 978-664-2565

N/S Street : Robinson Road E/W Street : Driveway / Christine Drive City/State : Hudson, NH Weather : Clear



Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1 Peak Hour for Each Approach Begins at:

			0		0 4 0 0 DL				0 4 00 DI				0 4 0 0 DI			
	04:45 PN				04:30 PN	1			04:30 PN	1			04:30 PN	1		ļ
+0 mins.	1	39	1	41	0	0	0	0	0	49	0	49	16	0	2	18
+15 mins.	0	55	3	58	0	0	1	1	0	41	0	41	9	0	2	11
+30 mins.	1	47	1	49	0	0	0	0	0	34	0	34	4	0	1	5
+45 mins.	0	47	0	47	1	0	1	2	0	47	0	47	9	0	1	10
Total Volume	2	188	5	195	1	0	2	3	0	171	0	171	38	0	6	44
% App. Total	1	96.4	2.6		33.3	0	66.7		0	100	0		86.4	0	13.6	
PHF	.500	.855	.417	.841	.250	.000	.500	.375	.000	.872	.000	.872	.594	.000	.750	.611
Cars	2	186	5	193	1	0	2	3	0	170	0	170	38	0	6	44
% Cars	100	98.9	100	99	100	0	100	100	0	99.4	0	99.4	100	0	100	100
Trucks	0	2	0	2	0	0	0	0	0	1	0	1	0	0	0	0
% Trucks	0	1.1	0	1	0	0	0	0	0	0.6	0	0.6	0	0	0	0

Accurate Counts 978-664-2565

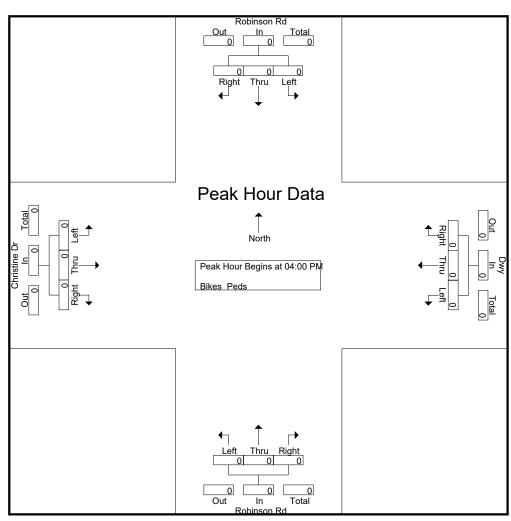
N/S Street : Robinson Road E/W Street : Driveway / Christine Drive City/State : Hudson, NH Weather : Clear

								Groups	s Printec	J- Bikes	Peds								
		Robins	son Rd			Dv	vy			Robins	on Rd			Christi	ne Dr				
		From I	North			From	East			From	South			From	West		L		
Start Time	Left	Thru	Right	Peds	Left	Thru	Right	Peds	Left	Thru	Right	Peds	Left	Thru	Right	Peds	Exclu. Total	Inclu. Total	Int. Total
04:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
04:15 PM	0	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0	1	0	1
04:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	1	0	1
04:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total	0	0	0	0	0	0	0	1	0	0	0	0	0	0	0	1	2	0	2
05:00 PM	0	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0	1	0	1
05:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
05:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
05:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total	0	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0	1	0	1
Grand Total	0	0	0	0	0	0	0	2	0	0	0	0	0	0	0	1	3	0	3
Apprch %	0	0	0		0	0	0		0	0	0		0	0	0				
Total %					l												100	0	

		Robins	son Rd			D	wy			Robin	son Rd			Chris	tine Dr		
		From	North			From	n East			From	South			From	n West		
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analy	/sis From	n 04:00 F	PM to 0	5:45 PM -	Peak 1 o	of 1											
Peak Hour for E	ntire Inte	rsection	Begins	at 04:00 F	PM												
04:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
04:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
04:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
04:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total Volume	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
% App. Total	0	0	0		0	0	0		0	0	0		0	0	0		
PHF	.000	.000	.000	.000	.000	.000	.000	.000	.000	.000	.000	.000	.000	.000	.000	.000	.000

Accurate Counts 978-664-2565

N/S Street : Robinson Road E/W Street : Driveway / Christine Drive City/State : Hudson, NH Weather : Clear



Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1 Peak Hour for Each Approach Begins at:

	04:00 PN	I			04:00 PN	1			04:00 PN	1			04:00 PN	1		
+0 mins.	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
+15 mins.	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
+30 mins.	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
+45 mins.	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total Volume	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
% App. Total	0	0	0		0	0	0		0	0	0		0	0	0	
PHF	.000	.000	.000	.000	.000	.000	.000	.000	.000	.000	.000	.000	.000	.000	.000	.000



Appendix C: Monthly Traffic Volumes

Year 2019 Monthly Data

Group 4 Averages: Urban Highways

		Adjustment	Adjustment
<u>Month</u>	<u>ADT</u>	to Average	<u>to Peak</u>
January	11,431	1.12	1.23
February	11,848	1.08	1.18
March	12,141	1.06	1.15
April	12,860	1.00	1.09
May	13,551	0.95	1.03
June	13,785	0.93	1.02
July	13,942	0.92	1.01
August	14,016	0.92	1.00
September	13,379	0.96	1.05
October	13,339	0.96	1.05
November	12,265	1.05	1.14
December	11,496	1.12	1.22
Average ADT:	12,838		
Peak ADT:	14,016		



Appendix D: Vehicle Speeds

Location : Robinson Road

Location : Robinson Road Location : South of Christine Drive City/State: Hudson, NH Direction: SB,

Miccuon. OD,														
6/29/2021	0 - 15	> 15 -	> 20 -	> 25 -	> 30 -	> 35 -	> 40 -	> 45 -	> 50 -	> 55 -	> 60 -	> 65 -	> 70	
Time	MPH	20 MPH	25 MPH	30 MPH	35 MPH	40 MPH	45 MPH	50 MPH	55 MPH	60 MPH	65 MPH	70 MPH	MPH	Total
12:00 AM	0	0	0	0	1	2	0	0	0	0	0	0	0	3
1:00	0	0	0	0	0	4	2	0	1	0	0	0	0	7
2:00	0	0	0	0	1	0	2	0	0	0	0	0	0	3
3:00	0	0	0	1	0	1	0	0	0	0	0	0	0	2
4:00	0	0	0	0	3	4	2	1	0	0	0	0	0	10
5:00	0	0	0	0	12	22	11	3	0	0	0	0	0	48
6:00	0	0	0	1	15	34	30	3	0	0	0	0	0	83
7:00	0	0	0	1	8	42	33	5	2	0	0	0	0	91
8:00	0	0	0	2	18	52	28	1	0	0	0	0	0	101
9:00	0	1	0	2	21	47	26	5	0	0	0	0	0	102
10:00	0	0	0	2	15	36	27	7	0	0	0	0	0	87
11:00	0	0	0	2	26	59	34	12	1	0	0	0	0	134
12:00 PM	0	0	0	2	29	58	34	7	0	0	0	0	0	130
1:00	0	0	1	3	14	47	26	6	2	0	0	0	0	99
2:00	1	0	1	3	18	56	38	9	1	0	0	0	1	128
3:00	0	0	0	0	25	65	46	4	4	1	0	0	0	145
4:00	0	0	3	1	17	64	55	12	0	0	0	0	0	152
5:00	0	0	0	0	18	87	58	13	1	1	0	0	0	178
6:00	0	0	0	2	15	49	33	4	2	0	0	0	0	105
7:00	0	0	1	1	18	42	27	4	1	0	0	0	0	94
8:00	0	0	0	0	28	34	12	2	0	0	0	0	0	76
9:00	0	0	0	1	15	23	10	3	3	1	0	0	0	56
10:00	0	0	0	1	7	9	6	3	0	0	0	0	0	26
11:00	0	0	0	2	2	5	2	1	0	0	1	0	0	13
Total	1	1	6	27	326	842	542	105	18	3	1	0	1	1873
			Percentile	15th	50th	85th	95th							

38.4

42.8 45.9

Speed 34.1 Mean Speed (Average) 10 MPH Pace Speed 38.7 35-44 Number in Pace 1380 Percent in Pace 73.7% Number > 40 MPH 670

Percent > 40 MPH 35.8% 15570001

Location : Robinson Road Location : South of Christine Drive City/State: Hudson, NH Direction: SB,

<u>Biroononi 05</u> ,														
6/30/2021	0 - 15	> 15 -	> 20 -	> 25 -	> 30 -	> 35 -	> 40 -	> 45 -	> 50 -	> 55 -	> 60 -	> 65 -	> 70	
Time	MPH	20 MPH	25 MPH	30 MPH	35 MPH	40 MPH	45 MPH	50 MPH	55 MPH	60 MPH	65 MPH	70 MPH	MPH	Total
12:00 AM	0	0	0	0	2	3	2	0	0	0	0	0	0	7
1:00	0	0	0	0	0	1	0	1	0	0	0	0	0	2
2:00	0	0	0	2	0	0	1	0	0	0	0	0	0	3
3:00	0	0	0	0	1	1	0	0	0	0	0	0	0	2
4:00	0	0	0	0	1	6	0	1	1	0	0	0	0	9
5:00	0	0	0	2	4	15	13	3	2	0	0	0	0	39
6:00	0	0	0	4	9	36	24	6	1	0	0	0	0	80
7:00	0	0	0	0	12	36	25	4	0	0	0	0	0	77
8:00	0	2	1	3	10	48	44	4	2	0	0	0	0	114
9:00	0	0	0	0	26	46	28	4	0	0	0	0	0	104
10:00	0			0	17	48	28	3	1	1	0	0	0	98
11:00	0		0	3	23	54	41	11	1	0	0	0	0	134
12:00 PM	0		0	3	38	52	32	5	0	0	0	0	0	130
1:00	0			1	17	50	39	5	0	0	0	0	0	112
2:00	0		0	3	15	52	46	8	1	0	1	0	0	126
3:00	0		0	3	25	48	48	8	1	0	0	0	0	134
4:00	0			1	30	90	50	7	0	0	0	0	0	178
5:00	0		0	2	26	89	46	9	1	0	0	0	0	173
6:00	0		Õ	2	15	45	25	4	1	0	0	ů 0	0	92
7:00	0			1	16	25	11	3	0	0	0	ů 0	0	56
8:00	0			1	14	43	8	3	0	0	0	ů 0	0	69
9:00	0			0	12	21	4	4	0	0	0	0	0	41
10:00	0			0	4	11	6	1	0	0	0	0	0	22
11:00	0			0	2	8	2	1	0	0	0	0	0	13
Total	0		-	31	319	828	523	95	12	1	1	0	0	1815
10101		-	Percentile	15th	50th	85th	95th		14		· ·			1010
			Speed	34.1	38.4	42.8	45.3							
	Me	an Speed		38.5	00.4	42.0	40.0							
) MPH Pa		35-44										
			er in Pace	1347										
			nt in Pace	74.2%										
			> 40 MPH	632										
			> 40 MPH	34.8%										
Grand Total	1	5		58	645	1670	1065	200	30	4	2	0	1	3688
Stats			Percentile	15th	50th	85th	95th	200			2	0		0000
Oldio			Speed	34.1	38.4	42.8	45.9							
	Me	an Speed		38.6	00.4	42.0	40.0							
) MPH Pa		35-44										
			er in Pace	2727										
			nt in Pace	73.9%										
			> 40 MPH	1302										
			> 40 MPH	35.3%										
				55.5%										

Location : Robinson Road

Location : Robinson Road Location : South of Christine Drive City/State: Hudson, NH Direction: NB,

COUCH. ND,														
6/29/2021	0 - 15	> 15 -	> 20 -	> 25 -	> 30 -	> 35 -	> 40 -	> 45 -	> 50 -	> 55 -	> 60 -	> 65 -	> 70	
Time	MPH	20 MPH	25 MPH	30 MPH	35 MPH	40 MPH	45 MPH	50 MPH	55 MPH	60 MPH	65 MPH	70 MPH	MPH	Total
12:00 AM	0	0	0	0	0	2	2	0	0	0	0	0	0	4
1:00	0	0	0	0	2	4	1	0	0	0	0	0	0	7
2:00	0	0	0	0	2	3	0	1	0	0	0	0	0	6
3:00	0	0	0	1	1	1	1	1	0	0	0	0	0	5
4:00	0	0	0	0	5	11	7	2	1	0	0	0	0	26
5:00	0	0	0	3	5	32	25	12	2	0	0	0	0	79
6:00	0	0	1	1	14	42	35	7	3	0	0	0	0	103
7:00	0	0	3	2	23	73	65	12	1	0	0	0	0	179
8:00	0	0	0	1	17	64	48	8	0	0	0	0	0	138
9:00	0	0	0	4	23	50	47	6	1	0	1	0	1	133
10:00	0	0	0	0	19	35	45	11	3	0	0	0	0	113
11:00	0	1	1	2	8	48	46	8	1	0	0	0	0	115
12:00 PM	0	0	0	3	20	71	35	5	0	0	0	0	0	134
1:00	0	0	1	1	14	33	38	15	2	1	0	0	0	105
2:00	0	1	0	2	20	48	37	12	4	0	0	0	2	126
3:00	0	0	0	6	17	57	58	14	2	2	0	0	0	156
4:00	0	1	1	2	12	57	60	27	1	0	0	0	0	161
5:00	0	0	0	2	14	44	67	12	6	0	0	0	0	145
6:00	0	0	0	4	4	40	46	11	1	0	0	0	0	106
7:00	0	1	0	1	13	24	26	10	2	0	0	0	0	77
8:00	0	0	0	2	10	28	13	6	1	1	0	0	0	61
9:00	0	0	0	1	7	15	2	1	2	0	0	0	0	28
10:00	0	0	0	0	1	8	5	0	0	2	0	0	0	16
11:00	0	0	0	0	3	4	1	0	1	0	0	0	0	9
Total	0	4	7	38	254	794	710	181	34	6	1	0	3	2032
		l	Percentile	15th		85th								
			Speed	35.3	39.7	44	47.1							

35.3 Speed Mean Speed (Average) 10 MPH Pace Speed 39.8 35-44 Number in Pace 1495 73.6% Percent in Pace Number > 40 MPH 935

Percent > 40 MPH 46.0% 15570001

Location : Robinson Road Location : South of Christine Drive City/State: Hudson, NH Direction: NB,

6/30/2021	0 - 15	> 15 -	> 20 -	> 25 -	> 30 -	> 35 -	> 40 -	> 45 -	> 50 -	> 55 -	> 60 -	> 65 -	> 70	
Time	MPH	20 MPH	25 MPH	30 MPH	35 MPH	40 MPH	45 MPH	50 MPH	55 MPH	60 MPH	65 MPH	70 MPH	MPH	Total
12:00 AM	0	0	0	0	1	0	1	0	0	0	0	0	0	2
1:00	0	0	0	0	0	1	1	0	0	0	0	0	0	2
2:00	0	0	0	0	0	3	1	0	0	0	0	0	0	4
3:00	0	0	0	1	0	3	2	0	0	0	0	0	0	6
4:00	0	0	1	1	7	7	10	1	0	0	0	1	0	28
5:00	0	0	0	0	15	27	20	5	6	2	0	0	0	75
6:00	0	1	0	3	14	52	34	9	2	0	0	0	2	117
7:00	0	0	0	9	20	64	68	10	2	0	0	0	0	173
8:00	0	0	0	4	22	82	49	9	0	0	1	0	0	167
9:00	0	0	0	3	22	56	34	9	2	0	0	0	1	127
10:00	0	0	0		22	38	39	14		0	0	0	0	122
11:00	0	0	0	3	15	51	30	10	3	0	0	0	1	113
12:00 PM	0	0	0	2	15	45	46	11	0	0	0	0	1	120
1:00	0	0	0	2	21	43	42	9	3	1	0	0	1	122
2:00	1	0	0	3	15	35	40	14	1	0	0	0	0	109
3:00	0	0	0	5	27	63	46	15	0	0	0	0	1	157
4:00	0	0	0	3	26	52	57	17	0	0	0	0	0	155
5:00	0	1	0	0	18	46	27	14	2	0	0	0	0	108
6:00	0	0	0	0	11	24	40	12	4	1	0	0	0	92
7:00	0	0	0	2	7	23	15	6	1	0	0	0	0	54
8:00	0	0	0	2	14	17	14	1	0	0	0	0	0	48
9:00	0	0	0	1	8	14	3	1	0	0	0	0	0	27
10:00	0	0	0	1	3	10	3	1	0	0	0	0	0	18
11:00	1	0	0	0	1	3	2			0	0	0	0	7
Total	2	2	1	50		759	624		30	4	1	1	7	1953
		l	Percentile	15th	50th	85th	95th							
			Speed	34.1	39	44	47.1							
		an Speed		39.7										
	10	0 MPH Pa	ice Speed	35-44										
		Numbe	er in Pace	1377										
		Percer	nt in Pace	70.5%										
		Number >	> 40 MPH	835										
			> 40 MPH	42.8%										
Grand Total	2	6	8	88			1334		64	10	2	1	10	3985
Stats		l	Percentile	15th		85th	95th							
			Speed	34.7		44	47.1							
		an Speed		39.7										
	10	0 MPH Pa	ice Speed	35-44										
			er in Pace	2871										
			nt in Pace	72.0%										
		Number >		1770										
		Percent >	> 40 MPH	44.4%										



Appendix E: Capacity-Analysis Worksheets

Intersection

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		\$			\$			\$			\$		
Traffic Vol, veh/h	2	0	0	0	0	0	0	186	0	0	117	19	
Future Vol, veh/h	2	0	0	0	0	0	0	186	0	0	117	19	
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0	
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free	
RT Channelized	-	-	None										
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-	
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-	
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-	
Peak Hour Factor	50	50	50	90	90	90	87	87	87	88	88	88	
Heavy Vehicles, %	100	0	0	2	2	2	0	2	0	2	4	6	
Mvmt Flow	4	0	0	0	0	0	0	214	0	0	133	22	

Major/Minor	Minor2		ſ	Minor1		1	Major1		Ν	/lajor2			
Conflicting Flow All	358	358	144	358	369	214	155	0	0	214	0	0	
Stage 1	144	144	-	214	214	-	-	-	-	-	-	-	
Stage 2	214	214	-	144	155	-	-	-	-	-	-	-	
Critical Hdwy	8.1	6.5	6.2	7.12	6.52	6.22	4.1	-	-	4.12	-	-	
Critical Hdwy Stg 1	7.1	5.5	-	6.12	5.52	-	-	-	-	-	-	-	
Critical Hdwy Stg 2	7.1	5.5	-	6.12	5.52	-	-	-	-	-	-	-	
Follow-up Hdwy	4.4	4	3.3	3.518	4.018	3.318	2.2	-	-	2.218	-	-	
Pot Cap-1 Maneuver	451	572	909	597	560	826	1438	-	-	1356	-	-	
Stage 1	672	782	-	788	725	-	-	-	-	-	-	-	
Stage 2	610	729	-	859	769	-	-	-	-	-	-	-	
Platoon blocked, %								-	-		-	-	
Mov Cap-1 Maneuver	451	572	909	597	560	826	1438	-	-	1356	-	-	
Mov Cap-2 Maneuver	451	572	-	597	560	-	-	-	-	-	-	-	
Stage 1	672	782	-	788	725	-	-	-	-	-	-	-	
Stage 2	610	729	-	859	769	-	-	-	-	-	-	-	
A													

Approach	EB	WB	NB	SB	
HCM Control Delay, s	13.1	0	0	0	
HCM LOS	В	А			

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1W	/BLn1	SBL	SBT	SBR
Capacity (veh/h)	1438	-	-	451	-	1356	-	-
HCM Lane V/C Ratio	-	-	-	0.009	-	-	-	-
HCM Control Delay (s)	0	-	-	13.1	0	0	-	-
HCM Lane LOS	А	-	-	В	А	А	-	-
HCM 95th %tile Q(veh)	0	-	-	0	-	0	-	-

2

Intersection

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		¢			\$			\$			\$		
Traffic Vol, veh/h	41	0	6	1	0	2	0	183	0	2	185	6	
Future Vol, veh/h	41	0	6	1	0	2	0	183	0	2	185	6	
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0	
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free	
RT Channelized	-	-	None										
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-	
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-	
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-	
Peak Hour Factor	61	61	61	38	38	38	87	87	87	78	78	78	
Heavy Vehicles, %	0	2	0	0	2	0	2	1	2	0	2	0	
Mvmt Flow	67	0	10	3	0	5	0	210	0	3	237	8	

Major/Minor	Minor2		Ν	/linor1			Major1		Ν	/lajor2				
Conflicting Flow All	460	457	241	462	461	210	245	0	0	210	0	0		
Stage 1	247	247	-	210	210	-	-	-	-	-	-	-		
Stage 2	213	210	-	252	251	-	-	-	-	-	-	-		
Critical Hdwy	7.1	6.52	6.2	7.1	6.52	6.2	4.12	-	-	4.1	-	-		
Critical Hdwy Stg 1	6.1	5.52	-	6.1	5.52	-	-	-	-	-	-	-		
Critical Hdwy Stg 2	6.1	5.52	-	6.1	5.52	-	-	-	-	-	-	-		
Follow-up Hdwy	3.5	4.018	3.3	3.5	4.018	3.3	2.218	-	-	2.2	-	-		
Pot Cap-1 Maneuver	515	500	803	513	497	835	1321	-	-	1373	-	-		
Stage 1	761	702	-	797	728	-	-	-	-	-	-	-		
Stage 2	794	728	-	757	699	-	-	-	-	-	-	-		
Platoon blocked, %								-	-		-	-		
Mov Cap-1 Maneuver	510	499	803	505	496	835	1321	-	-	1373	-	-		
Mov Cap-2 Maneuver	510	499	-	505	496	-	-	-	-	-	-	-		
Stage 1	761	700	-	797	728	-	-	-	-	-	-	-		
Stage 2	789	728	-	745	697	-	-	-	-	-	-	-		
Awwasseh	FD									CD				

Approach	EB	WB	NB	SB	
HCM Control Delay, s	12.9	10.3	0	0.1	
HCM LOS	В	В			

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1\	WBLn1	SBL	SBT	SBR
Capacity (veh/h)	1321	-	-	535	686	1373	-	-
HCM Lane V/C Ratio	-	-	-	0.144	0.012	0.002	-	-
HCM Control Delay (s)	0	-	-	12.9	10.3	7.6	0	-
HCM Lane LOS	А	-	-	В	В	А	А	-
HCM 95th %tile Q(veh)	0	-	-	0.5	0	0	-	-

Intersection

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		\$			\$			\$			\$		
Traffic Vol, veh/h	2	0	0	0	0	0	0	188	0	0	118	19	
Future Vol, veh/h	2	0	0	0	0	0	0	188	0	0	118	19	
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0	
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free	
RT Channelized	-	-	None										
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-	
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-	
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-	
Peak Hour Factor	50	50	50	90	90	90	87	87	87	88	88	88	
Heavy Vehicles, %	100	0	0	2	2	2	0	2	0	2	4	6	
Mvmt Flow	4	0	0	0	0	0	0	216	0	0	134	22	

Major/Minor	Minor2		ſ	Minor1		1	Major1		Ν	/lajor2				
Conflicting Flow All	361	361	145	361	372	216	156	0	0	216	0	0		
Stage 1	145	145	-	216	216	-	-	-	-	-	-	-		
Stage 2	216	216	-	145	156	-	-	-	-	-	-	-		
Critical Hdwy	8.1	6.5	6.2	7.12	6.52	6.22	4.1	-	-	4.12	-	-		
Critical Hdwy Stg 1	7.1	5.5	-	6.12	5.52	-	-	-	-	-	-	-		
Critical Hdwy Stg 2	7.1	5.5	-	6.12	5.52	-	-	-	-	-	-	-		
Follow-up Hdwy	4.4	4	3.3	3.518	4.018	3.318	2.2	-	-	2.218	-	-		
Pot Cap-1 Maneuver	449	569	908	595	558	824	1436	-	-	1354	-	-		
Stage 1	671	781	-	786	724	-	-	-	-	-	-	-		
Stage 2	608	728	-	858	769	-	-	-	-	-	-	-		
Platoon blocked, %								-	-		-	-		
Mov Cap-1 Maneuver	449	569	908	595	558	824	1436	-	-	1354	-	-		
Mov Cap-2 Maneuver	449	569	-	595	558	-	-	-	-	-	-	-		
Stage 1	671	781	-	786	724	-	-	-	-	-	-	-		
Stage 2	608	728	-	858	769	-	-	-	-	-	-	-		

Approach	EB	WB	NB	SB	
HCM Control Delay, s	13.1	0	0	0	
HCM LOS	В	А			

Minor Lane/Major Mvmt	NBL	NBT	NBRI	EBLn1W	/BLn1	SBL	SBT	SBR
Capacity (veh/h)	1436	-	-	449	-	1354	-	-
HCM Lane V/C Ratio	-	-	-	0.009	-	-	-	-
HCM Control Delay (s)	0	-	-	13.1	0	0	-	-
HCM Lane LOS	А	-	-	В	Α	А	-	-
HCM 95th %tile Q(veh)	0	-	-	0	-	0	-	-

2

Intersection

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		4			4			4			4		
Traffic Vol, veh/h	41	0	6	1	0	2	0	185	0	2	187	6	
Future Vol, veh/h	41	0	6	1	0	2	0	185	0	2	187	6	
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0	
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free	
RT Channelized	-	-	None										
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-	
Veh in Median Storage	,# -	0	-	-	0	-	-	0	-	-	0	-	
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-	
Peak Hour Factor	61	61	61	38	38	38	87	87	87	78	78	78	
Heavy Vehicles, %	0	2	0	0	2	0	2	1	2	0	2	0	
Mvmt Flow	67	0	10	3	0	5	0	213	0	3	240	8	

Major/Minor	Minor2		Ν	1inor1			Major1		Ν	/lajor2				
Conflicting Flow All	466	463	244	468	467	213	248	0	0	213	0	0		
Stage 1	250	250	-	213	213	-	-	-	-	-	-	-		
Stage 2	216	213	-	255	254	-	-	-	-	-	-	-		
Critical Hdwy	7.1	6.52	6.2	7.1	6.52	6.2	4.12	-	-	4.1	-	-		
Critical Hdwy Stg 1	6.1	5.52	-	6.1	5.52	-	-	-	-	-	-	-		
Critical Hdwy Stg 2	6.1	5.52	-	6.1	5.52	-	-	-	-	-	-	-		
Follow-up Hdwy	3.5	4.018	3.3	3.5	4.018	3.3	2.218	-	-	2.2	-	-		
Pot Cap-1 Maneuver	510	496	800	509	493	832	1318	-	-	1369	-	-		
Stage 1	759	700	-	794	726	-	-	-	-	-	-	-		
Stage 2	791	726	-	754	697	-	-	-	-	-	-	-		
Platoon blocked, %								-	-		-	-		
Mov Cap-1 Maneuver	505	495	800	501	492	832	1318	-	-	1369	-	-		
Mov Cap-2 Maneuver	- 505	495	-	501	492	-	-	-	-	-	-	-		
Stage 1	759	698	-	794	726	-	-	-	-	-	-	-		
Stage 2	786	726	-	742	695	-	-	-	-	-	-	-		

Approach	EB	WB	NB	SB	
HCM Control Delay, s	12.9	10.3	0	0.1	
HCM LOS	В	В			

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1\	WBLn1	SBL	SBT	SBR
Capacity (veh/h)	1318	-	-	530	682	1369	-	-
HCM Lane V/C Ratio	-	-	-	0.145	0.012	0.002	-	-
HCM Control Delay (s)	0	-	-	12.9	10.3	7.6	0	-
HCM Lane LOS	А	-	-	В	В	А	А	-
HCM 95th %tile Q(veh)	0	-	-	0.5	0	0	-	-

Intersection

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		¢			\$			\$			\$		
Traffic Vol, veh/h	2	0	0	0	0	0	0	208	0	0	131	19	
Future Vol, veh/h	2	0	0	0	0	0	0	208	0	0	131	19	
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0	
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free	
RT Channelized	-	-	None										
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-	
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-	
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-	
Peak Hour Factor	50	50	50	90	90	90	87	87	87	88	88	88	
Heavy Vehicles, %	100	0	0	2	2	2	0	2	0	2	4	6	
Mvmt Flow	4	0	0	0	0	0	0	239	0	0	149	22	

Major/Minor	Minor2		ſ	Minor1		N	/lajor1		Ν	/lajor2			
Conflicting Flow All	399	399	160	399	410	239	171	0	0	239	0	0	
Stage 1	160	160	-	239	239	-	-	-	-	-	-	-	
Stage 2	239	239	-	160	171	-	-	-	-	-	-	-	
Critical Hdwy	8.1	6.5	6.2	7.12	6.52	6.22	4.1	-	-	4.12	-	-	
Critical Hdwy Stg 1	7.1	5.5	-	6.12	5.52	-	-	-	-	-	-	-	
Critical Hdwy Stg 2	7.1	5.5	-	6.12	5.52	-	-	-	-	-	-	-	
Follow-up Hdwy	4.4	4	3.3	3.518	4.018	3.318	2.2	-	-	2.218	-	-	
Pot Cap-1 Maneuver	421	542	890	561	531	800	1418	-	-	1328	-	-	
Stage 1	657	769	-	764	708	-	-	-	-	-	-	-	
Stage 2	589	711	-	842	757	-	-	-	-	-	-	-	
Platoon blocked, %								-	-		-	-	
Mov Cap-1 Maneuver	· 421	542	890	561	531	800	1418	-	-	1328	-	-	
Mov Cap-2 Maneuver	· 421	542	-	561	531	-	-	-	-	-	-	-	
Stage 1	657	769	-	764	708	-	-	-	-	-	-	-	
Stage 2	589	711	-	842	757	-	-	-	-	-	-	-	
Approach	EB			WB			NB			SB			

				05	
HCM Control Delay, s	13.6	0	0	0	
HCM LOS	В	А			

Minor Lane/Major Mvmt	NBL	NBT	NBR E	BLn1W	/BLn1	SBL	SBT	SBR
Capacity (veh/h)	1418	-	-	421	-	1328	-	-
HCM Lane V/C Ratio	-	-	-	0.01	-	-	-	-
HCM Control Delay (s)	0	-	-	13.6	0	0	-	-
HCM Lane LOS	А	-	-	В	А	А	-	-
HCM 95th %tile Q(veh)	0	-	-	0	-	0	-	-

2

Intersection

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		\$			\$			\$			\$		
Traffic Vol, veh/h	41	0	6	1	0	2	0	204	0	2	206	6	
Future Vol, veh/h	41	0	6	1	0	2	0	204	0	2	206	6	
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0	
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free	
RT Channelized	-	-	None										
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-	
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-	
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-	
Peak Hour Factor	61	61	61	38	38	38	87	87	87	78	78	78	
Heavy Vehicles, %	0	2	0	0	2	0	2	1	2	0	2	0	
Mvmt Flow	67	0	10	3	0	5	0	234	0	3	264	8	

Major/Minor	Minor2		Ν	1inor1			Major1			Major2				
Conflicting Flow All	511	508	268	513	512	234	272	0	0	234	0	0		
Stage 1	274	274	-	234	234	-	-	-	-	-	-	-		
Stage 2	237	234	-	279	278	-	-	-	-	-	-	-		
Critical Hdwy	7.1	6.52	6.2	7.1	6.52	6.2	4.12	-	-	4.1	-	-		
Critical Hdwy Stg 1	6.1	5.52	-	6.1	5.52	-	-	-	-	-	-	-		
Critical Hdwy Stg 2	6.1	5.52	-	6.1	5.52	-	-	-	-	-	-	-		
Follow-up Hdwy	3.5	4.018	3.3	3.5	4.018	3.3	2.218	-	-	2.2	-	-		
Pot Cap-1 Maneuver	476	468	776	475	465	810	1291	-	-	1345	-	-		
Stage 1	736	683	-	774	711	-	-	-	-	-	-	-		
Stage 2	771	711	-	732	680	-	-	-	-	-	-	-		
Platoon blocked, %								-	-		-	-		
Mov Cap-1 Maneuver	472	467	776	468	464	810	1291	-	-	1345	-	-		
Mov Cap-2 Maneuver	472	467	-	468	464	-	-	-	-	-	-	-		
Stage 1	736	681	-	774	711	-	-	-	-	-	-	-		
Stage 2	766	711	-	721	678	-	-	-	-	-	-	-		

Approach	EB	WB	NB	SB	
HCM Control Delay, s	13.6	10.6	0	0.1	
HCM LOS	В	В			

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1\	WBLn1	SBL	SBT	SBR
Capacity (veh/h)	1291	-	-	497	651	1345	-	-
HCM Lane V/C Ratio	-	-	-	0.155	0.012	0.002	-	-
HCM Control Delay (s)	0	-	-	13.6	10.6	7.7	0	-
HCM Lane LOS	А	-	-	В	В	А	А	-
HCM 95th %tile Q(veh)	0	-	-	0.5	0	0	-	-

Intersection

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR		
Lane Configurations		4			4			4			4			
Traffic Vol, veh/h	2	0	0	1	0	0	0	190	1	8	137	19		
Future Vol, veh/h	2	0	0	1	0	0	0	190	1	8	137	19		
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0		
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free		
RT Channelized	-	-	None											
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-		
Veh in Median Storage,	,# -	0	-	-	0	-	-	0	-	-	0	-		
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-		
Peak Hour Factor	50	50	50	90	90	90	87	87	87	88	88	88		
Heavy Vehicles, %	100	0	0	2	2	2	0	2	0	2	4	6		
Mvmt Flow	4	0	0	1	0	0	0	218	1	9	156	22		

Major/Minor	Minor2		ſ	Minor1		1	Major1		Ν	lajor2			
Conflicting Flow All	404	404	167	404	415	219	178	0	0	219	0	0	
Stage 1	185	185	-	219	219	-	-	-	-	-	-	-	
Stage 2	219	219	-	185	196	-	-	-	-	-	-	-	
Critical Hdwy	8.1	6.5	6.2	7.12	6.52	6.22	4.1	-	-	4.12	-	-	
Critical Hdwy Stg 1	7.1	5.5	-	6.12	5.52	-	-	-	-	-	-	-	
Critical Hdwy Stg 2	7.1	5.5	-	6.12	5.52	-	-	-	-	-	-	-	
Follow-up Hdwy	4.4	4	3.3	3.518	4.018	3.318	2.2	-	-	2.218	-	-	
Pot Cap-1 Maneuver	418	539	882	557	528	821	1410	-	-	1350	-	-	
Stage 1	635	751	-	783	722	-	-	-	-	-	-	-	
Stage 2	605	726	-	817	739	-	-	-	-	-	-	-	
Platoon blocked, %								-	-		-	-	
Mov Cap-1 Maneuver	· 416	535	882	554	524	821	1410	-	-	1350	-	-	
Mov Cap-2 Maneuver	· 416	535	-	554	524	-	-	-	-	-	-	-	
Stage 1	635	746	-	783	722	-	-	-	-	-	-	-	
Stage 2	605	726	-	811	734	-	-	-	-	-	-	-	

Approach	EB	WB	NB	SB	
HCM Control Delay, s	13.7	11.5	0	0.4	
HCM LOS	В	В			

Minor Lane/Major Mvmt	NBL	NBT	NBR E	BLn1	WBLn1	SBL	SBT	SBR
Capacity (veh/h)	1410	-	-	416	554	1350	-	-
HCM Lane V/C Ratio	-	-	-	0.01	0.002	0.007	-	-
HCM Control Delay (s)	0	-	-	13.7	11.5	7.7	0	-
HCM Lane LOS	А	-	-	В	В	А	А	-
HCM 95th %tile Q(veh)	0	-	-	0	0	0	-	-

Intersection

Int Delay, s/veh	0.5					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	۰¥		4			- 4
Traffic Vol, veh/h	1	2	189	3	19	118
Future Vol, veh/h	1	2	189	3	19	118
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage	e, # 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	3	3	3	3	3	3
Mvmt Flow	1	2	210	3	21	131

Major/Minor	Minor1	N	lajor1	Ν	/lajor2	
Conflicting Flow All	385	212	0	0	213	0
Stage 1	212	-	-	-	-	-
Stage 2	173	-	-	-	-	-
Critical Hdwy	6.43	6.23	-	-	4.13	-
Critical Hdwy Stg 1	5.43	-	-	-	-	-
Critical Hdwy Stg 2	5.43	-	-	-	-	-
Follow-up Hdwy	3.527	3.327	-		2.227	-
Pot Cap-1 Maneuver	616	826	-	-	1351	-
Stage 1	821	-	-	-	-	-
Stage 2	855	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver		826	-	-	1351	-
Mov Cap-2 Maneuver	606	-	-	-	-	-
Stage 1	821	-	-	-	-	-
Stage 2	840	-	-	-	-	-
Approach	\//R		NR		SB	

Approach	WB	NB	SB	
HCM Control Delay, s	9.9	0	1.1	
HCM LOS	А			

Minor Lane/Major Mvmt	NBT	NBRV	VBLn1	SBL	SBT
Capacity (veh/h)	-	-	737	1351	-
HCM Lane V/C Ratio	-	-	0.005	0.016	-
HCM Control Delay (s)	-	-	9.9	7.7	0
HCM Lane LOS	-	-	А	А	А
HCM 95th %tile Q(veh)	-	-	0	0	-

Intersection

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		4			4			4			4		_
Traffic Vol, veh/h	41	0	6	1	0	7	0	201	0	2	188	6	
Future Vol, veh/h	41	0	6	1	0	7	0	201	0	2	188	6	
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0	
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free	
RT Channelized	-	-	None										
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-	
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-	
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-	
Peak Hour Factor	61	61	61	38	38	38	87	87	87	78	78	78	
Heavy Vehicles, %	0	2	0	0	2	0	2	1	2	0	2	0	
Mvmt Flow	67	0	10	3	0	18	0	231	0	3	241	8	

Major/Minor	Minor2		Ν	/linor1			Major1			Majo	r2			
Conflicting Flow All	491	482	245	487	486	231	249	0	0	2	31	0	0	
Stage 1	251	251	-	231	231	-	-	-	-		-	-	-	
Stage 2	240	231	-	256	255	-	-	-	-		-	-	-	
Critical Hdwy	7.1	6.52	6.2	7.1	6.52	6.2	4.12	-	-	- 4	.1	-	-	
Critical Hdwy Stg 1	6.1	5.52	-	6.1	5.52	-	-	-	-		-	-	-	
Critical Hdwy Stg 2	6.1	5.52	-	6.1	5.52	-	-	-	-		-	-	-	
Follow-up Hdwy	3.5	4.018	3.3	3.5	4.018	3.3	2.218	-	-	- 2	.2	-	-	
Pot Cap-1 Maneuver	491	484	799	494	481	813	1317	-	-	134	19	-	-	
Stage 1	758	699	-	776	713	-	-	-	-		-	-	-	
Stage 2	768	713	-	753	696	-	-	-	-		-	-	-	
Platoon blocked, %								-	-			-	-	
Mov Cap-1 Maneuver	479	483	799	487	480	813	1317	-	-	134	19	-	-	
Mov Cap-2 Maneuver	479	483	-	487	480	-	-	-	-		-	-	-	
Stage 1	758	697	-	776	713	-	-	-	-		-	-	-	
Stage 2	751	713	-	741	694	-	-	-	-		-	-	-	

Approach	EB	WB	NB	SB	
HCM Control Delay, s	13.4	9.9	0	0.1	
HCM LOS	В	А			

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1\	VBLn1	SBL	SBT	SBR
Capacity (veh/h)	1317	-	-	505	750	1349	-	-
HCM Lane V/C Ratio	-	-	-	0.153	0.028	0.002	-	-
HCM Control Delay (s)	0	-	-	13.4	9.9	7.7	0	-
HCM Lane LOS	А	-	-	В	А	А	А	-
HCM 95th %tile Q(veh)	0	-	-	0.5	0.1	0	-	-

Intersection

Int Delay, s/veh	0.4						
Movement	WBL	WBR	NBT	NBR	SBL	SBT	
Lane Configurations	Y		4			<u>स</u> ्	
Traffic Vol, veh/h	2	16	185	1	1	194	
Future Vol, veh/h	2	16	185	1	1	194	
Conflicting Peds, #/hr	0	0	0	0	0	0	
Sign Control	Stop	Stop	Free	Free	Free	Free	
RT Channelized	-	None	-	None	-	None	
Storage Length	0	-	-	-	-	-	
Veh in Median Storage	e, # 0	-	0	-	-	0	
Grade, %	0	-	0	-	-	0	
Peak Hour Factor	90	90	90	90	90	90	
Heavy Vehicles, %	2	2	2	2	2	2	
Mvmt Flow	2	18	206	1	1	216	

Major/Minor	Minor1	Ν	lajor1	Ν	/lajor2	
Conflicting Flow All	425	207	0	0	207	0
Stage 1	207	-	-	-	-	-
Stage 2	218	-	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	4.12	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	-	-	2.218	-
Pot Cap-1 Maneuver	586	833	-	-	1364	-
Stage 1	828	-	-	-	-	-
Stage 2	818	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver		833	-	-	1364	-
Mov Cap-2 Maneuver	585	-	-	-	-	-
Stage 1	828	-	-	-	-	-
Stage 2	817	-	-	-	-	-
-						

Approach	WB	NB	SB
HCM Control Delay, s	9.6	0	0
HCM LOS	А		

Minor Lane/Major Mvmt	NBT	NBRV	/BLn1	SBL	SBT
Capacity (veh/h)	-	-	796	1364	-
HCM Lane V/C Ratio	-	-	0.025	0.001	-
HCM Control Delay (s)	-	-	9.6	7.6	0
HCM Lane LOS	-	-	А	А	А
HCM 95th %tile Q(veh)	-	-	0.1	0	-

Intersection

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		4			4			4			4		
Traffic Vol, veh/h	2	0	0	1	0	0	0	210	1	8	150	19	
Future Vol, veh/h	2	0	0	1	0	0	0	210	1	8	150	19	
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0	
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free	
RT Channelized	-	-	None										
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-	
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-	
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-	
Peak Hour Factor	50	50	50	90	90	90	87	87	87	88	88	88	
Heavy Vehicles, %	100	0	0	2	2	2	0	2	0	2	4	6	
Mvmt Flow	4	0	0	1	0	0	0	241	1	9	170	22	

Major/Minor	Minor2		ſ	Minor1		1	Major1			Major2			
Conflicting Flow All	441	441	181	441	452	242	192	0	0	242	0	0	
Stage 1	199	199	-	242	242	-	-	-	-	-	-	-	
Stage 2	242	242	-	199	210	-	-	-	-	-	-	-	
Critical Hdwy	8.1	6.5	6.2	7.12	6.52	6.22	4.1	-	-	4.12	-	-	
Critical Hdwy Stg 1	7.1	5.5	-	6.12	5.52	-	-	-	-	-	-	-	
Critical Hdwy Stg 2	7.1	5.5	-	6.12	5.52	-	-	-	-	-	-	-	
Follow-up Hdwy	4.4	4	3.3	3.518	4.018	3.318	2.2	-	-	2.218	-	-	
Pot Cap-1 Maneuver	392	513	867	527	503	797	1394	-	-	1324	-	-	
Stage 1	623	740	-	762	705	-	-	-	-	-	-	-	
Stage 2	586	709	-	803	728	-	-	-	-	-	-	-	
Platoon blocked, %								-	-		-	-	
Mov Cap-1 Maneuver	r 390	509	867	524	499	797	1394	-	-	1324	-	-	
Mov Cap-2 Maneuver	r 390	509	-	524	499	-	-	-	-	-	-	-	
Stage 1	623	734	-	762	705	-	-	-	-	-	-	-	
Stage 2	586	709	-	797	722	-	-	-	-	-	-	-	

Approach	EB	WB	NB	SB	
HCM Control Delay, s	14.3	11.9	0	0.3	
HCM LOS	В	В			

Minor Lane/Major Mvmt	NBL	NBT	NBR E	BLn1	WBLn1	SBL	SBT	SBR
Capacity (veh/h)	1394	-	-	390	524	1324	-	-
HCM Lane V/C Ratio	-	-	-	0.01	0.002	0.007	-	-
HCM Control Delay (s)	0	-	-	14.3	11.9	7.7	0	-
HCM Lane LOS	А	-	-	В	В	А	А	-
HCM 95th %tile Q(veh)	0	-	-	0	0	0	-	-

Intersection

Int Delay, s/veh	0.5						
Movement	WBL	WBR	NBT	NBR	SBL	SBT	
Lane Configurations	۰¥		4			- 4	
Traffic Vol, veh/h	1	2	209	3	19	131	
Future Vol, veh/h	1	2	209	3	19	131	
Conflicting Peds, #/hr	0	0	0	0	0	0	
Sign Control	Stop	Stop	Free	Free	Free	Free	
RT Channelized	-	None	-	None	-	None	
Storage Length	0	-	-	-	-	-	
Veh in Median Storage	,# 0	-	0	-	-	0	
Grade, %	0	-	0	-	-	0	
Peak Hour Factor	90	90	90	90	90	90	
Heavy Vehicles, %	3	3	3	3	3	3	
Mvmt Flow	1	2	232	3	21	146	

Major/Minor	Minor1	Ν	1ajor1	Ν	lajor2			
Conflicting Flow All	422	234	0	0	235	0		
Stage 1	234	-	-	-	-	-		
Stage 2	188	-	-	-	-	-		
Critical Hdwy	6.43	6.23	-	-	4.13	-		
Critical Hdwy Stg 1	5.43	-	-	-	-	-		
Critical Hdwy Stg 2	5.43	-	-	-	-	-		
Follow-up Hdwy	3.527	3.327	-	-	2.227	-		
Pot Cap-1 Maneuver	586	803	-	-	1326	-		
Stage 1	802	-	-	-	-	-		
Stage 2	842	-	-	-	-	-		
Platoon blocked, %			-	-		-		
Mov Cap-1 Maneuver		803	-	-	1326	-		
Mov Cap-2 Maneuver	576	-	-	-	-	-		
Stage 1	802	-	-	-	-	-		
Stage 2	828	-	-	-	-	-		

Approach	WB	NB	SB
HCM Control Delay, s	10.1	0	1
HCM LOS	В		

Minor Lane/Major Mvmt	NBT	NBRV	VBLn1	SBL	SBT
Capacity (veh/h)	-	-	710	1326	-
HCM Lane V/C Ratio	-	-	0.005	0.016	-
HCM Control Delay (s)	-	-	10.1	7.8	0
HCM Lane LOS	-	-	В	А	А
HCM 95th %tile Q(veh)	-	-	0	0	-

Intersection

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		4			4			4			4		
Traffic Vol, veh/h	41	0	6	1	0	7	0	220	0	2	207	6	
Future Vol, veh/h	41	0	6	1	0	7	0	220	0	2	207	6	
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0	
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free	
RT Channelized	-	-	None										
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-	
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-	
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-	
Peak Hour Factor	61	61	61	38	38	38	87	87	87	78	78	78	
Heavy Vehicles, %	0	2	0	0	2	0	2	1	2	0	2	0	
Mvmt Flow	67	0	10	3	0	18	0	253	0	3	265	8	

Major/Minor	Minor2		Ν	1inor1			Major1		Ν	/lajor2			
Conflicting Flow All	537	528	269	533	532	253	273	0	0	253	0	0	
Stage 1	275	275	-	253	253	-	-	-	-	-	-	-	
Stage 2	262	253	-	280	279	-	-	-	-	-	-	-	
Critical Hdwy	7.1	6.52	6.2	7.1	6.52	6.2	4.12	-	-	4.1	-	-	
Critical Hdwy Stg 1	6.1	5.52	-	6.1	5.52	-	-	-	-	-	-	-	
Critical Hdwy Stg 2	6.1	5.52	-	6.1	5.52	-	-	-	-	-	-	-	
Follow-up Hdwy	3.5	4.018	3.3	3.5	4.018	3.3	2.218	-	-	2.2	-	-	
Pot Cap-1 Maneuver	458	456	775	461	453	791	1290	-	-	1324	-	-	
Stage 1	736	683	-	756	698	-	-	-	-	-	-	-	
Stage 2	747	698	-	731	680	-	-	-	-	-	-	-	
Platoon blocked, %								-	-		-	-	
Mov Cap-1 Maneuver	r 446	455	775	454	452	791	1290	-	-	1324	-	-	
Mov Cap-2 Maneuver	r 446	455	-	454	452	-	-	-	-	-	-	-	
Stage 1	736	681	-	756	698	-	-	-	-	-	-	-	
Stage 2	730	698	-	720	678	-	-	-	-	-	-	-	

Approach	EB	WB	NB	SB	
HCM Control Delay, s	14.1	10.1	0	0.1	
HCM LOS	В	В			

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1\	WBLn1	SBL	SBT	SBR
Capacity (veh/h)	1290	-	-	472	724	1324	-	-
HCM Lane V/C Ratio	-	-	-	0.163	0.029	0.002	-	-
HCM Control Delay (s)	0	-	-	14.1	10.1	7.7	0	-
HCM Lane LOS	А	-	-	В	В	А	А	-
HCM 95th %tile Q(veh)	0	-	-	0.6	0.1	0	-	-

Intersection

Int Delay, s/veh	0.4						
Movement	WBL	WBR	NBT	NBR	SBL	SBT	
Lane Configurations	Y		4			<u>स</u> ्	•
Traffic Vol, veh/h	2	16	204	1	1	213	
Future Vol, veh/h	2	16	204	1	1	213	
Conflicting Peds, #/hr	0	0	0	0	0	0	
Sign Control	Stop	Stop	Free	Free	Free	Free	!
RT Channelized	-	None	-	None	-	None	
Storage Length	0	-	-	-	-	-	
Veh in Median Storage	, # 0	-	0	-	-	0	
Grade, %	0	-	0	-	-	0	
Peak Hour Factor	90	90	90	90	90	90	
Heavy Vehicles, %	2	2	2	2	2	2	
Mvmt Flow	2	18	227	1	1	237	

Major/Minor	Minor1	Ν	1ajor1	Ν	/lajor2		
Conflicting Flow All	467	228	0	0	228	0	
Stage 1	228	-	-	-	-	-	
Stage 2	239	-	-	-	-	-	
Critical Hdwy	6.42	6.22	-	-	4.12	-	
Critical Hdwy Stg 1	5.42	-	-	-	-	-	
Critical Hdwy Stg 2	5.42	-	-	-	-	-	
Follow-up Hdwy	3.518	3.318	-	-	2.218	-	
Pot Cap-1 Maneuver	554	811	-	-	1340	-	
Stage 1	810	-	-	-	-	-	
Stage 2	801	-	-	-	-	-	
Platoon blocked, %			-	-		-	
Mov Cap-1 Maneuver	553	811	-	-	1340	-	
Mov Cap-2 Maneuver	- 553	-	-	-	-	-	
Stage 1	810	-	-	-	-	-	
Stage 2	800	-	-	-	-	-	
Approach	WB		NB		SB		

Minor Lane/Major Mvmt	NBT	NBRW	BLn1	SBL	SBT	
Capacity (veh/h)	-	-	771	1340	-	
HCM Lane V/C Ratio	-	- (0.026	0.001	-	
HCM Control Delay (s)	-	-	9.8	7.7	0	
HCM Lane LOS	-	-	А	А	А	
HCM 95th %tile Q(veh)	-	-	0.1	0	-	



Appendix F: Comments and Responses



August 6, 2021

Mr. Brian Groth Town Planner Town of Hudson 12 School Street Hudson, NH 03051

Re: Town of Hudson Planning Board Review – Traffic Study Review SL Chasse Steel Site Plan, Robinson Road Tax Map 105 Lot 17-2 & 17-3; Acct. #1350-532 Reference No. 20030249.2020

Dear Mr. Groth:

Fuss & O'Neill (F&O) has reviewed the Traffic Impact and Access Study received on July 23, 2021, related to the above-referenced project. The scope of this review letter is related to the traffic study only. Site plan, subdivision, and other review elements were previously provided.

The following items are noted:

4. Traffic

Fuss & O'Neill, Inc. has reviewed the traffic impact and access prepared by Transportation Engineering, Planning and Policy, LLC (TEPP) for Keach-Nordstrom Associates, Inc. (KNA) dated July 22, 2021, for the proposed commercial redevelopment on Robinson Road in Hudson, New Hampshire (Tax Map 115 Lots 17-2 and 17-3). The project proposes to replace one existing single-family housing unit with a 79,200 square foot light-industrial land use. Access and egress to the site will be provided via two proposed driveways—one on the east side of Robinson Road directly across from Christine Drive and one along Robinson Road 250 feet south of its intersection with Christine Drive. The two driveways will be connected internally to the site.

50 Commercial Street Manchester, NH 03101 † 603.668.8223 800.286.2469

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California Connecticut Maine Massachusetts New Hampshire Rhode Island Vermont In review of the TEPP report, we would suggest the following:

- Based on the provided ATR vehicle speeds, it appears that the mean speed and 85th percentile speed for the northbound direction should be 39.7 mph and 44 mph, respectively. These values are represented correctly in Table 2 of the Vehicle Speeds section of the report, but are summarized incorrectly in the text immediately after. The text should be revised to reflect what is shown in Table 2.
- The last column of Table 8 is incorrectly labeled as 2032 No Build. This label should be revised to reflect 2032 Build conditions.



Mr. Brian Groth August 6, 2021 Page 2 of 2

Overall, the procedures that the TEPP report uses are reasonable, with the appropriate ITE trip generation information used for the scenario provided. The analysis provided evaluated any impacts to Robinson Road at the driveway intersections. It should be noted that this site is within 1,200 feet of the Derry Road signalized intersection with Robinson Road and West Road. It's assumed that the majority of traffic from this site would need to travel through this signalized intersection, which would be up to an additional 27 vehicles in the AM either coming from Derry Road northbound or southbound approach, and up to an additional 21 vehicles in the PM existing operations of this signalized intersection, we would recommend that the traffic study add additional discussion of the site trips accessing the Derry Road intersection. Since the trips generated by the proposed site are relatively low, a full signalized intersection analysis may not be warranted, but an idea of what the additional queue lengths would be with the addition of the site could be estimated with an evaluation of existing conditions, including some type of adjustment for COVID if Derry Road's current traffic is not back to pre COVID levels.

We concur with TEPP's overall conclusion that, given the relatively low number of new trips expected to be generated by the site's proposed land use (35 during the weekday morning peak hour and 25 during the weekday evening peak hour), there should be minimal impacts on traffic operations of Robinson Road. However, the report does not include the full impacted roadway network, only the adjacent site roadway, so further evaluation as described above may be warranted.

Steven W. Bigitally signed by Steven W. Reichert, PE DR. cn=Steven W. Reichert, PE, c=US, o=Fus & O'Neil, Inc., ou=Fus & a Chefus & O'Neil, Inc., ou=Fus & a Chefus & Chefus & Chefus & a Dete: 2021 0806 17:252 c.9400 Dete: 2021 0806 17:252 c.9400

Please feel free to call if you have any questions.

Very truly yours,

Steven W. Reichert, P.E.

SWR:

Enclosure

cc:

Town of Hudson Engineering Division – File Keach- Nordstrom Associates, Inc. - alewis@keachnordstrom.com

RESPONSES BY TEPP LLC

<u>Comment.</u> Based on the provided ATR vehicle speeds, it appears that the mean speed and 85th percentile speed for the northbound direction should be 39.7 mph and 44 mph, respectively. These values are represented correctly in Table 2 of the Vehicle Speeds section of the report, but are summarized incorrectly in the text immediately after. The text should be revised to reflect what is shown in Table 2.

Response. Page 9 shows the revisions.

The last column of Table 8 is incorrectly labeled as 2032 No Build. This label should be revised to reflect 2032 Build conditions.

Response. Page 23, Table 8, shows the revision.

Comment. Overall, the procedures that the TEPP report uses are reasonable, with the appropriate ITE trip generation information used for the scenario provided. The analysis provided evaluated any impacts to Robinson Road at the driveway intersections. It should be noted that this site is within 1,200 feet of the Derry Road signalized intersection with Robinson Road and West Road. It's assumed that the majority of traffic from this site would need to travel through this signalized intersection, which would be up to an additional 27 vehicles in the AM either coming from Derry Road northbound or southbound approach, and up to an additional 21 vehicles in the PM existing the site via the eastbound approach. Should the Town have any concerns about the existing operations of this signalized intersection, we would recommend that the traffic study add additional discussion of the site trips accessing the Derry Road intersection. Since the trips generated by the proposed site are relatively low, a full signalized intersection analysis may not be warranted, but an idea of what the additional queue lengths would be with the addition of the site could be estimated with an evaluation of existing conditions, including some type of adjustment for COVID if Derry Road's current traffic is not back to pre COVID levels.

Response. Pages 15 and 19 show the response.

The Derry Road/Robinson Road/West Road signalized intersection is about 1,200 ft north of the Robinson Road/Christine Drive intersection. Traffic-volume increases at the former intersection due to the proposed redevelopment are up to:

- for the weekday AM-street-peak hour, up to 27 vehicle-trips entering for the site
- for the weekday PM-street-peak hour, up to 21 vehicle-trips leaving the site

This approximates less than one vehicle-trip per signal cycle and constitutes no significant impact. Therefore, no further analysis of the Derry Road/Robinson Road/West Road intersection is warranted.

S.L. CHASSE STEEL – 199 ROBINSON ROAD

SITE PLAN APPLICATION #04-21

STAFF REPORT #4

September 8, 2021

SITE: 199 Robinson Road; Map 105 Lot 17-3

ZONING: General-One (G-1)

PURPOSE OF PLANS: To show three proposed industrial buildings totaling 50,400 SF (including 2,520 SF office space) and associated parking on Robinson Road.

PLANS UNDER REVIEW: Non-residential Site Plan, S.L. Chasse Steel Contractor Buildings, Map 105 Lot 17-3, Robinson Road, Hudson, New Hampshire; prepared by Keach-Nordstrom Associates, Inc., 10 Commerce Park North, Suite 3, Bedford, New Hampshire 03110; prepared for SLC Development, LLC, 8 Christine Drive, Hudson, New Hampshire 03051; consisting of 12 sheets plus a cover page, with general notes 1-41 on Sheet 1; dated April 6, 2021, last revised September 2, 2021.

Note: Included in this packet is revised Sheet 5 showing fire suppression storage tanks.

ATTACHMENTS:

- A. Third Round of Peer Review of SP #04-21 by Fuss & O'Neill, dated August 30, 2021
- B. CAP Fee worksheet.

APPLICATION TRACKING:

- April 7, 2021 Site Plan applications received.
- May 28, 2021 Conditional Use Permit application received.
- June 1, 2021 Revised plans received.
- June 9, 2021 Public hearing scheduled, SP applications accepted, continued to June 23, 2021.
- June 23, 2021 Public hearing scheduled, applicant requested continuance to July 28, 2021.
- July 28, 2021 Public hearing scheduled, applicant requested continuance to August 25, 2021.
- August 17, 2021 Revised plan sets received.
- August 25, 2021 Public hearing scheduled.

WAIVER REQUESTS

The Applicant is seeking relief from two land use regulations:

§276-11.1.B(25) – This regulation permits the Planning Board to allow access ways across side lot lines. This enables the application to connect to the Owner's neighboring Lot 17-3 which is the subject of SP #04-21.

COMMENTS:

FIRE SUPPRESSION SUPPLY

Upon conferring with the Fire Department and Engineering Department, we have found the following:

- 1. The total fire suppression need to cover both sites is 60,000 gallons/hour for 3 hours, or 180,000 gallons
- 2. Our consultant's conservative estimate is that 500 gallons/minute are available, equating to 90,000 gallons over 3 hours.
- 3. This leaves a gap of 90,000 gallons.
- 4. 3x 30,000 gallon tanks would cover the supply gap for both sites.
- 5. Engineering requests an offsite improvement, a surge valve, in order to ensure the 500 gallons/minute is available from town supply. The Town would handle installation.

Additionally, during final design of the sprinkler systems for the respective buildings, if a "quick-response" system is designed and built; further forgiveness on the fire flow may become available. This has the potential to reduce the needs described above. Accordingly, this may mean starting the building permit review prior to recording the final Mylar in order to determine the final tank design.

A revised plan sheet has been provided showing proposed tank locations, if necessary.

(Draft Motions on the follow pages)

DRAFT MOTIONS

To GRANT a waiver:

I move to grant a waiver from §276-11.1.B(25), to allow access across the side lot line between lot 17-2 and Lot 17-4, based on the Board's discussion, the testimony of the Applicant's representative, and in accordance with the language included in the submitted Waiver Request Form for said waiver.

Motion by: ______Second: _____Carried/Failed: _____

<u>CONTINUE</u> the public hearing to a date certain:

I move to continue the public hearing for the conditional use permit application and site plan applications for S.L. Chasse Steel at 199; Map 105 Lot 17-3 to date certain, _____, 2021.

Motion by: _____ Second: _____ Carried/Failed:

<u>APPROVE</u> the site plan application:

I move to approve the site plan for Non-residential Site Plan, S.L. Chasse Steel Contractor Buildings, Map 105 Lot 17-3, Robinson Road, Hudson, New Hampshire; prepared by Keach-Nordstrom Associates, Inc., 10 Commerce Park North, Suite 3, Bedford, New Hampshire 03110; prepared for SLC Development, LLC, 8 Christine Drive, Hudson, New Hampshire 03051; consisting of 12 sheets plus a cover page, with general notes 1-41 on Sheet 1; dated April 6, 2021, last revised September 2, 2021; subject to, and revised per, the following stipulations:

- 1. All stipulations of approval shall be incorporated into the Notice of Decision and the Development Agreement, which shall be recorded at the HCRD, together with the Plan.
- 2. All improvements shown on the Plan shall be completed in their entirety and at the expense of the applicant or the applicant's assigns.
- 3. A cost allocation procedure (CAP) amount of \$69,552 shall be paid prior to the issuance of a Certificate of Occupancy.
- 4. An offsite improvement, a Surge Valve for the Route 102 Booster Station, is necessitated by this application in tandem with SP #03-21. This shall be coordinated with the Engineering Department.
- 5. Prior to the issuance of a Certificate of Occupancy, an L.L.S. Certified "as-built" site plan shall be provided to the Planning Department, confirming that the site conforms to the Plan approved by the Planning Board.

SP #04-21 Staff Report Page 3 of 4

- 6. The final design and size of the fire suppression water supply tanks shall be subject to the Fire Department's determination. The final Plan will reflect the needs identified during the building permit review process, which may begin prior to recording of the Plan. A building permit will not be issued until the Plan is recorded.
- 7. Prior to the Planning Board endorsement of the Plan, it shall be subject to final administrative review by Town Planner and Town Engineer.
- 8. The applicant shall schedule a pre-construction meeting with the Town Engineer prior to applying for a building permit.
- 9. Construction activities involving the subject lot shall be limited to the hours between 7:00 A.M. and 7:00 P.M. No exterior construction activities shall be allowed on Sundays.
- Blasting or ramming activities shall be limited to the hours between 9:00 A.M and 5:00 P.M, Monday through Friday. Blasting activities are prohibited on Saturday and Sunday.

Motion by: ______Second: _____Carried/Failed: _____



August 30, 2021

Mr. Brian Groth Town Planner Town of Hudson 12 School Street Hudson, NH 03051

Re: Town of Hudson Planning Board Review SL Chasse Steel Site Plan, Robinson Road Tax Map 105 Lot 17-3; Acct. #1350-532 Reference No. 20030249.2020

Dear Mr. Groth:

Fuss & O'Neill (F&O) has reviewed the second submission of the materials received on August 17, 2021, related to the above-referenced project. A list of items reviewed is enclosed. The scope of our review is based on the Site Plan Review Codes, Stormwater Codes, Driveway Review Codes, Sewer Use Ordinance 77, Zoning Regulations, and criteria outlined in the CLD Consulting Engineers Proposal approved September 16, 2003, revised September 20, 2004, June 4, 2007, September 3, 2008, and October 2015.

Please note that comments related to the proposed development at lot 17-2 will be forwarded with a separate letter. Also, the stormwater design documents provided as part of the review package incorporate both lots, so our drainage related comments have been provided separately.

Previous review comments that did not require further action or input have been removed from this letter for brevity/clarity.

The following items have outstanding issues:

4. Traffic

b. Former/Current Fuss & O'Neill Comment: The applicant should clarify the intent for the uses of each building related to truck loading. Large WB-50 and WB-67 trucks cannot access overhead loading door areas at each building. The applicant should confirm that smaller box trucks are intended for use at this site.

5. Utility Design/Conflicts

f. Former Fuss & O'Neill Comment: The applicant should review with the Hudson Fire Department whether hydrants should be installed along the proposed water main on Robinson Road. If required, hydrants shall meet the requirements of Hudson Engineering Technical Guidelines section 825.4.10. Current Fuss & O'Neill Comment: The applicant has stated that they are coordinating with the Town and the Fire Department.

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Mr. Brian Groth August 30, 2021 Page 2 of 4

The following items require Town evaluation or input:

1. Site Plan Review Codes (HR 275)

a. Former Fuss & O'Neill Comment: Hudson Regulation (HR) 275-6.I. The scope of this review does not include the adequacy of any fire protection provisions for the proposed buildings. Fuss & O'Neill defers to the Hudson Fire Department for review of proposed fire protection for this facility. We note that the site is proposed to be serviced by a private well. The Town should review the need for an onsite cistern depending on the well capacity.

Current Fuss & O'Neill Comment: The applicant has stated that they are coordinating with the Town and the Fire Department.

2. Administrative Review Codes (HR 276)

b. Former Fuss & O'Neill Comment: HR 276-11.1.B.(6) The owner's signature is not shown on the plan set.

Current Fuss & O'Neill Comment: The applicant noted that the owner will sign the final plan.

7. Zoning (ZO 334)

f. Former Fuss & O'Neill Comment: ZO 334-36.C.(2). The applicant is proposing installation of a water main along Robinson Road that impacts a wetlands buffer. The applicant should review with the Town to determine if the proposed water main requires a Conditional Use Permit in accordance with the Ordinance. Current Fuss & O'Neill Comment: The applicant has stated that a Conditional Use Permit had been submitted. No further Fuss & O'Neill comment.

The following items are resolved or have no further Fuss & O'Neill input:

1. Site Plan Review Codes (HR 275)

e. Former Fuss & O'Neill Comment: HR 275-8.C.(6). The applicant should show the proposed off-street loading spaces on the plan set. We note that 6 spaces are required and the applicant has noted that 7 spaces are provided, but they are not labeled. The applicant should show loading areas on the plan to be sure they do not conflict with circulation on the site and that the size is adequate. / The applicant has shown and labeled 6 loading spaces on the plan set but continued to note that 7 are provided. The applicant should revise the note or add an additional area to the plan set.

Current Fuss & O'Neill Comment: The applicant has revised the plan to show 7 loading spaces. No further Fuss & O'Neill comment.

g. Former Fuss & O'Neill Comment: HR 275-9.F. The applicant did not provide copies of any easements or deeds as part of the package received for review. / The applicant has provided a copy of the deed. We note no easements were provided, however, the deed references a State of New Hampshire easement.

Current Fuss & O'Neill Comment: The applicant has stated that the easement noted affected the lot before the 2020 subdivision and does not affect this parcel. No further Fuss & O'Neill comment.



Mr. Brian Groth August 30, 2021 Page 3 of 4

2. Administrative Review Codes (HR 276)

f. Former Fuss & O'Neill Comment: HR 276-11.1.B.(17). We were unable to locate any benchmarks within the Site plan.

Current Fuss & O'Neill Comment: The applicant has added the benchmark to the plan set. No further Fuss & O'Neill comment.

4. Traffic

a. Former Fuss & O'Neill Comment: HR 275-9.B. The applicant has not provided any traffic information as part of the review package. / The applicant has indicated that the Town has not requested any traffic information, and their understanding is that it is not required to this point. We note that between lots 2 and 3 there are over 130 parking spaces proposed, which may provide traffic impacts that warrant further review.

Current Fuss & O'Neill Comment: The applicant has submitted a traffic study and our review comments were provided to the Town on August 6, 2021, for the report dated July 23, 2021. Further traffic reviews will be responded to separately.

5. Utility Design/Conflicts

a. Former Fuss & O'Neill Comment: HR 275-9.E and 276-13. The applicant has provided a typical septic system detail and shown the approximate location. We note that no water/well details were provided. /The applicant has stated that they are currently working on a water connection to the site, and an off-site water main extension plan was provided. We note that water lines are now shown on the site but details, size and materials are not provided.

Current Fuss & O'Neill Comment: The applicant has shown details, size and materials are for the proposed water main. No further Fuss & O'Neill comment.

- d. Former Fuss & O'Neill Comment: The applicant should provide details for sewer manholes, sewer trench, and sewer/drain separation
 Current Fuss & O'Neill Comment: The applicant has added the recommended details to the plan set. No further Fuss & O'Neill comment.
- e. Former Fuss & O'Neill Comment: The applicant has provided a water main profile that shows a minimum of 4 feet of cover in some sections. The minimum cover required by the Town of Hudson is 5 feet. Current Fuss & O'Neill Comment: The applicant has revised the plan to provide the minimum 5 feet of cover. No further Fuss & O'Neill comment.

6. Drainage Design/Stormwater Management (HR 275-9.A./Chapter 290)

The review of the drainage design and Alteration of Terrain report was provided under separate letters from Fuss & O'Neill dated April 30, 2021, June 14, 2021, and August 30, 2021.



Mr. Brian Groth August 30, 2021 Page 4 of 4

Please feel free to call if you have any questions.

Very truly yours,



Steven W. Reichert, P.E.

SWR:

Enclosure

cc: Town of Hudson Engineering Division – File Keach- Nordstrom Associates, Inc. - svando@keachnordstrom.com

Chasse Steel Lot 3 - SP #04-21 - Attachment B



TOWN OF HUDSON

Planning Board



Timothy Malley, Chairman

12 School Street • Hudson, New Hampshire 03051 • Tel: 603-886-6008 • Fax: 603-594-1142

CAP FEE WORKSHEET - 2021

Date <u>:</u>	09-01-21	Zone #	1	Map/Lot: _	<u>105/017-003</u> 199 Robinson	Rd.
Project 1	Name:	S.L. Ch	asse Steel			114.
Propose	d ITE Use #	1: <u>Indus</u>	trial			
Propose	d Building A	Area (square	footage):_	50,4	00	<u>S.F.</u>
CAP FE	CES: (ONE C	CHECK NEE	DED)			
1.	(Bank	09)				

1.	(Bank 09) 2070-701	Light Industrical (50,400 s.f @ \$1.38 per s.f)		69,552.00
		Total CAP Fee	<u>\$</u>	69,552.00

Check should be made payable to the <u>Town of Hudson</u>.

Thank you,

Brooke Dubowik

Planning Administrative Aide

S.L. CHASSE STEEL WATERLINE EXTENSION

CONDITIONAL USE PERMIT APPLICATION #07-21

STAFF REPORT #2

September 8, 2021

SITE: 199-201 Robinson Road; Map 105 Lot 017-002 & Map 105 Lot 017-003

ZONING: General-One (G-1)

PURPOSE OF PLAN: To show public water connection from the existing stub approximately 900 feet north of the site to lots 17-2 & 17-3.

PLANS UNDER REVIEW: Offsite Water Main Extension Plan and Profile, S.L. Chasse Steel, Map 105 Lot 17-2 & 17-3, Robinson Road, Hudson, New Hampshire; prepared by Keach-Nordstrom Associates, Inc., 10 Commerce Park North, Suite 3, Bedford, New Hampshire 03110; prepared for Steel Properties, LLC, 8 Christine Drive, Hudson, New Hampshire 03051; consisting of 1 sheet with construction notes 1-4 on Sheet 1; dated May 25, 2021.

ATTACHMENTS:

A. Hudson Conservation Commission's Input, dated June 14, 2021.

APPLICATION TRACKING:

- May 28, 2021 Application received.
- June 14, 2021 Site Walk and Recommendation the Conservation Commission
- June 23, 2021 Public hearing scheduled.
- June 23, 2021 Public hearing scheduled, applicant requested continuance to July 28, 2021.
- July 28, 2021 Public hearing scheduled, applicant requested continuance to August 25, 2021.
- August 25, 2021 Public hearing scheduled, continued to September 8, 2021
- September 8, 2021 Public hearing scheduled.

COMMENTS & RECOMMENDATIONS:

BACKGROUND

The applicant is proposing development on Lot 017-002 and Lot 017-003, which requires an offsite extension of the waterline along the public right-of-way on Robinson Road to provide town water to both lots. The extension work will run alongside Robinson Road from Map 105 Lot 018-000 to Map 105 Lot 017-003.

The proposed waterline extension will traverse over portions of both delineated wetland and wetland buffer area that run across the road between Map 105 Lot 017-000 and Map 105 Lot 017-001, requiring a conditional use permit from the Planning Board.

CUP #07-21 Staff Report Page 1 of 3

STAFF COMMENTS

1. Use within Wetland Conservation District (§ 334-36): The proposed waterline extension is a conditional use permitted under § 334-36:C(2). However, such conditional use shall be located and constructed in such a way as to minimize the potential for detrimental impact to the District, and may be permitted only when no viable alternative is available.

The proposed extension is laid out to limit impact to the District by avoiding direct impacts to the wetland. A stipulation of approval is recommended in the event a direct impact occurs in the field.

2. Hudson Conservation Commission Comments

- a. The HCC found the proposed project complies with the Hudson Zoning Ordinance Article IX, §334-36 (C) (2) and §334-37 (2).
- b. The HCC voted to recommend a favorable acceptance to the Planning Board for the proposed waterline extension (3-0).
- c. The HCC asked the following recommendations and or notes be added to the plan set as part of the approval process:
 - 1. Construction and restoration shall comply with Best Management Practices set forth in New Hampshire Storm Water Manual Volume 3: Erosion and Sediment Control
 - 2. Prior to the start of construction erosion control barriers shall be installed and maintained to the satisfaction of the Town Engineer
 - 3. This motion is based on the plan(s) submitted by the applicant. It is recommended that if additional impacts are necessary the plan be returned to the Conservation Commission for further review.
- 3. Lot Number and Lot Line: Several off-site lot numbers and lines shown on the plan appear to be incorrect.
 - a. Map 105 Lot 11 shown on the plan should be two different lots: Map 105 Lot 011-000 and Map 105 Lot 011-001.
 - b. The lot label for Map 105 Lot 017-000 is missing (the designated wetland is located on this lot).
 - c. Town of Hudson's formal lot number consists of two sets of 3-digit numbers (e.g. Map 105 Lot 011-000). Since the submitted plan includes multiple lots with similar numbering, staff suggests the applicant relabel the lots using the two full sets of 3-digit numbers to avoid confusion.

DRAFT MOTIONS

ACCEPT the conditional use permit application:

I move to accept the conditional use permit application for S.L. Chasse Steel Waterline Extension along Robinson Road from the existing water main to Map 105 Lot 17-3.

Motion by: ______Second: _____Carried/Failed: _____

<u>CONTINUE</u> the public hearing to a date certain:

I move to continue the conditional use permit application for S.L. Chasse Steel Waterline Extension along Robinson Road from the existing water main to Map 105 Lot 17-3.to date certain, ______, 2021.

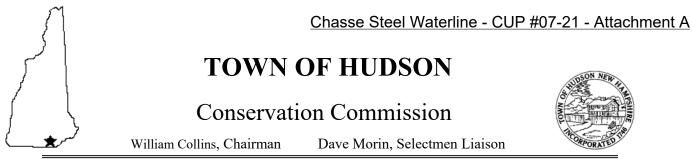
Motion by: _____ Second: _____ Carried/Failed:

<u>APPROVE</u> the site plan application:

I move to approve the conditional use permit for Offsite Water Main Extension Plan and Profile, S.L. Chasse Steel, Map 105 Lot 17-2 & 17-3, Robinson Road, Hudson, New Hampshire; prepared by Keach-Nordstrom Associates, Inc., 10 Commerce Park North, Suite 3, Bedford, New Hampshire 03110; prepared for Steel Properties, LLC, 8 Christine Drive, Hudson, New Hampshire 03051; consisting of 1 sheet with construction notes 1-4 on Sheet 1; dated May 25, 2021; subject to, and revised per, the following stipulations:

- 1. All stipulations of approval shall be incorporated into the Notice of Decision and the Development Agreement, which shall be recorded at the HCRD, together with the Plan.
- 2. Prior to the Planning Board endorsement of the plans, it shall be subject to final administrative review by Town Planner and Town Engineer, including the plan revisions identified by the Staff Report.
- 3. Construction activities involving the subject lot shall be limited to the hours between 7:00 A.M. and 7:00 P.M. No exterior construction activities shall be allowed on Sundays.
- 4. Construction and restoration shall comply with Best Management Practices set forth in New Hampshire Storm Water Manual Volume 3: Erosion and Sediment Control
- 5. Prior to the start of construction erosion control barriers shall be installed and maintained to the satisfaction of the Town Engineer.
- 6. In the event a wetland impact occurs, the Applicant or its assigns shall notify the Town of Engineering Department and the Department of Environmental Services.

Motion by:	Second:	Carried/Failed:



12 School Street • Hudson, New Hampshire 03051 • Tel: 603-886-6008 • Fax: 603-816-1291

Date: June 14, 2021

Case: Robinson Rd. Water Main Extension Hudson, New Hampshire Map 205, Lot 17-2 & 17-3 Zone: General One (G1)

Description of work to be performed: The project entails extending an existing water main located near the intersection of Robinson Road and Derry Road (Route 102) approximately 900 feet southerly along Robison Road to supply water service for Map 105 lots 17-2 and 17-3. Total wetland buffer impact if accepted will equal 1,760 sq. ft. total.

Conservation Members Stepping Down: None

Alternates Seated: None

Applicant Representative(s): Tony Basso, Keach-Nordstrom Associates, Inc.

Motion to "Recommend"

Mr. Dickinson moved to recommend a favorable acceptance to the Planning Board for the proposed water main extension along Robinson Road that will be used to service Map 105 lots 17-2 and 17-3. After review the conservation commission members find that the proposed project is in compliance with the Hudson Zoning Ordinance Article IX, §334-36 (C) (2) and §334-37 (2). The HCC does ask that the following recommendations and or notes be added to the plan set as part of the approval process.

- 1. Construction and restoration shall comply with Best Management Practices set forth in New Hampshire Storm Water Manual Volume 3: Erosion and Sediment Control
- 2. Prior to the start of construction erosion control barriers shall be installed and maintained to the satisfaction of the Town Engineer

3. This motion is based on the plan(s) submitted by the applicant. It is recommended that if additional impacts are necessary the plan be returned to the Conservation Commission for further review.

Motion Second: Mr. Kallgren

Vote: William Kallgren Yes, Ken Dickinson Yes, William Collins Yes

William Collins

William Collins HCC Chairman

A copy of this recommendation/motion shall be stapled to the CUP application and forward it to the Town Planning Office for inclusion in the Planning Board Member Packets.

AROMA JOES

SITE PLAN APPLICATION #08-21

STAFF REPORT

September 8, 2021

SITE: 56 Derry Street; Map 173 Lot 029-000

ZONING: Business (B)

PURPOSE OF PLANS: Propose an Aroma Joe's drive-thru coffee shop at 56 Derry Street with associated parking and drives.

PLANS UNDER REVIEW:

Non-Residential Site Plan, Aroma Joe's; prepared by Keach-Nordstrom Associates, Inc., 10 Commerce Park North, Suite 3B, Bedford, NH 03110; prepared for owner: Steve S. & Hsiang Hwa W. Pan, 13 King Henry Drive, Londonderry, NH 03053 and owner/applicant: Scott Ziefelder, 169 Cannan Back Road, Barrington, NH 03825; consisting of 16 sheets including a cover sheet, with general notes 1-32 on Sheet 1; dated June 22, 2021, last revised August 30, 2021. [Plan set attached hereto]

AND: Revised Sheet 3, responsive to Staff comment in coordination of pedestrian facilities.

ATTACHMENTS:

- A. Peer Review of Traffic Study, dated August 26, 2021
- B. Applicant response to peer review, received August 30, 2021
- C. Revised Traffic Impact and Access Study, prepared by TEPP, August 30, 2021

APPLICATION TRACKING:

- June 23, 2021 Application received.
- July 20, 2021 Traffic Impact and Access Study received.
- July 28, 2021 Application accepted, public hearing held, waiver granted for relief from residential buffer, continued to 8/25/21.
- August 17, 2021 Revised plans received.
- August 25, 2021 Awaiting peer review of traffic study, deferred to September 8, 2021
- August 31, 2021 Revised plans and studies received.
- September 8, 2021 Public hearing scheduled.

COMMENTS:

TRAFFIC

Fuss & O'Neill completed their review of the traffic study; this review is **Attachment A**. The review identifies some typos, but more significantly, several points that require additional detail.

SP #08-21 Staff Report Page 1 of 3 TEPP, the Applicant's traffic engineer, provided a revised Traffic Study (Attachment C); the final two pages are direct responses to the peer review.

Staff hopes to have a peer review of this response in advance of the meeting.

ENGINEERING

See Attachment B for the Applicant's response to the engineering peer review. There appears to be different perspectives on drainage analysis, for which Staff will seek additional input in advance of the meeting.

PLANNING

When designing the crosswalk spanning the driveway, the Applicant encountered a challenge with MUTCD standards (Manual on Uniform Traffic Control Devices issued by the Federal Highway Administration). These standards do not allow a vehicular stop bar to be placed immediately after a crosswalk. Instead, the Applicant removed the "jog" from the sidewalk, allowing the stop bar to be placed before the crosswalk. See the revised Sheet 3 for more detail. As a result, the easement will likely changed but shall be reviewed and approved by Town Counsel.

In practice, it is likely that vehicles will inch up across the sidewalk as they look for opening to exit the site. However, as pedestrians arrive the markings will clearly delineate their right to pass thereby achieving the crosswalk's objective.

DRAFT MOTIONS <u>CONTINUE</u> the public hearing to a date certain:

I move to continue the public hearing for the site plan application for Aroma Joes at 56 Derry Street; Map 173 Lot 029-000 to date certain, _____, 2021.

Motion by: ______ Second: ______ Carried/Failed: ______

<u>APPROVE</u> the site plan application:

I move to approve the Site Plan: Non-Residential Site Plan, Aroma Joe's; prepared by Keach-Nordstrom Associates, Inc., 10 Commerce Park North, Suite 3B, Bedford, NH 03110; prepared for owner: Steve S. & Hsiang Hwa W. Pan, 13 King Henry Drive, Londonderry, NH 03053 and owner/applicant: Scott Ziefelder, 169 Cannan Back Road, Barrington, NH 03825; consisting of 16 sheets including a cover sheet, with general notes 1-32 on Sheet 1; dated June 22, 2021, last revised August 30, 2021; subject to, and revised per, the following stipulations:

- 1. All stipulations of approval shall be incorporated into the Development Agreement, which shall be recorded at the HCRD, together with the Plan.
- 2. All improvements shown on the Plan shall be completed in their entirety and at the expense of the applicant or the applicant's assigns.

- 3. Prior to the issuance of a Certificate of Occupancy, an L.L.S. Certified "as-built" site plan shall be provided to the Town of Hudson Land Use Development, confirming that the site conforms to the Plan approved by the Planning Board.
- 4. A cost allocation procedure (CAP) amount of \$20,696.00 shall be paid prior to the issuance of a Certificate of Occupancy
- 5. The onsite drainage system shall be constructed and maintained in compliance with NHDES requirements for such systems.
- 6. Prior to the Planning Board endorsement of the Plan, it shall be subject to final administrative review by Town Planner and Town Engineer.
- 7. The applicant shall schedule a pre-construction meeting with the Town Engineer prior to beginning work on the site.
- 8. Hours of refuse removal shall be exclusive to the hours between 7:00 A.M. and 7:00 P.M., Monday through Friday only.

Motion by: _____Second: _____Carried/Failed: _____

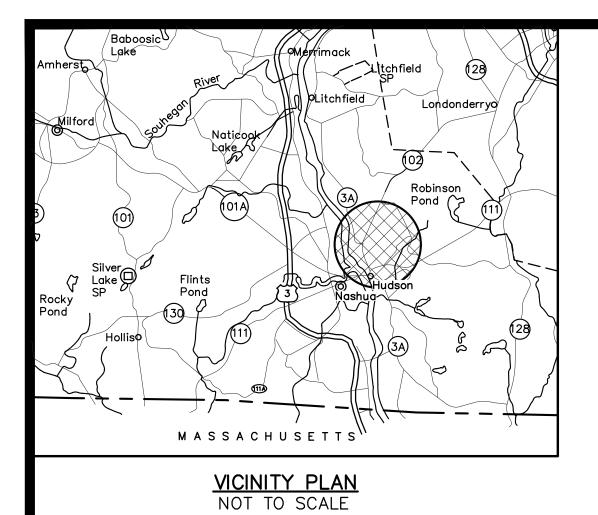
REVISED DRAFT MOTION, CHANGE IN BOLD.

<u>APPROVE</u> the site plan application:

I move to approve the Site Plan: Non-Residential Site Plan, Aroma Joe's; prepared by Keach-Nordstrom Associates, Inc., 10 Commerce Park North, Suite 3B, Bedford, NH 03110; prepared for owner: Steve S. & Hsiang Hwa W. Pan, 13 King Henry Drive, Londonderry, NH 03053 and owner/applicant: Scott Ziefelder, 169 Cannan Back Road, Barrington, NH 03825; consisting of 16 sheets including a cover sheet, with general notes 1-32 on Sheet 1; dated June 22, 2021, last revised August 30, 2021; subject to, and revised per, the following stipulations:

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- 4. A cost allocation procedure (CAP) amount of \$20,696.00 shall be paid prior to the issuance of a Certificate of Occupancy
- 5. The onsite drainage system shall be constructed and maintained in compliance with NHDES requirements for such systems.
- 6. Prior to the Planning Board endorsement of the Plan, it shall be subject to final administrative review by Town Planner and Town Engineer.
- 7. The applicant shall schedule a pre-construction meeting with the Town Engineer prior to beginning work on the site.
- 8. Hours of refuse removal shall be exclusive to the hours between 7:00 A.M. and 7:00 P.M., Monday through Friday only.

Motion by: _____Second: _____Carried/Failed: _____

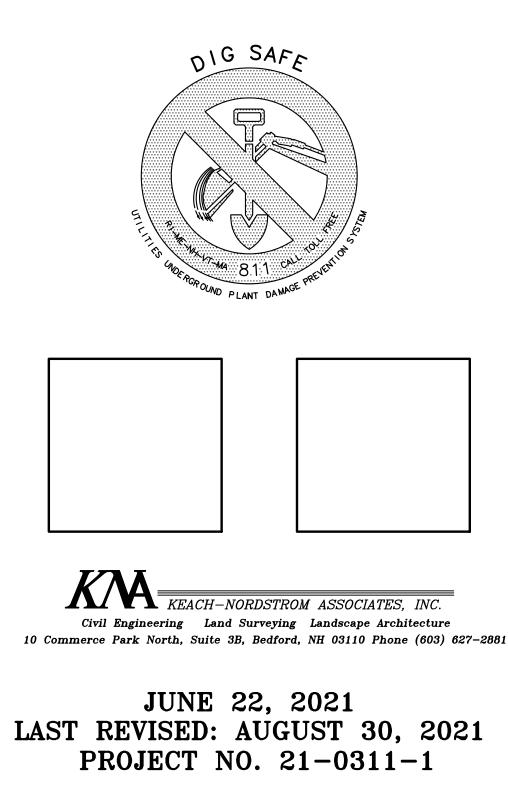


OWNER: STEVE S. & HSIANG HWA W. PAN **13 KING HENRY DRIVE** LONDONDERRY, NH 03053

OWNER/APPLICANT: SCOTT ZIEFELDER 169 CANAAN BACK ROAD BARINGTON, NH 03825

PREPARED BY: KEACH-NORDSTROM ASSOCIATES, INC. 10 COMMERCE PARK NORTH, SUITE 3B BEDFORD, NEW HAMPSHIRE 03110 (603) 627-2881

NON RESIDENTIAL SITE PLAN AROMA JOE'S MAP 173; LOTS 29 56 DERRY STREET HUDSON, NEW HAMPSHIRE

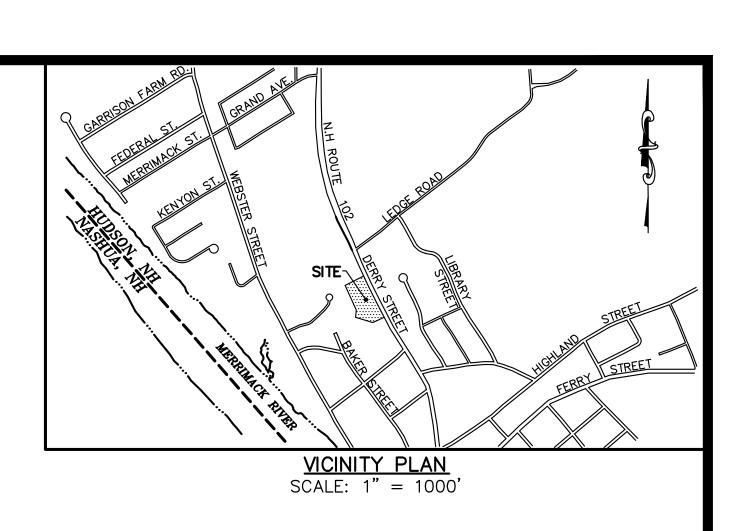


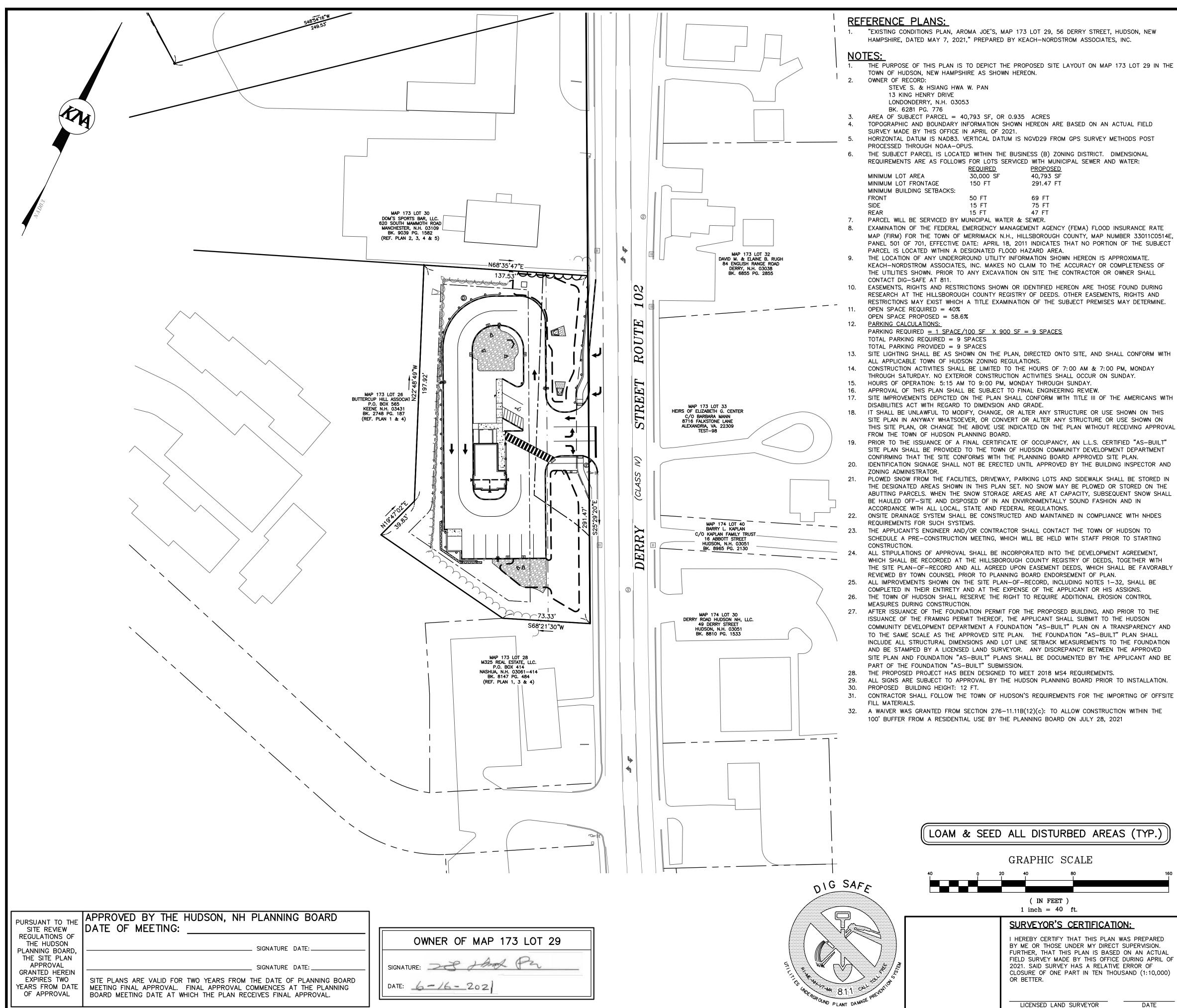
SHEET TITLE

MASTER PLAN EXISTING CONDIT NON-RESIDENTIAI GRADING, DRAINA **EROSION CONTRO** LANDSCAPE PLAN LIGHTING PLAN SIGHT DISTANCE CONSTRUCTION DI

EASEMENT PLAN

	SHEET No.
	1
TIONS/REMOVALS PLAN	2
L SITE PLAN	3
AGE, AND UTILITY PLAN	4
DL PLAN	5
I	6
	7
PLAN	8
DETAILS	9-14
	E1





THE SUBJECT PARCEL IS LOCATE	ED WITHIN TH	E BUSINESS (B) ZONING	DISTRICT.
REQUIREMENTS ARE AS FOLLOWS	S FOR LOTS S	SERVICED WITH MUNICIPAL	SEWER A
	<u>REQUIRED</u>	PROPOSED	

MINIMUM LOT AREA	30,000 SF	40,793 SF
MINIMUM LOT FRONTAGE	150 FT	291.47 FT
MINIMUM BUILDING SETBACKS:		
FRONT	50 FT	69 FT
SIDE	15 FT	75 FT
REAR	15 FT	47 FT

DIMENSIONAL

AND WATER:

E	CAREADON LAND AND AND AND AND AND AND AND AND AND
	VICINITY PLAN

SCALE: 1'' = 1000'

<u>LEGEN</u>	<u>D</u>			
I GB-F	GRANITE	BOUND	FOUND	

OWNER OF RECORD:

STEVE S. & HSIANG HWA W. PAN

13 KING HENRY DRIVE

LONDONDERRY, N.H. 03053

BK. 6281 PG. 776

⊚ IP−F	IRON PIN FOUND
⊚ IP−S W/CAP	IRON PIN SET WITH CAP
0	UTILITY POLE
x-o	STREET LIGHT
GV	GAS VALVE
Ŵ	WATER VALVE
S	SEWER MANHOLE
\bigcirc	DRAINAGE MANHOLE
11	CATCH BASIN
	ABUTTER LINE
	PROPERTY LINE
OHU	OVERHEAD UTILITIES
	DRAINAGE LINE
	TREELINE
	RETAINING WALL
EOP	EDGE OF PAVEMENT
VGC	VERTICAL GRANITE CURB
	SETBACK
	100' RESIDENTIAL BUFFER
· ·	GREEN SPACE BUFFER
-	PROPOSED SIGN
₽¢÷	PROPOSED LIGHT
₿¥ I	PROPOSED GAS VALVE
Ŵ	PROPOSED WATER VALVE
0 0 0 0 0 .	PROPOSED WOOD GUARDRAIL
	PROPOSED TREELINE
	PROPOSED EDGE OF PAVEMENT
	PROPOSED RETAINING WALL
	PROPOSED VERTICAL GRANITE CU
	PROPOSED OUTLET STRUCTURE

MASTER SITE PLAN AROMA JOE'S MAP 173 LOT 29 56 DERRY STREET HUDSON, NEW HAMPSHIRE HILLSBOROUGH COUNTY

KAA KEACH-NORDSTROM ASSOCIATES, INC.

Civil Engineering Land Surveying Landscape Architecture

10 Commerce Park North, Suite 3B, Bedford, NH 03110 Phone (603) 627-2881

DATE

REVISIONS						
No.	No. DATE DESCRIPTION					
1	07/23/2021	07/23/2021 REVISED PER REVIEW COMMENTS				
2	08/12/2021 REVISED PER PLANNING BOARD COMMENTS					
3	08/30/2021	REVISED PER I	REVISED PER REVIEW COMMENTS			
DATI	SCALE: 1"=40'					
PRO	JECT NO: 2	1-0311-1	SHEET 1 OF 14			

APPLICANT:

SCOTT ZIELFELDER

169 CANAAN BACK ROAD BARRINGTON, NH 03825



GRANITE BOUND FOUND

IRON PIN SET WITH CAP

IRON PIN FOUND

UTILITY POLE

STREET LIGHT

GAS VALVE

WATER VALVE

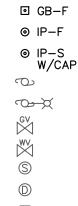
CATCH BASIN

PROPERTY LINE

EDGE OF PAVEMENT

SEWER MANHOLE

DRAINAGE MANHOLE



----- ABUTTER LINE -----G------ GAS LINE -----W------W------WATER LINE = = = = = = = DRAINAGE LINE

RETAINING WALL FOP VERTICAL GRANITE CURB _____ VGC _____ 10' CONTOUR _____2' CONTOUR

••••••••••••• SOIL LINE

SCS SOILS LEGEND WdB WINDSOR LOAMY SAND

3 TO 8% SLOPES WINDSOR-URBAN LAND COMPLEX WnC 3 TO 15% SLOPES

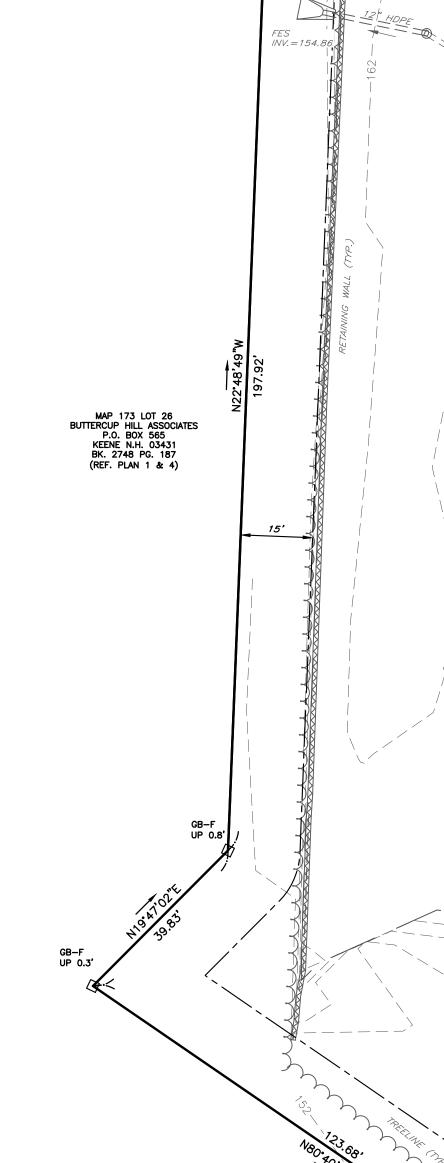
SOURCE: USDA-SCS WEB SOIL SURVEY HILLSBOROUGH COUNTY

DIG SAFF



UTILITY NOTE

THE UNDERGROUND UTILITIES DEPICTED HEREON HAVE BEEN DRAWN FROM FIELD SURVEY INFORMATION AND OR PLOTTED FROM EXISTING DRAWINGS. KEACH-NORDSTROM ASSOCIATES, INC. MAKES NO GUARANTEES THAT THE UNDERGROUND UTILITIES DEPICTED COMPRISE ALL SUCH UTILITIES IN THE AREA, EITHER IN SERVICE OR ABANDONED. FURTHER, KEACH-NORDSTROM ASSOCIATES, INC. DOES NOT WARRANT THAT THE UNDERGROUND UTILITIES SHOWN ARE IN THE EXACT LOCATION INDICATED ALTHOUGH THEY ARE LOCATED AS ACCURATELY AS POSSIBLE FROM THE INFORMATION AVAILABLE. KEACH-NORDSTROM ASSOCIATES, INC. HAS NOT PHYSICALLY LOCATED THE UNDERGROUND PORTIONS OF THE UTILITIES.



MAP 173 LOT 30 DOM'S SPORTS BAR, LLC. 620 SOUTH MAMMOTH ROAD MANCHESTER, N.H. 03109 BK. 9039 PG. 1582

(REF. PLAN 2, 3, 4 & 5)

IP-S W/CAP

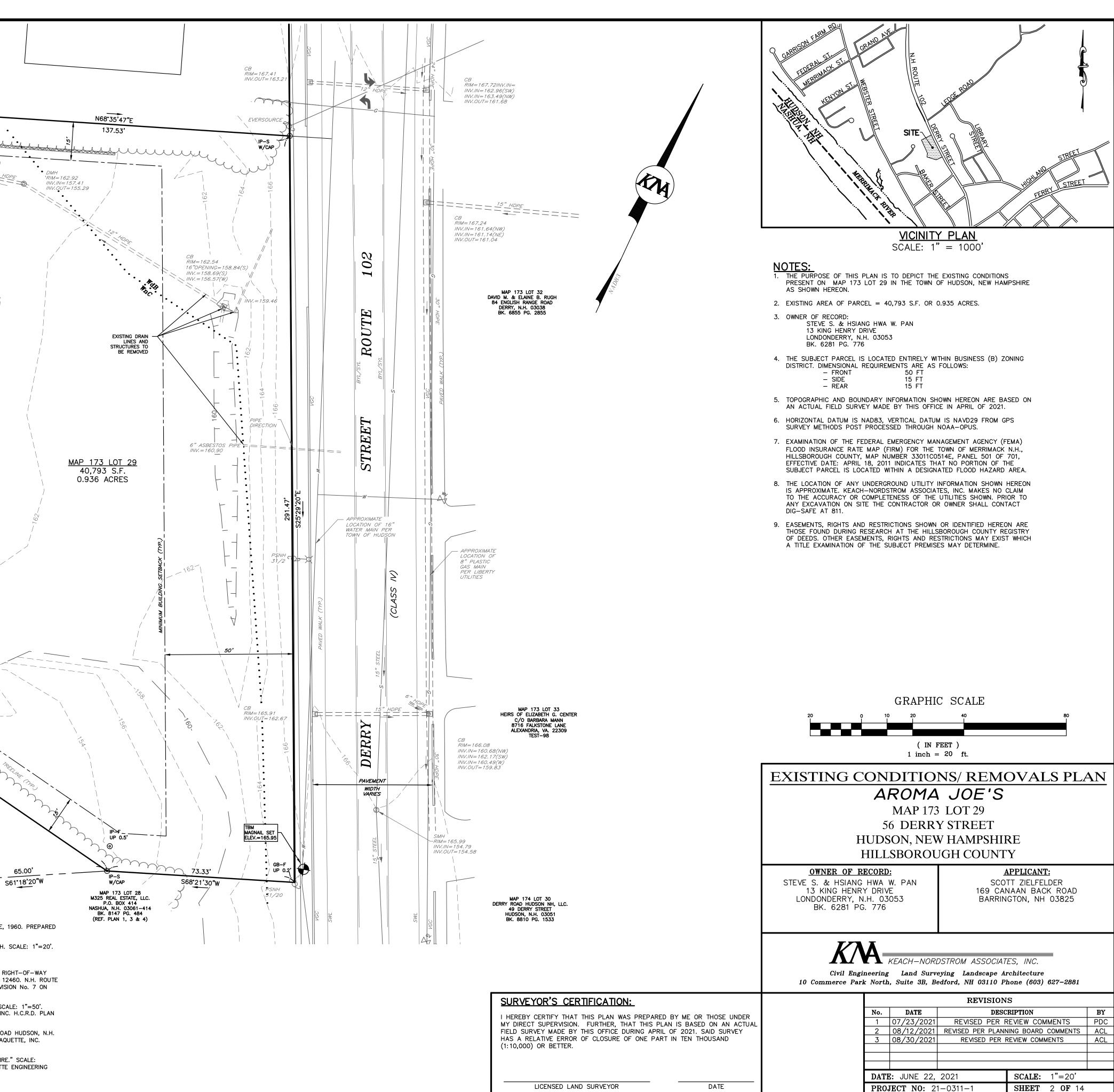
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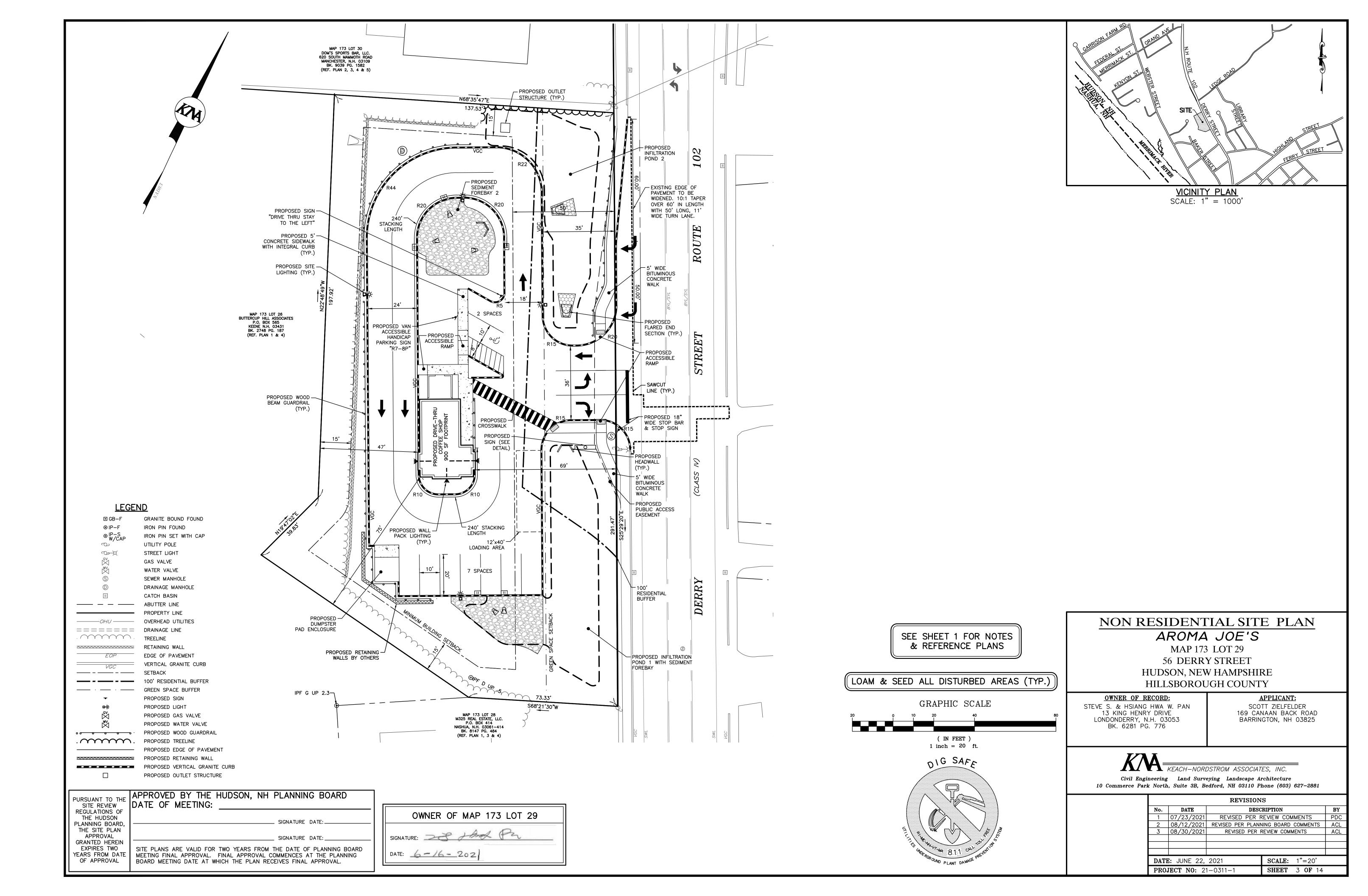
REFERENCE PLANS

1. "PLAN OF DAW ACRES," HUDSON, N.H. SCALE: 1"=50'. DATED: JUNE, 1960. PREPARED BY: NED SPAULDING H.C.R.D. PLAN #2473

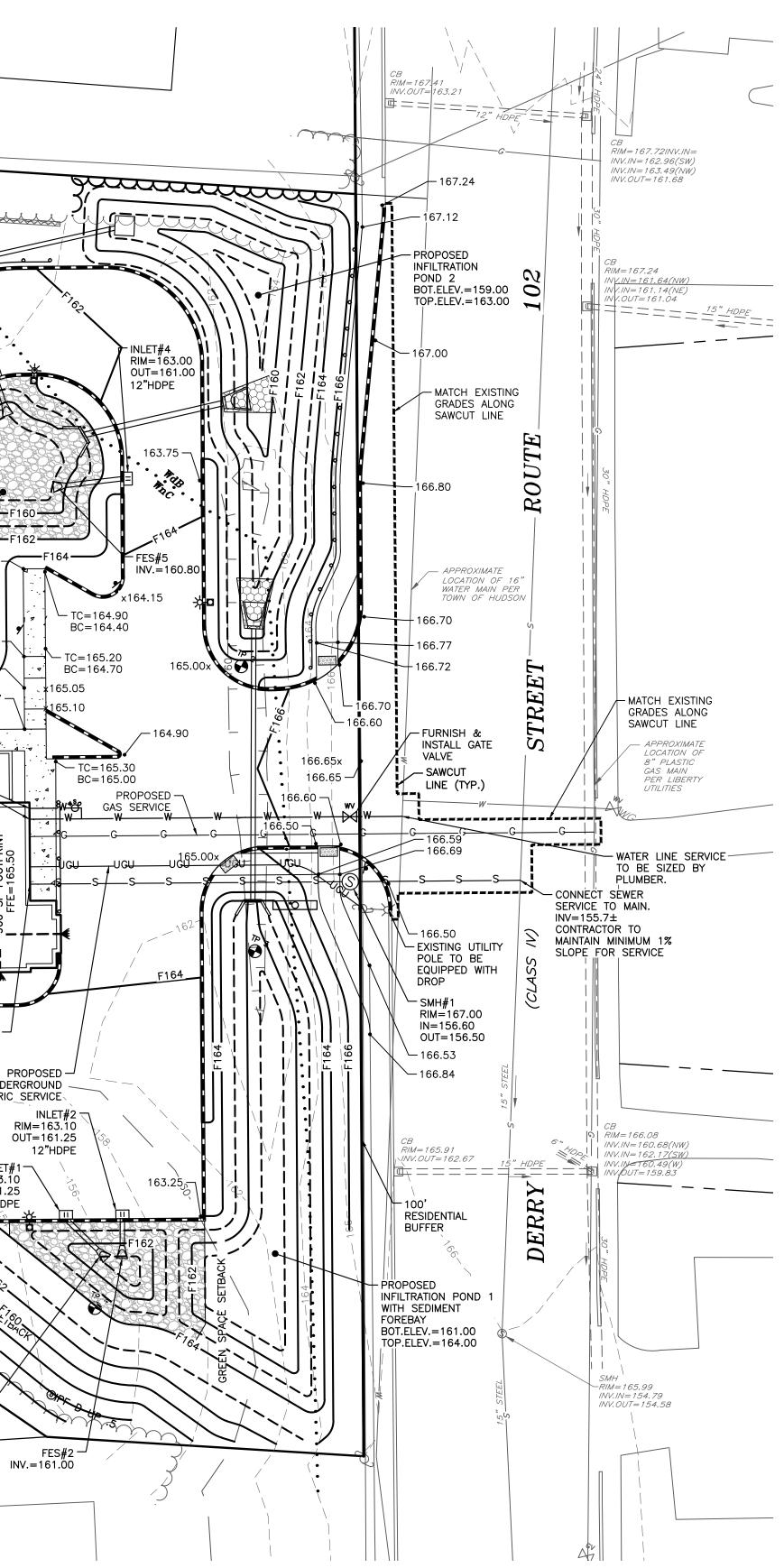
65.00**'**

- 2. "CONSOLIDATION & SUBDIVISION PLAN," DERRY STREET, HUDSON, N.H. SCALE: 1"=20'. DATED: JUNE, 1978. PREPARED BY: A.E. MAYNARD CIVIL ENGINEER. H.C.R.D. PLAN #11484 3. "THE STATE OF NEW HAMPSHIRE DEPARTMENT OF TRANSPORTATION RIGHT-OF-WAY
- PLANS FEDERAL AID PROJECT STP-X-000S(216) N.H. PROJECT NO. 12460. N.H. ROUTE 102" REVISED ROW PURCHASE PLANS DATE: D SEPT . 09, 2005, REVISION No. 7 ON FILE WITH NHDOT, NOT RECORDED.
- 4. "BOUNDARY & CONSOLIDATION PLAN, HUDSON ELDERLY HOUSING." SCALE: 1"=50'. DATED: SEPTEMBER 12, 1979. PREPARED BY: ALLAN H. SWANSON, INC. H.C.R.D. PLAN #12828
- 5. "SITE PLAN, PROPOSED RESTAURANT," PIZZA HUT INC. 62 DERRY ROAD HUDSON, N.H. SCALE: 1"=20'. DATED: JUNE, 1978. PREPARED BY: MAYNARD & PAQUETTE, INC. H.C.R.D. PLAN #15190
- 6. "SITE PLAN, GREEN TEA, 56 DERRY STREET, HUDSON, NEW HAMPSHIRE." SCALE: 1"=20'. DATED: JUNE 7, 2002. PREPARED BY: MAYNARD & PAQUETTE ENGINEERING ASSOCIATES, LLC. H.C.R.D. PLAN #32223

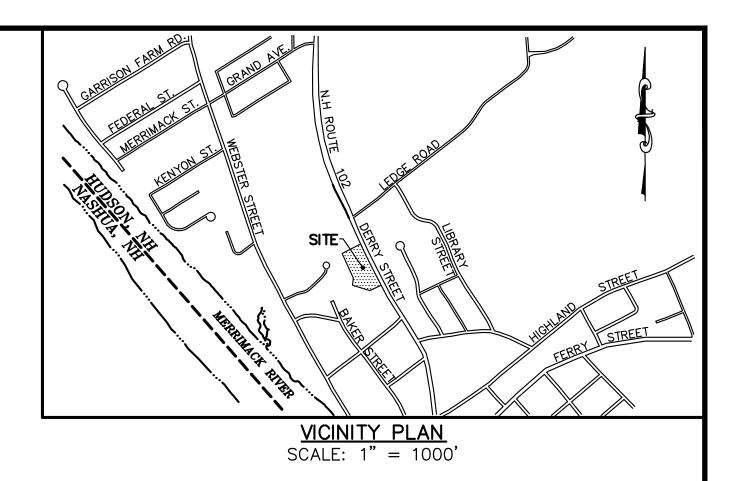




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⊚ IP−S W/CAP		
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X	WATER VALVE	
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	CATCH BASIN	EXISTING FLARED
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	- PROPERTY LINE	
GG_	GAS LINE	
WW		EXISTING DRAIN
<i>SS</i>		MANHOLE
OHU		162.00
======		INLET#4 —
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EOP	EDGE OF PAVEMENT	12"HDPE
	VERTICAL GRANITE CURB	FES#4 INV.=159.75
<i>VGC</i>		INV.=159.75
· ·		
-	PROPOSED SIGN	F162
ъ¢	PROPOSED LIGHT	INLET#3 E RIM=162.00
Š	PROPOSED GAS VALVE	OUT=160.10 12"HDPE
×	PROPOSED WATER VALVE	
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	PROPOSED EDGE OF PAVEMENT PROPOSED RETAINING WALL	
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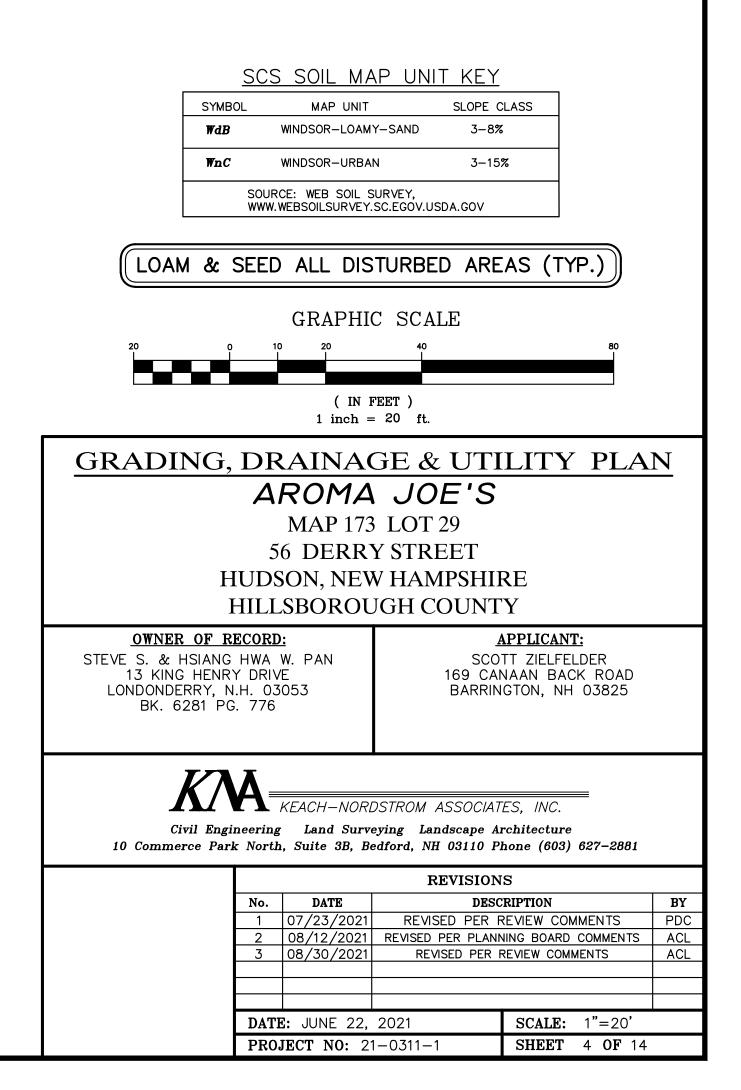


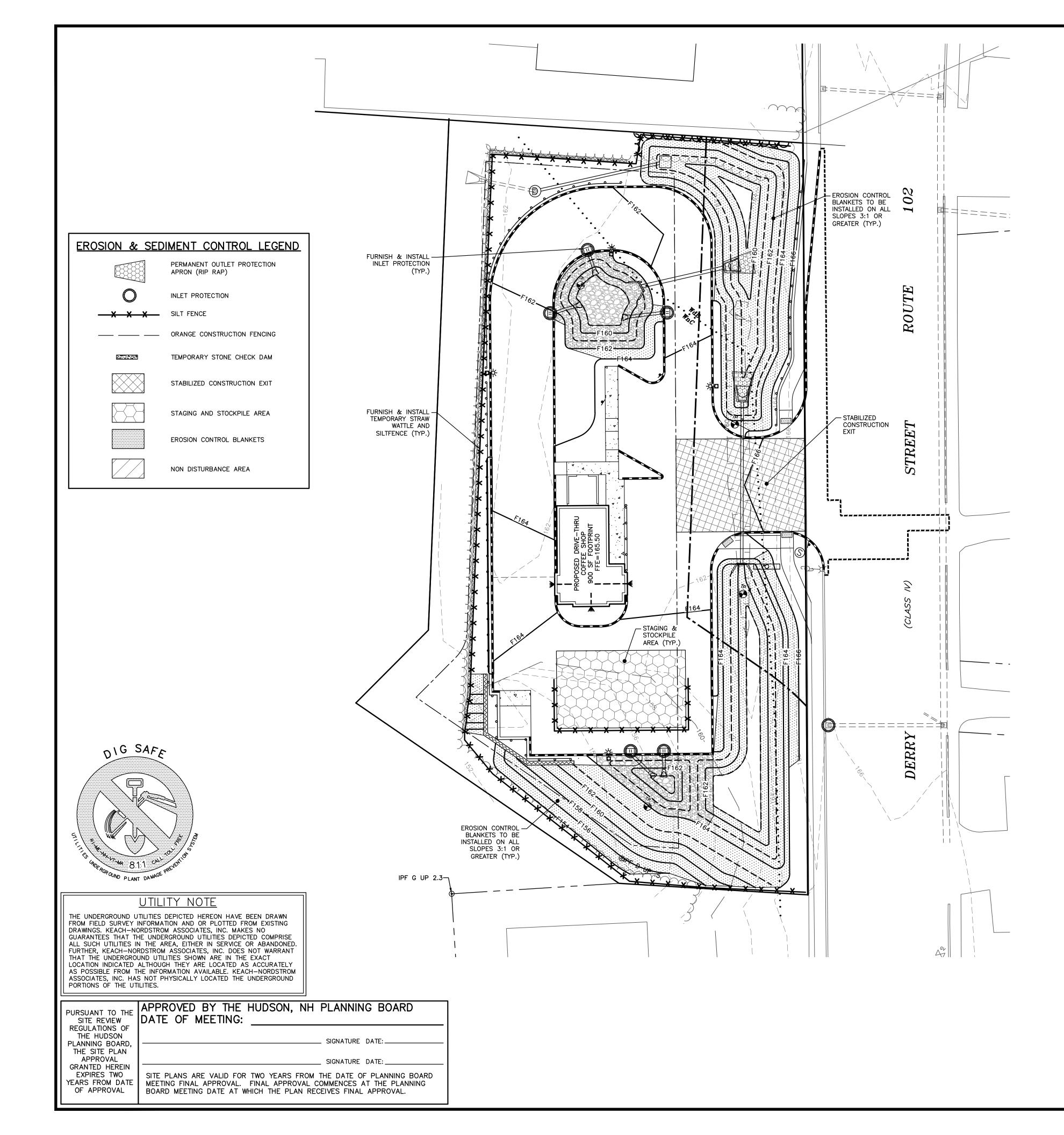
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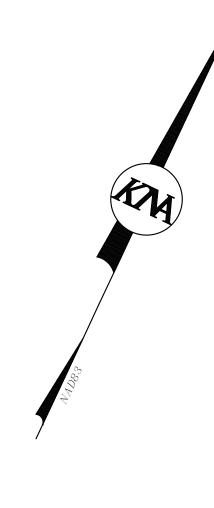


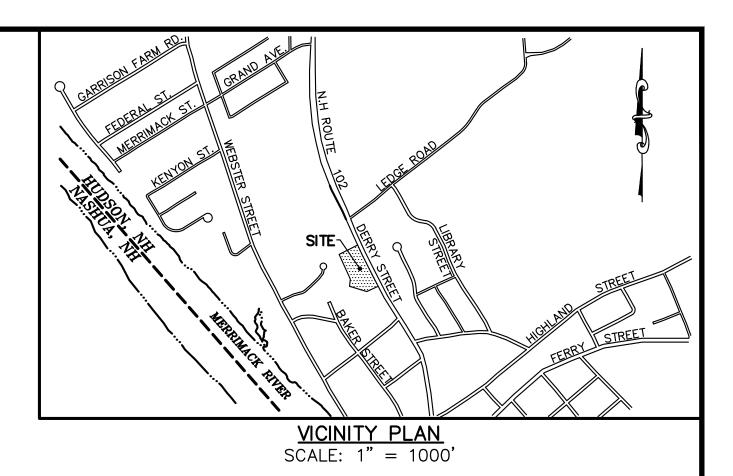
CONSTRUCTION NOTES:

- 1. THE PURPOSE OF THIS PLAN IS TO SHOW THE PROPOSED GRADING, DRAINAGE, AND UTILITY SYSTEMS FOR THIS SITE.
- 2. ALL WORK SHALL CONFORM TO THE APPLICABLE REGULATIONS AND STANDARDS OF THE TOWN OF HUDSON, AND SHALL BE BUILT IN A WORKMANLIKE MANNER IN ACCORDANCE WITH THE PLANS AND SPECIFICATIONS. ALL WORK PERFORMED IN THE NEW HAMPSHIRE DEPARTMENT OF TRANSPORTATION RIGHT-OF-WAY SHALL CONFORM TO THE STANDARD SPECIFICATIONS FOR ROAD AND BRIDGE CONSTRUCTION, STATE OF NEW HAMPSHIRE, DEPARTMENT OF TRANSPORTATION, APPROVED AND ADOPTED 2016 ARE HEREBY INCORPORATED BY REFERENCE.
- 3. CONSTRUCTION SHALL CONFORM TO THE TYPICAL SECTIONS AND DETAILS SHOWN ON THE PLANS, AND SHALL MEET THE REQUIREMENTS AND SPECIFICATIONS FOR ROAD CONSTRUCTION, PUBLIC WORKS DEPARTMENT, HUDSON, NEW HAMPSHIRE. ALL DRAINAGE PIPES SHOWN SHALL BE HDPE.
- 4. THE CONTRACTOR SHALL BE RESPONSIBLE FOR VERIFYING AND DETERMINING THE LOCATION, SIZE AND ELEVATION OF ALL EXISTING UTILITIES, SHOWN OR NOT SHOWN ON THESE PLANS, PRIOR TO THE START OF ANY CONSTRUCTION THE ENGINEER SHALL BE NOTIFIED IN WRITING OF ANY UTILITIES FOUND INTERFERING WITH THE PROPOSED CONSTRUCTION, AND APPROPRIATE REMEDIAL ACTION TAKEN BEFORE PROCEEDING WITH THE WORK. THE CONTRACTOR SHALL BE RESPONSIBLE FOR CONTACTING "DIG SAFE" AT 811 AT LEAST 72 HOURS BEFORE DIGGING.
- 5. ALL DRAINAGE PIPE SHALL BE INSTALLED FOLLOWING MANUFACTURER'S INSTALLATION INSTRUCTIONS.
- 6. THE WATER, SANITARY SEWER, AND ELECTRICAL UTILITIES SHOWN HERE SHALL BE COORDINATED WITH THE FINAL BUILDING PLANS PRIOR TO CONSTRUCTION. THE DESIGN ENGINEER SHALL BE NOTIFIED OF ANY DISCREPANCIES.
- 7. PLANS TO COMPLY WITH THE TOWN OF HUDSON'S MS4 PERMIT.
- 8. OWNER IS SOLELY RESPONSIBLE FOR MAINTAINING THE STORMWATER MANAGEMENT SYSTEM AS OUTLINED IN THE OPERATION & MAINTENANCE PLAN.



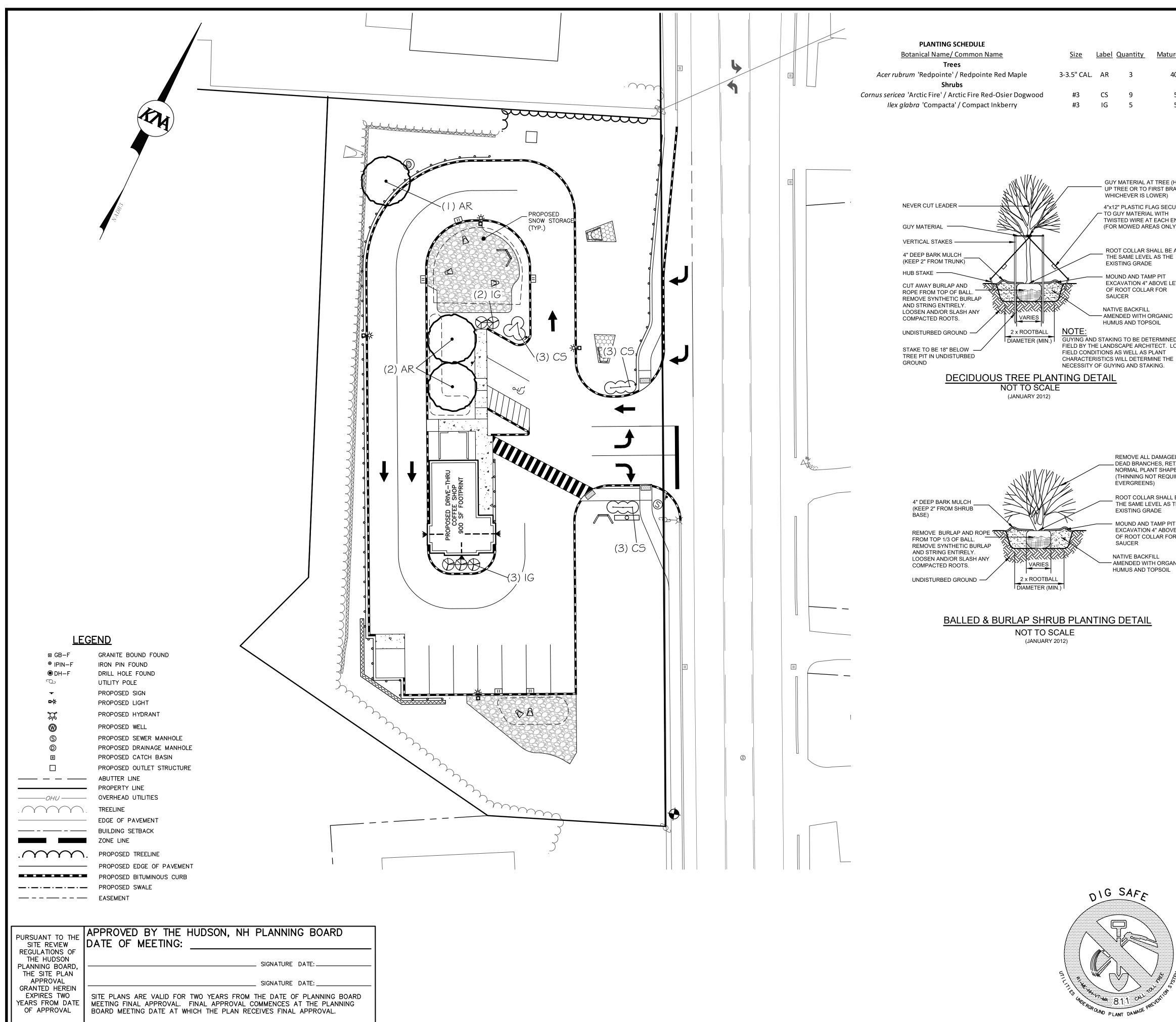






- EROSION CONTROL NOTES: 1. THE PURPOSE OF THIS PLAN IS TO DEPICT THE REQUIRED ONSITE TEMPORARY CONSTRUCTION EROSION CONTROL MEASURES AS WELL AS THE PERMANENT EROSION CONTROL MEASURES.
- 2. ALL MEASURES IN THE PLAN SHALL MEET AS A MINIMUM THE BEST MANAGEMENT PRACTICES SET FORTH IN VOLUME 3 OF THE NEW HAMPSHIRE STORMWATER MANUEL TITLED "EROSION AND SEDIMENT CONTROLS DURING CONSTRUCTION," DATED DECEMBER 2010, AS AMENDED FROM TIME TO TIME
- 3. WHENEVER PRACTICAL, NATURAL VEGETATION SHALL BE RETAINED, PROTECTED OR SUPPLEMENTED. THE STRIPPING OF VEGETATION SHALL BE DONE IN A MANNER THAT MINIMIZES SOIL EROSION.
- 4. APPROPRIATE EROSION AND SEDIMENT CONTROL MEASURES SHALL BE INSTALLED PRIOR TO LAND DISTURBANCE.
- 5. THE AREA OF DISTURBANCE SHALL BE KEPT TO A MINIMUM. DISTURBED AREAS REMAINING IDLE FOR MORE THAN 30 DAYS SHALL BE STABILIZED.
- 6. MEASURES SHALL BE TAKEN TO CONTROL EROSION WITHIN THE PROJECT AREA. SEDIMENT IN RUNOFF WATER SHALL BE TRAPPED AND RETAINED WITHIN THE PROJECT AREA USING APPROVED MEASURES. WETLAND AREAS AND SURFACE WATERS SHALL BE PROTECTED FROM SEDIMENT.
- 7. OFFSITE SURFACE WATER AND RUNOFF FROM UNDISTURBED AREAS SHALL BE DIVERTED AWAY FROM DISTURBED AREAS WHERE FEASIBLE OR CARRIED NON-EROSIVELY THROUGH THE PROJECT AREA. INTEGRITY OF DOWNSTREAM DRAINAGE SYSTEMS SHALL BE MAINTAINED.
- 8. ALL TEMPORARY EROSION AND SEDIMENT CONTROL MEASURES SHALL BE MAINTAINED IN FUNCTIONING CONDITION UNTIL FINAL SITE STABILIZATION IS ACCOMPLISHED.
- 9. ALL TEMPORARY EROSION AND SEDIMENT CONTROL MEASURES SHALL BE REMOVED AFTER FINAL SITE STABILIZATION. TRAPPED SEDIMENT AND OTHER DISTURBED SOIL AREAS RESULTING FROM THE REMOVAL OF TEMPORARY MEASURES SHALL BE PERMANENTLY STABILIZED WITHIN 30 DAYS UNLESS CONDITIONS DICTATE OTHERWISE.
- 10. THE TOWN OF HUDSON SHALL RESERVE THE RIGHT TO REQUIRE FURTHER EROSION CONTROL PRACTICES DURING CONSTRUCTION SHOULD THEY FIND IT NECESSARY.
- 11. INFILTRATION AREAS ARE TO BE PROTECTED FROM OVER-COMPACTION DURING CONSTRUCTION.

LOAM & SEED ALL DISTURBED AREAS (TYP.)							
	10 	20 (IN 1	C SCALE	80			
EROSION CONTROL PLAN AROMA JOE'S MAP 173 LOT 29 56 DERRY STREET HUDSON, NEW HAMPSHIRE HILLSBOROUGH COUNTY							
OWNER OF RECORD:APPLICANT:STEVE S. & HSIANG HWA W. PANSCOTT ZIELFELDER13 KING HENRY DRIVE169 CANAAN BACK ROADLONDONDERRY, N.H. 03053BARRINGTON, NH 03825BK. 6281 PG. 776BARRINGTON, NH 03825							
KEACH-NORDSTROM ASSOCIATES, INC. Civil Engineering Land Surveying Landscape Architecture 10 Commerce Park North, Suite 3B, Bedford, NH 03110 Phone (603) 627-2881							
			REVISION	IS			
	No.	DATE	DESC	CRIPTION	BY		
		07/23/2021		REVIEW COMMENTS	PDC		
		08/12/2021		NING BOARD COMMENTS	ACL		
		08/30/2021	REVISED PER	REVIEW COMMENTS	ACL		
				· · · · ·			
		JUNE 22,		SCALE: 1"=20'			
PROJECT NO: 21–0311–1 SHEET 5 OF 14							



uantity_	<u>Mature Height</u>
3	40-60'
9 5	5-6' 5-6'

GUY MATERIAL AT TREE (HALF UP TREE OR TO FIRST BRANCH, WHICHEVER IS LOWER)

4"x12" PLASTIC FLAG SECURED - TO GUY MATERIAL WITH TWISTED WIRE AT EACH END (FOR MOWED AREAS ONLY)

ROOT COLLAR SHALL BE AT THE SAME LEVEL AS THE EXISTING GRADE

- MOUND AND TAMP PIT EXCAVATION 4" ABOVE LEVEL OF ROOT COLLAR FOR

NATIVE BACKFILL — AMENDED WITH ORGANIC HUMUS AND TOPSOIL

GUYING AND STAKING TO BE DETERMINED IN THE FIELD BY THE LANDSCAPE ARCHITECT. LOCAL

> REMOVE ALL DAMAGED AND _ DEAD BRANCHES, RETAINING NORMAL PLANT SHAPE (THINNING NOT REQUIRED ON EVERGREENS)

ROOT COLLAR SHALL BE AT THE SAME LEVEL AS THE EXISTING GRADE

- MOUND AND TAMP PIT EXCAVATION 4" ABOVE LEVEL OF ROOT COLLAR FOR SAUCER

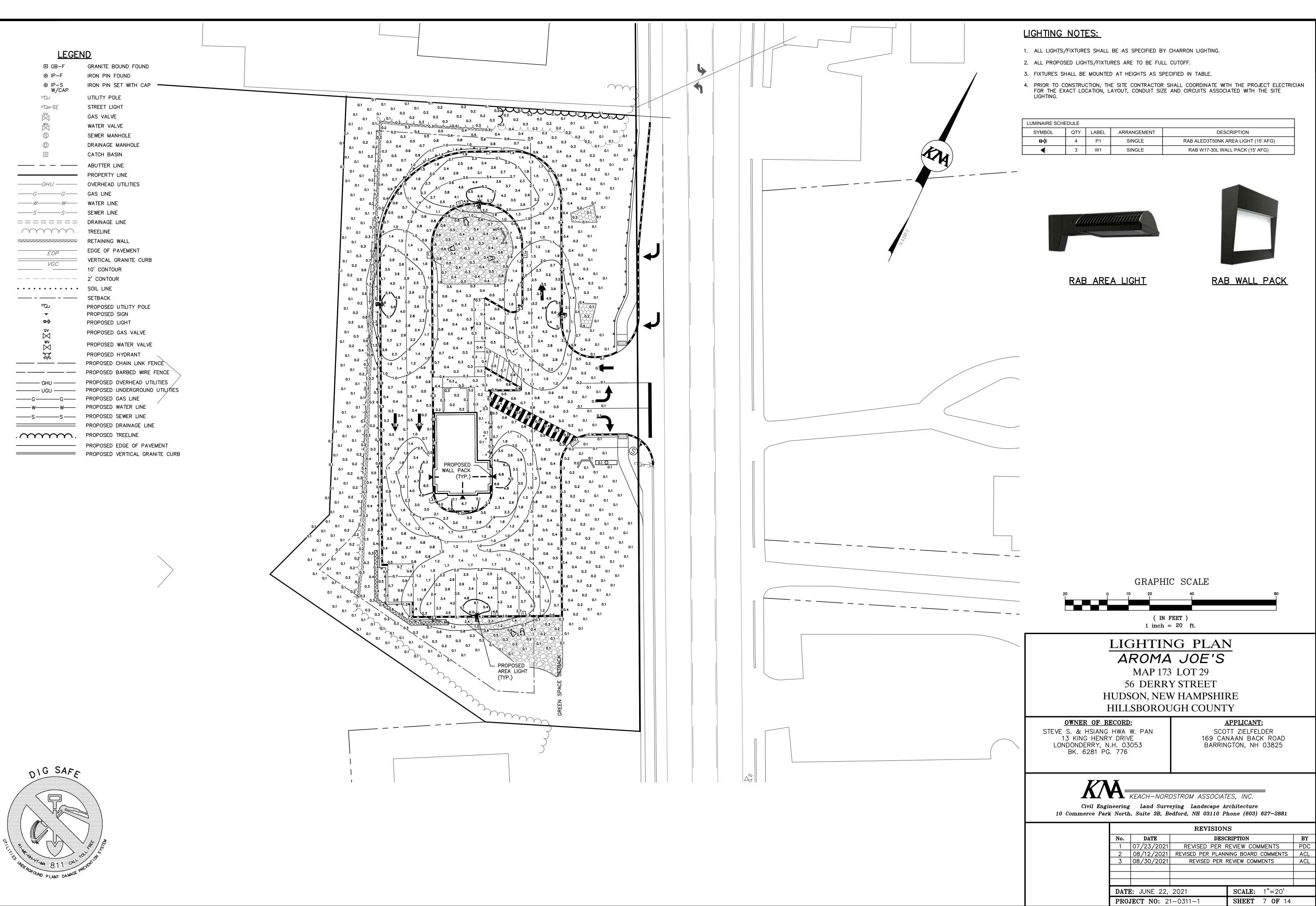
NATIVE BACKFILL - AMENDED WITH ORGANIC HUMUS AND TOPSOIL

LANDSCAPE NOTES: 1. THE PURPOSE OF THIS PLAN IS TO SHOW THE PROPOSED SITE LANDSCAPE WHICH PROVIDES CLIMATIC RELIEF AND AESTHETIC APPEAL.

- 2. ALL PLANT MATERIALS USED SHALL BE NURSERY STOCK AND SHALL BE GUARANTEED FOR A PERIOD OF ONE (1) YEAR FROM DATE OF INSTALLATION. ANY MATERIAL WHICH DIES OR DOES NOT SHOWN HEALTHY APPEARANCE WITHIN THIS TIME SHALL BE REPLACED AT THE CONTRACTOR'S EXPENSE; WITH SAME WARRANTY REQUIREMENTS AS THE ORIGINAL. WARRANTIES TYPICALLY DO NOT COVER LOSS DUE TO INSECT INFESTATION OR MECHANICAL DAMAGE (I.E. SNOW STORAGE).
- 3. IF THE SOIL CONDITIONS ARE EXTREMELY SANDY, ALL TREES SHALL HAVE A 6" LAYER OF COMPACTED TOPSOIL PLACED IN THE BASE OF THE PLANT PIT AS A MOISTURE RETENTION LAYER. THE PLANT PIT SIDEWALLS SHALL BE OVER EXCAVATED BY AN ADDITIONAL 12" BEYOND THE NORMAL OUTSIDE RADIUS OF THE HOLE. A TOPSOIL MIXTURE SHALL BE USED TO BACKFILL THE HOLE AS FOLLOWED; ORGANIC TOPSOIL, AMENDED WITH 10% WOOD ASH, 10% MANURE, 30% PEATMOSS AND A GRANULAR HYDROGEL TO ABSORB AND RETAIN WATER.
- 4. PLANTING BEDS AND SAUCERS SHALL RECEIVE A 4" MINIMUM THICKNESS OF PINE/HEMLOCK BARK MULCH OVER A 5oz. POLYPROPYLENE WEED CONTROL FABRIC.
- 5. PAVEMENT AND ROAD BASE MATERIAL ENCOUNTERED IN ANY LAWN OR PLANTING BED SHALL BE REMOVED AND DISPOSED OF BY THE CONTRACTOR AND SUITABLE AMENDED SOIL INSTALLED AS SPECIFIED IN THE TURF ESTABLISHMENT SCHEDULE.
- 6. PLANT TYPES SHOWN ARE SUBJECT TO AVAILABILITY. SUBSTITUTE MATERIALS CAN BE IMPLEMENTED WITH APPROVAL FROM KEACH NORDSTROM ASSOCIATES PRIOR TO CONSTRUCTION.

LANDSCAPE CALCULATIONS	
REQUIRED PARKING LOT INTERIOR LANDSCAPE PROPOSED PARKING AREA PAVED:	5,151 SF
10% REQUIRED LANDSCAPE AREA: PROVIDED LANDSCAPE AREA:	515 SF 1,496 SF
REQUIRED PARKING LOT SHADE TREES AND SI	
PROPOSED PAVED AREA: SHADE TREES REQUIRED (5,151/1,600):	5,151 SF 3 TREES REQUIRED
(OR 1 TREE/5 PROP. PARKING SPACES) SHADE TREES PROVIDED:	2 TREES REQUIRED 3 TREES PROPOSED
SHRUBS REQUIRED (5,151/200): (OR 1.6 x 9 PROP. PARKING SPACES)	26 SHRUBS, OR 14 SHRUBS REQUIRED
SHRUBS PROVIDED:	14 SHRUBS PROPOSED

AIL	LOAM & SEED ALL DISTURBED AREAS (TYP.)					
	GRAPHIC SCALE 20 0 10 20 40 80 (IN FEET) 1 inch = 20 ft.					
	LANDSCAPE PLAN AROMA JOE'S MAP 173 LOT 29 56 DERRY STREET HUDSON, NEW HAMPSHIRE HILLSBOROUGH COUNTY					
	OWNER OF RECORD:APPLICANT:STEVE S. & HSIANG HWA W. PANSCOTT ZIELFELDER13 KING HENRY DRIVE169 CANAAN BACK ROADLONDONDERRY, N.H. 03053BARRINGTON, NH 03825BK. 6281 PG. 776BARRINGTON, NH 03825					
E	KEACH-NORDSTROM ASSOCIATES, INC. Civil Engineering Land Surveying Landscape Architecture 10 Commerce Park North, Suite 3B, Bedford, NH 03110 Phone (603) 627-2881					
ALE PRETERINO	No.DATEDESCRIPTIONBY107/23/2021REVISED PER REVIEW COMMENTSPDC208/12/2021REVISED PER PLANNING BOARD COMMENTSACL308/30/2021REVISED PER REVIEW COMMENTSACL					
AGE PRECY	DATE: JUNE 22, 2021 SCALE: 1"=20' PROJECT NO: 21-0311-1 SHEET 6 OF 14					

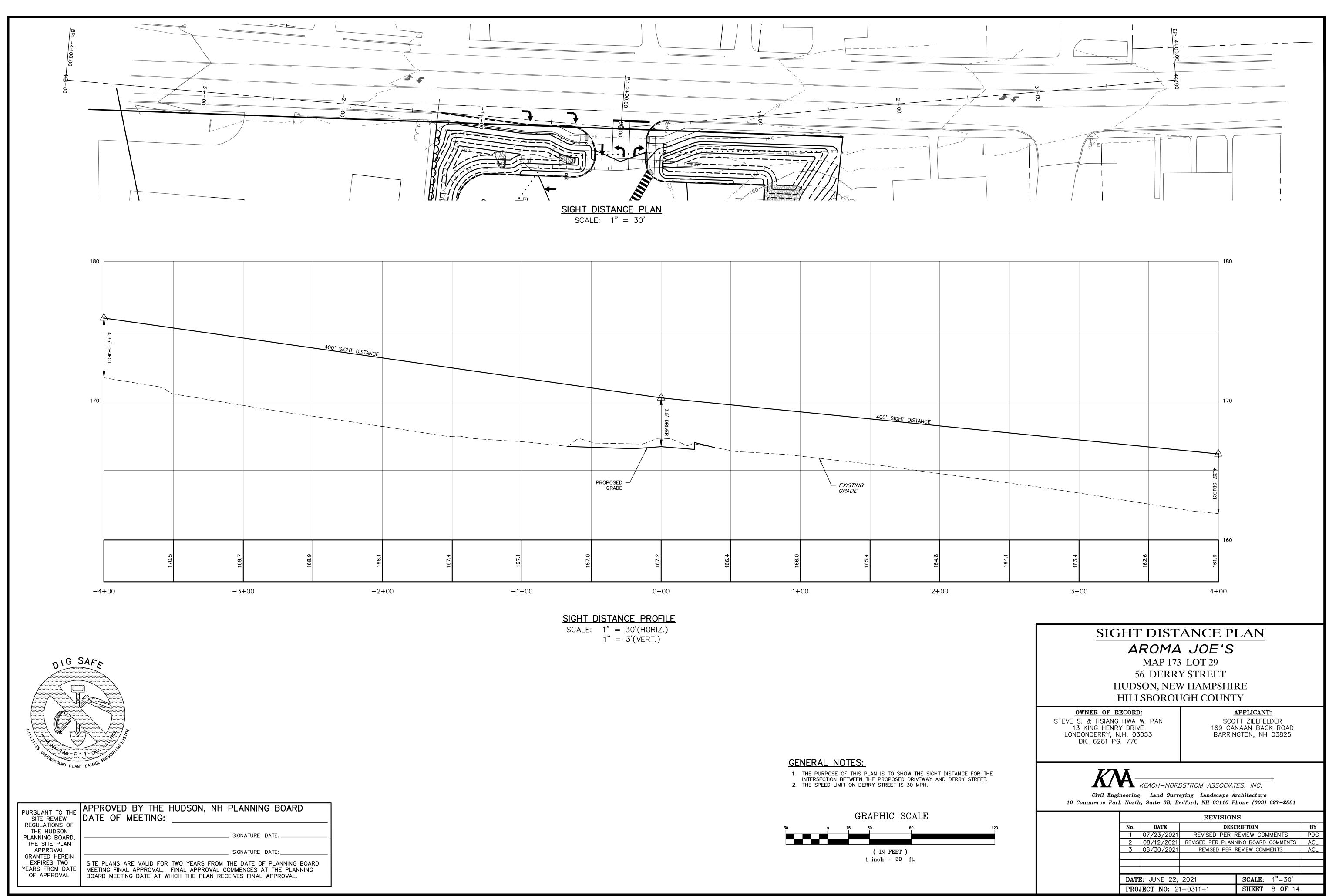


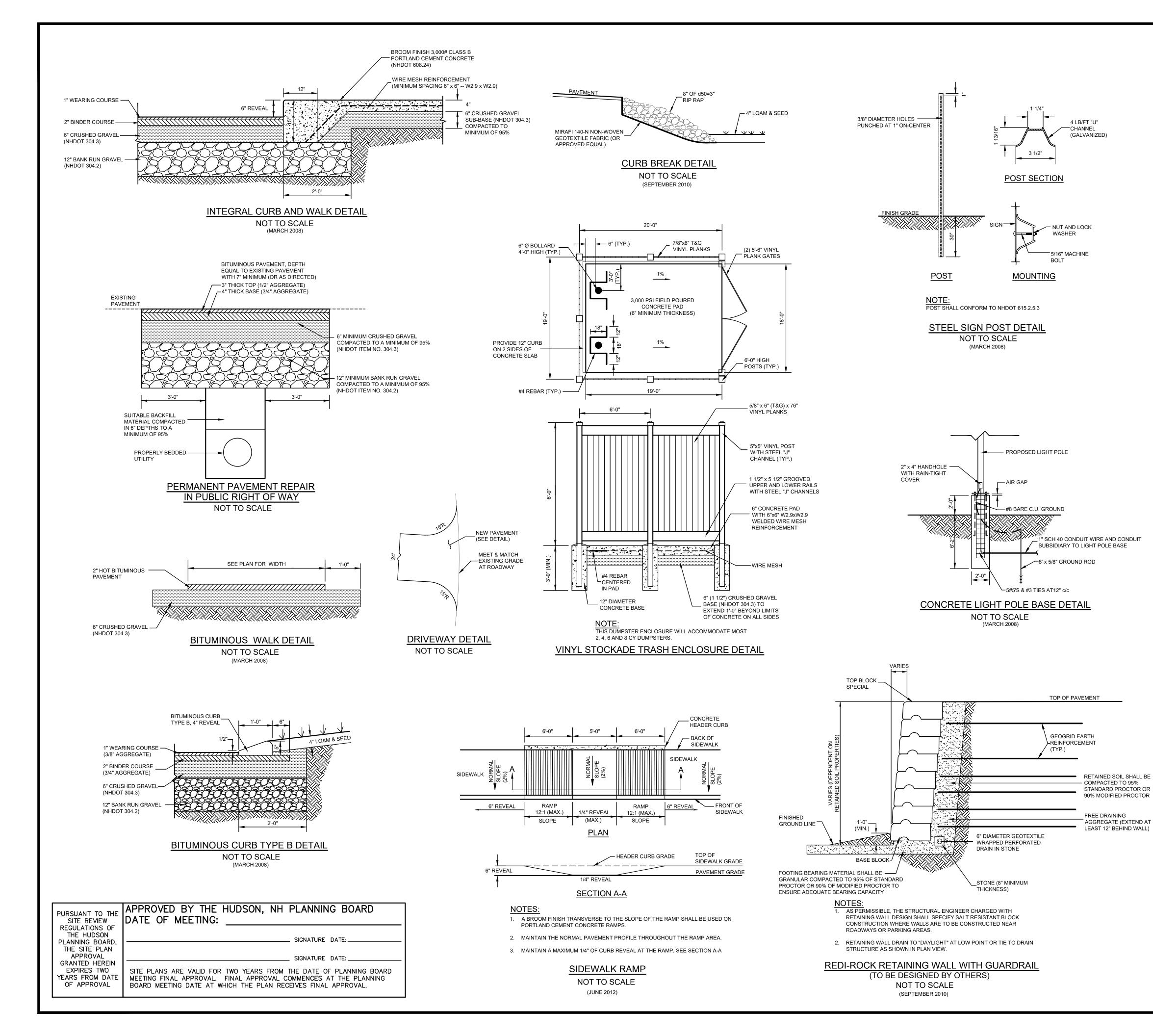


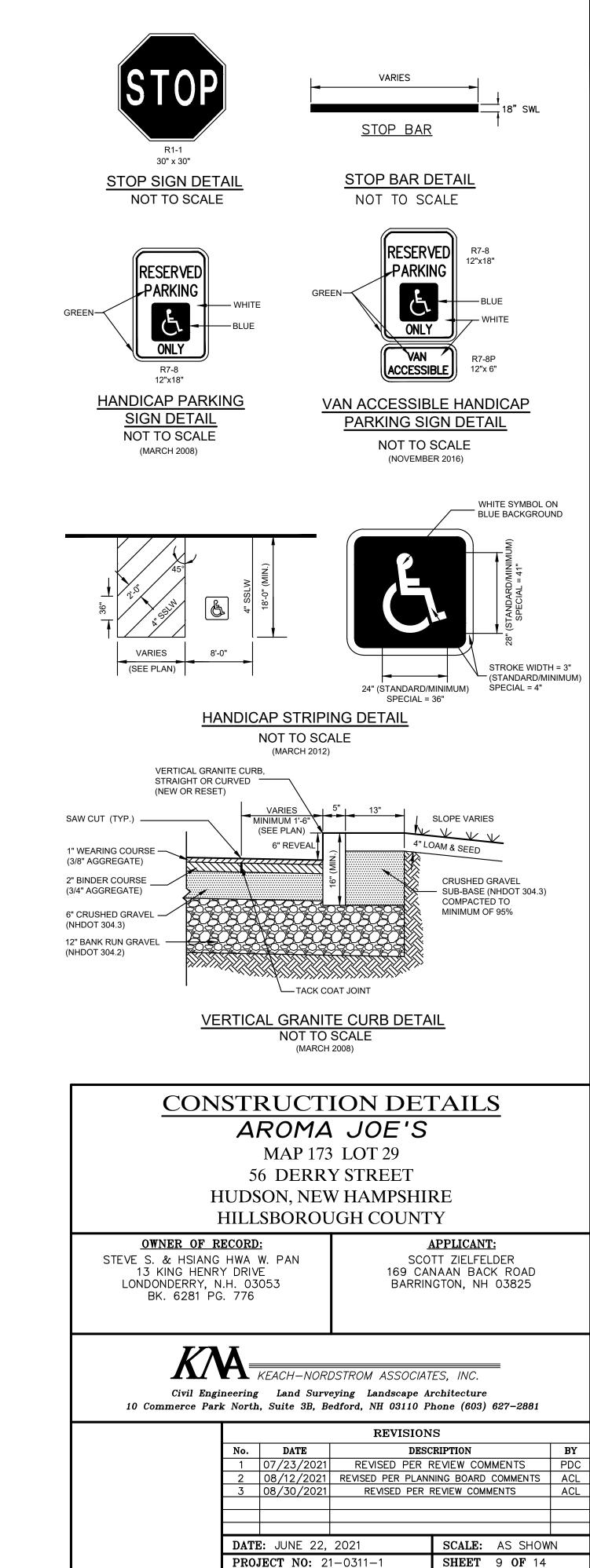
LUMINAIRE SCHEDULE				
SYMBOL	QTY	LABEL	ARRANGEMENT	DESCRIPTION
₽₩	4	P1	SINGLE	RAB ALED3T50NK AREA LIGHT (15' AFG)
€	3	W1	SINGLE	RAB W17-30L WALL PACK (15' AFG)

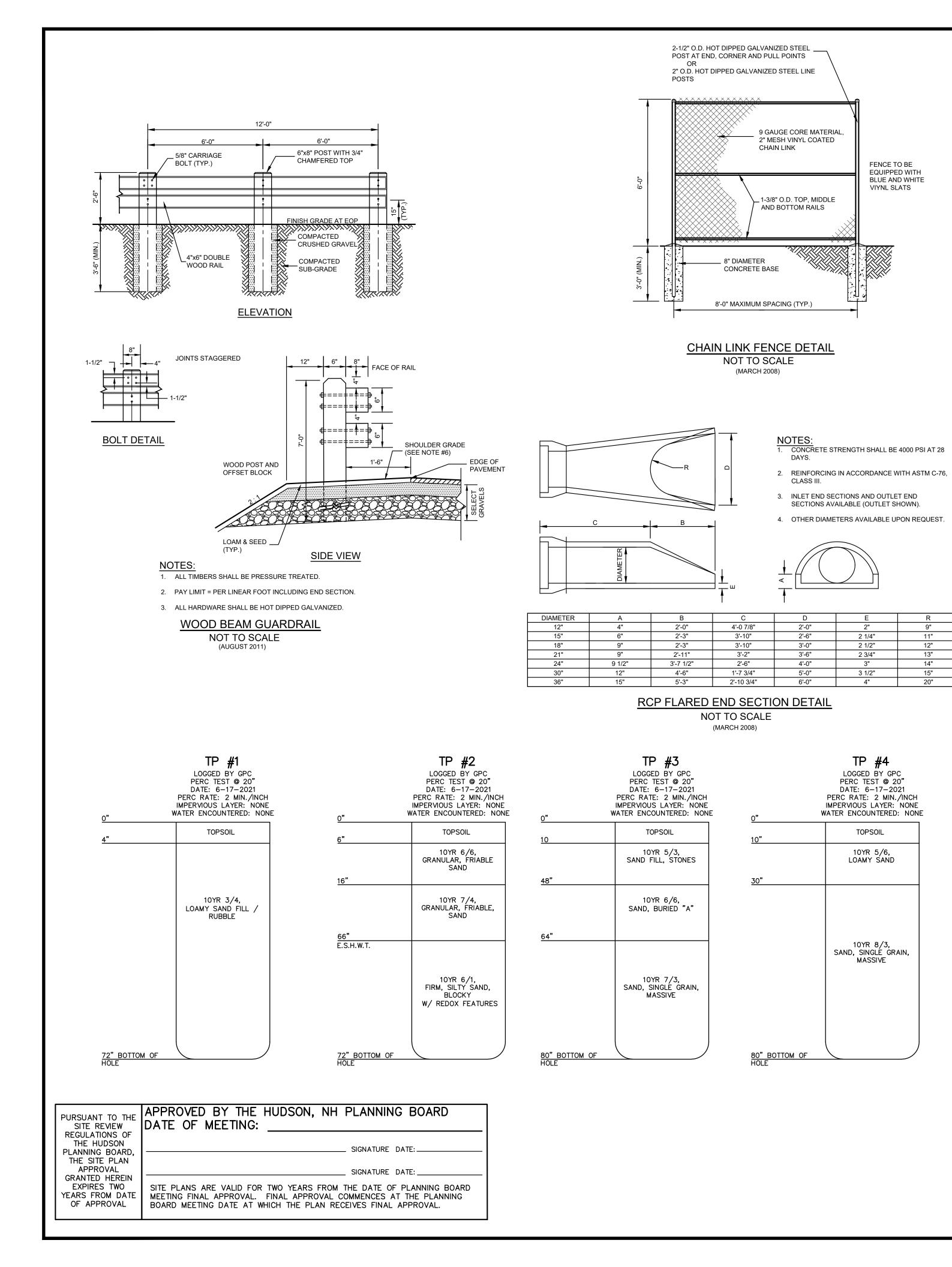


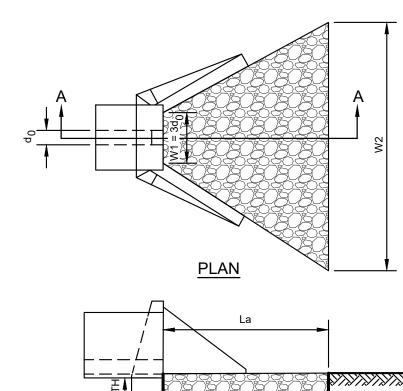


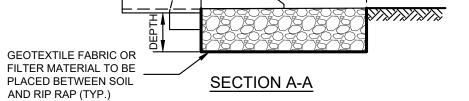






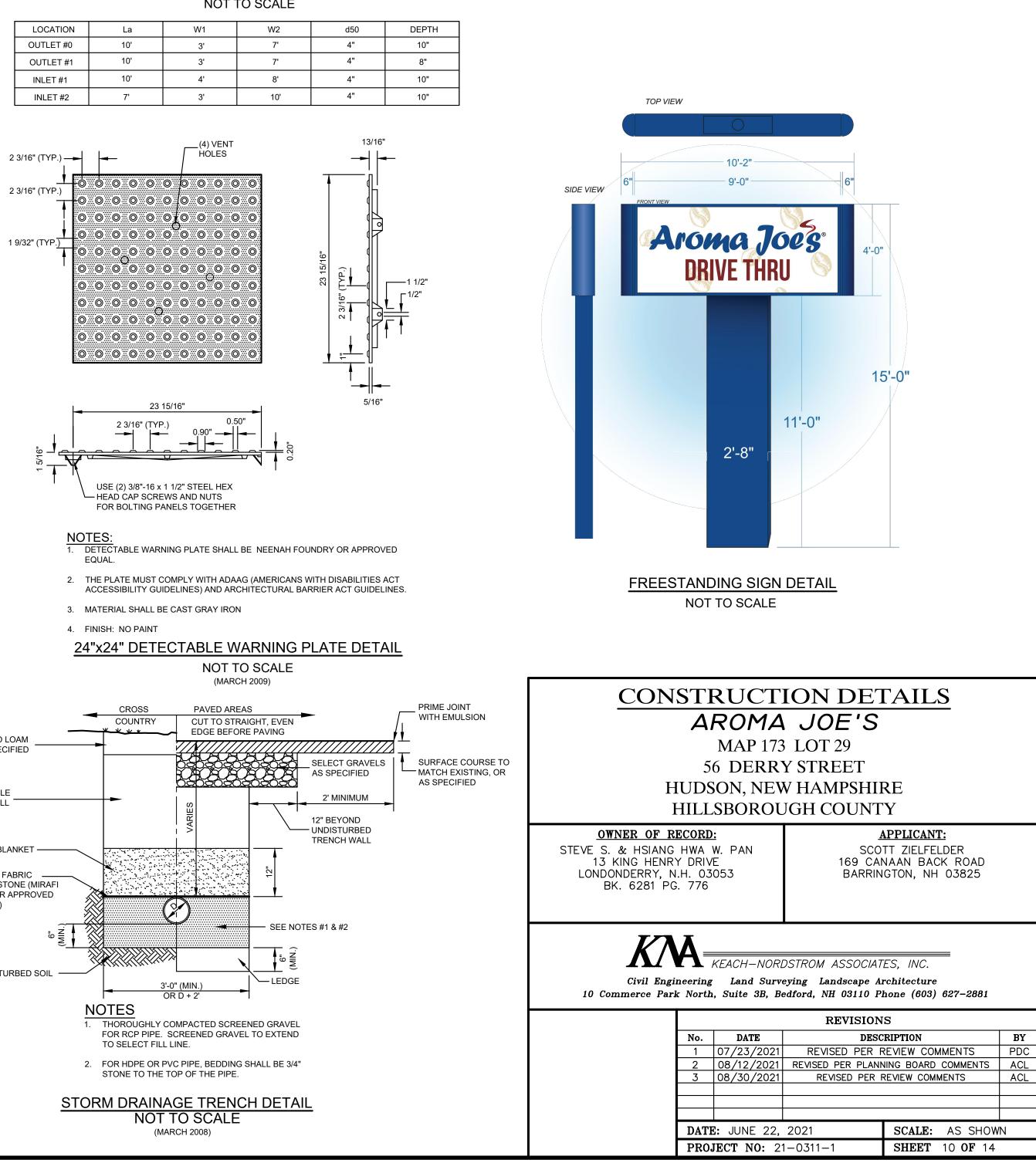






PIPE OUTLET TO FLAT AREA WITH NO DEFINED CHANNEL NOT TO SCALE

LOCATION	La	W1	W2	d50	
OUTLET #0	10'	3'	7'	4"	
OUTLET #1	10'	3'	7'	4"	
INLET #1	10'	4'	8'	4"	
INLET #2	7'	3'	10'	4"	



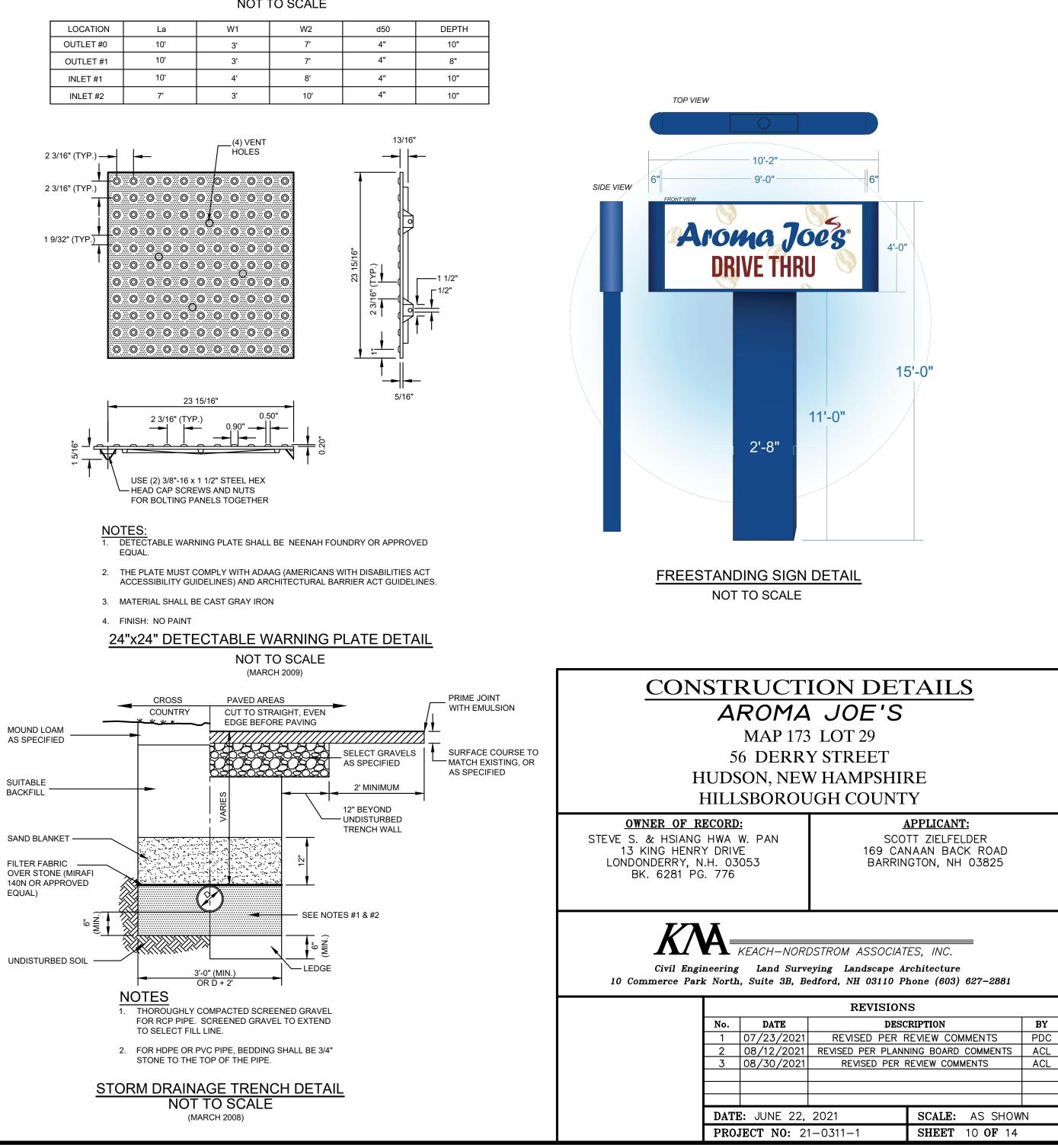


TABLE 7-24 RECOMMENDED RIP RAP GRADATION RANGES		
PERCENT OF WEIGHT SMALLER THAN THE <u>GIVEN SIZE</u>	SIZE OF STONE	
100% 85% 50% 15%	1.5 TO 2.0 d50 1.3 TO 1.8 d50 1.0 TO 1.5 d50 0.3 TO 0.5 d50	

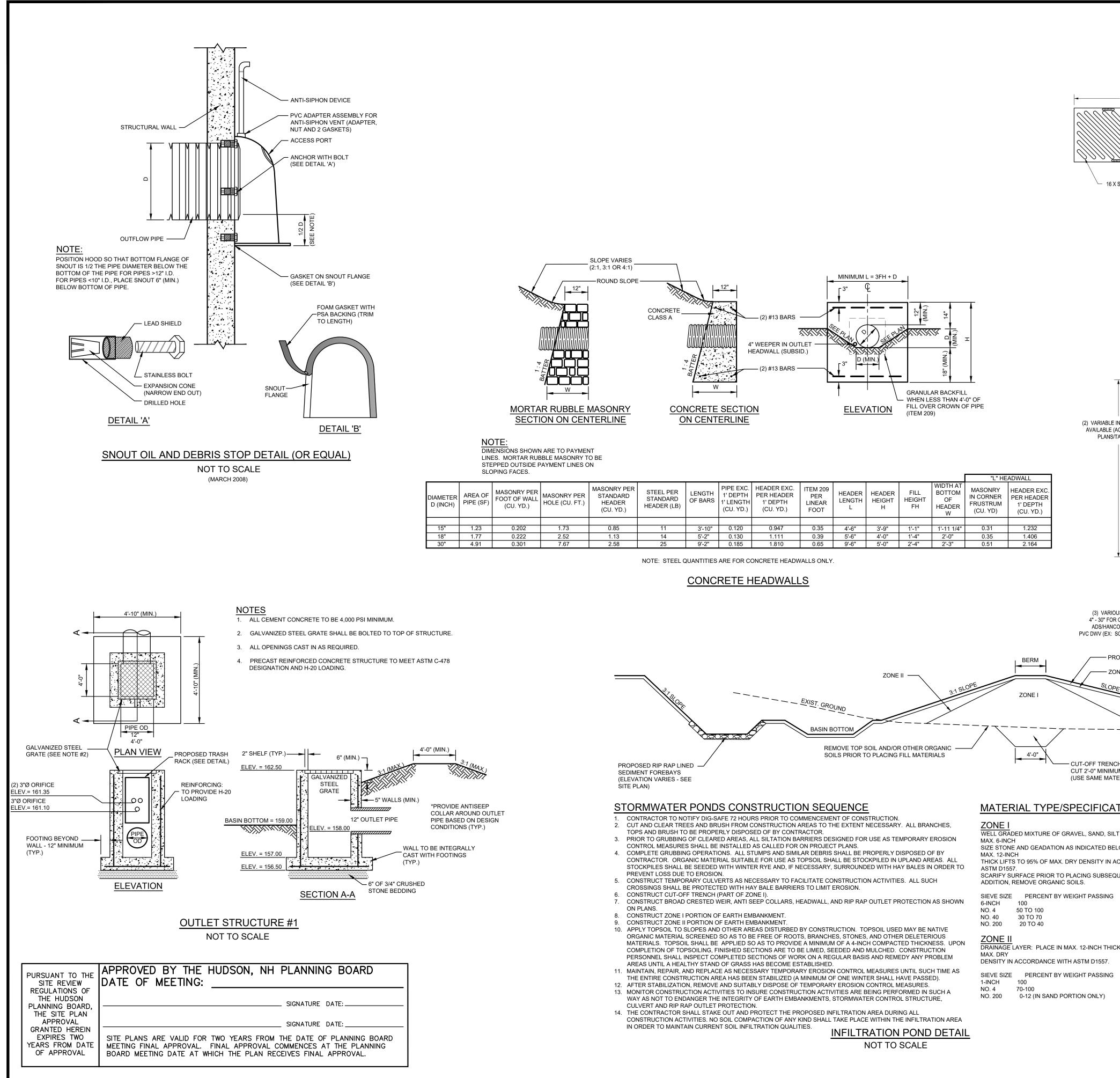
CONSTRUCTION SPECIFICATIONS:

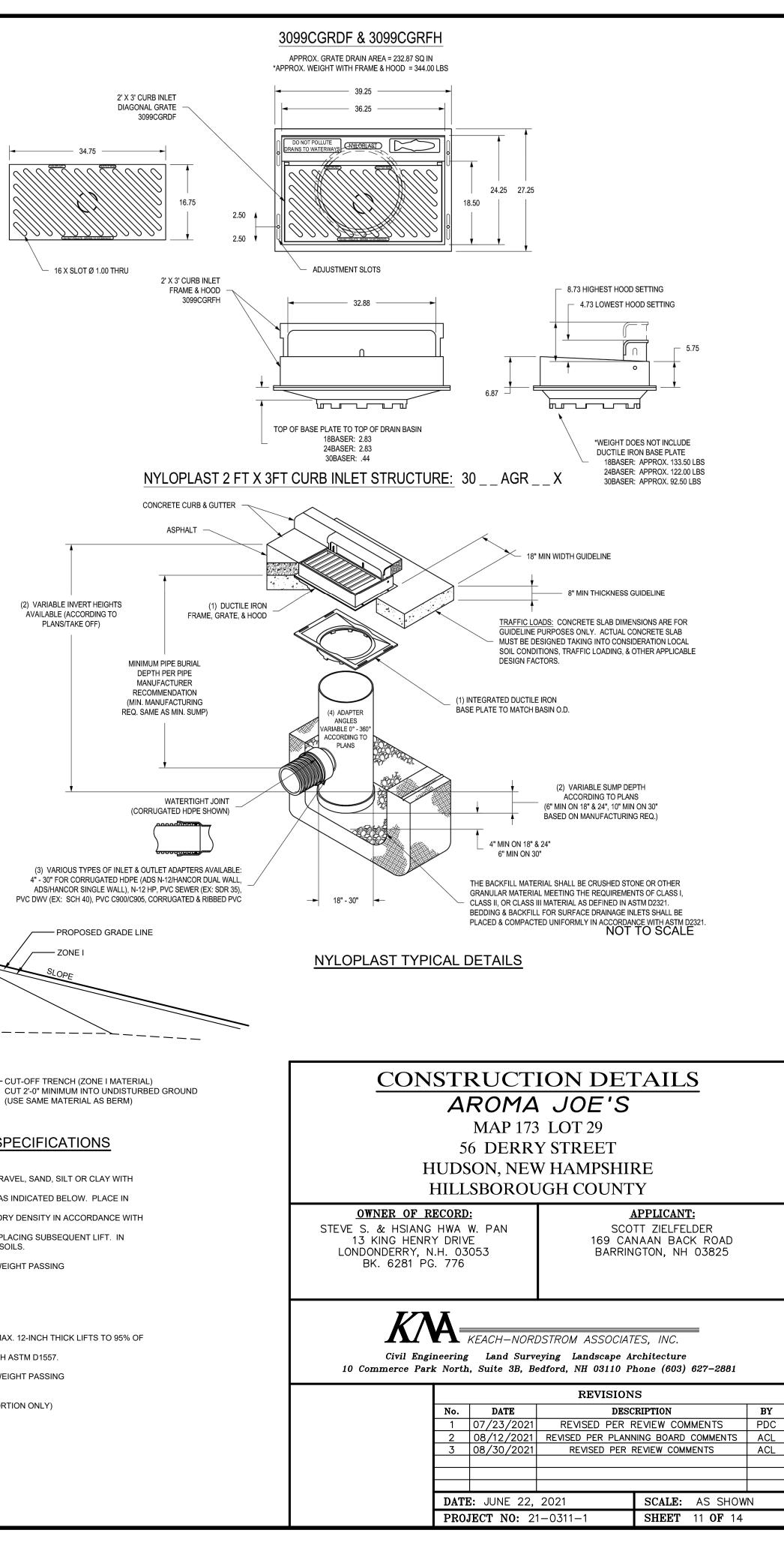
1. THE SUBGRADE FOR THE FILTER MATERIAL, GEOTEXTILE FABRIC AND RIP RAP SHALL BE PREPARED TO THE LINES AND GRADES SHOWN ON THE PLANS.

- 2. THE ROCK OR GRAVEL USED FOR FILTER OR RIP RAP SHALL CONFORM TO THE SPECIFIED GRADATION.
- 3. GEOTEXTILE FABRICS SHALL BE PROTECTED FROM PUNCTURE OR TEARING DURING THE PLACEMENT OF THE ROCK RIP RAP. DAMAGED AREAS IN THE FABRIC SHALL BE REPAIRED BY PLACING A PIECE OF FABRIC OVER THE DAMAGED AREA OR BY COMPLETE REPLACEMENT OF THE FABRIC. ALL OVERLAPS REQUIRED FOR REPAIRS OR JOINING TWO PIECES OF FABRIC SHALL BE A MINIMUM OF 12 INCHES.
- 4. STONE FOR THE RIP RAP MAY BE PLACED BY EQUIPMENT AND SHALL BE CONSTRUCTED TO THE FULL LAYER THICKNESS IN ONE OPERATION AND IN SUCH A MANNER AS TO PREVENT SEGREGATION OF THE STONE SIZES.

MAINTENANCE:

THE OUTLET PROTECTION SHOULD BE CHECKED AT LEAST ANNUALLY AND AFTER EVERY MAJOR RAIN EVENT. IF THE RIP RAP HAS BEEN DISPLACED, UNDERMINED, OR DAMAGED, IT SHOULD BE REPAIRED IMMEDIATELY. THE CHANNEL IMMEDIATELY BELOW THE OUTLET SHOULD BE CHECKED TO SEE THAT EROSION IS NOT OCCURRING. THE DOWNSTREAM CHANNEL SHOULD BE KEPT CLEAR OF OBSTRUCTIONS SUCH AS FALLEN TREES, DEBRIS, AND SEDIMENT THAT COULD CHANGE FLOW PATTERNS AND/OR TAILWATER DEPTHS ON THE PIPES. REPAIRS MUST BE CARRIED OUT IMMEDIATELY TO AVOID ADDITIONAL DAMAGE TO THE OUTLET PROTECTION APRON.





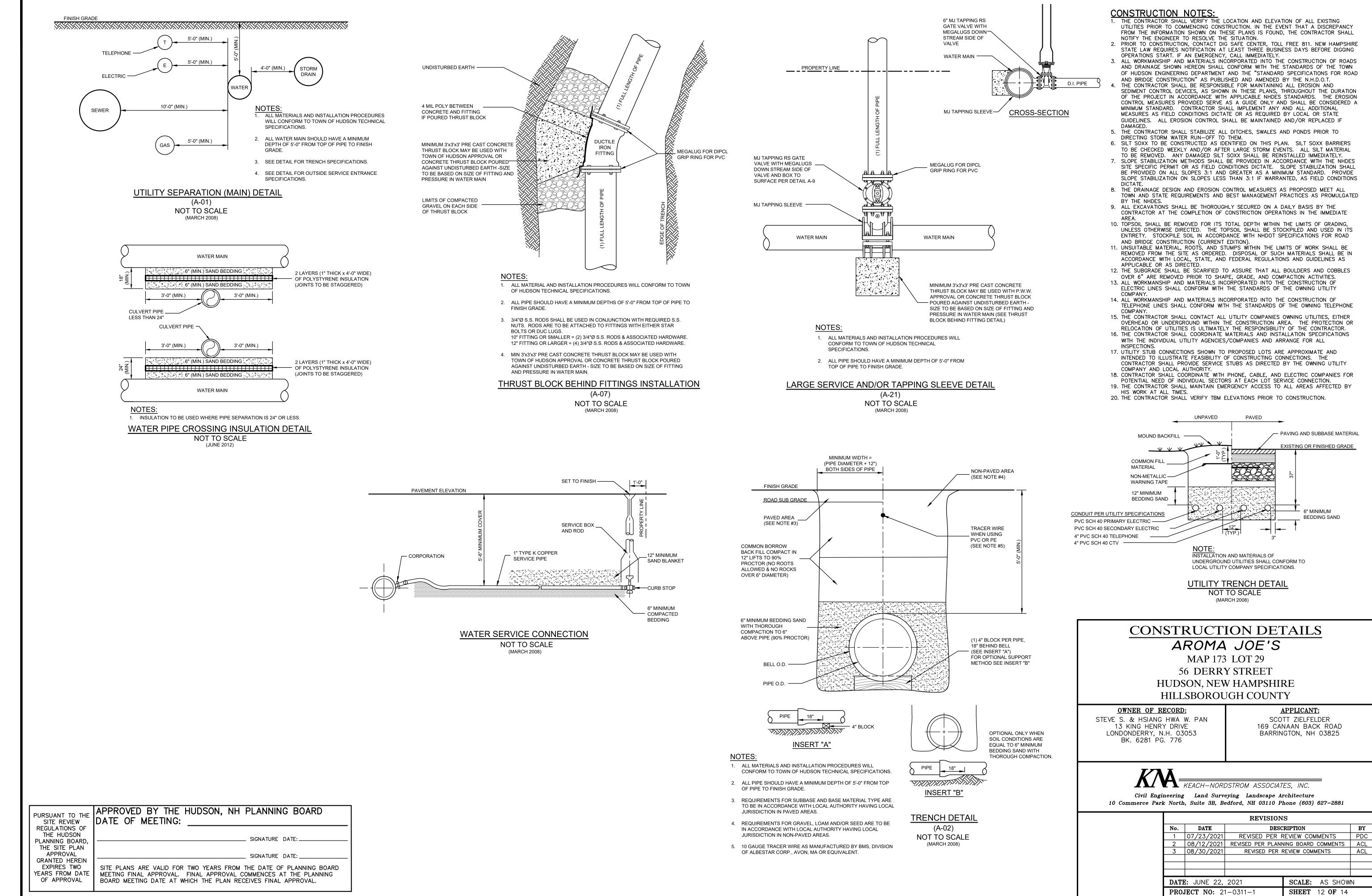
MATERIAL TYPE/SPECIFICATIONS

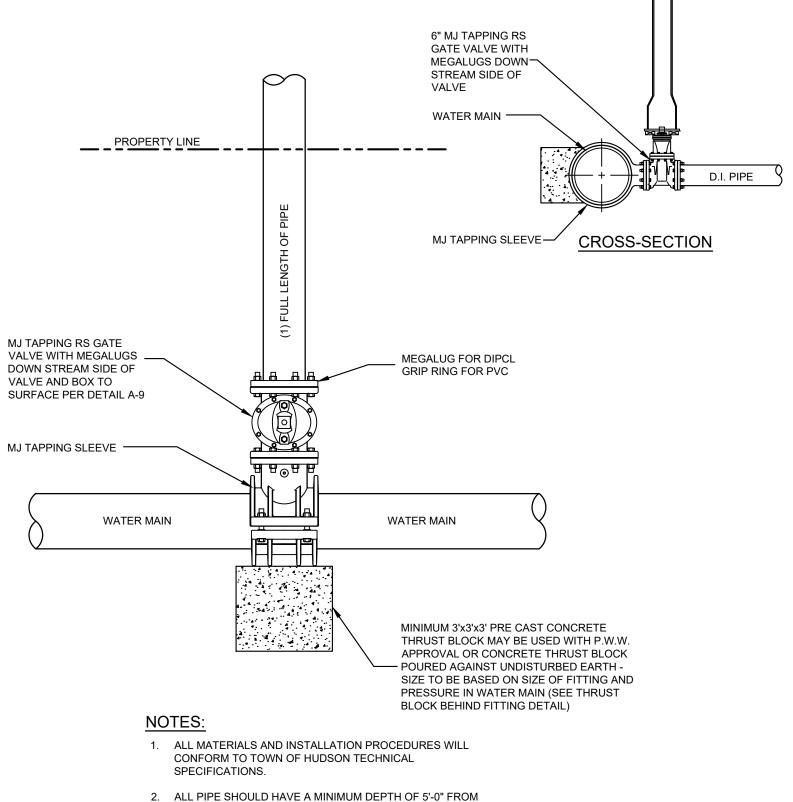
WELL GRADED MIXTURE OF GRAVEL, SAND, SILT OR CLAY WITH SIZE STONE AND GEADATION AS INDICATED BELOW. PLACE IN THICK LIFTS TO 95% OF MAX. DRY DENSITY IN ACCORDANCE WITH SCARIFY SURFACE PRIOR TO PLACING SUBSEQUENT LIFT. IN

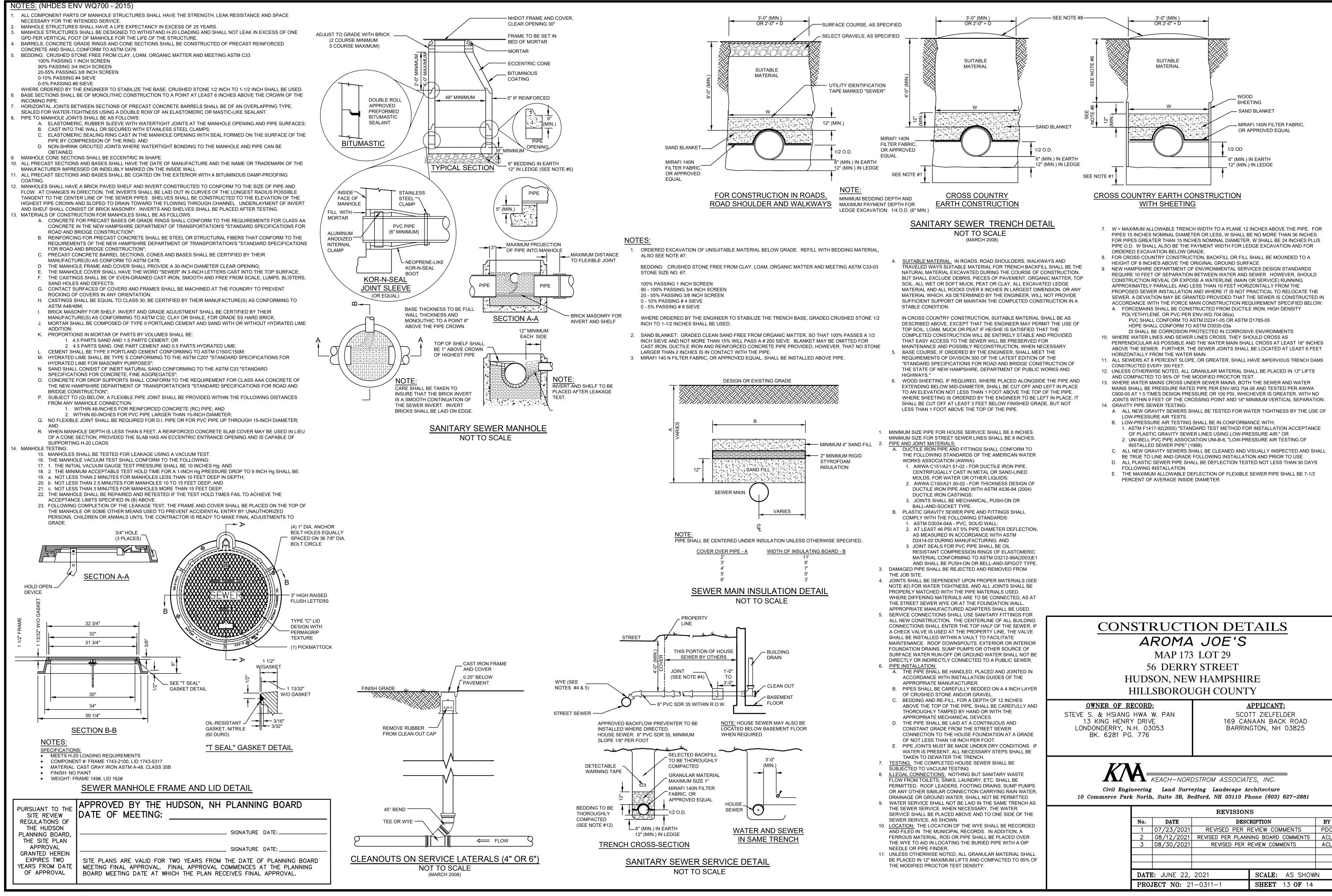
SIEVE SIZE	PERCENT BY WEIGHT PASSING	
6-INCH	100	
NO. 4	50 TO 100	
NO. 40	30 TO 70	
NO. 200	20 TO 40	

RAINAGE LAYER: PLACE IN MAX. 12-INCH THICK LIFTS TO 95% OF

SIEVE SIZE	PERCENT BY WEIGHT PASSING
1-INCH	100
NO. 4	70-100
NO 200	0-12 (IN SAND PORTION ONLY)







TURF ESTABLISHMENT SCHEDULE

PURPOSE

TO ESTABLISH AND MAINTAIN PERMANENT AND TEMPORARY TURF AREAS, RESTORE GROWTH TO EXISTING TURF AREAS DISTURBED DURING CONSTRUCTION AND CONTROL SOIL EROSION.

PREPARATION AND EXECUTION:

- RAKE THE SUBGRADE OF ALL AREAS TO BE LOAMED AND SEEDED TO REMOVE RUBBISH, STICKS, ROOTS AND STONES LARGER THAN 1 INCH PLACE LOAM OVER AREAS TO BE SEEDED AND SPREAD.
- FINE GRADE SURFACE AND SUPPLEMENT WITH SUITABLE LOAM WHERE NEEDED TO CREATE A UNIFORM SURFACE ACCORDING TO THE FINISH GRADES INDICATED; TOP AND BOTTOM OF SLOPES SHALL BE ROUNDED. NO LOAM SHALL BE SPREAD IF THE SUBGRADE IS EXCESSIVELY WET OR FR07FN
- 4. IF THE pH OF THE SOIL NEEDS TO BE RAISED, APPLY LIME EVENLY OVER LOAM SURFACE AND THOROUGHLY INCORPORATE LIME INTO THE LOAM BY HEAVY RAKING TO AT LEAST ONE-HALF THE DEPTH OF THE LOAM. APPLY FERTILIZER AND MIX WITH THE UPPER 2 INCHES OF LOAM.
- DETERMINE APPROPRIATE MIXTURE FOR AREA TO BE SEEDED BASED ON EXAMINATION OF PROJECT PLANS. UNIFORMLY SPREAD THE SEED BY BROADCASTING OR HYDROSEEDING. IF BROADCASTING. LIGHTLY RAKE INTO THE PREPARED SURFACE AND ROLL. IF. HYDROSEEDING. USE 4 TIMES THE RECOMMENDED RATE OF INOCULANT. AFTER SEED IS SPREAD, WATER THOROUGHLY WITH A FINE SPRAY
- SEEDING AND INITIAL FERTILIZING SHALL BE DONE BETWEEN APRIL 1 AND JUNE 1 OR BETWEEN AUGUST 15 AND OCTOBER 14, OR AS PERMITTED. SEEDING SHALL NOT BE DONE DURING WINDY WEATHER OR WHEN THE GROUND IS FROZEN, EXCESSIVELY WET OR OTHERWISE UNTILLABLE.
- WITHIN 24 HOURS AFTER SEEDING OPERATION, UNIFORMLY MULCH THE AREA WITH STRAW. ANCHOR MULCH ON ALL SLOPES EXCEEDING 3 : 1 USING MULCH NETTING INSTALLED IN ACCORDANCE WITH THE MANUFACTURER.
- PROTECT AND PREVENT AGAINST WASHOUTS, ANY WASHOUTS WHICH OCCUR SHALL BE PROMPTLY REGRADED AND RESEEDED.
- 10. WHEN IT IS IMPRACTICAL TO ESTABLISH PERMANENT GROWTH ON DISTURBED EARTH BY OCTOBER 14, A TEMPORARY SEED MIXTURE SHALL BE USED. WHEN TEMPORARY SEEDING CANNOT ESTABLISH VISIBLE GROWTH, THE DISTURBED AREA SHALL BE COVERED WITH SIX INCHES OF MULCH FOR THE WINTER

MAINTENANCE

ALL SEEDED AREAS SHALL BE KEPT WATERED AND IN GOOD CONDITION. RESEED AS NECESSARY TO ESTABLISH HEALTHY UNIFORM GROWTH OVER THE ENTIRE SEEDED AREA. MAINTAIN SEEDED AREAS IN AN APPROVED CONDITION UNTIL FINAL ACCEPTANCE. MAINTENANCE SHALL INCLUDE REPAIRS FOR DAMAGE CAUSED BY EROSION.

APPLICATION RATES

LOAM SHALL BE APPLIED AT A MINIMUM COMPACTED THICKNESS OF 4 INCHES. 2. LIME SHALL BE USED WHEN NECESSARY TO RAISE THE pH OF THE SOIL AND APPLIED AT ONE OF THE FOLLOWING RATES

EXISTING SOIL Ph	TONS/ACRE	POUNDS/CUBIC YARD
4.0 - 4.4	3	1.2
4.5 - 4.9	2	0.8
5.0 - 5.4	1	0.4

3. FERTILIZER SHALL BE APPLIED AT THE FOLLOWING RATE:

		-
INITIAL APPLICATION	POUNDS/1,000 SF	MEASUREMENT FACTOR
10-10-10	20.0	1.0
15-15-15	13.4	1.5
19-19-19	10.5	1.9
REFERTILIZATION	POUNDS/1,000 SF	MEASUREMENT FACTOR
10-3-6	20.0	1.0
12-2-8	16.7	1.2
12-4-8	16.7	1.2

4. MULCH SHALL BE APPLIED AT A RATE OF 13 CUBIC YARDS PER 1,000 S.F. OF LANDSCAPE BED. MATERIALS:

- LOAM SHALL CONSIST OF LOOSE, FRIABLE TOPSOIL WITH NO ADMIXTURE OF REFUSE OR MATERIAL TOXIC TO PLANT GROWTH. LOAM SHALL BE FREE OF VIABLE PARTS OF PROHIBITED INVASIVE PLANTS AND BE GENERALLY FREE OF STONES, LUMPS, STUMPS AND SIMILAR OBJECTS LARGER THAN 2 INCHES IN GREATEST DIAMETER, SUBSOIL, ROOTS AND WEEDS. THE MINIMUM AND MAXIMUM pH VALUE SHALL BE FROM 5.5 TO 7.6.
- LIME SHALL BE A CALCIC OR DOLOMITIC GROUND AGRICULTURAL LIMESTONE CONTAINING NOT LESS THAN 95% OF EITHER CALCIUM OR MAGNESIUM CARBONATE, OR BOTH, IT SHALL CONFORM TO THE STANDARDS OF THE ASSOCIATION OF OFFICIAL AGRICULTURAL CHEMISTS AND SHALL COMPLY WITH ALL STATE AND FEDERAL RULES AND REGULATIONS.
- FERTILIZER SHALL BE STANDARD COMMERCIAL GRADE FERTILIZER CONFORMING TO ALL STATE AND FEDERAL RULES AND REGULATIONS AND TO THE STANDARDS OF THE ASSOCIATION OF OFFICIAL AGRICULTURAL CHEMISTS. EXCEPT AS PERMITTED, THE ANALYSIS RATIO SHALL BE 1:1:1 FOR INITIAL APPLICATION AND 3:1:2 FOR REFERTILIZATION APPLICATION.
- GRASS SEED SHALL MEET THE REQUIREMENTS OF THE NEW HAMPSHIRE AGRICULTURAL AND VEGETABLE SEED LAWS AND SHALL INCLUDE NO "PRIMARY NOXIOUS WEED SEEDS." SEED MIXTURE FOR LAWN AREAS SHALL CONSIST OF THE FOLLOWING

KIND OF SEED	MINIMUM PURITY (%)	MINIMUM GERMANATION (%)	POUNDS/ACRE (TOTAL 120 POUNDS)
CREEPING RED FESCUE	96	85	40
PERENNIAL RYEGRASS	98	90	50
KENTUCKY BLUEGRASS	97	85	25
REDTOP	95	80	5

6. SEED MIXTURE FOR SLOPE AREAS SHALL CONSIST OF THE FOLLOWING:

KIND OF SEED	MINIMUM PURITY (%)	MINIMUM GERMANATION (%)	POUNDS/ACRE (TOTAL 95 POUNDS)
CREEPING RED FESCUE	96	85	35
PERENNIAL RYEGRASS	98	90	30
REDTOP	95	80	5
ALSIKE CLOVER	97	90	5
BIRDSFOOT TREFOIL	98	80	5
LANCE-LEAVED COREOPSIS	95	80	4
OXEYE DAISY	95	80	3
BLACKEYED SUSAN	95	80	4
WILD LUPINE	95	80	4

BY NORTH AMERICAN GREEN OR	×*
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NOTES:	
. SLOPE SURFACE SHALL BE FREE OF ROC	KS.
CLODS, STICKS AND GRASS. MATS/ BLAN	

MATS/BLANKETS SHOULD BE

DOWNSLOPE AND SHALL BE

DOUBLE NET STRAW BLANKETS

BY NORTH AMERICAN GREEN OR

INSTALLED VERTICALLY

- SHALL HAVE GOOD SOIL CONTACT. 2. APPLY PERMANENT SEEDING BEFORE PLACING
- BLANKETS 3. LAY BLANKETS LOOSELY AND STAKE OR STAPLE
- TO MAINTAIN DIRECT CONTACT WITH THE SOIL. DO NOT STRETCH. **EROSION CONTROL BLANKETS - SLOPE INSTALLATION** NOT TO SCALE

(AUGUST 2011)

- TEMPORARY SEEDING MIXTURE SHALL BE APPLIED AT A RATE OF 2 POUNDS PER 1,000 SF AND SHALL BE AN APPROVED CONSERVATION MIX OR CONSIST OF THE FOLLOWING:
- 15% BLACKWELL OR SHELTER SWITCHGRASS 30% NIAGRA OR KAW BIG BLUESTEM
- 30% CAMPER OR BLAZE LITTLESTEM
- 15% NE-27 OR BLAZE SAND LOVEGRASS 10% VIKING BIRDSFOOT TREFOIL

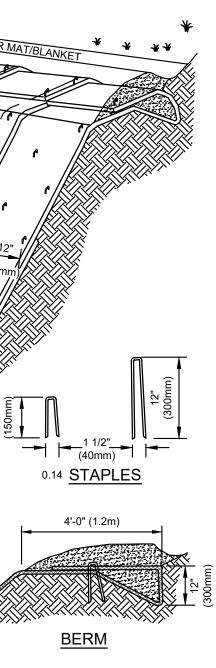
INOCULUM SPECIFIC TO BIRDSFOOT TREFOIL MUST BE USED WITH THIS MIXTURE. IF SEEDING BY HAND, A STICKING AGENT SHALL BE USED. IF SEEDING WITH A HYDROSEEDER, USE FOUR TIMES THE RECOMMENDED AMOUNT OF INOCULUM.

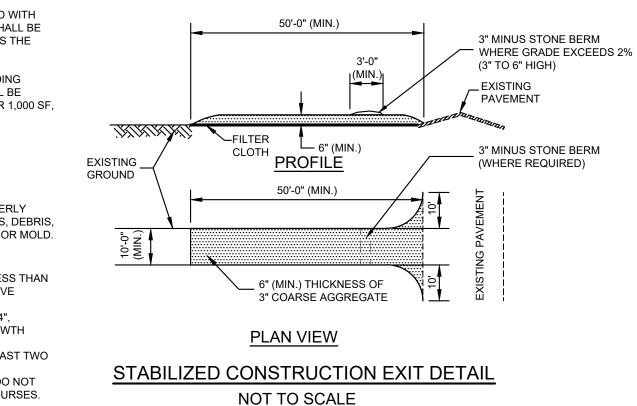
- SEED MIXTURE FOR STORMWATER MANAGEMENT AREAS, INCLUDING DETENTION BASINS AND VEGETATED TREATMENT SWALES, SHALL BE APPLIED AT A RATE OF 70 POUNDS PER ACRE OR 1.6 POUNDS PER 1,000 SF AND SHALL CONSIST OF THE FOLLOWING
 - 25% CREEPING RED FESCUE
 - 15% SWITCH GRASS 15% FOX SEDGE
 - **15% CREEPING BENTGRASS**
 - 10% FLATPEA 20% WILDFLOWER VARIETY
- STRAW USED FOR MULCH SHALL CONSIST OF MOWED AND PROPERLY CURED GRASS OR LEGUME MOWINGS FREE FROM WEEDS TWIGS DEBRIS INVASIVE SPECIES OR OTHER DELETERIOUS MATERIAL AND ROT OR MOLD.

SOD SPECIFICATIONS

- SOD SHALL BE PROVIDED WITH A STRONG ROOT SYSTEM, NOT LESS THAN TWO YEARS OLD AND SHALL BE FREE OF ANY UNDESIRABLE NATIVE GRASSES OR WEEDS
- SOD SHALL BE MACHINE CUT TO A THICKNESS NOT LESS THAN 3/4", EXCLUDING THATCH, AND SHALL BE CAPABLE OF VIGOROUS GROWTH WHEN PLANTED
- SOD PADS SHALL BE OF UNIFORM SIZE AND COMPOSED OF AT LEAST TWO LOCAL GRASS VARIETIES.
- LAY SOD TO FORM A SOLID MASS WITH TIGHTLY FITTED JOINTS, DO NOT OVERLAP. STAGGER STRIPS TO OFFSET JOINTS IN ADJACENT COURSES.
- TAMP SOD TO ENSURE CONTACT WITH WITH SOIL WATER WITHIN ONE HOUR OF PLANTING WITH A FINE SPRAY.

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MAINTENANCE:

) SOIL PARTICLES WILL EVENTUALLY CLOG THE VOIDS IN THE CRUSHED STONE AND THE EFFECTIVENESS OF THE D STONE PAD WILL NOT BE SATISFACTORY. WHEN THIS OCCURS, THE PAD SHOULD BE TOPDRESSED WITH NEW D STONE OR COMPLETE REPLACEMENT OF THE PAD MAY BE NECESSARY WHEN THE PAD BECOMES COMPLETELY

(APRIL 2018)

IING FACILITIES ARE USED, THE SEDIMENT TRAPS SHOULD BE CLEANED OUT AS OFTEN AS NECESSARY TO ASSURE DEQUATE TRAPPING EFFICIENCY AND STORAGE VOLUME IS AVAILABLE. VEGETATIVE FILTER STRIPS SHOULD BE NED TO INSURE A VIGOROUS STAND OF VEGETATION AT ALL TIMES.

STRUCTION SPECIFICATIONS:

ONE FOR A STABILIZED CONSTRUCTION EXIT SHALL BE 1 TO 2 INCH STONE, RECLAIMED STONE OR RECYCLED NCRETE EQUIVALENT.

E LENGTH OF THE STABILIZED EXIT SHALL NOT BE LESS THAN 50 FEET, EXCEPT FOR A SINGLE RESIDENTIAL LOT IERE A 30 FOOT MINIMUM LENGTH WOULD APPLY.

THICKNESS OF THE STONE FOR THE STABILIZED EXIT SHALL NOT BE LESS THAN 6 INCHES.

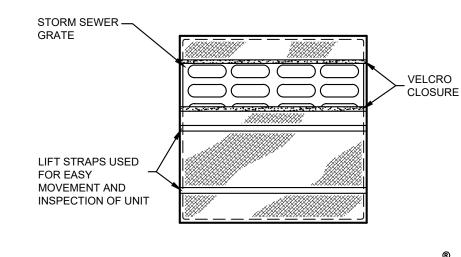
WIDTH OF THE EXIT SHALL NOT BE LESS THAN THE FULL WIDTH OF THE AREA WHERE INGRESS OR EGRESS CURS OR 10 FEET. WHICHEVER IS GREATER.

OTEXTILE FILTER CLOTH SHALL BE PLACED OVER THE ENTIRE AREA PRIOR TO PLACING THE STONE. FILTER CLOTH NOT REQUIRED FOR A SINGLE FAMILY LOT OR DUPLEX.

SURFACE WATER THAT IS FLOWING TO OR DIVERTED TOWARD THE CONSTRUCTION EXIT SHALL BE PIPED BENEATH E EXIT. IF PIPING IS IMPRACTICAL, A BERM WITH 5:1 SLOPES THAT CAN BE CROSSED BY VEHICLES MAY BE BSTITUTED FOR THE PIPE.

E EXIT SHALL BE MAINTAINED IN A CONDITION THAT WILL PREVENT TRACKING OR FLOWING OF SEDIMENT ONTO BLIC RIGHTS-OF-WAY. THIS MAY REQUIRE PERIODIC TOPDRESSING WITH ADDITIONAL STONE AS CONDITIONS MAND AND REPAIR AND/OR CLEANOUT OF ANY MEASURES USED TO TRAP SEDIMENT. ALL SEDIMENT SPILLED, SHED OR TRACKED ONTO PUBLIC RIGHT-OF-WAY MUST BE REMOVED PROMPTLY.

IEELS SHALL BE CLEANED TO REMOVE MUD PRIOR TO ENTRANCE ONTO PUBLIC RIGHTS-OF-WAY . WHEN WASHING REQUIRED, IT SHALL BE DONE ON AN AREA STABILIZED WITH STONE WHICH DRAINS INTO AN APPROVED SEDIMENT TRAPPING DEVICE.

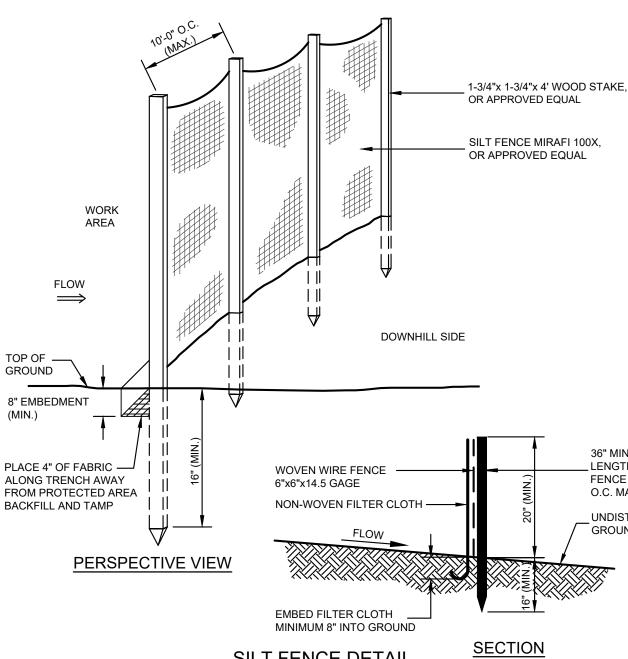


– DANDY BAG STORM SEWER · GRATE

HI-FLOW DANDY BAG[®] (SAFETY ORANGE)

MECHANICAL PROPERTIES	TEST METHOD	UNITS	MAF
GRAB TENSILE STRENGTH	ASTM D 4632	kN (lbs)	1.62 (365) x
GRAB TENSILE ELONGATION	ASTM D 4632	%	24 x
PUNCTURE STRENGTH	ASTM D 4833	kN (lbs)	0.40
MULLEN BURST STRENGTH	ASTM D 3786	kPa (psi)	3097 (
TRAPEZOID TEAR STRENGTH	ASTM D 4533	kN (lbs)	0.51 (115) >
UV RESISTANCE	ASTM D 4355	%	90
APPARENT OPENING SIZE	ASTM D 4751	Mm (US Std Sieve)	0.425
FLOW RATE	ASTM D 4491	1/min/m ² (gal/min/ft ²)	5907 (
PERMITTIVITY	ASTM D 4491	Sec ⁻¹	2.1

DANDY BAG NOT TO SCALE (APRIL 2010)



SILT FENCE DETAIL NOT TO SCALE

CONSTRUCTION SPECIFICATIONS:

1. THE GEOTEXTILE FABRIC SHALL MEET THE DESIGN CRITERIA FOR SILT FENCES.

- 2. THE FABRIC SHALL BE EMBEDDED A MINIMUM OF 8 INCHES INTO THE GROUND AND THE SOIL COM EMBEDDED FABRIC
- 3. WOVEN WIRE FENCE SHALL BE FASTENED SECURELY TO THE FENCE POSTS WITH WIRE TIE OR ST NOTED OR AS DIRECTED BY DESIGN ENGINEER.
- 4. FILTER CLOTH SHALL BE FASTENED SECURELY TO THE WOVEN WIRE FENCE WITH TIES SPACED E THE TOP, MIDSECTION AND BOTTOM.
- 5. WHEN TWO SECTIONS OF FILTER CLOTH ADJOIN EACH OTHER, THEY SHALL BE OVERLAPPED BY 6 AND STAPLED
- 6. FENCE POSTS SHALL BE A MINIMUM OF 36 INCHES LONG AND DRIVEN A MINIMUM OF 16 INCHES IN WOOD POSTS SHALL BE OF SOUND QUALITY HARDWOOD AND SHALL HAVE A MINIMUM CROSS SEC 3.0 SQUARE INCHES.
- 7. MAINTENANCE SHALL BE PERFORMED AS NEEDED AND MATERIAL REMOVED WHEN "BULGES" DEV FFNCF

MAINTENANCE

- 1. SILT FENCES SHALL BE INSPECTED IMMEDIATELY AFTER EACH RAINFALL AND AT LEAST DAILY DURI RAINFALL. ANY REPAIRS THAT ARE REQUIRED SHALL BE MADE IMMEDIATELY.
- 2. IF THE FABRIC ON A SILT FENCE SHOULD DECOMPOSE OR BECOME INEFFECTIVE DURING THE EXPE FENCE, THE FABRIC SHALL BE REPLACED PROMPTLY.
- 3. SEDIMENT DEPOSITS SHOULD BE INSPECTED AFTER EVERY STORM EVENT. THE DEPOSITS SHOULI WHEN THEY REACH APPROXIMATELY ONE-HALF THE HEIGHT OF THE BARRIER.
- 4. SEDIMENT DEPOSITS THAT ARE REMOVED OR LEFT IN PLACE AFTER THE FABRIC HAS BEEN REMOV GRADED TO CONFORM WITH THE EXISTING TOPOGRAPHY AND VEGETATED.

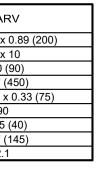
CONSTRUCTION SEQUENCE

- CONTRACTOR TO NOTIFY DIG-SAFE 72-HOURS PRIOR TO COMMENCEMENT OF CONSTRUCTION.
- PRIOR TO GRUBBING OF CLEARED AREAS, ALL SILTATION BARRIERS DESIGNED FOR USE AS TEMPORARY EROSION CONTROL MEASURES SHALL BE INSTALLED AS CALLED FOR ON PROJECT PLANS. INSTALL STABILIZED CONSTRUCTION EXIT AT LOCATION OF CONSTRUCTION ACCESS AT LOCATION OF INTERSECTION WITH EXISTING PAVEMENT
- 3. CUT AND CLEAR TREES AND BRUSH FROM CONSTRUCTION AREAS TO THE EXTENT NECESSARY. ALL BRANCHES, TOPS AND BRUSH TO BE PROPERLY DISPOSED OF BY CONTRACTOR. THIS PROJECT IS MANAGED TO MEET THE REQUIREMENTS AND INTENT OF RSA 430:53 AND AGR 3800 RELATIVE TO INVASIVE SPECIES. 4. COMPLETE GRUBBING OPERATIONS UNDER THE ROADWAY AND SLOPE SECTIONS . ALL STUMPS AND SIMILAR
- DEBRIS SHALL BE PROPERLY DISPOSED OF BY CONTRACTOR. ORGANIC MATERIAL SUITABLE FOR USE AS TOPSOIL SHALL BE STOCKPILED IN UPLAND AREAS. ALL STOCKPILES SHALL BE SEEDED WITH WINTER RYE AND, IF NECESSARY, SURROUNDED WITH HAY BALES IN ORDER TO PREVENT LOSS DUE TO EROSION. 5. CONSTRUCT TEMPORARY CULVERTS AS NECESSARY TO FACILITATE CONSTRUCTION ACTIVITIES. ALL SUCH
- CROSSINGS SHALL BE PROTECTED WITH HAY BALE BARRIERS TO LIMIT EROSION. RUNOFF MUST BE DIRECTED TO TEMPORARY PRACTICES UNTIL STORMWATER BEST MANAGEMENT PRACTICES ARE STABILIZED. 6. DO NOT DIRECT RUNOFF TO TREATMENT SWALES UNTIL THE SWALES AND ALL CONTRIBUTING AREAS HAVE
- BEEN FULLY STABILIZED. 7. STABILIZE ALL DITCHLINES PRIOR TO DIRECTING FLOW INTO THEM, CONSTRUCT DRAINAGE SYSTEM, SEWER AND OTHER SUBSURFACE UTILITIES.
- 8. PONDS/SWALES MUST BE INSTALLED BEFORE ROUGH GRADING OF THE SITE. 9. COMMENCE CONSTRUCTION OF ROADWAY, PERFORM EXCAVATION ACTIVITIES REQUIRED TO ACHIEVE SUBGRADE ELEVATION. ALL EXCAVATED EMBANKMENTS, DITCHES, SWALES AND ROADWAY CROSS CULVERTS SHALL BE INSTALLED AND STABILIZED. ALL SWALES AND DITCHLINES SHALL BE PROTECTED FROM EROSION BY IMPLEMENTATION OF HAY BALE SILTATION FENCES AS SHOWN ON PROJECT PLANS. DIVERT STORMWATER RUNOFF THROUGH THE USE OF TEMPORARY CULVERTS, OR OTHER MEANS NECESSARY PRIOR TO THE COMPLETIONS OF A FUNCTIONAL STORM DRAINAGE SYSTEM. SLOPES AND EMBANKMENTS SHALL BE STABILIZED BY TRACKING AND TEMPORARY SEEDING WITH WINTER RYE PRIOR TO TURF ESTABLISHMENT. ALL DITCHES AND SWALES SHALL BE STABILIZED PRIOR TO HAVING RUNOFF DIRECTED TO THEM. 10. COMPLETE CONSTRUCTION OF ROADWAY EMBANKMENTS BY ADDING APPROPRIATE BASE MATERIALS GRADED
- TO PROPER ELEVATION. 11. APPLY TOPSOIL TO ROADWAY SLOPES AND OTHER AREAS DISTURBED BY CONSTRUCTION. TOPSOIL USED MAY BE NATIVE ORGANIC MATERIAL SCREENED SO AS TO BE FREE OF ROOTS, BRANCHES, STONES AND OTHER DELETERIOUS MATERIALS. TOPSOIL SHALL BE APPLIED SO AS TO PROVIDE A MINIMUM OF A 4-INCH COMPACTED THICKNESS. UPON COMPLETION OF TOPSOILING, FINISHED SECTIONS ARE TO BE LIMED, SEEDED AND MULCHED. CONSTRUCTION PERSONNEL SHALL INSPECT COMPLETED SECTIONS OF WORK ON A REGULAR BASIS AND REMEDY ANY PROBLEM AREAS UNTIL A HEALTHY STAND OF GRASS HAS BECOME ESTABLISHED.
- 12. PERFORM FINE GRADING OF ROADWAY BASE MATERIALS. 13. MAINTAIN, REPAIR AND REPLACE AS NECESSARY TEMPORARY EROSION CONTROL MEASURES UNTIL SUCH TIME AS THE ENTIRE CONSTRUCTION AREA HAS BEEN STABILIZED (A MINIMUM OF ONE WINTER SHALL HAVE PASSED). 14. AFTER STABILIZATION, REMOVE AND SUITABLY DISPOSE OF TEMPORARY EROSION CONTROL MEASURES. 15. MONITOR CONSTRUCTION ACTIVITIES ON INDIVIDUAL LOTS TO INSURE CONSTRUCTION ACTIVITIES ARE BEING PERFORMED IN SUCH A WAY AS NOT TO ENDANGER THE INTEGRITY OF ROADWAY EMBANKMENTS, STORMWATER SYSTEMS AND UTILITIES. ALL DRIVEWAYS ACROSS DITCHLINES SHALL HAVE CULVERTS
- INSTALLED IN ACCORDANCE WITH LOCAL REQUIREMENTS. 16. LOT DISTURBANCE, OTHER THAN THAT SHOWN ON THE APPROVED PLANS, SHALL NOT COMMENCE UNTIL AFTER THE ROADWAY HAS THE BASE COURSE TO DESIGN ELEVATION AND THE ASSOCIATED DRAINAGE IS COMPLETE AND STABLE
- 17. PRIOR TO CONSTRUCTION A STORMWATER PROTECTION PLAN SHALL BE PREPARED PER FEDERAL REGULATRIONS
- 18. SINCE THIS SITE WILL DISTURB MORE THAN 5 ACRES AT ONE TIME WEEKLY INSPECTION SHALL OCCUR, AS WELL AS DURING ANY RAIN EVENT IN WHICH 0.5 INCH OF PRECIPITATION OR MORE FALLS WITHIN A 24 HOUR PERIOD. PROVIDED THAT IF THE MONITOR IS UNABLE TO BE PRESENT DURING SUCH A STORM, THE MONITOR SHALL INSPECT THE SITE WITHIN 24 HOURS OF THE RAIN EVENT.
- 19. A REPORT FOR EACH INSPECTION SHALL BE STAMPED BY A QUALIFIED ENGINEER OR A CPESC SPECIALIST AND SUBMITTED WITHIN 24 HOURS OF EACH INSPECTION.

EROSION CONTROL NOTES:

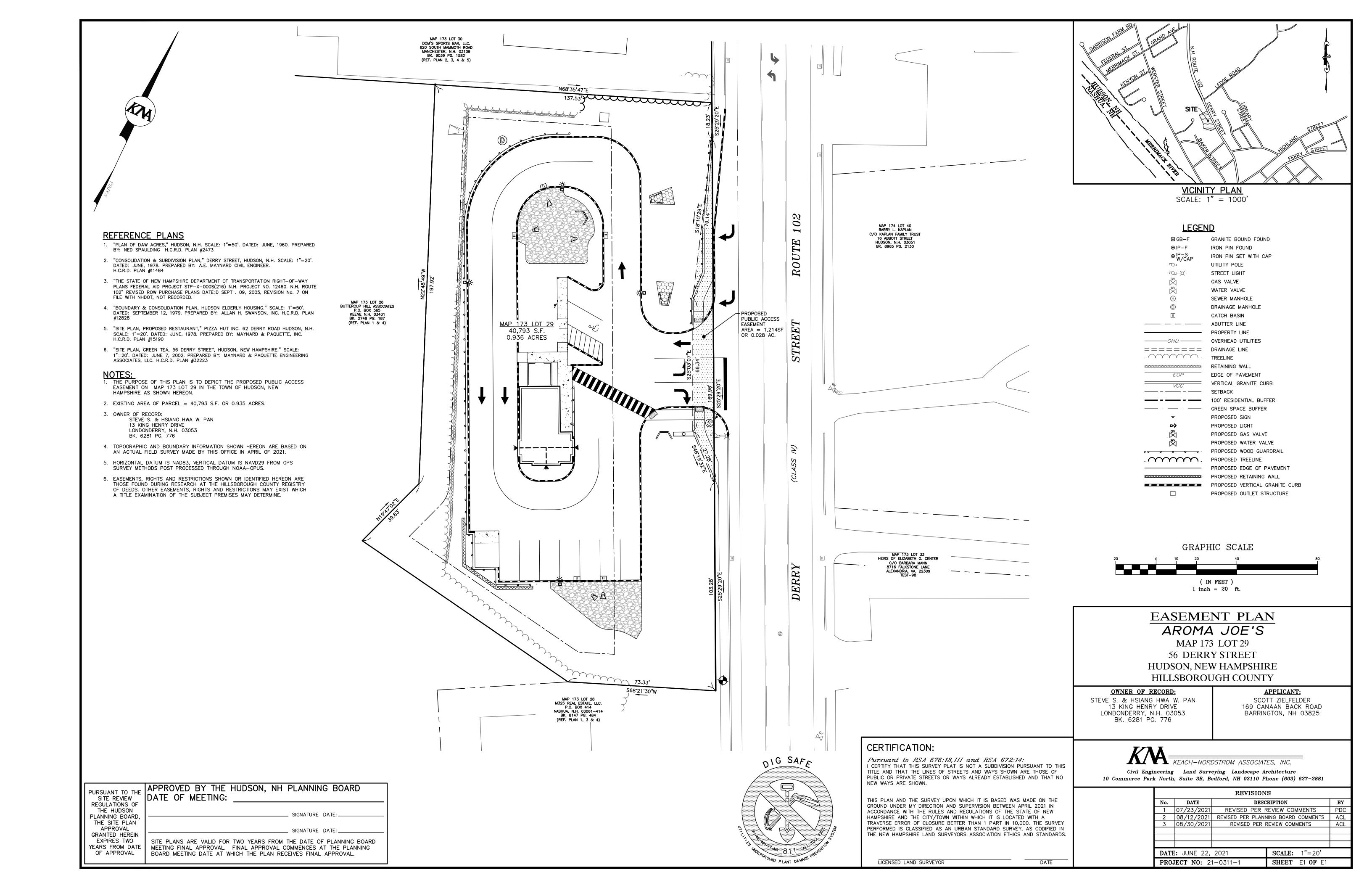
- EXPOSED EARTHWORK SHALL BE CONFINED TO AS LIMITED AN AREA AS IS PRACTICAL AT ANY GIVEN TIME THROUGHOUT THE CONSTRUCTION SEQUENCE. AT NO TIME SHALL MORE THAN FIVE (5) ACRES OF SITE AREA BE IN AN UNSTABLE CONDITION. NO GIVEN AREA OF THE SITE SHALL BE LEFT IN AN UNSTABILIZED CONDITION FOR A PERIOD OF TIME EXCEEDING THIRTY (30) CALENDAR DAYS.
- TEMPORARY EROSION CONTROL MEASURES SHALL BE INSTALLED IN STRICT ACCORDANCE WITH PROJECT PLANS. II ADDITION, SIMILAR MEASURES SHALL BE INSTALLED WHERE AND WHEN THE FIELD CONDITION, OR FIELD OPERATION OF THE INDIVIDUAL SITE CONTRACTOR, MAY WARRANT, ALL TEMPORARY EROSION CONTROL MEASURES USED SHALL BE INSPECTED WEEKLY AND WITHIN 24 HOURS AFTER 0.25" OF RAINFALL OR MORE. THEY SHALL BE CLEANED AND MAINTAINED AND OTHERWISE KEPT IN AN EFFECTIVE OPERATING MANNER THROUGHOUT THE CONSTRUCTION PFRIOD
- 3. ALL DISTURBED AREAS DESIGNATED TO BE TURF, SHALL RECEIVE A MINIMUM APPLICATION OF 4 INCHES OF LOAM (COMPACTED THICKNESS), PRIOR TO FINAL SEEDING AND MULCHING. ALL SWALES AND DITCHLINES SHALL BE PERIODICALLY CLEANED OF DEPOSITED SEDIMENT SO AS TO MAINTAIN AN
- EFFECTIVE GRADE AND CROSS SECTION. ALL SWALES AND DITCHLINES SHALL BE FULLY STABILIZED PRIOR TO HAVING STORMWATER DIRECTED TOWARDS THEM 5. IN THE EVENT THAT, DURING CONSTRUCTION OF ANY PORTION OF THIS PROJECT, A WINTER SHUTDOWN IS
- NECESSARY, THE CONTRACTOR SHALL STABILIZE ALL INCOMPLETE WORK AND PROVIDE FOR SUITABLE METHODS OF DIVERTING RUNOFF IN ORDER TO ELIMINATE SHEET FLOW ACROSS FROZEN SURFACES. 6. AN AREA SHALL BE CONSIDERED STABLE IF ONE OF THE FOLLOWING HAS OCCURRED:
 - A. BASE COURSE GRAVELS ARE INSTALLED IN AREAS TO BE PAVED;
 - B. A MINIMUM OF 85% VEGETATED GROWTH HAS BEEN ESTABLISHED;
- A MINIMUM OF 3" OF NON-EROSIVE MATERIAL SUCH AS STONE OR RIP RAP HAS BEEN INSTALLED; OR EROSION CONTROL BLANKETS HAVE BEEN PROPERLY INSTALLED. 7. DUST SHALL BE CONTROLLED BY THE USE OF WATER AS NECESSARY THROUGHOUT THE CONSTRUCTION PERIOD, I
- ACCORDANCE WITH ENV-A 1000. IN NO WAY ARE THOSE TEMPORARY EROSION CONTROL MEASURES INDICATED ON THESE PLANS TO BE CONSIDERI ALL INCLUSIVE. THE CONTRACTOR SHALL USE JUDGEMENT IN INSTALLING SUPPLEMENTARY EROSION CONTROL MEASURES WHERE AND WHEN SPECIFIC SITE CONDITIONS AND/OR CONSTRUCTION METHODOLOGIES MAY
- WARRANT AREAS HAVING FINISH GRADE SLOPES OF 3 : 1 OR STEEPER, SHALL BE STABILIZED WITH JUTE MATTING WHEN AND I FIELD CONDITIONS WARRANT, OR IF SO ORDERED. JUTE MATTING INSTALLED TO CONFORM WITH THE RECOMMENDED BEST MANAGEMENT PRACTICE OUTLINED IN VOLUME 3 OF THE NEW HAMPSHIRE STORMWATER MANUAL "EROSION AND SEDIMENT CONTROLS DURING CONSTRUCTION." 10. ALL DETENTION PONDS AND TREATMENT SWALES SHALL BE CONSTRUCTED PRIOR TO ANY EARTH MOVING
- ACTIVITIES THAT WILL INFLUENCE STORMWATER RUNOFF.
- 11 ALL ROADWAYS AND PARKING AREAS SHALL BE STABILIZED WITHIN 72 HOURS OF ACHIEVING FINISHED GRADE LENGTH 1-3/4" x1-3/4" 12. ALL CUT AND FILL SLOPES SHALL BE STABILIZED WITHIN 72 HOURS OF ACHIEVING FINISHED GRADE.

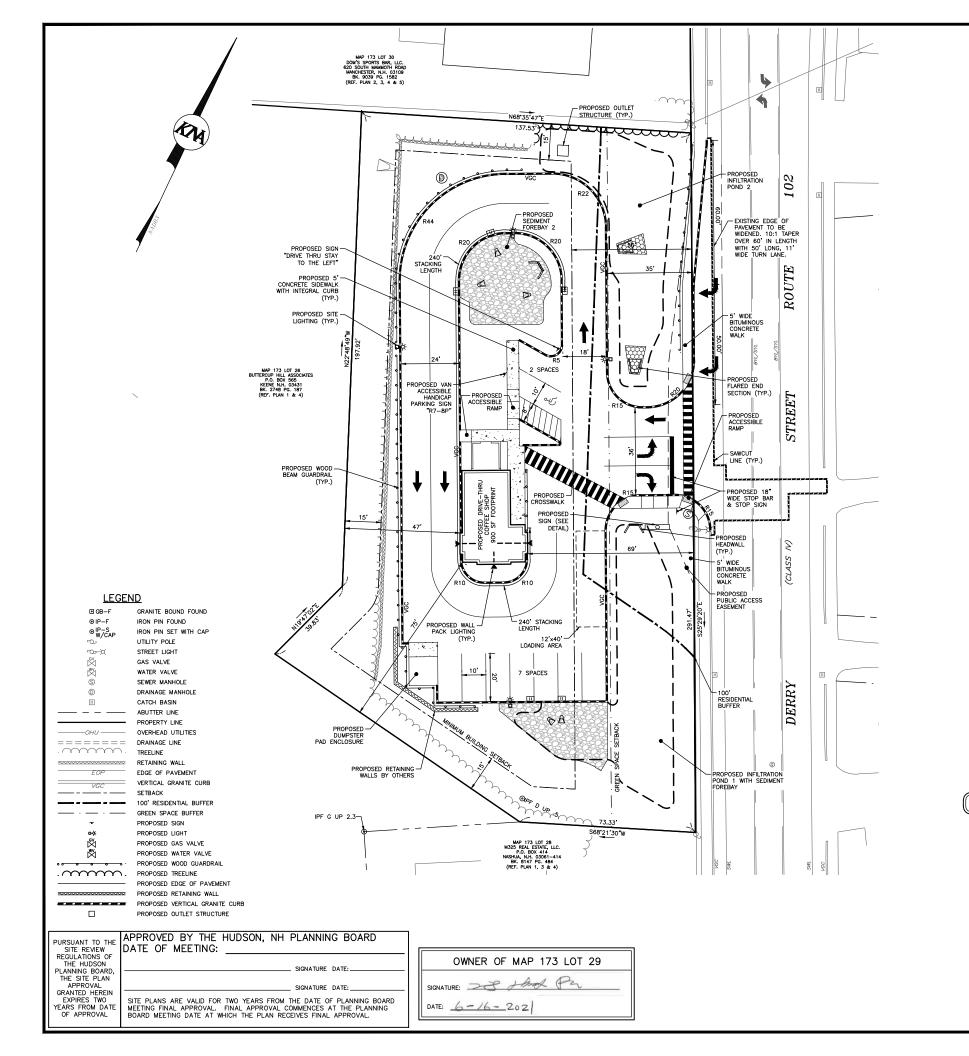
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			MAP 173	5 LOT 29			
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	HU	JDS	SON, NEV	V HAMPSHII	RE		
	HILLSBOROUGH COUNTY						
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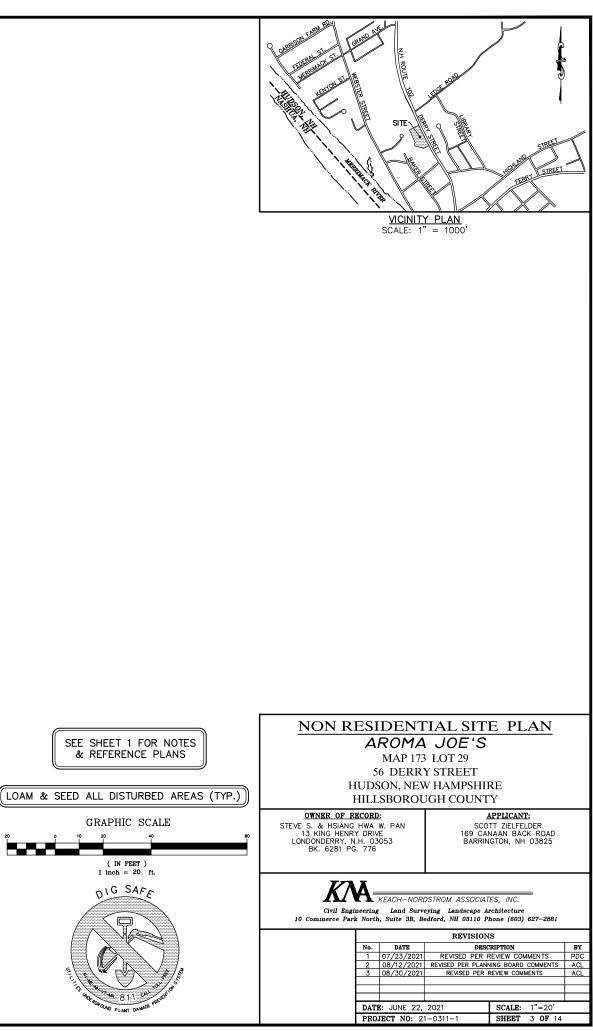


36" MINIMUM

FENCE POST 10'-0"











August 26, 2021

Mr. Brian Groth Town Planner Town of Hudson 12 School Street Hudson, NH 03051

Re: Town of Hudson Planning Board Review Aroma Joe's Site Plan, 56 Derry Street, Traffic Study Tax Map 173 Lot 29; Acct. #1350-970 Reference No. 20030249.2040

Dear Mr. Groth:

Fuss & O'Neill, Inc. has reviewed the Traffic Impact and Access Study prepared by Transportation Engineering, Planning and Policy (TEPP) for Keach-Nordstrom Associates, Inc. (KNA) dated July 1, 2021, for the proposed commercial development at 56 Derry Road in Hudson, New Hampshire (Tax Map 173 Lot 29). The project proposes the development of a 900 square foot (sf) drivethrough coffee shop on the currently vacant land at 56 Derry Road. Access and egress to the site will be provided via a proposed driveway on the west side of Derry Road, with one lane for access. For egress, a dedicated left turn lane and a dedicated right turn lane will be provided.

Please note that site plan, stormwater, and other project related comments were provided under separate letters dated July 9, 2021, and August 3, 2021.

In review of the TEPP report the following items are noted:

4. Traffic

50 Commercial Street Manchester, NH 03101 t 603.668.8223 800.286.2469

www.fando.com

California Connecticut Maine Massachusetts New Hampshire

> Rhode Island Vermont

- a. Provide more detail regarding the proposed use in the introduction of the report; the introduction omits the size of the coffee shop and that the shop will not provide indoor seating.
- b. Revise the street name displayed in Table 3 Sight Distances to Derry Road instead of Portland Street and revise the data in the table accordingly if what is currently shown in the table is sight distance data for a different traffic impact study.
- c. In the summary of Table 4 Calculated Weekday Vehicle Trip-Generation, the 2032 total vehicle-trips are presented as "117 (58 in and 539 out)" for the weekday AM peak hour. The number of trips exiting the site should be revised to match what is shown in Table 4.



Mr. Brian Groth August 26, 2021 Page 2 of 3

- d. Although we agree that the trip generation information compiled in the ITE Trip Generation Manual for land use code 938 is not applicable to the proposed land use due to the small sample size, it is unclear if the method used in the TEPP report as an alternative is acceptable. The TEPP report mentions that Stephen G. Pernaw & Company, Inc. has published appropriate trip generation information for the proposed land use. The report then uses the information presumably from the Stephen G. Pernaw & Company, Inc. publication but does not provide the publication itself or specify the project the information comes from, so it is unclear whether the information used in the TEPP report to calculate traffic volumes generated by the proposed development is correct or appropriate. Relevant information from the Stephen G. Pernaw publication should be provided and the methodology used for determining the proposed development's generated trips should be further explained in the report.
- e. Table 5 Trip Distribution and Network Assignment assigns all site-generated traffic coming from/going to the south on Derry Road. This should be revised to reflect the applied distributions.
- f. The title of the last column of Table 8 Capacity Analysis Summary should be corrected to the 2032 Build condition.
- g. The v/c ratio and queue length for the northbound left turn movement at the Derry Road/Site Driveway intersection during the 2032 Build in Table 8 are flipped according to the attached Synchro reports. This should be revised.
- h. The attached site plan shows a proposed southbound right turn storage lane at the Derry Road/Site Driveway intersection for access into the site. The Build condition Synchro reports and capacity analysis results in the TEPP report do not appear to reflect this lane configuration change and should be revised accordingly. If this lane configuration change is being proposed, it should also be discussed in the report.
- i. The site plan should include signage per MUTCD for the proposed right turn lane.
- j. We recommend the applicant and Town consider the installation of a left turn arrow in the center turning lane for northbound Derry Street traffic to formalize the site entrance, similar to turn arrows further north along the Derry Street center turning lane.

Fuss & O'Neill needs clarification on the methodology used of the trip generation calculations used in the TEPP report to determine if TEPP's overall conclusion, that there should be minimal impacts on traffic operations at the Derry Road and Ledge Road intersection, is appropriate. As for



Mr. Brian Groth August 26, 2021 Page 3 of 3

the Derry Road and Site Driveway intersection, we suggest revising the analysis to include any proposed roadway improvements before further review.

Please feel free to call if you have any questions.

Very truly yours,

Steven W. Bicket, PE. C=US, Orbeit, Inc., cu=Fus & Orbeit, Inc., cu=

Steven W. Reichert, P.E.

SWR:

cc: Town of Hudson Engineering Division – File Keach- Nordstrom Associates, Inc. - alewis@keachnordstrom.com



August 30, 2021

Brian Groth Town Planner Town of Hudson 12 School Street Hudson, New Hampshire 03051

Subject: Response to Fuss & O'Neill Review Letter Aroma Joe's Site Plan, 56 Derry Street Tax Map 173, Lot 29 KNA Project No. 21-0311-1

Dear Mr. Groth:

Our office is in receipt of Fuss & O'Neill's review comments dated August 16, 2021. Based on the comments, we have made the required modifications to the plan set and attached a copy for final review. A response to each comment has been provided below.

Site Plan Review Codes (HR 275)

a. The applicant has added a loading space to the plan set. We note that the space shown is 40 feet long instead of the standard 60 feet. The applicant should confirm that the size of anticipated delivery vehicles will fit in this location.

The space shown in the Site Plan Sheet will not be a painted space, and therefore the delivery truck will be able to utilize the entire length of curb as well as the parking space to park. The anticipated truck length is 53 feet and will have enough room along that curb line to park and pull out of the site from that location.

b. The applicant has provided spot grades for the parking lot ramp locations. We continue to recommend spot grades be provided for the sidewalk ramps as well.

Spot grades have been added and can be seen on the Grading, Drainage and Utility Plan.

c. The applicant has stated that no easements or deeds are required. We continue to recommend a sidewalk easement be provided to the Town for the relocated sidewalk.

A draft easement has been created and sent to the Town of Hudson to review. The draft easement has been included in this submittal.

Civil Engineering	Land Surveying	Landso	cape Architecture
10 Commerce Park North, Suite 3B	Bedford, NH 03110	Phone (603) 627-2881	Fax (603) 627-2915

Driveway Review Codes (HR 275-8)

a. We understand that the Town has requested a crosswalk be installed from the Derry Street sidewalk to the sidewalk at the proposed building. We recommend that the applicant provide appropriate pedestrian signage both internally for this crosswalk and for vehicles approaching Derry Street at the right turn lane.

A crosswalk has been added from the sidewalk to the proposed building and can be seen on the Site Plan. Signage has not been added at this time, as KNA believes the painted crosswalk and detectable warning plates to be adequate for pedestrian traffic.

Drainage Design/Stormwater Management (HR 275-9.A.)

a. The applicant has noted that test pits are consistent within the site and within close proximity to the infiltration basin areas. We note test pit #2 has an existing elevation of 160.0, with 66" depth to ESHWT as noted within the Test Pit data and BMP worksheet for Infiltration Pond 2. Applying this test pit data to the entire Infiltration Pond#2: Bottom of basin is 159.0, existing elevation at bottom of basin is 164.0. Calculating ESHWT to be 66" below 164.0, computes to 158.5 and not 156.0 as noted within the BMP worksheet. This does not meet the 3.0' required separation from ESHWT. The applicant should provide additional test pit information to support the use of an infiltration basin situated upon the site in respect to existing elevations.

While this method is one of the standard methods for calculating seasonal high water table depth for a pond on an undisturbed site, this site is different. This site has been cleared, graded, and a retaining wall put in many years ago. Three out of the four test pits found no seasonal high-water table at up to 80" deep. KNA believes this to be a more accurate representation of the water table than the single test pit that does show the seasonal high. Taking into account the other test pit data the following elevations for season high water table were found (using the bottom of the test pit depth as no water table was encountered): TP # I - 156.000, TP # 3 - 153.33, and TP # 4 - 148.33. As the site is relatively flat, the elevation of 156.00 was conservatively used for the water table depth for the entire site.

b. The applicant has provided BMP worksheets. We continue to recommend the applicant provide the required Infiltration Feasibility Report.

An infiltration feasibility report has not been included in this submittal as it is not required and KNA believes the infiltration to be adequate based on HydoCAD calculations and BMP worksheets. All drainage calculations were done in accordance with state regulations.

c. The applicant has stated that a letter was provided under separate cover. We note that a letter was not provided as part of the package received for review.

A letter from the Wetland Scientist has been included in this submittal.

d. The applicant has illustrated two snow storage areas upon the landscaping plan. The applicant should review the need for fencing or signage to ensure snow

Civil Engineering	Land Surveying	Landso	cape Architecture
10 Commerce Park North, Suite 3B	Bedford, NH 03110	Phone (603) 627-2881	Fax (603) 627-2915

storage does not occur within the footprint of the infiltration basins.

Snow storage areas have been updated and can be seen on the Landscape Plan.

e. The applicant has updated the infiltration rate to be 3.0 in/hr, from the previously utilized 6.0 in/hr. The Test Pit information provided within the Stormwater Management and Erosion Control Plan illustrates a perc rate of 2.0 min/inch, which is equivalent to the utilized 3.0 in/hr. The applicant should provide additional information as to why this rate was utilized and a factor of safety is not being accounted for within the calculations.

The 3 in./hr used is with the factor of safety taken into account for the soil type. Also, 2.0 min/inch percolation rate equals about 30 inches per hour, therefore KNA feels that the 3 in./hr used after the factor of safety was subtracted is a conservative infiltration rate.

f. The applicant has provided rip rap calculations. The applicant should also provide all outlet protection apron calculations, as illustrated upon Plan Sheet 10 of the Plan Set.

Rip rap calculations have been updated and the detail on sheet 10 has been updated as well and included in this submittal.

g. The applicant has provided information demonstrating that the pipes are selfcleaning within the HydroCAD 2-year report. The applicant should review this with the Town to ensure this is an acceptable variation from the Regulations.

No comment.

Landscaping (HR 275-8.C.(7))

a. The applicant has stated that the lights will be on a timer and scheduled around the operating hours of the business. We note that applicant should update the business hours on the plan set per the Planning Board meeting on July 28, 2021.

Proposed business hours have been updated and included in the Master Plan as note #15.

Other

a. The applicant should update Derry Road plan references to Derry Street.

All references to Derry Road have been updated to Derry Street

Respectfully,

Allison Lewis, EIT Project Engineer Keach Nordstrom Associates, Inc. 10 Commerce Park North, Suite 3 Bedford, NH 03110

Civil Engineering	Land Surveying	Landso	cape Architecture
10 Commerce Park North, Suite 3B	Bedford, NH 03110	Phone (603) 627-2881	Fax (603) 627-2915

SP #08-21 - Aroma Joe's - Attachment B

KEACH-NORDSTROM ASSOCIATES, INC.
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RIP RAP OUTLET PROTECTION APRON CALCULATIONS

Project: KNA #:	Aroma Joes 21-0311-1		Date:		8/30/2021
The purpo required di protection	se of this sprea uring the SCS/I will be sized fo	idsheet is to calcu NRCS <u>25-year</u> type r the SCS/NRCS <u>2</u>	The purpose of this spreadsheet is to calculate the dimensions of Inlet/Outlet Protection apron (riprap) required during the SCS/NRCS <u>25-year</u> type III 24-th storm event. The spillway weir(s) inlet/outlet apron protection will be sized for the SCS/NRCS <u>25-year</u> type III <u>24-hr</u> storm event.	n (ripra utlet a	p) pron
Required In	E	T≰ DΩ	peak flow in CFS diameter in feet of outlet or width of channel tall water at end of apron		
Depending	on the tail wat	er conditions, eith	Depending on the tail water conditions, either column 1 or column 2 is used for calculations		
	Column One	Column One where Tw<1/2Do	Column Two where Tw>1/2Do		

W2 = 3*Do + La	W1 = 3*Do	Width of Apron at outfall	La = (1.8Q/Do^3/2)+7Do	Length of Apron
W2 = 3*Do+0.4*La	W1 = 3*Do		La = 3*Q/Do^3/2+7Do	

If defined channel, then use channel width for W1 and W2

Rock Rip Rap Size: d50 = (0.02*Q^4/3)/(Tw*Do)

RIRAP GRADATION ENVELOPE

Inlet #2	Inlet #1	Outlet #	Outlet	Description	Input to Chart	Calculation
2 Infrilation Pond 2 Inlet from Infil 1	1 Infiltration Pond 2 Inlet from S.F.	\$1 Sediment Forebay #2	9 Sediment Forebay	ion (Optional)	Chart	ion Summary Table:
0.11	0.68	1.15	1.07	(cfs)	Q-25	
1.00	1.25	1.00	1.00	Do (ft)		
0.50	1.67	1.32	1.60	Tw (ft)		
7	10	10	10	La	Calculated C	
ω	4	ω	ω	W1	Output	
10	8	7	7	no channel	W2	
0.0	0.0	0.0	0.0	d50, ft		
0.03	0.07	0.22	0.16	d50 in		
4	4	3	4	d50 in.	USE	
6	6	ς'n	6	5	FROM	a
8	8	6	8	3.		d100
5	თ	4	თ	3'	FROM	d
7	7	51	7	3.	ТО	d85
4	4	ω	4	5.	FROM	05p
6	თ	сл	6	3.	б	50
-		-	-	3.	FROM	a
2	N	2	2	2.	ТО	15
10	10	7.5	10	3.	depth	
10	10	8	10	in,	Depth	
7	10	10	10	ft	Length	S.O.
3	4	3	з	ft	W1	SE
10	~	7	7	₽	W2	

*All rip rap for this project is sized to the largest flow outlet as a conservative measure.

* Center Apron with Headwall and Outlet Pipe (All Cases)

Line Apron with 6.0 oz. Geotextile Fabric (All Cases)



July 9, 2020

Re: Environmental Services 56 Derry Street Hudson, NH KNA # 21-0311-1

Dear Ms. Lewis,

Joshua Brien, CWS 256 of Keach-Nordstrom Associates, Inc. (KNA) has completed an on-site investigation on the above referenced parcel. This investigation was completed in spring of 2021. These services were requested as part of an investigation into presence or lack thereof jurisdictional wetlands within the Tax map 173, Lot 29 also known as 56 Derry Street, Hudson, New Hampshire.

Wetland identification was performed according to the methodology presented in the Corps of Engineers Wetland Delineation Manual (Technical report Y-87-1) Jan 1987 and the Regional Supplement to the Corps of Engineers Wetland Delineation Manual: Northcentral and North-East Region, version 2.0. January 2012, US Army Corps of Engineers. This methodology requires the presence of indicators for three parameters: hydric soils, hydrophytic vegetation, and evidence of hydrology at or near the surface for 14 days during the growing season.

At this time, I am pleased to report that, there are no areas found within the parcel that qualify as jurisdictional wetlands under the aforementioned criteria.

If you have any further questions or concerns, please do not hesitate to contact me at (603) 627-2881 or via email at jbrien@keachnordstrom.com

Sincerely,

Joshua M Brien, CWS 256 Keach-Nordstrom Associates, Inc.



Civil Engineering

Land Surveying

Landscape Architecture

Return to: Town of Hudson 12 School Street Hudson, New Hampshire 03051

EASEMENT DEED

Scott Zielfelder, an individual, of 169 Canaan Back Road, Barrington, New Hampshire 02825, for consideration paid, grants to the Town of Hudson, a body corporate and politic organized and existing under the laws of the State of New Hampshire, with a mailing address of 12 School Street, Hudson, County of Hillsborough, State of New Hampshire 03051 ("Grantee"), with QUITCLAIM COVENANTS:

Certain rights and easements, described below, over land situated in Hudson, Hillsborough County, New Hampshire, as follows:

1. Perpetual, non-exclusive rights and easements for a sidewalk, identified as Proposed Public Access Easement (the "Easement Area") on a plan entitled "Easement Plan, Aroma Joe's, Map 173, Lot 29, 56 Derry Street, Hudson, New Hampshire", dated June 22, 2021, through revision #2 dated 8/12/2021, prepared by Keach-Nordstrom Associates, Inc. and recorded in the Hillsborough County Registry of Deeds as Plan No. ______ (hereinafter "Plan").

2. The aforesaid easement shall be on the following conditions:

a. This Easement Area shall include the perpetual right to enter upon the real estate described at any time that the Grantee, its successors or assigns, may see fit to construct and maintain a public sidewalk and public walkway within the Easement Area. This Easement Area shall allow public pedestrian and non-motorized traffic and shall permit maintenance, repair and replacement of the pavement in the easement area.

b. All construction within said Easement Area shall be undertaken in a commercially reasonable manner.

B. Use of said Easement Area shall, except where required for construction and maintenance, not interfere with the use and enjoyment of the traveled portions of the property of which the Easement Areas are a part.

C. The Grantor, for itself and its successors and assigns, hereby covenants that it will not erect or permit any building or any other structure upon the Easement Area (except as shown on the Plan) which in the reasonable judgment of the Grantee, might unreasonably interfere with the proper maintenance of underground facilities actually constructed in the Easement Areas by Grantee, or service in connection therewith, but otherwise, Grantor shall be prohibited from use of the Easement Areas for any purpose including ingress and egress, driveways, parking areas, landscaping and the like.

The above premises is not homestead property of the Grantor herein.

This transfer is exempt from documentary tax stamps pursuant to RSA 78-B:2, I, as it consists of a transfer of title to a municipality in the State of New Hampshire.

For Grantor's title see deed recorded in the Hillsborough County Registry of Deeds at Book ______, Page _____.

Signatures on Pages to Follow

GRANTOR:

Scott Zielfelder

STATE OF NEW HAMPSHIRE

_____, SS

_____, 202___

Then personally appeared Scott Zielfelder, known to me, or satisfactorily proven to be the person whose name is subscribed to the foregoing instrument and acknowledged that he executed the same for the purposes contained therein, before me

> Notary Public My commission expires:

GRANTEE:

Town of Hudson

By: _____

STATE OF NEW HAMPSHIRE COUNTY OF HILLSBOROUGH

The foregoing instrument was acknowledged before me this _____ day of ____, 202__, by _____, as _____ of the Town of Hudson.

Justice of the Peace / Notary Public My Commission Expires: _____

TRAFFIC IMPACT AND ACCESS STUDY

56 DERRY ROAD Hudson, New Hampshire

August 31, 2021

Prepared for Keach-Nordstrom Associates, Inc.



TRANSPORTATION ENGINEERING, PLANNING AND POLICY

TRAFFIC-IMPACT AND ACCESS STUDY

56 DERRY ROAD Hudson, New Hampshire

Revised August 31, 2021



Prepared for Keach-Nordstrom Associates, Inc.

TEPP LLC

TRANSPORTATION ENGINEERING, PLANNING AND POLICY

93 Stiles Road, Suite 201, Salem, New Hampshire 03079 USA 800 Turnpike Street, Suite 300, North Andover, Massachusetts 01845 USA Phone (603) 212-9133 and Fax (603) 226-4108 Email tepp@teppllc.com and Web www.teppllc.com



CONTENTS

SUMMARY1
Project Description1
Study Scope1
Trip Generation2
Capacity Analysis2
Traffic Impacts
INTRODUCTION
Project Description
Study Approach5
EXISTING CONDITIONS
Introduction
Physical Conditions
Traffic Volumes10
Vehicle Speeds11
Sight Distances
FUTURE CONDITIONS
Introduction15
Planned Road Improvements15
Background-Traffic Growth15
No-Build Traffic Volumes15
tRIP Generation
Trip Distribution and Network Assignment
Build Traffic Volumes
Traffic-Volume Changes
CAPACITY ANALYSIS
Introduction
Methods



Results		
CONCLUSION		
Project Descri	ption	
Trip Generation	on	
Capacity Anal	lysis	
Traffic Impact	ts	
APPENDIX		
Appendix A:	Project Plan	
Appendix B:	Traffic Counts	
Appendix C:	Monthly Traffic Volumes	

- Appendix D: Vehicle Speeds
- Appendix E: Trip Generation
- Appendix F: Capacity-Analysis Worksheets
- Appendix G: Comments and Responses

TABLES

Table 1.	2021 existing traffic volumes	11
Table 2.	Vehicle speeds.	13
Table 3.	Sight distances.	14
Table 4.	Calculated weekday vehicle-trip generation	19
Table 5.	Trip distribution and network assignment.	20
Table 6.	Traffic-volume changes.	25
Table 7.	Level-of-service criteria for intersections.	27
Table 8.	Capacity-analysis summary.	29



FIGURES

Figure 1.	Site location.	6
Figure 2.	2021 existing traffic volumes	. 12
Figure 3.	2022 no-build traffic volumes	. 16
Figure 4.	2032 no-build traffic volumes	. 17
Figure 5.	2022 site traffic volumes	. 21
Figure 6.	2032 site traffic volumes	. 22
Figure 7.	2022 build traffic volumes.	. 23
Figure 8.	2032 build traffic volumes.	. 24

TFPP

SUMMARY

PROJECT DESCRIPTION

Keach-Nordstrom Associates, Inc. (KNA) has retained TEPP LLC to prepare this traffic impact and access study (TIAS) for a proposed commercial redevelopment in the Town of Hudson, New Hampshire.

The proposed redevelopment will:

- be at 56 Derry Road
- provide one drive-through coffee shop with no indoor seating and a floor area of about 900 square feet (sf)
- have one driveway to the west side of Derry Road, with a one-lane entrance and a twolane exit, and with a potential right-turn lane on the Derry Road southbound approach

STUDY SCOPE

The TIAS study area includes the following unsignalized intersections:

- Derry Road/Ledge Road
- Derry Road/driveway

This TIAS analyzes the following conditions as applicable:

- 2021 existing
- 2022 and 2032 no-build, with background-traffic growth
- 2022 and 2032 build, with background-traffic growth and the proposed redevelopment

This TIAS analyzes traffic operations for the following hours as applicable:

- weekday AM street-peak hour
- weekday PM street-peak hour



TRIP GENERATION

Total trips appear on the site driveway but not all are added to Derry Road near the site. 2022 total vehicle-trips are:

- weekday daily, 629 (total of in and out)
- weekday AM-street-peak hour, 106 (53 in and 53 out)
- weekday PM-street-peak hour, 40 (20 in and 20 out)

2032 total vehicle-trips are:

- weekday daily, 694 (total of in and out)
- weekday AM-street-peak hour, 117 (58 in and 539 out)
- weekday PM-street-peak hour, 44 (22 in and 22 out)

Primary trips are added to Derry Road near the site. 2022 primary vehicle-trips are:

- weekday daily, 69 (total of in and out)
- weekday AM-street-peak hour, 12 (6 in and 6 out)
- weekday PM-street-peak hour, 4 (2 in and 2 out)

2032 primary vehicle-trips are:

- weekday daily, 78 (total of in and out)
- weekday AM-street-peak hour, 13 (6 in and 7 out)
- weekday PM-street-peak hour, 6 (3 in and 3 out)

CAPACITY ANALYSIS

Capacity analysis shows, for the Derry Road/Ledge Road intersection

- low delays for left turns from Derry Road
- moderate delays or delayed operations for movements from Ledge Road
- insignificant project impacts

Capacity analysis shows, for the Derry Road/driveway intersection:

- low delays for left turns from Derry Road
- moderate delays or delayed operations for movements from the driveway

Delayed operations on minor-street approaches to high-volume arterials are typical and acceptable.

TRAFFIC IMPACTS

Analysis indicates no significant area impact due to the proposed redevelopment.

TFPP

INTRODUCTION

PROJECT DESCRIPTION

KNA has retained TEPP LLC to prepare this TIAS for a proposed commercial redevelopment in the Town of Hudson, New Hampshire.

The proposed redevelopment will:

- be at 56 Derry Road
- provide one drive-through coffee shop with no indoor seating and a floor area of about 900 sf
- have one driveway to the west side of Derry Road, with a one-lane entrance and a twolane exit, and with a potential right-turn lane on the Derry Road southbound approach

Figure 1 shows site location. The project plan is in Appendix A.

STUDY APPROACH

This TIAS assesses traffic impacts and access for the proposed redevelopment.

The TIAS study area includes the following unsignalized intersections:

- Derry Road/Ledge Road
- Derry Road/driveway

This TIAS analyzes the following conditions as applicable:

- 2021 existing
- 2022 and 2032 no-build, with background-traffic growth
- 2022 and 2032 build, with background-traffic growth and the proposed redevelopment

This TIAS analyzes traffic operations for the following hours as applicable:

- weekday AM street-peak hour
- weekday PM street-peak hour





Figure 1. Site location.

Differences in traffic operations between the no-build and build conditions approximate traffic impacts of the proposed redevelopment.

EXISTING CONDITIONS

INTRODUCTION

Existing conditions include:

- physical conditions of the transportation network, roads, and intersections
- traffic volumes
- other relevant information

PHYSICAL CONDITIONS

INTRODUCTION

Figure 1 shows the transportation network.

The TIAS study area includes the following existing unsignalized intersection: Derry Road/Ledge Road.

Description of the TIAS study area follows.

DERRY ROAD

Derry Road:

- is oriented approximately north-south
- functions as an arterial street
- is also known as New Hampshire Routes (NH) 3A and 102
- to the south, connects with the Town Center and New Hampshire Route 111 (NH 111), an arterial highway that leads to the City of Nashua and Towns of Windham and Salem
- to the north, connects with NH 102, an arterial highway that leads to the Towns of Londonderry and Derry, and NH 3A, an arterial highway that leads to the Town of Litchfield and the City of Manchester
- has a horizontal alignment includes minor to moderate horizontal curvature, but is essentially tangent at the proposed driveway location
- has a near-level vertical alignment

- has a three-lane cross-section with one travel lane per direction, a center-two-way-left-turn lane (TWLTL), and paved shoulders
- has asphaltic-cement concrete (ACC) pavement in overall good condition
- has curb and sidewalk along both sides
- includes utility poles along the west side, some with luminaires
- has a posted speed limit of 30 miles per hour (mph)
- has nearby commercial and residential development
- is under the jurisdiction of the Town

DERRY ROAD/LEDGE ROAD INTERSECTION

The intersection:

- is three legged
- has Derry Road as the major north-south street
- has Ledge Road as the minor east leg
- on Derry Road, has one travel lane per direction and one center TWLTL
- on the Ledge Road approach, has one lane
- has a STOP sign on the Ledge Road approach
- is illuminated
- has commercial and residential development nearby

TRAFFIC VOLUMES

TRAFFIC COUNTS

TEPP LLC obtained an automatic traffic counter (ATR) count:

- on Derry Road along the site frontage
- from Wednesday, June 2, to Thursday, June 3, 2021

The ATR data are in Appendix B.

ADJUSTMENTS

The June 2021 traffic counts were adjusted to reflect peak-month and non-pandemic conditions.

The increase to peak month was 2.0 percent, based on based on NHDOT 2019 monthly volumes for Group 4 (Urban Highways) averages in Appendix C,

The increase to pre-pandemic was 5.6 percent. NHDOT continuous count station 82229031, on Daniel Webster Highway north of Hilton Drive, in the Town of Merrimack showed May 2021 two-way average-daily traffic (ADT) of 15,404 vehicles. The station showed May 2019 pre-pandemic two-way ADT of 16,260 vehicles, which is 5.6 percent greater.

The combined increase was 7.7 percent.

RESULTS

Table 1 and Figure 2 show 2021 existing traffic volumes.

Table 1.2021 existing traffic volumes.					
Location and Time Period	Vehicles ^a	K-factor ^b	Percent Direction		
Derry Road near Site Frontage					
Weekday Daily	28,667				
Weekday AM-Street-Peak Hour	2,157	7.5	58 Southbound		
Weekday PM-Street-Peak Hour	2,290	8.0	54 Northbound		

^a Two-way-total volumes.

^b K = hour volume as a percent of daily volume.

Derry Road near the site frontage showed about:

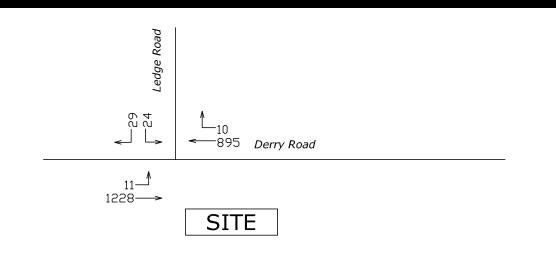
- 28,667 weekday-daily vehicles
- 2,157 vehicles during the weekday AM street-peak hour, predominantly southbound
- 2,290 vehicles during the weekday PM street-peak hour, predominantly northbound

VEHICLE SPEEDS

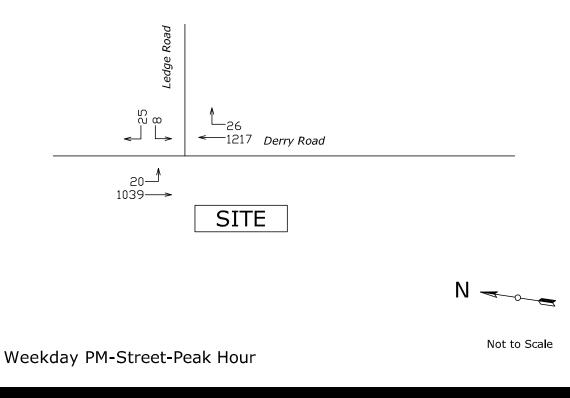
The ATR collected vehicle speeds:

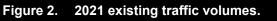
- on Derry Road along the site frontage
- from Wednesday, June 2, to Thursday, June 3, 2021





Weekday AM-Street-Peak Hour





The data are in Appendix D and are summarized in Table 2.

Table 2 indicates that on Derry Road:

Table 2. Vehicle speed	ds.						
		Speeds (mpl	h)				
Location and Direction	Speed Limit	Mean ^a	85 th Percentile ^a				
Derry Road along Site Frontage							
Northbound	30	35.3	39.0				
Southbound	30	33.6	37.2				

^a From ATR conducted from Wednesday, June 2, to Thursday, June 3, 2021.

- the posted speed limit was 30 mph
- the northbound the mean speed was 35.3 mph and the 85th percentile speed was 39.0 mph
- for southbound the mean speed was 33.6 mph and the 85th percentile speed was 37.2 mph

SIGHT DISTANCES

The American Association of State Highway and Transportation Officials (AASHTO) has established authoritative policy for sight distances at unsignalized intersections¹ in terms of:

- stopping sight distance (SSD)
- optional intersection sight distance (ISD)

SSD:²

- provides for safety
- enables a driver, on the major road, to perceive and react accordingly to a vehicle entering the major road from a minor road
- is conservative because it encompasses a wide range of brake-reaction times and deceleration rates

¹ AASHTO, *A Policy on Geometric Design of Highways and Streets*, 6th Edition (Washington, DC, 2011), pages 9-28 to 9-29.

 $^{^{2}}$ AASHTO, pages 3-2 to 3-6.



Optional ISD:³

- is ordinarily greater than SSD and may enhance traffic operations
- is not required for safety

Table 3 shows relevant available sight distances that are at least 400 ft, per NHDOT practice, and are adequate. Appendix A includes a sight-distance plan.

Table 3. Sight distances.							
Intersection, Movements, and View	Available Sight	Speeds (miles per hour)					
	Distance (ft) ^a	Limit	SSD Provides For	ISD Provides For			
Derry Road/Driveway for Driveway Movements							
Derry Road to/from South	At Least 400	30	45+	36+			
Derry Road to/from North	At Least 400	30	45+	36+			

^a With appropriate roadside and vegetation maintenance.

³ AASHTO, pages 9-22 to 9-55.

FUTURE CONDITIONS

INTRODUCTION

Future conditions include:

- planned road improvements independent of the proposed redevelopment
- future no-build traffic volumes, with background-traffic growth and without the proposed redevelopment
- future build traffic volumes, with background-traffic growth and with the proposed redevelopment

PLANNED ROAD IMPROVEMENTS

TEPP LLC identified no significant planned road improvement in the study area independent of the project.

BACKGROUND-TRAFFIC GROWTH

Background-traffic growth:

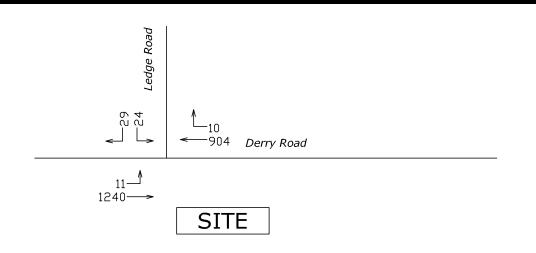
- is independent of the proposed redevelopment
- is related to land development in the immediate area, population and economic development in the region, and changes in travel patterns in the region
- typically considers two factors: a general traffic-growth rate and specific planned land developments in the immediate area

This TIAS uses a 1.0-percent annual growth rate. This yields about 11.6-percent growth between 2021 and 2032.

NO-BUILD TRAFFIC VOLUMES

The background-traffic growth described above was applied to 2021 existing traffic volumes. Figures 3 and 4 show 2022 and 2032 no-build traffic volumes.





Weekday AM-Street-Peak Hour

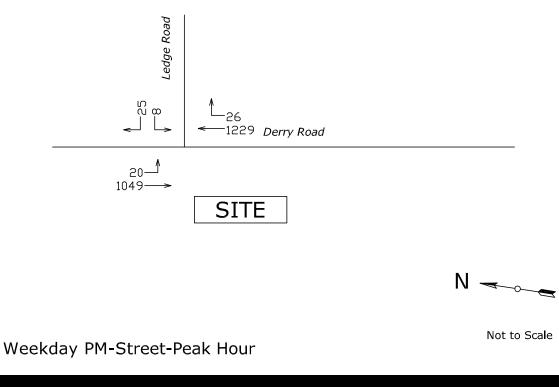
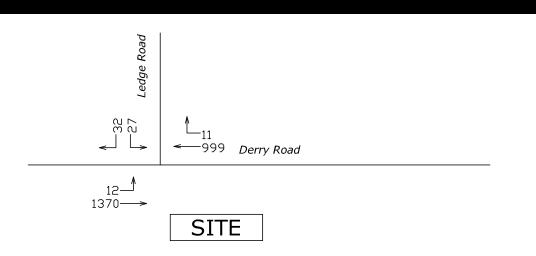
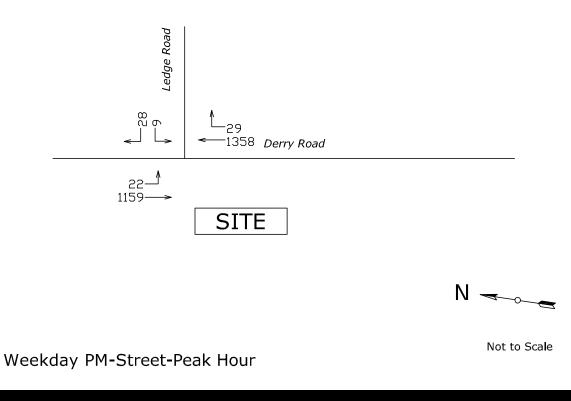
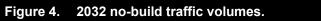


Figure 3. 2022 no-build traffic volumes.



Weekday AM-Street-Peak Hour







TRIP GENERATION

BASIC TRIP GENERATION

The Institute of Transportation Engineers (ITE) compiles and publishes trip-generation information for a variety of land uses in *Trip Generation Manual*.⁴ This guide for estimating site traffic includes coffee/donut shop with drive-through window and no indoor seating, land use 938, based on floor area.⁵ However, this information is based on sites with floor areas of 90 sf and is not applicable to the proposed land use, with a floor area of about 900 sf.

Stephen G. Pernaw & Company, Inc. has published appropriate and applicable trip-generation information specific to this land use, which estimates trip generation based on traffic volumes passing the site.⁶ Basic trip generation is based on this information, which is in Appendix E.

TRIP TYPES

Total trips appear on site driveways but not all are added to roads near the site. Accordingly, ITE compiles information on three trip types, based on empirical data for many land uses, in the authoritative Hooper, *Trip Generation Handbook*.⁷ These three trip types are:

- primary trips that are added to the area and are primarily for visiting the site
- diverted trips that not added to the general area; these trips are from existing traffic on roads near the site
- pass-by trips that are not added to the general area; these trips are from existing traffic passing the site⁸

RESULTS

Table 4 shows calculated weekday vehicle-trip generation for the site.

⁴ ITE, *Trip Generation Manual*, 10th edition (Washington DC, September 2017).

⁵ ITE, *Trip Generation Manual*, V Volume 2, Data, Services (Land Uses 900-999), pages 250 and 251, pages 249 to 254.

⁶ Stephen G. Pernaw & Company, Inc., *Traffic Impact Assessment, Proposed Drive-Thru Coffee Shop, Northwood, New Hampshire* (Concord, New Hampshire, October 2019), page 10 and Appendix E.

⁷ Kevin G. Hooper, P.E., Principal Editor, *Trip Generation Handbook*, 3rd edition (Washington DC: Institute of Transportation Engineers, September 2017).

⁸ Definitions of primary trips, diverted trips, and pass-by trips are in Hooper, page 93. Relevant data on primary trips, diverted trips and pass-by trips are in Hooper, 3rd edition, page 216.

		AM-	Street-Peak I	Hour	PM-Street-Peak Hour				
	Daily ^a	Total ^b	In	Out	Total ^c	In	Out		
2022 Vehicle-Tr	ips								
Primary	69	12	6	6	4	2	2		
Pass-Byd	<u>560</u>	<u>94</u>	<u>47</u>	<u>47</u>	<u>36</u>	<u>18</u>	<u>18</u>		
Total	629	106	53	53	40	20	20		
2032 Vehicle-Tr	ips								
Primary	78	13	6	7	6	3	3		
Pass-Byd	<u>616</u>	<u>104</u>	<u>52</u>	<u>52</u>	<u>38</u>	<u>19</u>	<u>19</u>		
Total	694	117	58	59	44	22	22		

^a Estimated total weekday daily trips are 5.93 times weekday AM-street-peak hour trips, based on ITE, *Trip Generation Manual*, Volume 2, Data, Services (Land Uses 900-999), pages 250 and 251.

^b Total weekday AM-street-peak hour trips are 0.0488 times 2021 no-build weekday AM-street-peak hour volume on Derry Road along the site frontage. Stephen G. Pernaw & Company, Inc., Appendix E.

^c Total weekday PM-street-peak hour trips are 0.0172 times 2021 no-build weekday PM-street-peak hour volume on Derry Road along the site frontage. Stephen G. Pernaw & Company, Inc., Appendix E.

^d Pass-by trip percentage is 89. Based on Hooper, *Trip Generation Handbook*, 3rd edition, page 216, coffee/donut shop with drive-through window and no indoor seating, land use 938.

Total trips appear on the site driveway but not all are added to Derry Road near the site. 2022 total vehicle-trips are:

- weekday daily, 629 (total of in and out)
- weekday AM-street-peak hour, 106 (53 in and 53 out)
- weekday PM-street-peak hour, 40 (20 in and 20 out)

2032 total vehicle-trips are:

- weekday daily, 694 (total of in and out)
- weekday AM-street-peak hour, 117 (58 in and 59 out)
- weekday PM-street-peak hour, 44 (22 in and 22 out)

Primary trips are added to Derry Road near the site. 2022 primary vehicle-trips are:

- weekday daily, 69 (total of in and out)
- weekday AM-street-peak hour, 12 (6 in and 6 out)



• weekday PM-street-peak hour, 4 (2 in and 2 out)

2032 primary vehicle-trips are:

- weekday daily, 78 (total of in and out)
- weekday AM-street-peak hour, 13 (6 in and 7 out)
- weekday PM-street-peak hour, 6 (3 in and 3 out)

TRIP DISTRIBUTION AND NETWORK ASSIGNMENT

Trip distribution and network assignment of vehicle-trips to and from the site may consider such factors as existing site distribution, travel patterns, population, regional land development, and site access. Trip distribution and network assignment for this TIAS considered the 2021 existing volumes.

Table 5 shows trip distribution and network assignment for primary trips. Pass-by trips were assigned reflecting peak-hour directional distributions on Derry Road: 58-percent southbound for the weekday AM-street-peak hour and 54-percent northbound for the weekday PM-street-peak hour. Figures 5 and 6 show site traffic volumes.

Table 5. Trip distribution and network	vork assignment.				
Road and Direction (To/From)	Approximate Percent				
Derry Road to/from North	45				
Derry Road to/from South	<u>55</u>				
Total	100				

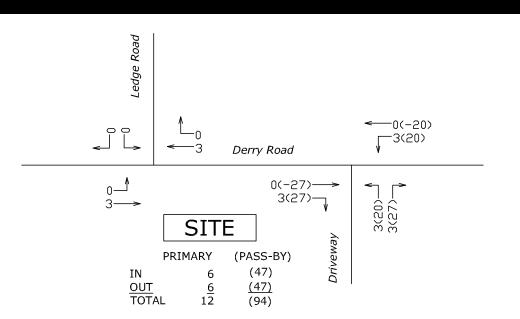
BUILD TRAFFIC VOLUMES

Site traffic volumes were superimposed on the no-build traffic volumes to estimate build traffic volumes. Figures 7 and 8 show the resulting 2022 and 2032 build traffic volumes.

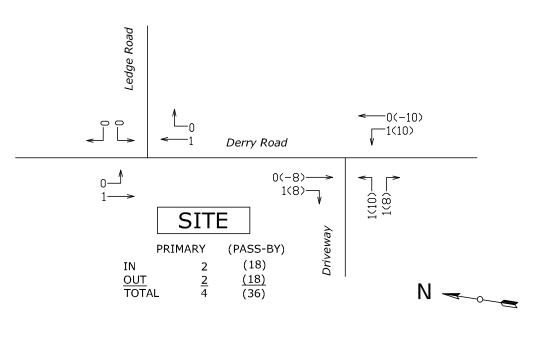
TRAFFIC-VOLUME CHANGES

Table 6 presents calculated traffic-volume changes due to the proposed redevelopment for the:

- weekday AM-street-peak hour
- weekday PM-street-peak hour



Weekday AM-Street-Peak Hour

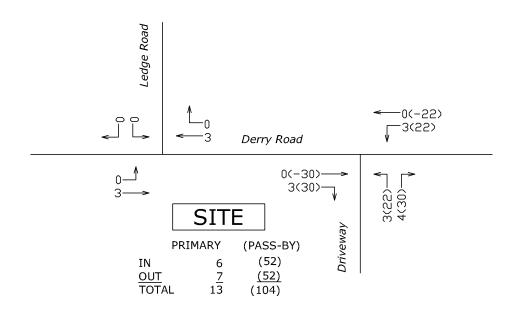


Weekday PM-Street-Peak Hour

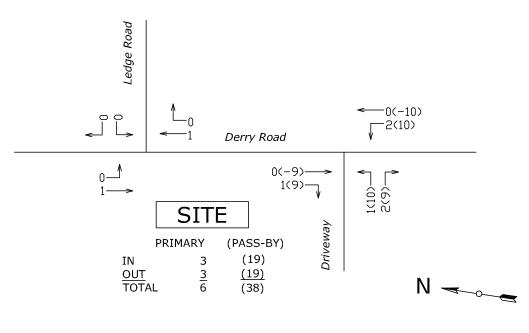


Figure 5. 2022 site traffic volumes.





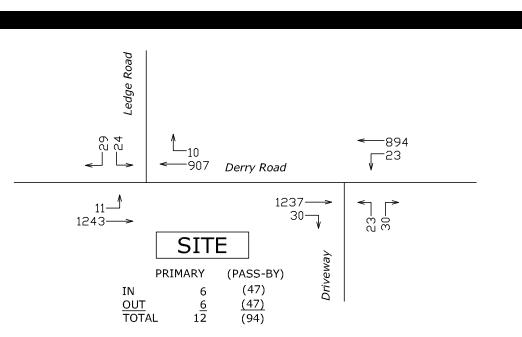
Weekday AM-Street-Peak Hour



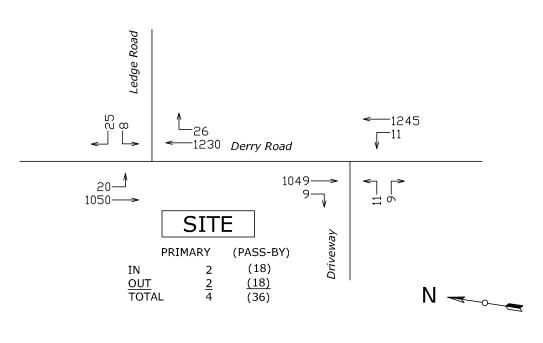
Weekday PM-Street-Peak Hour



Figure 6. 2032 site traffic volumes.

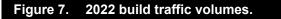


Weekday AM-Street-Peak Hour

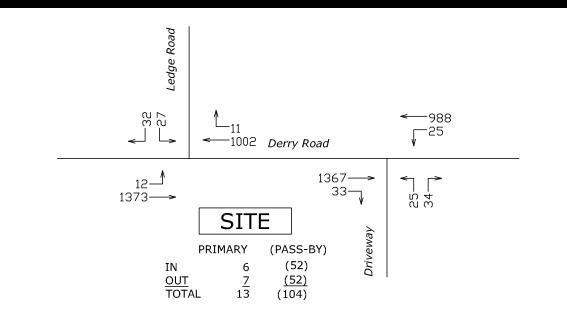


Weekday PM-Street-Peak Hour

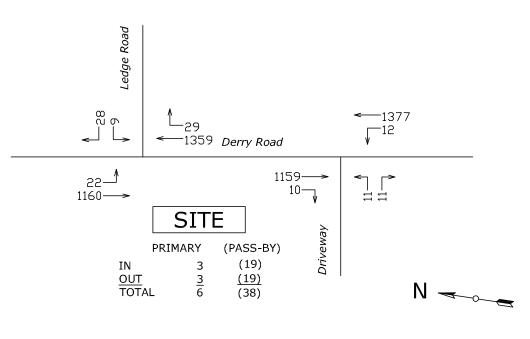








Weekday AM-Street-Peak Hour



Weekday PM-Street-Peak Hour



Figure 8. 2032 build traffic volumes.

Table 6. Traffic-volume changes. 2022 Traffic Volumes (vph)^a 2032 Traffic Volumes (vph) Location and Time Period No-Build Build Change No-Build Build Change Derry Road North of Driveway Weekday AM-Street-Peak Hour 2,178 2,184 6 2,407 2.413 6 Weekday PM-Street-Peak Hour 2,312 2,314 2 2,555 2,557 2 Derry Road South of Driveway 7 Weekday AM-Street-Peak Hour 2,178 2,184 6 2,407 2,414 2 Weekday PM-Street-Peak Hour 2,312 2,314 2,555 2,559 4

^a Two-way total volumes.

Table 6 shows peak-hour-traffic-volume increases:

- of 2 to 7 vehicle-trips
- constituting averages about one vehicle-trip per 8 to 30 minutes
- that are further split by northbound and southbound direction on Derry Road

CAPACITY ANALYSIS

INTRODUCTION

This TIAS has quantified existing, future-no-build and future-build traffic volumes. Capacity analysis models the quality of traffic operations. Comparing build conditions to the no-build conditions indicates impacts of the proposed redevelopment on quality of traffic operations.

METHODS

Capacity analysis calculates LOS for transportation facilities. LOS indicates the quality of traffic operations based on delay and other measures. The six LOS are designated A to F. LOS A represents the best or highest operating conditions. LOS F is the lowest, but does not necessarily connote failure.

LOS is a function of traffic volumes and traffic control. Because these volumes can vary, LOS of a transportation facility can differ by time of day, day of the week, or month. For example, a transportation facility with a low LOS during peak hours may have a high LOS during other hours. The operational analysis methods of the Transportation Research Board (TRB)⁹ models LOS for intersections based on calculated delay per vehicle, as shown in Table 7. Synchro analysis software was used.

Method inputs include:

- intersection geometry
- traffic control, such as YIELD sign, two-way STOP sign, all-way STOP sign, roundabout, or signal (including phasing, timing, and progression)
- traffic volumes
- vehicle composition, such as passenger cars and trucks

The methods are all approximate. In particular, the method for two-way STOP-sign control can be conservative, with observed delays and queuing shorter than those modeled.

⁹ TRB, *Highway Capacity Manual 2000* (Washington DC 2000) and *Highway Capacity Manual 2010* (Washington DC, 2010).

тғрр

Table 7. Level-of-se	Table 7. Level-of-service criteria for intersections.											
	Control Delay (seconds/vehicle)											
Level of Service	Unsignalized Intersections ^a	Signalized Intersections										
А	≤10.0	≤10.0										
В	$>10.0 \text{ and } \le 15.0$	$>10.0 \text{ and } \le 20.0$										
С	>15.0 and ≤25.0	>20.0 and ≤35.0										
D	>25.0 and ≤35.0	>35.0 and ≤55.0										
Е	>35.0 and ≤ 50.0	>55.0 and ≤80.0										
F	>50	>80										

From Transportation Research Board, Highway Capacity Manual 2010 (Washington D.C., 2010).

^a For YIELD sign, two-way STOP sign or all-way STOP sign, control delay defines LOS. For roundabout approaches and overall intersection, control delay defines LOS. For roundabout lanes with volume/capacity ratio ≤1.0, control delay defines LOS. For roundabout lanes with volume/capacity ratio > 1.0, LOS is F regardless of control delay.

RESULTS

Table 8 shows computed LOS, delays, and queues at study-area intersections for the:

- weekday AM-street-peak hour
- weekday PM-street-peak hour

The analysis is under the following conditions, as applicable:

- 2021 existing
- 2022 and 2032 no build
- 2022 and 2032 build
- 2022 and 2032 build with a potential right-turn lane on the Derry Road southbound approach to the driveway intersection

Capacity-analysis worksheets that give detail and explanation are in Appendix E.

Table 8 shows, for the Derry Road/Ledge Road intersection

- low delays for left turns from Derry Road
- moderate delays or delayed operations for movements from Ledge Road
- insignificant project impacts

Table 8 shows, for the Derry Road/driveway intersection:

- low delays for left turns from Derry Road
- moderate delays or delayed operations for movements from the driveway

Delayed operations on minor-street approaches to high-volume arterials are typical and acceptable.

Intersection, Control,		2021 E	xisting			2022 1	No Build			2032 No Build			2022 Build				2032 Build			
Hour and Movement	LOS ^a	Delay ^b	V/C ^c	Queued	LOS	Delay	V/C	Queue	LOS	Delay	V/C	Queue	LOS	Delay	V/C	Queue	LOS	Delay	V/C	Queue
Derry Road/Ledge Road	Intersection,	Unsignalize	d, Weekda	y AM-Street-	Peak Hour															
Derry Road SB L	В	10.7	0.019	0.1	В	10.9	0.020	0.1	В	11.6	0.024	0.1	В	11.0	0.020	0.1	В	11.6	0.024	0.1
Ledge Road WB LR	D	33.3	0.370	1.6	D	31.8	0.307	1.2	Е	41.2	0.402	1.8	Е	35.2	0.333	1.4	Е	41.6	0.405	1.8
Derry Road/Ledge Road	Intersection,	Unsignalize	d, Weekda	y PM-Street-	Peak Hour															
Derry Road SB L	В	11.7	0.037	0.1	В	12.6	0.045	0.1	В	13.7	0.056	0.2	В	12.6	0.045	0.1	В	13.7	0.056	0.2
Ledge Road WB LR	D	29.6	0.250	1.0	D	33.4	0.225	0.8	Е	42.9	0.305	1.2	D	33.6	0.226	0.8	Е	43.3	0.307	1.2
Derry Road/Driveway Int	ersection, N	o Right-Turi	n Lane on I	Derry Road S	outhbound	Approach, U	Jnsignalized	, Weekday A	M-Street-P	eak Hour										
Derry Road NB L													В	12.9	0.053	0.2	В	14.1	0.066	0.02
Driveway EB L													Е	37.5	0.188	0.7	Е	46.9	0.246	0.9
Driveway EB R													D	30.5	0.192	0.7	Е	39.3	0.266	1.0
Derry Road/Driveway Int	ersection, N	o Right-Turr	n Lane on I	Derry Road Se	outhbound A	Approach, U	Jnsignalized	, Weekday P	M-Street-P	eak Hour										
Derry Road NB L													В	11.2	0.021	0.1	В	11.9	0.025	0.1
Driveway EB L													Е	36.1	0.095	0.3	Е	42.5	0.113	0.4
Driveway EB R													С	21.0	0.043	0.1	С	24.3	0.061	0.2
Derry Road/Driveway Int	ersection, w	ith Right-Tu	rn Lane on	Derry Road	Southbound	Approach,	Unsignalize	d, Weekday	AM-Street-	Peak Hour										
Derry Road NB L													В	12.9	0.053	0.2	В	14.1	0.066	0.02
Driveway EB L													Е	37.2	0.187	0.7	Е	46.0	0.242	0.9
Driveway EB R													D	29.8	0.187	0.7	Е	38.1	0.259	1.0
Derry Road/Driveway Int	ersection, w	ith Right-Tu	rn Lane on	Derry Road	Southbound	Approach,	Unsignalize	d, Weekday I	PM-Street-	Peak Hour										
Derry Road NB L													В	11.2	0.021	0.1	В	11.9	0.025	0.1
Driveway EB L													Е	36.1	0.095	0.3	Е	42.5	0.113	0.4
Driveway EB R													С	20.9	0.042	0.1	С	24.2	0.061	0.2

^a LOS = level of service.

^b Delay = average delay in seconds per vehicle.

^c V/C = volume/capacity ratio.

^d 95th percentile queue in vehicles.

EB = eastbound, WB = westbound, SB = southbound, NB = northbound, L = left, T = through, R = right.

SP #08-21 - Aroma Joe's - Attachment C

TEPP

CONCLUSION

PROJECT DESCRIPTION

The proposed redevelopment will:

- be at 56 Derry Road
- provide one drive-through coffee shop with no indoor seating and a floor area of about 900 sf
- have one driveway to the west side of Derry Road, with a one-lane entrance and a twolane exit, and with a potential right-turn lane on the Derry Road southbound approach

TRIP GENERATION

Total trips appear on the site driveway but not all are added to Derry Road near the site. 2022 total vehicle-trips are:

- weekday daily, 629 (total of in and out)
- weekday AM-street-peak hour, 106 (53 in and 53 out)
- weekday PM-street-peak hour, 40 (20 in and 20 out)

2032 total vehicle-trips are:

- weekday daily, 694 (total of in and out)
- weekday AM-street-peak hour, 117 (58 in and 539 out)
- weekday PM-street-peak hour, 44 (22 in and 22 out)

Primary trips are added to Derry Road near the site. 2022 primary vehicle-trips are:

- weekday daily, 69 (total of in and out)
- weekday AM-street-peak hour, 12 (6 in and 6 out)
- weekday PM-street-peak hour, 4 (2 in and 2 out)

2032 primary vehicle-trips are:

- weekday daily, 78 (total of in and out)
- weekday AM-street-peak hour, 13 (6 in and 7 out)

• weekday PM-street-peak hour, 6 (3 in and 3 out)

CAPACITY ANALYSIS

Capacity analysis shows, for the Derry Road/Ledge Road intersection

- low delays for left turns from Derry Road
- moderate delays or delayed operations for movements from Ledge Road
- insignificant project impacts

Capacity analysis shows, for the Derry Road/driveway intersection:

- low delays for left turns from Derry Road
- moderate delays or delayed operations for movements from the driveway

Delayed operations on minor-street approaches to high-volume arterials are typical and acceptable.

TRAFFIC IMPACTS

Analysis indicates no significant area impact due to the proposed redevelopment.

SP #08-21 - Aroma Joe's - Attachment C

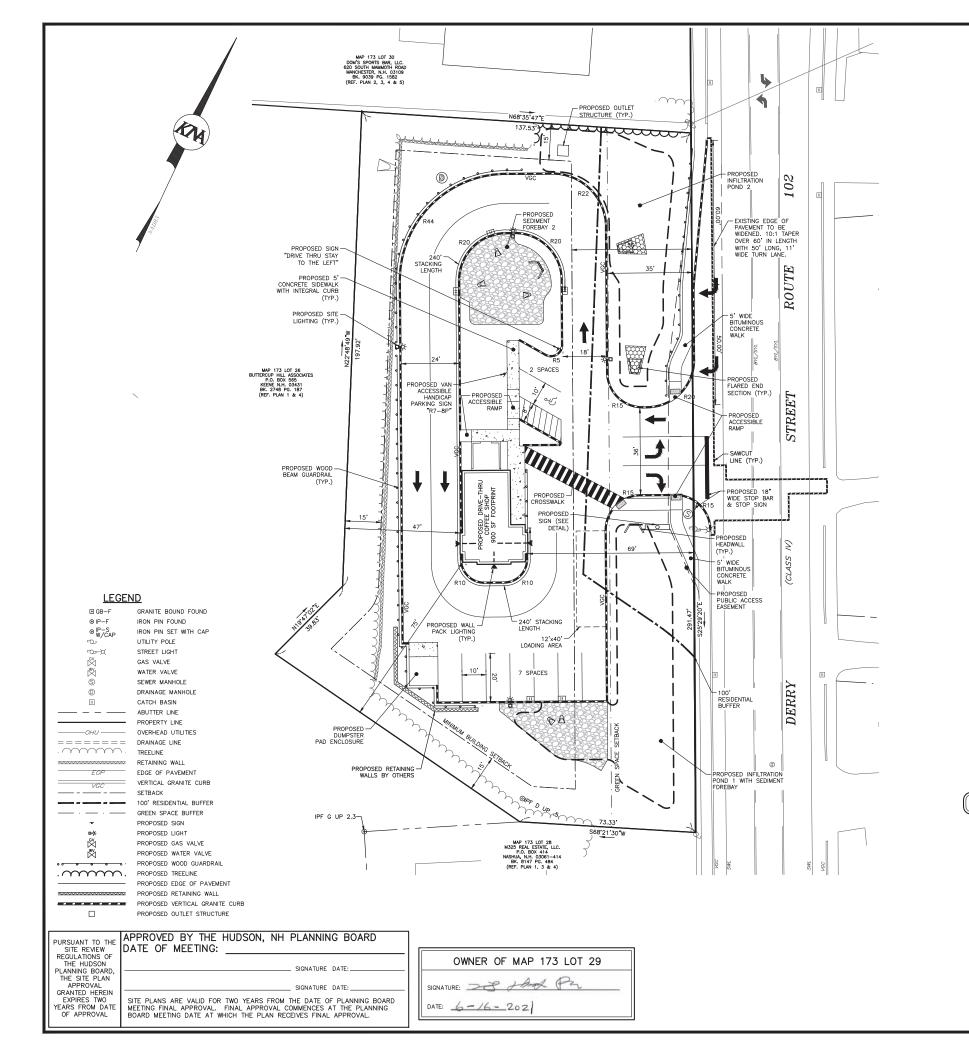


APPENDIX

1553 20210831 TIAS Revised Body.docx



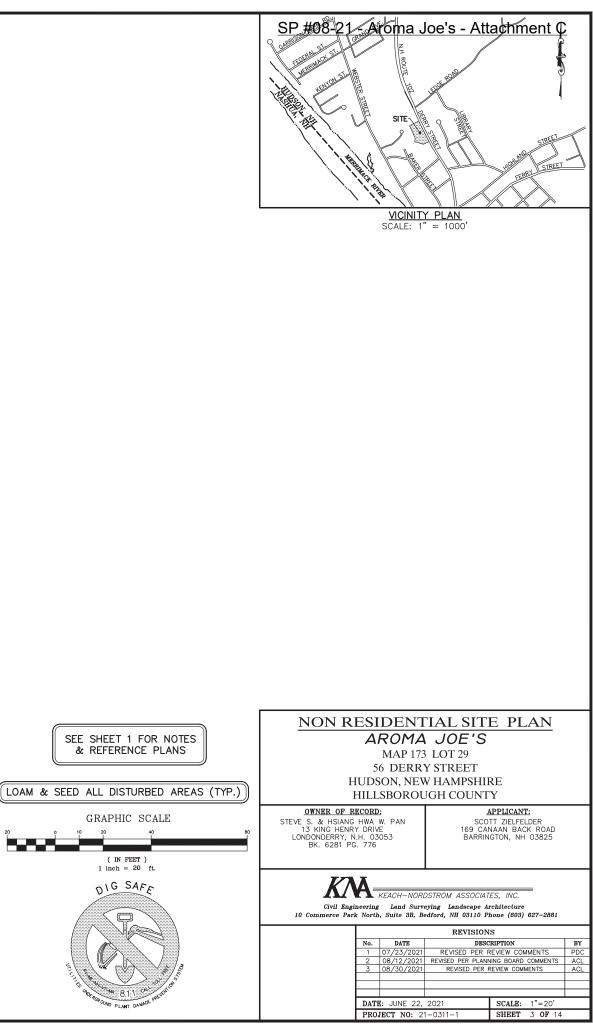
Appendix A: Project Plan

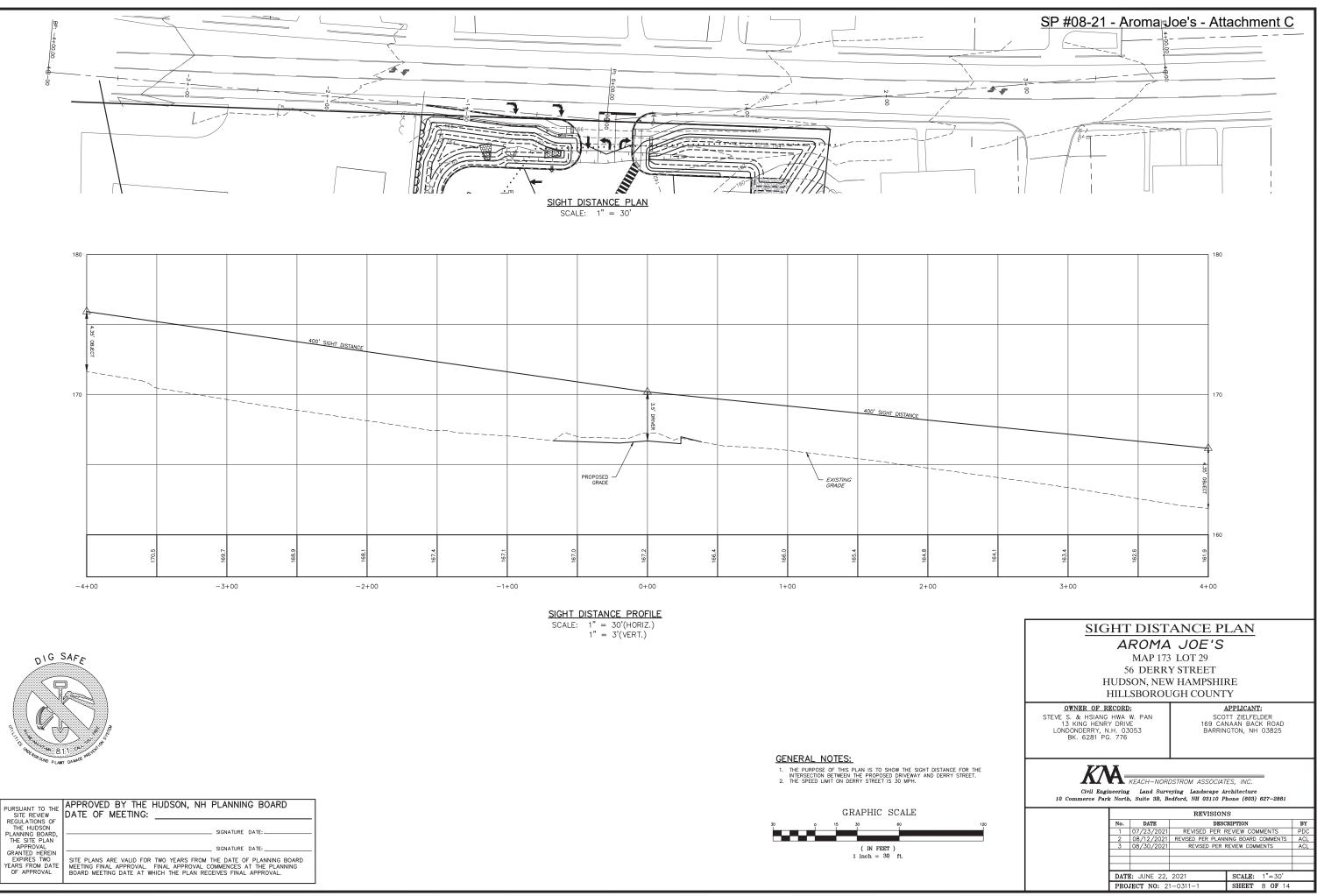


SEE SHEET 1 FOR NOTES & REFERENCE PLANS

GRAPHIC SCALE

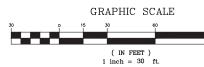
(IN FEET) 1 inch = 20 ft. DIG SAFE







	APPROVED BY THE HUDSON, NH PLANNING BOARD DATE OF MEETING:
REGULATIONS OF THE HUDSON PLANNING BOARD, THE SITE PLAN	SIGNATURE DATE:
APPROVAL	SIGNATURE_DATE:
GRANTED HEREIN EXPIRES TWO YEARS FROM DATE OF APPROVAL	SITE PLANS ARE VALID FOR TWO YEARS FROM THE DATE OF PLANNING BOARD MEETING FINAL APPROVAL. FINAL APPROVAL COMMENCES AT THE PLANNING BOARD MEETING DATE AT WHICH THE PLAN RECEIVES FINAL APPROVAL.





Appendix B: Traffic Counts

15530001

Accurate Counts 978-664-2565

Location : Derry Road Location : South of Ledge Road

LUCATION .	30utii 0i	Leuge Roau	
City/State	Hudson	NH	

5/31/2021	Monday		Tuesc		Wednes		Thurso		Frid		Saturo		Sunda		Week Average	
Time	SB, N	۱B,	SB,	NB,	SB,	NB,	SB,	NB,	SB,	NB,	SB,	NB,	SB,	NB,	SB,	NB,
12:00 AM	*	*	*	*	33	41	44	51	*	*	*	*	*	*	38	46
1:00	*	*	*	*	27	22	28	27	*	*	*	*	*	*	28	24
2:00	*	*	*	*	18	19	36	23	*	*	*	*	*	*	27	21
3:00	*	*	*	*	46	40	49	23	*	*	*	*	*	*	48	32
4:00	*	*	*	*	155	75	138	91	*	*	*	*	*	*	146	83
5:00	*	*	*	*	428	241	421	264	*	*	*	*	*	*	424	252
6:00	*	*	*	*	759	544	744	490	*	*	*	*	*	*	752	517
7:00	*	*	*	*	1048	842	1026	871	*	*	*	*	*	*	1037	856
8:00	*	*	*	*	956	735	945	774	*	*	*	*	*	*	950	754
9:00	*	*	*	*	767	654	721	664	*	*	*	*	*	*	744	659
10:00	*	*	*	*	813	640	748	673	*	*	*	*	*	*	780	656
11:00	*	*	*	*	738	755	810	690	*	*	*	*	*	*	774	722
12:00 PM	*	*	*	*	840	799	819	814	*	*	*	*	*	*	830	806
1:00	*	*	*	*	798	808	825	877	*	*	*	*	*	*	812	842
2:00	*	*	*	*	1013	921	1008	986	*	*	*	*	*	*	1010	954
3:00	*	*	*	*	981	1065	979	1072	*	*	*	*	*	*	980	1068
4:00	*	*	*	*	927	1163	1009	1120	*	*	*	*	*	*	968	1142
5:00	*	*	*	*	943	1106	950	1098	*	*	*	*	*	*	946	1102
6:00	*	*	*	*	783	883	700	841	*	*	*	*	*	*	742	862
7:00	*	*	*	*	605	642	528	638	*	*	*	*	*	*	566	640
8:00	*	*	*	*	448	507	396	447	*	*	*	*	*	*	422	477
9:00	*	*	*	*	267	329	264	304	*	*	*	*	*	*	266	316
10:00	*	*	*	*	154	165	164	171	*	*	*	*	*	*	159	168
11:00	*	*	*	*	72	99	78	104	*	*	*	*	*	*	75	102
Total	0	0	0	0	13619	13095	13430	13113	0	0	0	0	0	0	13524	13101
Day	0		0		2671	4	2654	3	0		0		0		2662	25
AM Peak					7:00	7:00	7:00	7:00							7:00	7:00
Volume					1048	842	1026	871							1037	856
PM Peak					2:00	4:00	4:00	4:00							2:00	4:00
Volume					1013	1163	1009	1120							1010	1142
Comb Total	0		0		2671	4	2654	3	0		0		0		2662	25
ADT	ADT: 2	26,628	AAD	T: 26,628												

N/S Street : Derry Road E/W Street : Ledge Road City/State : Hudson, NH Weather : Clear

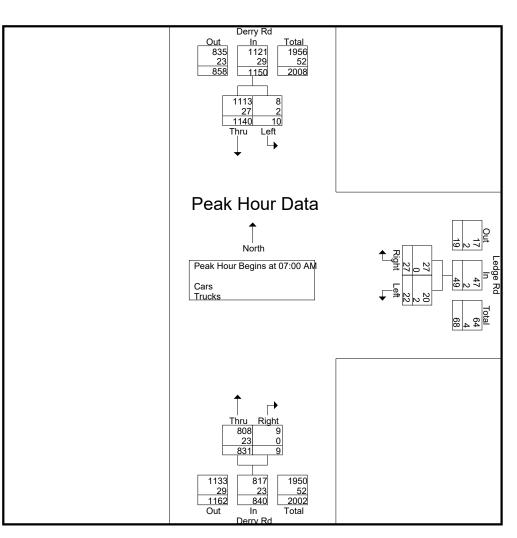
		Grov	ups Printed- Cars -	- Trucks			
	Derry Rd		Ledge	e Rd	Derry		
	From North	<u>ה </u>	From E	East	From S	South	
Start Time	Left	Thru	Left	Right	Thru	Right	Int. Total
07:00 AM	3	246	7	10	211	3	480
07:15 AM	3	297	7	5	225	0	537
07:30 AM	1	319	5	4	191	3	523
07:45 AM	3	278	3	8	204	3	499
Total	10	1140	22	27	831	9	2039
				1		1	I
08:00 AM	2	251	3	3	162	4	425
08:15 AM	2	250	2	4	153	3	414
08:30 AM	2	288	7	2	175	2	476
08:45 AM	2	240	3	5	212	6	468
Total	8	1029	15	14	702	15	1783
Grand Total	18	2169	37	41	1533	24	3822
Apprch %	0.8	99.2	47.4	52.6	98.5	1.5	UULL
Total %	0.5	56.8	ب ، ب ب 1	1.1	40.1	0.6	
Cars	16	2092	34	40	1492	23	3697
% Cars	88.9	96.4	91.9	97.6	97.3	23 95.8	96.7
		90.4					
Trucks	2		3		41	1	125
% Trucks	11.1	3.6	8.1	2.4	2.7	4.2	3.3

		Derry Rd From North			Ledge Rd From East			Derry Rd From South				
Start Time	Left	Thru	App. Total	Left	Right	App. Total	Thru	Right	App. Total	Int. Total		
Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1												
Peak Hour for Entire Inter	section Begir	ns at 07:00 Al	M									
07:00 AM	3	246	249	7	10	17	211	3	214	480		
07:15 AM	3	297	300	7	5	12	225	0	225	537		
07:30 AM	1	319	320	5	4	9	191	3	194	523		
07:45 AM	3	278	281	3	8	11	204	3	207	499		
Total Volume	10	1140	1150	22	27	49	831	9	840	2039		
% App. Total	0.9	99.1		44.9	55.1		98.9	1.1				
PHF	.833	.893	.898	.786	.675	.721	.923	.750	.933	.949		
Cars	8	1113	1121	20	27	47	808	9	817	1985		
% Cars	80.0	97.6	97.5	90.9	100	95.9	97.2	100	97.3	97.4		
Trucks	2	27	29	2	0	2	23	0	23	54		
% Trucks	20.0	2.4	2.5	9.1	0	4.1	2.8	0	2.7	2.6		

Accurate Counts 978-664-2565



File Name : 15530001 Site Code : 15530001 Start Date : 6/2/2021 Page No : 2



Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1 Peak Hour for Each Approach Begins at:

Feak Hour for Lacit Appr	bach Degins a	L.							
	07:15 AM			07:00 AM			07:00 AM		
+0 mins.	3	297	300	7	10	17	211	3	214
+15 mins.	1	319	320	7	5	12	225	0	225
+30 mins.	3	278	281	5	4	9	191	3	194
+45 mins.	2	251	253	3	8	11	204	3	207
Total Volume	9	1145	1154	22	27	49	831	9	840
% App. Total	0.8	99.2		44.9	55.1		98.9	1.1	
PHF	.750	.897	.902	.786	.675	.721	.923	.750	.933
Cars	8	1120	1128	20	27	47	808	9	817
% Cars	88.9	97.8	97.7	90.9	100	95.9	97.2	100	97.3
Trucks	1	25	26	2	0	2	23	0	23
% Trucks	11.1	2.2	2.3	9.1	0	4.1	2.8	0	2.7

SP #08-21 - Aroma Joe's - Attachment C

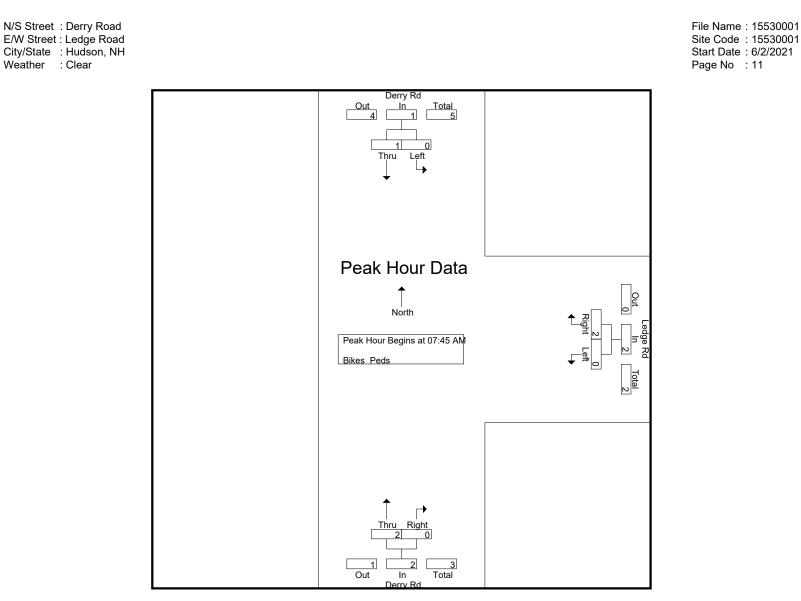
N/S Street : Derry Road E/W Street : Ledge Road City/State : Hudson, NH Weather : Clear

File Name : 15530001 Site Code : 15530001 Start Date : 6/2/2021 Page No : 10

	Groups Printed- Bikes Peds													
	D	erry Rd		L	edge Rd		Γ	Derry Rd						
	Fre	om North		From East			From South							
Start Time	Left	Thru	Peds	Left	Right	Peds	Thru	Right	Peds	Exclu. Total	Inclu. Total	Int. Total		
07:00 AM	0	0	0	0	0	0	0	0	0	0	0	0		
07:15 AM	0	0	0	0	0	0	0	0	0	0	0	0		
07:30 AM	0	0	0	0	0	0	0	0	0	0	0	0		
07:45 AM	0	0	0	0	0	0	2	0	0	0	2	2		
Total	0	0	0	0	0	0	2	0	0	0	2	2		
08:00 AM	0	0	0	0	0	0	0	0	0	0	0	0		
08:15 AM	0	0	0	0	0	0	0	0	0	0	0	0		
08:30 AM	0	1	0	0	2	1	0	0	0	1	3	4		
08:45 AM	0	0	0	0	0	0	0	0	0	0	0	0		
Total	0	1	0	0	2	1	0	0	0	1	3	4		
Grand Total	0	1	0	0	2	1	2	0	0	1	5	6		
Apprch %	0	100		0	100		100	0						
Total %	0	20		0	40		40	0		16.7	83.3			

		Derry Rd			Ledge Rd						
		From North			From East						
Start Time	Left	Thru	App. Total	Left	Right	App. Total	Thru	Right	App. Total	Int. Total	
Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1											
Peak Hour for Entire Inter	rsection Begir	ns at 07:45 AN	1								
07:45 AM	Ō	0	0	0	0	0	2	0	2	2	
08:00 AM	0	0	0	0	0	0	0	0	0	0	
08:15 AM	0	0	0	0	0	0	0	0	0	0	
08:30 AM	0	1	1	0	2	2	0	0	0	3	
Total Volume	0	1	1	0	2	2	2	0	2	5	
% App. Total	0	100		0	100		100	0			
PHF	.000	.250	.250	.000	.250	.250	.250	.000	.250	.417	

Accurate Counts 978-664-2565



Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1 Peak Hour for Each Approach Begins at:

Feak Hour for Each Appr	Dacit Degins a	ι.								
	07:45 AM			07:45 AM			07:00 AM			
+0 mins.	0	0	0	0	0	0	0	0	0	
+15 mins.	0	0	0	0	0	0	0	0	0	
+30 mins.	0	0	0	0	0	0	0	0	0	
+45 mins.	0	1	1	0	2	2	2	0	2	
Total Volume	0	1	1	0	2	2	2	0	2	
% App. Total	0	100		0	100		100	0		
PHF	.000	.250	.250	.000	.250	.250	.250	.000	.250	

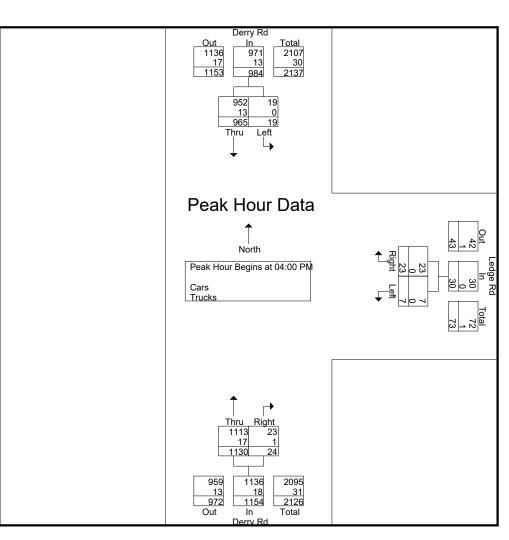
N/S Street : Derry Road E/W Street : Ledge Road City/State : Hudson, NH Weather : Clear

		Grou	ups Printed- Cars - T	rucks			
	Derry Rd		Ledge R	Rd	Derry		
	From North		From Ea	JSt	From Se	outh	
Start Time	Left	Thru	Left	Right	Thru	Right	Int. Total
04:00 PM	2	251	2	6	284	8	553
04:15 PM	2	239	1	6	277	6	531
04:30 PM	6	227	1	3	287	5	529
04:45 PM	9	248	3	8	282	5	555
Total	19	965	7	23	1130	24	2168
05:00 PM	3	237	1	6	258	6	511
05:15 PM	6	269	4	5	282	6	572
05:30 PM	8	220	2	8	261	6	505
05:45 PM	3	244	- 1	5	277	4	534
Total	20	970	8	24	1078	22	2122
Grand Total	39	1935	15	47	2208	46	4290
Apprch %	2	98	24.2	75.8	98	2	
Total %	0.9	45.1	0.3	1.1	51.5	1.1	
Cars	39	1913	15	47	2188	45	4247
% Cars	100	98.9	100	100	99.1	97.8	4247 99
				0	20		43
Trucks	0	22	0	-			43
% Trucks	0	1.1	0	0	0.9	2.2	1

		Derry Rd From North			Ledge Rd From East					
Start Time	Left	Thru	App. Total	Left	Right	App. Total	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From	04:00 PM to	05:45 PM - P	eak 1 of 1							
Peak Hour for Entire Inter	section Begin	ns at 04:00 PN	Λ							
04:00 PM	2	251	253	2	6	8	284	8	292	553
04:15 PM	2	239	241	1	6	7	277	6	283	531
04:30 PM	6	227	233	1	3	4	287	5	292	529
04:45 PM	9	248	257	3	8	11	282	5	287	555
Total Volume	19	965	984	7	23	30	1130	24	1154	2168
% App. Total	1.9	98.1		23.3	76.7		97.9	2.1		
PHF	.528	.961	.957	.583	.719	.682	.984	.750	.988	.977
Cars	19	952	971	7	23	30	1113	23	1136	2137
% Cars	100	98.7	98.7	100	100	100	98.5	95.8	98.4	98.6
Trucks	0	13	13	0	0	0	17	1	18	31
% Trucks	0	1.3	1.3	0	0	0	1.5	4.2	1.6	1.4

Accurate Counts 978-664-2565

N/S Street : Derry Road E/W Street : Ledge Road City/State : Hudson, NH Weather : Clear File Name : 15530001 Site Code : 15530001 Start Date : 6/2/2021 Page No : 2



Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1 Peak Hour for Each Approach Begins at:

Feak Hour for Lach Appr	ouch Degins at	•							
	04:30 PM			04:45 PM			04:00 PM		
+0 mins.	6	227	233	3	8	11	284	8	292
+15 mins.	9	248	257	1	6	7	277	6	283
+30 mins.	3	237	240	4	5	9	287	5	292
+45 mins.	6	269	275	2	8	10	282	5	287
Total Volume	24	981	1005	10	27	37	1130	24	1154
% App. Total	2.4	97.6		27	73		97.9	2.1	
PHF	.667	.912	.914	.625	.844	.841	.984	.750	.988
Cars	24	975	999	10	27	37	1113	23	1136
% Cars	100	99.4	99.4	100	100	100	98.5	95.8	98.4
Trucks	0	6	6	0	0	0	17	1	18
% Trucks	0	0.6	0.6	0	0	0	1.5	4.2	1.6

SP #08-21 - Aroma Joe's - Attachment C

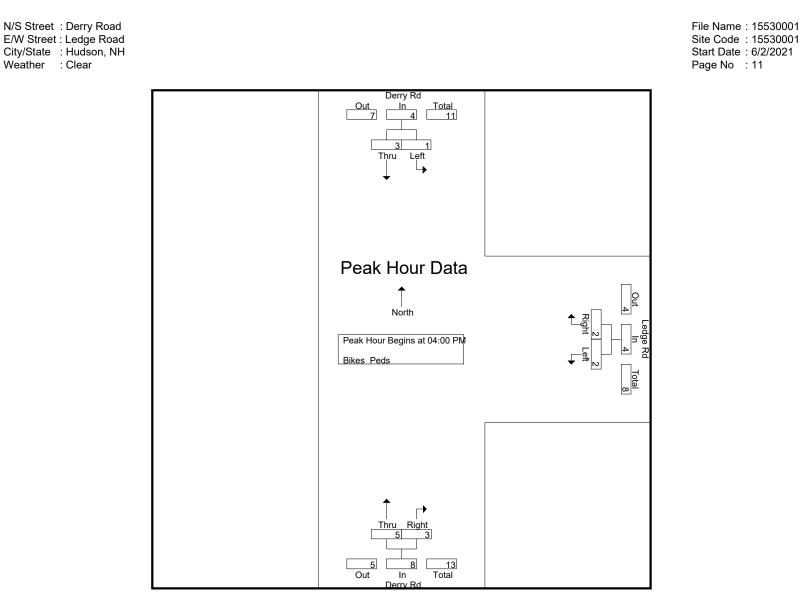
N/S Street : Derry Road E/W Street : Ledge Road City/State : Hudson, NH Weather : Clear

File Name : 15530001 Site Code : 15530001 Start Date : 6/2/2021 Page No : 10

					Groups Prin	ted- Bikes	Peds					
	D	erry Rd		Ledge Rd			C	Derry Rd				
	Fro	om North		Fi	rom East		Fr	om South				
Start Time	Left	Thru	Peds	Left	Right	Peds	Thru	Right	Peds	Exclu. Total	Inclu. Total	Int. Total
04:00 PM	0	0	0	0	0	0	1	0	1	1	1	2
04:15 PM	0	2	1	0	1	0	1	1	0	1	5	6
04:30 PM	1	1	0	0	0	0	3	2	0	0	7	7
04:45 PM	0	0	0	2	1	0	0	0	0	0	3	3
Total	1	3	1	2	2	0	5	3	1	2	16	18
05:00 PM	0	0	0	0	0	0	0	1	0	0	1	1
05:15 PM	0	1	0	0	0	0	0	0	0	0	1	1
05:30 PM	0	1	0	0	0	0	0	0	0	0	1	1
05:45 PM	0	0	0	4	0	2	0	0	0	2	4	6
Total	0	2	0	4	0	2	0	1	0	2	7	9
Grand Total	1	5	1	6	2	2	5	4	1	4	23	27
Apprch %	16.7	83.3		75	25		55.6	44.4				
Total %	4.3	21.7		26.1	8.7		21.7	17.4		14.8	85.2	

	Derry Rd				Ledge Rd					
		From North	า		From East			From South		
Start Time	Left	Thru	App. Total	Left	Right	App. Total	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From	04:00 PM to	05:45 PM - I	Peak 1 of 1							
Peak Hour for Entire Inter	section Begi	ns at 04:00 P	Μ							
04:00 PM	Ō	0	0	0	0	0	1	0	1	1
04:15 PM	0	2	2	0	1	1	1	1	2	5
04:30 PM	1	1	2	0	0	0	3	2	5	7
04:45 PM	0	0	0	2	1	3	0	0	0	3
Total Volume	1	3	4	2	2	4	5	3	8	16
% App. Total	25	75		50	50		62.5	37.5		
PHF	.250	.375	.500	.250	.500	.333	.417	.375	.400	.571

Accurate Counts 978-664-2565



Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1 Peak Hour for Each Approach Begins at:

	04:00 PM			04:00 PM			04:00 PM			
+0 mins.	0	0	0	0	0	0	1	0	1	
+15 mins.	0	2	2	0	1	1	1	1	2	
+30 mins.	1	1	2	0	0	0	3	2	5	
+45 mins.	0	0	0	2	1	3	0	0	0	
Total Volume	1	3	4	2	2	4	5	3	8	
% App. Total	25	75		50	50		62.5	37.5		
PHF	.250	.375	.500	.250	.500	.333	.417	.375	.400	



Appendix C: Monthly Traffic Volumes

Year 2019 Monthly Data

Group 4 Averages:

Urban Highways

		Adjustment	Adjustment
<u>Month</u>	<u>ADT</u>	to Average	<u>to Peak</u>
January	11,431	1.12	1.23
February	11,848	1.08	1.18
March	12,141	1.06	1.15
April	12,860	1.00	1.09
May	13,551	0.95	1.03
June	13,785	0.93	1.02
July	13,942	0.92	1.01
August	14,016	0.92	1.00
September	13,379	0.96	1.05
October	13,339	0.96	1.05
November	12,265	1.05	1.14
December	11,496	1.12	1.22
Average ADT:	12,838		
Peak ADT:	14,016		



Appendix D: Vehicle Speeds

Accurate Counts 978-664-2565

Location : Derry Road Location : South of Ledge Road

City/State: Hudson, NH

Direction: SB,

6/2/2021 > 15 -> 20 -> 25 -> 30 -> 35 -> 40 -> 45 -> 50 -> 55 -> 60 -> 65 -> 70 0 - 15 MPH 20 MPH 25 MPH 30 MPH 35 MPH 40 MPH 45 MPH 50 MPH 55 MPH 60 MPH 65 MPH 70 MPH MPH Time Total 12:00 AM 1:00 2:00 3:00 4:00 5:00 6:00 7:00 8:00 9:00 10:00 11:00 12:00 PM 1:00 2:00 3:00 4:00 5:00 6:00 7:00 8:00 9:00 10:00 11:00 Total Percentile 15th 50th 85th 95th

29.7 33.5

37.8 40.3

Mean Speed (Average) 33.7 10 MPH Pace Speed 30-39 Number in Pace 10644 Percent in Pace 78.2%

Speed

 Number > 30 MPH
 11402

 Percent > 30 MPH
 83.7%

Accurate Counts 978-664-2565

Location : Derry Road

Location .	oouun oi	Lougo Road
City/State:	Hudson,	NH

Direction: SB,

> 65 -6/3/2021 > 20 -> 25 -> 30 -> 35 -> 40 -> 45 -> 50 -> 55 -> 60 -> 70 > 15 -0 - 15 20 MPH 25 MPH 30 MPH 35 MPH 40 MPH 45 MPH 50 MPH 55 MPH 60 MPH 65 MPH 70 MPH MPH Time MPH Total 12:00 AM 1:00 2:00 3:00 4:00 5:00 6:00 7:00 8:00 9:00 10:00 11:00 12:00 PM 1:00 2:00 3:00 4:00 5:00 6:00 7:00 8:00 9:00 10:00 11:00 Total Percentile 15th 50th 85th 95th Speed 29.7 33.5 37.2 39.7 Mean Speed (Average) 33.5 10 MPH Pace Speed 30-39 Number in Pace Percent in Pace 76.6% Number > 30 MPH Percent > 30 MPH 81.9% Grand Total Stats Percentile 15th 50th 85th 95th 29.7 39.7 Speed 33.5 37.2 Mean Speed (Average) 33.6 10 MPH Pace Speed 30-39 Number in Pace Percent in Pace 77.4% Number > 30 MPH Percent > 30 MPH 82.8%

Accurate Counts 978-664-2565

Location : Derry Road Location : South of Ledge Road

Loodalon i Coali oi Loago i i	
City/State: Hudson, NH	
Direction: NB,	

Direction: NB,														
6/2/2021	0 - 15	> 15 -	> 20 -	> 25 -	> 30 -	> 35 -	> 40 -	> 45 -	> 50 -	> 55 -	> 60 -	> 65 -	> 70	
Time	MPH	20 MPH	25 MPH	30 MPH	35 MPH	40 MPH	45 MPH	50 MPH	55 MPH	60 MPH	65 MPH	70 MPH	MPH	Total
12:00 AM	0	0	1	1	7	23	7	2	0	0	0	0	0	41
1:00	0	0	0	0	10	10	2	0	0	0	0	0	0	22
2:00	0	0	0	2	4	6	5	2	0	0	0	0	0	19
3:00	0	0	0	0	7	18	7	7	0	0	0	0	1	40
4:00	0	0	2	0	11	29	27	4	0	0	0	0	2	75
5:00	0	0	2	3	38	102	77	17	0	1	0	0	1	241
6:00	0	5	21	61	133	217	94	11	2	0	0	0	0	544
7:00	3	1	17	87	235	404	86	7	1	0	0	0	1	842
8:00	3	1	24	63	221	352	68	1	0	0	0	0	2	735
9:00	1	2	12	34	204	323	70	5	0	0	0	1	2	654
10:00	3	6	8	30	238	274	72	7	0	0	0	0	2	640
11:00	2	6	8	63	275	312	80	6	0	0	0	1	2	755
12:00 PM	2	7	16	39	296	362	72	5	0	0	0	0	0	799
1:00	6	2	17	90	312	317	60	3	1	0	0	0	0	808
2:00	3	2	4	75	349	411	70	5	0	0	0	1	1	921
3:00	4	4	20	86	423	431	94	3	0	0	0	0	0	1065
4:00	6	8	24	204	498	389	32	1	0	0	0	0	1	1163
5:00	0	3	6	105	408	493	86	5	0	0	0	0	0	1106
6:00	3	5	3	45	293	418	104	11	0	0	0	0	1	883
7:00	0	3	3	28	179	348	70	9	2	0	0	0	0	642
8:00	0	1	5	41	189	222	42	7	0	0	0	0	0	507
9:00	1	0	0	13	103	159	50	3	0	0	0	0	0	329
10:00	0	0	2	10	44	81	25	2	0	1	0	0	0	165
11:00	0	0	3	3	23	51	15	4	0	0	0	0	0	99
Total	37	56	198	1083	4500	5752	1315	127	6	2	0	3	16	13095
			Percentile	15th	50th	85th	95th							

35.3

39

41.5

Speed 31 Mean Speed (Average) 35.4 10 MPH Pace Speed 30-39 Number in Pace 10159 Percent in Pace 77.6%

Number > 30 MPH 11721 Percent > 30 MPH 89.5%

15530001

Accurate Counts 978-664-2565

Location : Derry Road Location : South of Ledge Road

City/State: Hudson, NH Direction: NB,
Direction NB
Biroodon. HD,

Direction. ND,														
6/3/2021	0 - 15	> 15 -	> 20 -	> 25 -	> 30 -	> 35 -	> 40 -	> 45 -	> 50 -	> 55 -	> 60 -	> 65 -	> 70	
Time	MPH	20 MPH	25 MPH	30 MPH	35 MPH	40 MPH	45 MPH	50 MPH	55 MPH	60 MPH	65 MPH	70 MPH	MPH	Total
12:00 AM	0	1	0	5	9	27	7	0	0	0	1	1	0	51
1:00	0	0	0	2	7	8	8	1	1	0	0	0	0	27
2:00	0	0	1	1	4	7	7	3	0	0	0	0	0	23
3:00	0	0	0	1	5	9	7	0	1	0	0	0	0	23
4:00	0	0	0	4	11	45	25	6	0	0	0	0	0	91
5:00	0	0	5	9	52	108	68	21	0	0	0	0	1	264
6:00	1	2	18	66	104	199	85	11	1	1	0	0	2	490
7:00	4	2	16	103	315	359	62	6	1	0	0	2	1	871
8:00	7	2	20	56	261	326	85	7	1	0	0	0	9	774
9:00	1	1	3	48	218	316	65	9	1	0	0	0	2	664
10:00	1	4	3	51	199	318	90	7	0	0	0	0	0	673
11:00	2	5	9	71	232	270	95	1	0	0	0	3	2	690
12:00 PM	3	3	7	50	310	358	75	8	0	0	0	0	0	814
1:00	2	4	16	74	373	349	56	3	0	0	0	0	0	877
2:00	5	7	15	129	431	347	47	2	0	0	1	2	0	986
3:00	5	2	17	180	447	366	53	1	1	0	0	0	0	1072
4:00	15	16	39	121	396	460	73	0	0	0	0	0	0	1120
5:00	0	5	12	68	433	471	104	5	0	0	0	0	0	1098
6:00	0	0	2	46	317	359	103	13	1	0	0	0	0	841
7:00	3	0	6	38	195	301	89	6	0	0	0	0	0	638
8:00	0	0	1	24	160	215	44	2	1	0	0	0	0	447
9:00	0	3	4	16	99	119	62	1	0	0	0	0	0	304
10:00	0	0	1	13	38	81	30	7	1	0	0	0	0	171
11:00	0	0	1	2	22	53	25	1	0	0		-	0	104
Total	49	-	196	1178	4638	5471	1365	121	10	1	2	8	17	13113
		I	Percentile	15th	50th	85th	95th							
			Speed	31	35.3	39	41.5							
	Mea	an Speed	(Average)	35.3										
	10) MPH Pa	ice Speed	30-39										
		Numbe	er in Pace	10023										
		Percer	nt in Pace	76.4%										
		Number >	> 30 MPH	11633										
		Percent >	> 30 MPH	88.7%										
Grand Total	86	113	394	2261	9138	11223	2680	248	16	3	2	11	33	26208
Stats			Percentile	15th	50th	85th	95th							
			Speed	31	35.3	39	41.5							
	Mea	an Speed	(Average)	35.3										
	10) MPH Pa	ice Speed	30-39										
		Numbe	er in Pace	20182										
		Percer	nt in Pace	77.0%										
		Number >	> 30 MPH	23354										
		Percent >	> 30 MPH	89.1%										



Appendix E: Trip Generation

TRAFFIC IMPACT ASSESSMENT

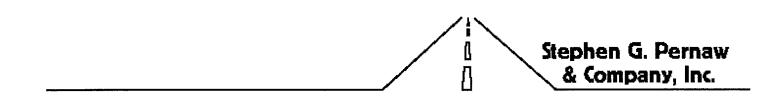
PROPOSED DRIVE-THRU COFFEE SHOP

Northwood, New Hampshire

October 2019

Prepared for

New Hampshire Land Consultants, PLLC



SITE GENERATED TRAFFIC

To estimate the quantity of vehicle trips that will be produced by the proposed drive-thru coffee shop, Pernaw & Company, Inc. considered using the standard trip generation rates published by the Institute of Transportation Engineers (ITE)¹. However, Land Use Code (LUC) 938 (Coffee/Donut Shop with Drive-Through Window and No Indoor Seating) pertains to sites with a very small gross floor area (90 sf average). For this type of land use our experience has confirmed that the traffic volume passing a coffee shop is a far better indicator of vehicle-trips than the square footage of the building.

Consequently, transaction data for a similar drive-thru Aroma Joe's in Tilton, New Hampshire was obtained and combined with Tilton traffic count data to establish a local "capture rate" for that store. The Tilton site is an excellent match as it is also located on a state-maintained highway with a high school located nearby. The AM and PM capture rates were then applied to the 2030 No-Build traffic volumes passing the Northwood site to arrive at the trip generation estimates for the proposed coffee shop. The vehicle-trips associated with the greenhouse shop, office, and single-family residence were estimated using ITE Land Use Codes 820, 712 and 210, respectively. Table 1 on Page 11 summarizes the trip generation estimates for the subject site. The site driveway on Bow Lake Road is expected to accommodate approximately 93 vehicle-trips (47 arrivals, 46 departures) during the morning peak hour and 35 vehicle-trips (17 arrivals, 18 departures) during the evening peak hour period.

It should be noted that the majority of the vehicle-trips generated by the site will be drawn from the existing traffic stream on US4 as "pass-by" trips. According to ITE statistics, approximately 89% of the coffee shop trips will be pass-by trips. The high percentage of pass-by traffic means that that net increases on the adjacent street system will be much less than the total trips shown in Table 1; particularly where the "primary" trips or new trips to the area will be split between points east and west on the highway (and on Bow Lake Road).

Appendix E shows the travel patterns and traffic volumes associated with both trips types, along with the derivation of the trip generation estimates.

 ¹ Institute of Transportation Engineers, *Trip Generation*, 10th edition (Washington, D.C., 2017).
 1951A
 10



Trip Generation Summary

Table 1

	Pass-By	Trips ⁵		40 veh	<u>40 veh</u>	80 trips		14 veh	<u>14 veh</u>	28 trips
	Primary	Trips		7 veh	<u>6 veh</u>	13 trips		3 veh	<u>4 veh</u>	7 trips
	2030	Total		47 veh	<u>46 veh</u>	93 trips		17 veh	<u>18 veh</u>	35 trips
Single-	Family	Residence ⁴		0 veh	<u>1 veh</u>	1 trips		1 veh	<u>0 veh</u>	1 trips
	Office ³	(203 sf)		1 veh	<u>0 veh</u>	1 trips		0 veh	<u>1 veh</u>	1 trips
	Retail ²	(250 sf)		1 veh	<u>0 veh</u>	1 trips		0 veh	<u>1 veh</u>	1 trips
pp (965 sf)	2030	Estimate ¹		45 veh	<u>45 veh</u>	90 trips		16 veh	<u>16 veh</u>	32 trips
Coffee Shop (965 sf)	2020	Estimate ¹		41 veh	<u>41 veh</u>	82 trips		14 veh	<u>14 veh</u>	28 trips
				Entering	Exiting	Total		Entering	Exiting	Total
			AM Peak Hour				PM Peak Hour			

¹ Trip Generation Computations (See Appendix E)

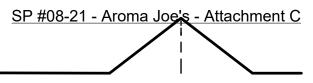
² ITE Land Use Code 820 - Shopping Center (250 sf)

 3 ITE Land Use Code 712 - Small Office Building (203 sf)

 4 ITE Land Use Code 210 - Single-Family Detached Housing (1 Dwelling Unit) 5 ITE Land Use Code 938 - Pass-By Trips = 89%, Page 216 of ITE Handbook

Appendix E

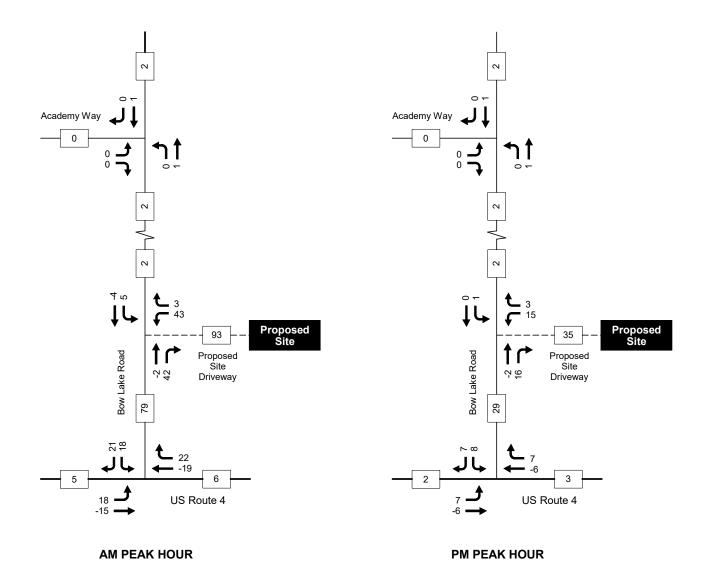
Site Generated Traffic Volumes / Trip Distribution



Pernaw & Company, Inc

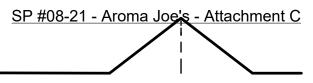
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NORTH



Appendix

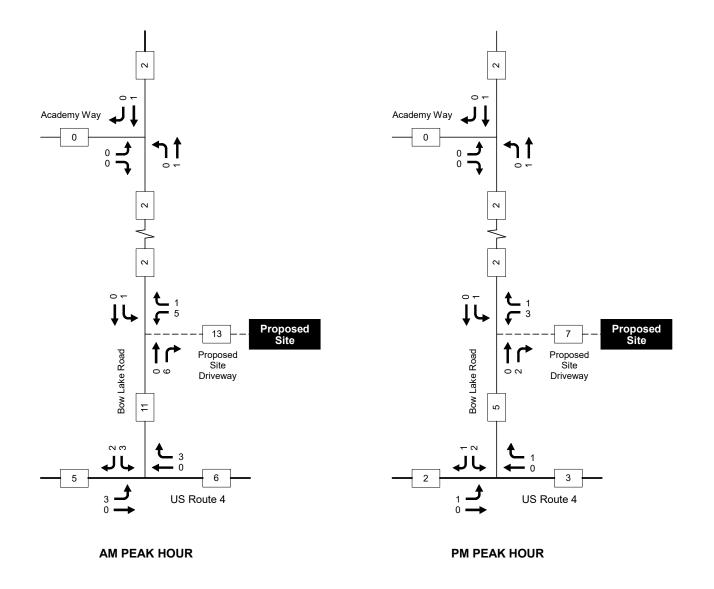
Site Generated Traffic Volumes - 2030 Total Trips Traffic Impact Assessment, Proposed Drive-Thru Coffee Shop, Northwood, New Hampshire



Pernaw & Company, Inc

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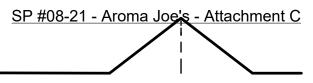
NORTH



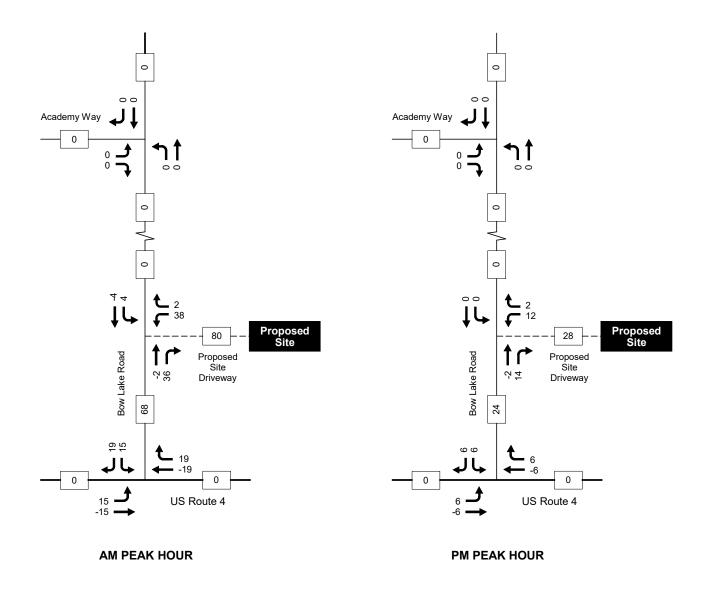
1951A

Appendix

Site Generated Traffic Volumes - 2030 Primary Trips Traffic Impact Assessment, Proposed Drive-Thru Coffee Shop, Northwood, New Hampshire



Pernaw & Company, Inc



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Appendix

1951A

Site Generated Traffic Volumes - 2030 Pass-By Trips Traffic Impact Assessment, Proposed Drive-Thru Coffee Shop, Northwood, New Hampshire

CALCULATION SHEEP #08-21 - Aroma Joe's - Attachment C



Stephen G. Pernaw & Company, Inc.

Project:	Coffee Shop	Job Number:	1951A
Calculated By:	SGP	Date:	10/8/2019
Checked By:	CFA	Date:	10/9/2019
Sheet No:	1	Of:	1
Subject:	Trip Generation Compute	ations	

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SP #08-21 - Aroma Joe's - Attachment C

48AJ- Printed on: 09/16/2019 11:20:05 Page

Store # 48AJ-

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Appendix F: Capacity-Analysis Worksheets

Intersection						
Int Delay, s/veh	1.1					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	۰¥		4		<u>۲</u>	↑
Traffic Vol, veh/h	24	29	895	10	11	1228
Future Vol, veh/h	24	29	895	10	11	1228
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	0	-
Veh in Median Storage	,# 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	72	72	93	93	90	90
Heavy Vehicles, %	9	0	3	0	20	2
Mvmt Flow	33	40	962	11	12	1364

Major/Minor	Minor1	Μ	ajor1	N	lajor2	
Conflicting Flow All	2356	968	0	0	973	0
Stage 1	968	-	-	-	-	-
Stage 2	1388	-	-	-	-	-
Critical Hdwy	6.49	6.2	-	-	4.3	-
Critical Hdwy Stg 1	5.49	-	-	-	-	-
Critical Hdwy Stg 2	5.49	-	-	-	-	-
Follow-up Hdwy	3.581	3.3	-	-	2.38	-
Pot Cap-1 Maneuver	37	311	-	-	642	-
Stage 1	358	-	-	-	-	-
Stage 2	223	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	36	311	-	-	642	-
Mov Cap-2 Maneuver	139	-	-	-	-	-
Stage 1	358	-	-	-	-	-
Stage 2	219	-	-	-	-	-
Annroach	W/R		MR		SR	

Approach	WB	NB	SB	
HCM Control Delay, s	33.3	0	0.1	
HCM LOS	D			

Minor Lane/Major Mvmt	NBT	NBRW	/BLn1	SBL	SBT
Capacity (veh/h)	-	-	199	642	-
HCM Lane V/C Ratio	-	-	0.37	0.019	-
HCM Control Delay (s)	-	-	33.3	10.7	-
HCM Lane LOS	-	-	D	В	-
HCM 95th %tile Q(veh)	-	-	1.6	0.1	-

Intersection						
Int Delay, s/veh	0.7					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	۰¥		el 👘		۳	•
Traffic Vol, veh/h	8	25	1217	26	20	1039
Future Vol, veh/h	8	25	1217	26	20	1039
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	0	-
Veh in Median Storage,	,# 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	68	68	99	99	96	96
Heavy Vehicles, %	0	0	2	4	0	1
Mvmt Flow	12	37	1229	26	21	1082

Major/Minor	Minor1	N	lajor1	Ν	/lajor2	
Conflicting Flow All	2366	1242	0	0	1255	0
Stage 1	1242	-	-	-	-	-
Stage 2	1124	-	-	-	-	-
Critical Hdwy	6.4	6.2	-	-	4.1	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	-	-	2.2	-
Pot Cap-1 Maneuver	39	215	-	-	561	-
Stage 1	275	-	-	-	-	-
Stage 2	313	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	38	215	-	-	561	-
Mov Cap-2 Maneuver	148	-	-	-	-	-
Stage 1	275	-	-	-	-	-
Stage 2	301	-	-	-	-	-
Approach	WB		NB		SB	

Appro	bach	WB	NB	SB	
HCM	Control Delay, s	29.6	0	0.2	
HCM	LOS	D			

Minor Lane/Major Mvmt	NBT	NBRW	/BLn1	SBL	SBT	
Capacity (veh/h)	-	-	194	561	-	
HCM Lane V/C Ratio	-	-	0.25	0.037	-	
HCM Control Delay (s)	-	-	29.6	11.7	-	
HCM Lane LOS	-	-	D	В	-	
HCM 95th %tile Q(veh)	-	-	1	0.1	-	

Intersection						
Int Delay, s/veh	0.8					
			NET			ODT
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	۰Y		- î÷		<u>۲</u>	↑
Traffic Vol, veh/h	24	29	904	10	11	1240
Future Vol, veh/h	24	29	904	10	11	1240
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	0	-
Veh in Median Storage	e, # 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	9	0	3	0	20	2
Mvmt Flow	27	32	1004	11	12	1378

Major/Minor	Minor1	N	lajor1	Ν	/lajor2	
Conflicting Flow All	2412	1010	0	0	1015	0
Stage 1	1010	-	-	-	-	-
Stage 2	1402	-	-	-	-	-
Critical Hdwy	6.49	6.2	-	-	4.3	-
Critical Hdwy Stg 1	5.49	-	-	-	-	-
Critical Hdwy Stg 2	5.49	-	-	-	-	-
Follow-up Hdwy	3.581	3.3	-	-	2.38	-
Pot Cap-1 Maneuver	34	294	-	-	618	-
Stage 1	342	-	-	-	-	-
Stage 2	220	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	33	294	-	-	618	-
Mov Cap-2 Maneuver	135	-	-	-	-	-
Stage 1	342	-	-	-	-	-
Stage 2	216	-	-	-	-	-
Annroach	W/R		NR		SB	

Approach	WB	NB	SB	
HCM Control Delay,	s 31.8	0	0.1	
HCM LOS	D			

Minor Lane/Major Mvmt	NBT	NBRWE	3Ln1	SBL	SBT
Capacity (veh/h)	-	-	192	618	-
HCM Lane V/C Ratio	-	- 0	.307	0.02	-
HCM Control Delay (s)	-	-	31.8	10.9	-
HCM Lane LOS	-	-	D	В	-
HCM 95th %tile Q(veh)	-	-	1.2	0.1	-

Intersection						
Int Delay, s/veh	0.6					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	۰¥		4		<u>۲</u>	↑
Traffic Vol, veh/h	8	25	1229	26	20	1049
Future Vol, veh/h	8	25	1229	26	20	1049
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	0	-
Veh in Median Storage	e,# 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	0	0	2	4	0	1
Mvmt Flow	9	28	1366	29	22	1166

Major/Minor	Minor1	N	lajor1	Ν	Najor2		
Conflicting Flow All	2591	1381	0	0	1395	0	
Stage 1	1381	-	-	-	-	-	
Stage 2	1210	-	-	-	-	-	
Critical Hdwy	6.4	6.2	-	-	4.1	-	
Critical Hdwy Stg 1	5.4	-	-	-	-	-	
Critical Hdwy Stg 2	5.4	-	-	-	-	-	
Follow-up Hdwy	3.5	3.3	-	-	2.2	-	
Pot Cap-1 Maneuver	28	178	-	-	497	-	
Stage 1	236	-	-	-	-	-	
Stage 2	285	-	-	-	-	-	
Platoon blocked, %			-	-		-	
Mov Cap-1 Maneuver	· 27	178	-	-	497	-	
Mov Cap-2 Maneuver	· 128	-	-	-	-	-	
Stage 1	236	-	-	-	-	-	
Stage 2	272	-	-	-	-	-	

Approach	WB	NB	SB
HCM Control Delay, s	33.4	0	0.2
HCM LOS	D		

Vinor Lane/Major Mvmt	NBT	NBRWB	Ln1	SBL	SBT
Capacity (veh/h)	-	-	163	497	-
HCM Lane V/C Ratio	-	- 0.1	225 (0.045	-
HCM Control Delay (s)	-	- 3	33.4	12.6	-
HCM Lane LOS	-	-	D	В	-
HCM 95th %tile Q(veh)	-	-	0.8	0.1	-

Intersection

Int Delay, s/veh	1					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	Y		et 👘		ľ	•
Traffic Vol, veh/h	27	32	999	11	12	1370
Future Vol, veh/h	27	32	999	11	12	1370
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	0	-
Veh in Median Storage,	# 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	9	0	3	0	20	2
Mvmt Flow	30	36	1110	12	13	1522

Major/Minor	Minor1	Ν	/lajor1	Ν	Major2	
Conflicting Flow All	2664	1116	0		1122	0
Stage 1	1116	-	-	-	-	-
Stage 2	1548	-	-	-	-	-
Critical Hdwy	6.49	6.2	-	-	4.3	-
Critical Hdwy Stg 1	5.49	-	-	-	-	-
Critical Hdwy Stg 2	5.49	-	-	-	-	-
Follow-up Hdwy	3.581	3.3	-	-	2.38	-
Pot Cap-1 Maneuver		255	-	-	561	-
Stage 1	303	-	-	-	-	-
Stage 2	186	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuve		255	-	-	561	-
Mov Cap-2 Maneuve		-	-	-	-	-
Stage 1	303	-	-	-	-	-
Stage 2	182	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay,	s 41.2		0		0.1	
HCM LOS	E					
Minor Lane/Major My	vmt	NBT	NBRW	/BLn1	SBL	SBT
Capacity (veh/h)		-	-	163	561	-
HCM Lane V/C Ratio	C	-	-	0.402	0.024	-
HCM Control Delay ((s)	-	-	41.2	11.6	-
HCM Lane LOS		-	-	E	В	-
HCM 95th %tile Q(ve	eh)	-	-	1.8	0.1	-

Notes

~: Volume exceeds capacity

\$: Delay exceeds 300s +: Computation Not Defined

*: All major volume in platoon

Intersection						
Int Delay, s/veh	0.7					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	۰¥		4		<u>۲</u>	↑
Traffic Vol, veh/h	9	28	1358	29	22	1159
Future Vol, veh/h	9	28	1358	29	22	1159
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	0	-
Veh in Median Storage	,# 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	0	0	2	4	0	1
Mvmt Flow	10	31	1509	32	24	1288

Major/Minor	Minor1	N	lajor1	Ν	/lajor2	
Conflicting Flow All	2861	1525	0	0	1541	0
Stage 1	1525	-	-	-	-	-
Stage 2	1336	-	-	-	-	-
Critical Hdwy	6.4	6.2	-	-	4.1	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	-	-	2.2	-
Pot Cap-1 Maneuver	19	147	-	-	437	-
Stage 1	200	-	-	-	-	-
Stage 2	248	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver		147	-	-	437	-
Mov Cap-2 Maneuver	107	-	-	-	-	-
Stage 1	200	-	-	-	-	-
Stage 2	234	-	-	-	-	-
Approach	WB		NB		SB	

Approach	WB	NB	SB	
HCM Control Delay, s	42.9	0	0.3	
HCM LOS	Е			

Minor Lane/Major Mvmt	NBT	NBRW	/BLn1	SBL	SBT
Capacity (veh/h)	-	-	135	437	-
HCM Lane V/C Ratio	-	-	0.305	0.056	-
HCM Control Delay (s)	-	-	42.9	13.7	-
HCM Lane LOS	-	-	E	В	-
HCM 95th %tile Q(veh)	-	-	1.2	0.2	-

Intersection						
Int Delay, s/veh	0.9					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	۰¥		4		<u>۲</u>	↑
Traffic Vol, veh/h	29	24	907	10	11	1243
Future Vol, veh/h	29	24	907	10	11	1243
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	0	-
Veh in Median Storage	e, # 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	9	0	3	0	20	2
Mvmt Flow	32	27	1008	11	12	1381

Major/Minor	Minor1	Ν	1ajor1	Ν	/lajor2	
Conflicting Flow All	2419	1014	0	0	1019	0
Stage 1	1014	-	-	-	-	-
Stage 2	1405	-	-	-	-	-
Critical Hdwy	6.49	6.2	-	-	4.3	-
Critical Hdwy Stg 1	5.49	-	-	-	-	-
Critical Hdwy Stg 2	5.49	-	-	-	-	-
Follow-up Hdwy	3.581	3.3	-	-	2.38	-
Pot Cap-1 Maneuver	34	292	-	-	616	-
Stage 1	340	-	-	-	-	-
Stage 2	219	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver		292	-	-	616	-
Mov Cap-2 Maneuver	134	-	-	-	-	-
Stage 1	340	-	-	-	-	-
Stage 2	215	-	-	-	-	-
Annroach	\//R		NR		SB	

Approach	WB	NB	SB	
HCM Control Delay, s	35.2	0	0.1	
HCM LOS	E			

Minor Lane/Major Mvmt	NBT	NBRWE	3Ln1	SBL	SBT	
Capacity (veh/h)	-	-	177	616	-	
HCM Lane V/C Ratio	-	- 0.	.333	0.02	-	
HCM Control Delay (s)	-	- :	35.2	11	-	
HCM Lane LOS	-	-	Е	В	-	
HCM 95th %tile Q(veh)	-	-	1.4	0.1	-	

IntersectionInt Delay, s/veh0.9MovementEBLEBRNBLNBTSBTSBRLane Configurations11111Traffic Vol, veh/h233023894123730Future Vol, veh/h233023894123730
MovementEBLEBRNBLNBTSBTSBRLane Configurations11111Traffic Vol, veh/h233023894123730
Lane ConfigurationsřřřTraffic Vol, veh/h233023894123730
Lane ConfigurationsřřřřTraffic Vol, veh/h233023894123730
Traffic Vol, veh/h 23 30 23 894 1237 30
Euture Vol veh/h 23 30 23 894 1237 30
Conflicting Peds, #/hr 0 0 0 0 0 0
Sign Control Stop Stop Free Free Free Free
RT Channelized - None - None - None
Storage Length 0 0 0
Veh in Median Storage, # 0 0 0 -
Grade, % 0 0 0 -
Peak Hour Factor 90 90 90 90 90 90
Heavy Vehicles, % 2 2 3 3 3 3
Mvmt Flow 26 33 26 993 1374 33

Major/Minor	Minor2		Major1	Ма	ijor2	
Conflicting Flow All	2436	1391	1407	0	-	0
Stage 1	1391	-	-	-	-	-
Stage 2	1045	-	-	-	-	-
Critical Hdwy	6.42	6.22	4.13	-	-	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	2.227	-	-	-
Pot Cap-1 Maneuver	35	174	482	-	-	-
Stage 1	231	-	-	-	-	-
Stage 2	339	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver		174	482	-	-	-
Mov Cap-2 Maneuver	136	-	-	-	-	-
Stage 1	219	-	-	-	-	-
Stage 2	339	-	-	-	-	-

Approach	EB	NB	SB
HCM Control Delay, s	33.5	0.3	0
HCM LOS	D		

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	EBLn2	SBT	SBR
Capacity (veh/h)	482	-	136	174	-	-
HCM Lane V/C Ratio	0.053	-	0.188	0.192	-	-
HCM Control Delay (s)	12.9	-	37.5	30.5	-	-
HCM Lane LOS	В	-	E	D	-	-
HCM 95th %tile Q(veh)	0.2	-	0.7	0.7	-	-

Intersection						
Int Delay, s/veh	0.6					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	۰¥		4		<u>۲</u>	↑
Traffic Vol, veh/h	8	25	1230	26	20	1050
Future Vol, veh/h	8	25	1230	26	20	1050
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	0	-
Veh in Median Storage	e, # 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	0	0	2	4	0	1
Mvmt Flow	9	28	1367	29	22	1167

Major/Minor	Minor1	N	lajor1	Ν	/lajor2	
Conflicting Flow All	2593	1382	0	0	1396	0
Stage 1	1382	-	-	-	-	-
Stage 2	1211	-	-	-	-	-
Critical Hdwy	6.4	6.2	-	-	4.1	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	-	-	2.2	-
Pot Cap-1 Maneuver	28	178	-	-	496	-
Stage 1	235	-	-	-	-	-
Stage 2	285	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver		178	-	-	496	-
Mov Cap-2 Maneuver	· 127	-	-	-	-	-
Stage 1	235	-	-	-	-	-
Stage 2	272	-	-	-	-	-
A						

Approach	WB	NB	SB	
HCM Control Delay, s	33.6	0	0.2	
HCM LOS	D			

Minor Lane/Major Mvmt	NBT	NBRV	/BLn1	SBL	SBT
Capacity (veh/h)	-	-	162	496	-
HCM Lane V/C Ratio	-	-	0.226	0.045	-
HCM Control Delay (s)	-	-	33.6	12.6	-
HCM Lane LOS	-	-	D	В	-
HCM 95th %tile Q(veh)	-	-	0.8	0.1	-

HCM 6th TWSC 5: Derry Road & Driveway

Intersection						
Int Delay, s/veh	0.3					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	ሻ	1	ሻ	↑	ef 👘	
Traffic Vol, veh/h	11	9	11	1245	1049	9
Future Vol, veh/h	11	9	11	1245	1049	9
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	0	0	-	-	-
Veh in Median Storage	,# 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	12	10	12	1383	1166	10

Major/Minor	Minor2	l	Major1	Ma	ijor2		
Conflicting Flow All	2578	1171	1176	0	-	0	
Stage 1	1171	-	-	-	-	-	
Stage 2	1407	-	-	-	-	-	
Critical Hdwy	6.42	6.22	4.12	-	-	-	
Critical Hdwy Stg 1	5.42	-	-	-	-	-	
Critical Hdwy Stg 2	5.42	-	-	-	-	-	
Follow-up Hdwy	3.518	3.318	2.218	-	-	-	
Pot Cap-1 Maneuver	28	235	594	-	-	-	
Stage 1	295	-	-	-	-	-	
Stage 2	226	-	-	-	-	-	
Platoon blocked, %				-	-	-	
Mov Cap-1 Maneuver	27	235	594	-	-	-	
Mov Cap-2 Maneuver	128	-	-	-	-	-	
Stage 1	289	-	-	-	-	-	
Stage 2	226	-	-	-	-	-	
					0.5		

Approach	EB	NB	SB	
HCM Control Delay, s	29.3	0.1	0	
HCM LOS	D			

Minor Lane/Major Mvmt	NBL	NBT EBLn1	EBLn2	SBT	SBR	
Capacity (veh/h)	594	- 128	235	-	-	
HCM Lane V/C Ratio	0.021	- 0.095	0.043	-	-	
HCM Control Delay (s)	11.2	- 36.1	21	-	-	
HCM Lane LOS	В	- E	С	-	-	
HCM 95th %tile Q(veh)	0.1	- 0.3	0.1	-	-	

Intersection						
Int Delay, s/veh	1.1					
			NDT		CDI	CDT
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	Y		4		<u></u>	1
Traffic Vol, veh/h	27	32	1002	11	12	1373
Future Vol, veh/h	27	32	1002	11	12	1373
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	0	-
Veh in Median Storage	e,# 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	9	0	3	0	20	2
Mvmt Flow	30	36	1113	12	13	1526
5 A 1 /5 A1			1 4		1 . 0	
	Minor1		Major1		Najor2	-
Conflicting Flow All	2671	1119	0	0	1125	0
Stage 1	1119	-	-	-	-	-
Stage 2	1552	-	-	-	-	-
Critical Hdwy	6.49	6.2	-	-	4.3	-
Critical Hdwy Stg 1	5.49	-	-	-	-	-
Critical Hdwy Stg 2	5.49	-	-	-	-	-
Follow-up Hdwy	3.581	3.3	-	-	2.38	-
Pot Cap-1 Maneuver	~ 23	254	-	-	559	-
Stage 1	302	-	-	-	-	-
Stage 2	185	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	~ 22	254	-	-	559	-
Mov Cap-2 Maneuver		-		-	-	-
Stage 1	302	-	_	_	-	-
Stage 2	181	-	-	_	_	_
Staye 2	101	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s	41.6		0		0.1	

HCM LOS E

Minor Lane/Major Mvmt	NBT	NBRWBLr	1 SBL	SBT		
Capacity (veh/h)	-	- 16		-		
HCM Lane V/C Ratio	-	- 0.40	5 0.024	-		
HCM Control Delay (s)	-	- 41	.6 11.6	-		
HCM Lane LOS	-	-	E B	-		
HCM 95th %tile Q(veh)	-	- 1	.8 0.1	-		
Notes						
~: Volume exceeds capacity	\$: De	alay exceeds	300s	+: Comp	outation Not Defined	*: All major volume in platoon

ne Configurations iffic Vol, veh/h iure Vol, veh/h inflicting Peds, #/hr in Control S Channelized irage Length h in Median Storage, # ade, % ak Hour Factor avy Vehicles, % mt Flow <u>jor/Minor Mil</u> nflicting Flow All 2 Stage 1 1 Stage 2 1 itical Hdwy Stg 1 itical Hdwy Stg 1 itical Hdwy Stg 2 low-up Hdwy 3 it Cap-1 Maneuver Stage 1 Stage 2 itoon blocked, % v Cap-2 Maneuver Stage 1 Stage 2 itoon blocked, % v Cap-2 Maneuver Stage 1 Stage 2 itoon blocked, %	0 90 2 28 <u>/linor2</u> 2692 1538 1154 6.42 5.42	Stop None 0 - - 90 2 38 - 1538 - - - - - - - - -	NBL 25 25 0 Free - 0 0 - - 90 3 28 Major1 1556 - - 4.13 - 2.227 422	NBT 988 988 0 Free None - 0 0 0 90 3 1098 N 0 - - - - - - - - - - - - -	SBT 1367 1367 0 Free - - 0 0 0 90 3 1519 Major2 - - - - - - - - - - - - - - - - - - -	SBR 33 33 0 Free None - - 90 3 37 37 0 0 - - - - - - - - - -				
ne Configurations iffic Vol, veh/h iure Vol, veh/h inflicting Peds, #/hr in Control S Channelized rage Length h in Median Storage, # ade, % ak Hour Factor avy Vehicles, % mt Flow <u>jor/Minor Min</u> nflicting Flow All 2 Stage 1 1 Stage 2 1 tical Hdwy Stg 1 tical Hdwy Stg 2 low-up Hdwy Stg 2 low-up Hdwy 3 t Cap-1 Maneuver Stage 1 Stage 2 low-up Hdwy 3 t Cap-1 Maneuver Stage 1 Stage 2 low-up Hdwy 3 t Cap-1 Maneuver Stage 1 Stage 2 low-up Hdwy 3 t Cap-1 Maneuver Stage 1 Stage 2 low-up Hdwy 3 t Cap-1 Maneuver Stage 1 Stage 2 proach M Control Delay, s M LOS	25 25 0 Stop - 0 # 0 0 90 2 28 4 inor2 2692 1538 1154 6.42 5.42	* 34 90 2 38 1538 - 6.22 - - -	 25 25 25 0 Free - 0 - 90 3 28 Major1 1556 - 4.13 - 2.227 	♦ 988 988 988 0 Free None - 0 0 0 90 3 1098	1367 1367 0 Free - - 0 0 0 90 3 1519 Major2 - - - - -	33 33 0 Free None - - - 90 3 37 37 0 0 - - - -				
ne Configurations iffic Vol, veh/h iure Vol, veh/h inflicting Peds, #/hr in Control S Channelized rage Length h in Median Storage, # ade, % ak Hour Factor avy Vehicles, % mt Flow <u>jor/Minor Min</u> nflicting Flow All 2 Stage 1 1 Stage 2 1 tical Hdwy Stg 1 tical Hdwy Stg 2 low-up Hdwy Stg 2 low-up Hdwy 3 t Cap-1 Maneuver Stage 1 Stage 2 low-up Hdwy 3 t Cap-1 Maneuver Stage 1 Stage 2 low-up Hdwy 3 t Cap-1 Maneuver Stage 1 Stage 2 low-up Hdwy 3 t Cap-1 Maneuver Stage 1 Stage 2 low-up Hdwy 3 t Cap-1 Maneuver Stage 1 Stage 2 proach M Control Delay, s M LOS	25 25 0 Stop - 0 # 0 0 90 2 28 4 inor2 2692 1538 1154 6.42 5.42	* 34 90 2 38 1538 - 6.22 - - -	 25 25 25 0 Free - 0 - 90 3 28 Major1 1556 - 4.13 - 2.227 	♦ 988 988 988 0 Free None - 0 0 0 90 3 1098	1367 1367 0 Free - - 0 0 0 90 3 1519 Major2 - - - - -	33 33 0 Free None - - - 90 3 37 37 0 0 - - - -				
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Stage 1 1 Stage 2 1 tical Hdwy 1 tical Hdwy Stg 1 1 tical Hdwy Stg 2 1 low-up Hdwy Stg 2 1 low-up Hdwy Stg 2 1 toal Hdwy Stg 2 1 stage 1 3 Stage 2 1 toon blocked, % 2 v Cap-1 Maneuver 3 v Cap-2 Maneuver 3 Stage 1 3 Stage 2 3 proach 3 M Control Delay, s 3 M LOS 3	1538 1154 6.42 5.42	- - 6.22 - -	- 4.13 - 2.227	-	- - -	- - -				
Stage 2 1 tical Hdwy tical Hdwy Stg 1 1 tical Hdwy Stg 2 low-up Hdwy Stg 2 low-up Hdwy 3 t Cap-1 Maneuver Stage 1 Stage 2 toon blocked, % v Cap-1 Maneuver v Cap-2 Maneuver Stage 1 Stage 1 Stage 2 proach M Control Delay, s M LOS nor Lane/Major Mvmt	1154 6.42 5.42	- 6.22 -	- 4.13 - 2.227	-	-	-				
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low-up Hdwy 3 t Cap-1 Maneuver Stage 1 Stage 2 toon blocked, % v Cap-1 Maneuver v Cap-2 Maneuver stage 1 Stage 2 proach M Control Delay, s M LOS	E 40			-	-	-				
t Cap-1 Maneuver Stage 1 Stage 2 toon blocked, % v Cap-1 Maneuver v Cap-2 Maneuver Stage 1 Stage 2 proach M Control Delay, s M LOS	5.42	3.318		-						
Stage 1 Stage 2 toon blocked, % v Cap-1 Maneuver v Cap-2 Maneuver Stage 1 Stage 2 proach M Control Delay, s M LOS	3.518		122		-	-				
Stage 2 toon blocked, % v Cap-1 Maneuver v Cap-2 Maneuver Stage 1 Stage 2 proach M Control Delay, s M LOS nor Lane/Major Mvmt	~ 24	142	722	-	-	-				
toon blocked, % v Cap-1 Maneuver v Cap-2 Maneuver Stage 1 Stage 2 proach M Control Delay, s M LOS	195	-	-	-	-	-				
v Cap-1 Maneuver v Cap-2 Maneuver Stage 1 Stage 2 proach M Control Delay, s M LOS nor Lane/Major Mvmt	300	-	-	-	-	-				
v Cap-2 Maneuver Stage 1 Stage 2 proach M Control Delay, s M LOS nor Lane/Major Mvmt				-	-	-				
v Cap-2 Maneuver Stage 1 Stage 2 proach M Control Delay, s M LOS nor Lane/Major Mvmt	~ 22	142	422	-	-	-				
Stage 1 Stage 2 proach M Control Delay, s M LOS nor Lane/Major Mvmt	113	-	-	-	-	-				
Stage 2 proach M Control Delay, s M LOS nor Lane/Major Mvmt	182	-	-	-	-	-				
proach M Control Delay, s M LOS nor Lane/Major Mvmt	300	-	-	-	-	-				
M Control Delay, s M LOS nor Lane/Major Mvmt										
M Control Delay, s M LOS nor Lane/Major Mvmt	EB		NB		SB					
M LOS	42.5		0.3		0					
nor Lane/Major Mvmt	E		0.0		5					
		NBL	NBT	EBLn1 I	FBI n2	SBT	SBR			
	ł	422	-		142	501				
M Lane V/C Ratio	t			0.246		-	-			
	t	0 044				-	-			
M Control Delay (s)	t	0.066	-		39.3	-	-			
M Lane LOS	1	14.1	-		E 1	-	-			
M 95th %tile Q(veh)	t	14.1 B				-	-			
tes	1	14.1	-	0.9						
Volume exceeds capa	1	14.1 B	-	0.9						
		14.1 B 0.2		ceeds 3		+: Com	outation Not	t Defined	*: All majo	r volume in

Intersection						
Int Delay, s/veh	0.8					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	۰¥		4		<u>۲</u>	↑
Traffic Vol, veh/h	9	28	1359	29	22	1160
Future Vol, veh/h	9	28	1359	29	22	1160
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	0	-
Veh in Median Storage	,# 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	0	0	2	4	0	1
Mvmt Flow	10	31	1510	32	24	1289

Major/Minor	Minor1	Ν	lajor1	Ν	/lajor2	
Conflicting Flow All	2863	1526	0	0	1542	0
Stage 1	1526	-	-	-	-	-
Stage 2	1337	-	-	-	-	-
Critical Hdwy	6.4	6.2	-	-	4.1	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	-	-	2.2	-
Pot Cap-1 Maneuver	19	146	-	-	436	-
Stage 1	200	-	-	-	-	-
Stage 2	247	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	18	146	-	-	436	-
Mov Cap-2 Maneuver	106	-	-	-	-	-
Stage 1	200	-	-	-	-	-
Stage 2	233	-	-	-	-	-
Approach	WB		NB		SB	

Approach	WB	NB	SB	
HCM Control Delay, s	43.3	0	0.3	
HCM LOS	E			

Minor Lane/Major Mvmt	NBT	NBRWBLn1	SBL	SBT
Capacity (veh/h)	-	- 134	436	-
HCM Lane V/C Ratio	-	- 0.307	0.056	-
HCM Control Delay (s)	-	- 43.3	13.7	-
HCM Lane LOS	-	- E	В	-
HCM 95th %tile Q(veh)	-	- 1.2	0.2	-

HCM 6th TWSC 5: Derry Road & Driveway

Intersection						
Int Delay, s/veh	0.3					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
			NDL	NDT		лас
Lane Configurations		7	1	↑	ર્ન	
Traffic Vol, veh/h	11	11	12	1377	1159	10
Future Vol, veh/h	11	11	12	1377	1159	10
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	0	0	-	-	-
Veh in Median Storage	,# 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	12	12	13	1530	1288	11

Major/Minor	Minor2	[Major1	Ma	ajor2	
Conflicting Flow All	2850	1294	1299	0	-	0
Stage 1	1294	-	-	-	-	-
Stage 2	1556	-	-	-	-	-
Critical Hdwy	6.42	6.22	4.12	-	-	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	2.218	-	-	-
Pot Cap-1 Maneuver	19	199	533	-	-	-
Stage 1	257	-	-	-	-	-
Stage 2	191	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	19	199	533	-	-	-
Mov Cap-2 Maneuver	108	-	-	-	-	-
Stage 1	251	-	-	-	-	-
Stage 2	191	-	-	-	-	-
Approach	EB		NB		SB	

Approach	EB	NB	SB	
HCM Control Delay, s	33.4	0.1	0	
HCM LOS	D			

Minor Lane/Major Mvmt	NBL	NBT E	BLn1 I	EBLn2	SBT	SBR
Capacity (veh/h)	533	-	108	199	-	-
HCM Lane V/C Ratio	0.025	-	0.113	0.061	-	-
HCM Control Delay (s)	11.9	-	42.5	24.3	-	-
HCM Lane LOS	В	-	Е	С	-	-
HCM 95th %tile Q(veh)	0.1	-	0.4	0.2	-	-

Intersection						
Int Delay, s/veh	0.9					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	ኘ	1	٦		↑	1
Traffic Vol, veh/h	23	30	23	894	1237	30
Future Vol, veh/h	23	30	23	894	1237	30
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	0	0	-	-	50
Veh in Median Storag	e, # 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	2	2	3	3	3	3
Mvmt Flow	26	33	26	993	1374	33

Major/Minor	Minor2	ļ	Major1	Ма	jor2	
Conflicting Flow All	2419	1374	1407	0	-	0
Stage 1	1374	-	-	-	-	-
Stage 2	1045	-	-	-	-	-
Critical Hdwy	6.42	6.22	4.13	-	-	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	2.227	-	-	-
Pot Cap-1 Maneuver	36	178	482	-	-	-
Stage 1	235	-	-	-	-	-
Stage 2	339	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	34	178	482	-	-	-
Mov Cap-2 Maneuver	137	-	-	-	-	-
Stage 1	222	-	-	-	-	-
Stage 2	339	-	-	-	-	-

Approach	EB	NB	SB
HCM Control Delay, s	33	0.3	0
HCM LOS	D		

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	EBLn2	SBT	SBR
Capacity (veh/h)	482	-	137	178	-	-
HCM Lane V/C Ratio	0.053	-	0.187	0.187	-	-
HCM Control Delay (s)	12.9	-	37.2	29.8	-	-
HCM Lane LOS	В	-	E	D	-	-
HCM 95th %tile Q(veh)	0.2	-	0.7	0.7	-	-

Intersection						
Int Delay, s/veh	0.3					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	<u>ار ا</u>	1	<u>ار</u>	•	•	1
Traffic Vol, veh/h	11	9	11	1245	1049	9
Future Vol, veh/h	11	9	11	1245	1049	9
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	0	0	-	-	50
Veh in Median Storage,	# 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	12	10	12	1383	1166	10

Major/Minor	Minor2		Major1	Ма	jor2	
Conflicting Flow All	2573	1166	1176	0	-	0
Stage 1	1166	-	-	-	-	-
Stage 2	1407	-	-	-	-	-
Critical Hdwy	6.42	6.22	4.12	-	-	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	2.218	-	-	-
Pot Cap-1 Maneuver	28	236	594	-	-	-
Stage 1	296	-	-	-	-	-
Stage 2	226	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	27	236	594	-	-	-
Mov Cap-2 Maneuver	128	-	-	-	-	-
Stage 1	290	-	-	-	-	-
Stage 2	226	-	-	-	-	-

Approach	EB	NB	SB
HCM Control Delay, s	29.3	0.1	0
HCM LOS	D		

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	EBLn2	SBT	SBR
Capacity (veh/h)	594	-	128	236	-	-
HCM Lane V/C Ratio	0.021	-	0.095	0.042	-	-
HCM Control Delay (s)	11.2	-	36.1	20.9	-	-
HCM Lane LOS	В	-	E	С	-	-
HCM 95th %tile Q(veh)	0.1	-	0.3	0.1	-	-

Intersection											
Int Delay, s/veh	1.1										
Movement	EBL	EBR	NBL	NBT	SBT	SBR					
Lane Configurations	<u> </u>										
Traffic Vol, veh/h	25	34	25	988	1367	33					
Future Vol, veh/h	25	34	25	988	1367	33					
Conflicting Peds, #/hr	0	0	0	0	0	0					
Sign Control	Stop	Stop	Free	Free	Free	Free					
RT Channelized	-	•	-	None	-	None					
Storage Length	0	0	0	-	-	50					
Veh in Median Storage	e,# 0	-	-	0	0	-					
Grade, %	0	-	-	0	0	-					
Peak Hour Factor	90	90	90	90	90	90					
Heavy Vehicles, %	2	2	3	3	3	3					
Mvmt Flow	28	38	28	1098	1519	37					
Major/Minor	Minor2		Major1	1	Major2					J	
Conflicting Flow All	2673	1519	1556	0	-	0					
Stage 1	1519	-	-	-	-	-					
Stage 2	1154	-	-	-	-	-					
Critical Hdwy	6.42	6.22	4.13	-	-	-					
Critical Hdwy Stg 1	5.42	-	-	-	-	-					
Critical Hdwy Stg 2	5.42	-	-	-	-	-					
Follow-up Hdwy	3.518	3.318	2.227	-	-	-					
Pot Cap-1 Maneuver	~ 25	146	422	-	-	-					
Stage 1	200	-	-	-	-	-					
Stage 2	300	-	-	-	-	-					
Platoon blocked, %				-	-	-					
Mov Cap-1 Maneuver	~ 23	146	422	-	-	-					
Mov Cap-2 Maneuver	115	-	-	-	-	-					
Stage 1	187	-	-	-	-	-					
Stage 2	300	-	-	-	-	-					
Approach	EB		NB		SB						
HCM Control Delay, s			0.3		0						
HCM LOS	41.4 E		0.5		0						
	L										
						0	0.005				
Minor Lane/Major Mvr	nt	NBL	NBT	EBLn1 I		SBT	SBR				
Capacity (veh/h)		422	-	115	146	-	-				
HCM Lane V/C Ratio		0.066	-	0.242		-	-				
HCM Control Delay (s)	14.1	-	46	38.1	-	-				
HCM Lane LOS		В	-	E	E	-	-				
HCM 95th %tile Q(veh	1)	0.2	-	0.9	1	-	-				
Notes											
~: Volume exceeds ca	pacity	\$: De	elav exc	ceeds 3	00s	+: Com	putation No	ot Define	ed		*: All majo
	- asity	φ. Β (2. 2011			/ iii 1.1.3j = 1

Interception						
Intersection						
Int Delay, s/veh	0.3					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	٦	1	٦	†	↑	1
Traffic Vol, veh/h	11	11	12	1377	1159	10
Future Vol, veh/h	11	11	12	1377	1159	10
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	0	0	-	-	50
Veh in Median Storage	,# 0	-	-	0	0	-
Grade, %	. 0	-	-	0	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	12	12	13	1530	1288	11

Major/Minor	Minor2	[Major1	Ma	ijor2	
Conflicting Flow All	2844	1288	1299	0	-	0
Stage 1	1288	-	-	-	-	-
Stage 2	1556	-	-	-	-	-
Critical Hdwy	6.42	6.22	4.12	-	-	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	2.218	-	-	-
Pot Cap-1 Maneuver	19	200	533	-	-	-
Stage 1	259	-	-	-	-	-
Stage 2	191	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	19	200	533	-	-	-
Mov Cap-2 Maneuver	108	-	-	-	-	-
Stage 1	253	-	-	-	-	-
Stage 2	191	-	-	-	-	-
Approach	FB		NB		SB	

Approach	EB	NB	SB	
HCM Control Delay, s	33.4	0.1	0	
HCM LOS	D			

Minor Lane/Major Mvmt	NBL	NBT EBLn1	EBLn2	SBT	SBR	
Capacity (veh/h)	533	- 108	200	-	-	
HCM Lane V/C Ratio	0.025	- 0.113	0.061	-	-	
HCM Control Delay (s)	11.9	- 42.5	24.2	-	-	
HCM Lane LOS	В	- E	С	-	-	
HCM 95th %tile Q(veh)	0.1	- 0.4	0.2	-	-	



Appendix G: Comments and Responses



August 26, 2021

Mr. Brian Groth Town Planner Town of Hudson 12 School Street Hudson, NH 03051

Re: Town of Hudson Planning Board Review Aroma Joe's Site Plan, 56 Derry Street, Traffic Study Tax Map 173 Lot 29; Acct. #1350-970 Reference No. 20030249.2040

Dear Mr. Groth:

Fuss & O'Neill, Inc. has reviewed the Traffic Impact and Access Study prepared by Transportation Engineering, Planning and Policy (TEPP) for Keach-Nordstrom Associates, Inc. (KNA) dated July 1, 2021, for the proposed commercial development at 56 Derry Road in Hudson, New Hampshire (Tax Map 173 Lot 29). The project proposes the development of a 900 square foot (sf) drivethrough coffee shop on the currently vacant land at 56 Derry Road. Access and egress to the site will be provided via a proposed driveway on the west side of Derry Road, with one lane for access. For egress, a dedicated left turn lane and a dedicated right turn lane will be provided.

Please note that site plan, stormwater, and other project related comments were provided under separate letters dated July 9, 2021, and August 3, 2021.

In review of the TEPP report the following items are noted:

4. Traffic

50 Commercial Street Manchester, NH 03101 t 603.668.8223 800.286.2469

www.fando.com

California Connecticut Maine Massachusetts New Hampshire Rhode Island

Vermont

- a. Provide more detail regarding the proposed use in the introduction of the report; the introduction omits the size of the coffee shop and that the shop will not provide indoor seating.
- b. Revise the street name displayed in Table 3 Sight Distances to Derry Road instead of Portland Street and revise the data in the table accordingly if what is currently shown in the table is sight distance data for a different traffic impact study.
- c. In the summary of Table 4 Calculated Weekday Vehicle Trip-Generation, the 2032 total vehicle-trips are presented as "117 (58 in and 539 out)" for the weekday AM peak hour. The number of trips exiting the site should be revised to match what is shown in Table 4.



Mr. Brian Groth August 26, 2021 Page 2 of 3

- d. Although we agree that the trip generation information compiled in the ITE Trip Generation Manual for land use code 938 is not applicable to the proposed land use due to the small sample size, it is unclear if the method used in the TEPP report as an alternative is acceptable. The TEPP report mentions that Stephen G. Pernaw & Company, Inc. has published appropriate trip generation information for the proposed land use. The report then uses the information presumably from the Stephen G. Pernaw & Company, Inc. publication but does not provide the publication itself or specify the project the information comes from, so it is unclear whether the information used in the TEPP report to calculate traffic volumes generated by the proposed development is correct or appropriate. Relevant information from the Stephen G. Pernaw publication should be provided and the methodology used for determining the proposed development's generated trips should be further explained in the report.
- e. Table 5 Trip Distribution and Network Assignment assigns all site-generated traffic coming from/going to the south on Derry Road. This should be revised to reflect the applied distributions.
- f. The title of the last column of Table 8 Capacity Analysis Summary should be corrected to the 2032 Build condition.
- g. The v/c ratio and queue length for the northbound left turn movement at the Derry Road/Site Driveway intersection during the 2032 Build in Table 8 are flipped according to the attached Synchro reports. This should be revised.
- h. The attached site plan shows a proposed southbound right turn storage lane at the Derry Road/Site Driveway intersection for access into the site. The Build condition Synchro reports and capacity analysis results in the TEPP report do not appear to reflect this lane configuration change and should be revised accordingly. If this lane configuration change is being proposed, it should also be discussed in the report.
- i. The site plan should include signage per MUTCD for the proposed right turn lane.
- j. We recommend the applicant and Town consider the installation of a left turn arrow in the center turning lane for northbound Derry Street traffic to formalize the site entrance, similar to turn arrows further north along the Derry Street center turning lane.

Fuss & O'Neill needs clarification on the methodology used of the trip generation calculations used in the TEPP report to determine if TEPP's overall conclusion, that there should be minimal impacts on traffic operations at the Derry Road and Ledge Road intersection, is appropriate. As for



Mr. Brian Groth August 26, 2021 Page 3 of 3

the Derry Road and Site Driveway intersection, we suggest revising the analysis to include any proposed roadway improvements before further review.

Please feel free to call if you have any questions.

Very truly yours,

Steven W. Peichert, PE Digitally signed by Steven W. Reichert, PE Di

Steven W. Reichert, P.E.

SWR:

cc: Town of Hudson Engineering Division – File Keach- Nordstrom Associates, Inc. - alewis@keachnordstrom.com

RESPONSES BY TEPP LLC

<u>Comment 4.a.</u> Provide more detail regarding the proposed use in the introduction of the report; the introduction omits the size of the coffee shop and that the shop will not provide indoor seating.

Response. Pages 1, 5, and 31 show the revisions.

<u>**Comment 4.b.**</u> Revise the street name displayed in Table 3 – Sight Distances to Derry Road instead of Portland Street and revise the data in the table accordingly if what is currently shown in the table is sight distance data for a different traffic impact study.

Response. Page 14, Table 3, shows the revision.

<u>Comment 4.c.</u> In the summary of Table 4 – Calculated Weekday Vehicle Trip-Generation, the 2032 total vehicle-trips are presented as "117 (58 in and 539 out)" for the weekday AM peak hour. The number of trips exiting the site should be revised to match what is shown in Table 4.

Response. Page 19 shows the revision.

Comment 4.d. Although we agree that the trip generation information compiled in the ITE Trip Generation Manual for land use code 938 is not applicable to the proposed land use due to the small sample size, it is unclear if the method used in the TEPP report as an alternative is acceptable. The TEPP report mentions that Stephen G. Pernaw & Company, Inc. has published appropriate trip generation information for the proposed land use. The report then uses the information presumably from the Stephen G. Pernaw & Company, Inc. publication but does not provide the publication itself or specify the project the information comes from, so it is unclear whether the information used in the TEPP report to calculate traffic volumes generated by the proposed development is correct or appropriate. Relevant information from the Stephen G. Pernaw publication should be provided and the methodology used for determining the proposed development's generated trips should be further explained in the report.

<u>Response.</u> Page 19, Table 8, footnotes show calculations. Appendix E includes relevant information from the publication.

<u>Comment 4.e.</u> Table 5 – Trip Distribution and Network Assignment assigns all site-generated traffic coming from/going to the south on Derry Road. This should be revised to reflect the applied distributions.

Response. Page 20, Table 5, shows the revisions.

<u>**Comment 4.f.</u>** The title of the last column of Table 8 – Capacity Analysis Summary should be corrected to the 2032 Build condition.</u>

Response. Page 29, Table 8, shows the revision.

Comment 4.g. The v/c ratio and queue length for the northbound left turn movement at the Derry Road/Site Driveway intersection during the 2032 Build in Table 8 are flipped according to the attached Synchro reports. This should be revised.

Response. Page 29, Table 8, shows the revisions.

Comment 4.h. The attached site plan shows a proposed southbound right turn storage lane at the Derry Road/Site Driveway intersection for access into the site. The Build condition Synchro reports and capacity analysis results in the TEPP report do not appear to reflect this lane configuration change and should be revised accordingly. If this lane configuration change is being proposed, it should also be discussed in the report.

Response. Pages 1, 5, and 31 shows the revisions. Page 29, Table 8, shows the revision.

<u>Comment 4.i.</u> The site plan should include signage per MUTCD for the proposed right turn lane.

Response. Keach-Nordstrom Associates, Inc. will be providing a plan that shows the signage.

<u>Comment 4.j.</u> We recommend the applicant and Town consider the installation of a left turn arrow in the center turning lane for northbound Derry Street traffic to formalize the site entrance, similar to turn arrows further north along the Derry Street center turning lane.

<u>Response.</u> TEPP LLC recommends that the Town consider retaining the simple and consistent two-way-left-turn lane treatment along the subject segment of Derry Road.