BARRETT HILL LLC OPEN SPACE DEVELOPMENT PLAN

SB# 08-23 STAFF REPORT

March 27, 2024

SITE: 75 Barretts Hill Road / Map 151 / Lot 059

ZONING: General -1 (G-1)

PURPOSE OF PLAN: To depict a thirteen (13) lot open space subdivision on Map 151, lot 059 and all associated improvements.

PLANS UNDER REVIEW:

Barrett Hill Subdivision SB# 08-23, Map 151/Lot 059, 75 Barretts Hill Road, Hudson, New Hampshire; prepared by: Keach-Nordstrom Associates, 10 Commerce Park North Suite 3B, Bedford, NH 03110; prepared for: Barrett Hill, LLC, 21 Continental Boulevard, Merrimack NH 03054, consisting of twenty-four sheets, and general notes 1-28 on sheet 1; dated December 6, 2023, last revised March 4, 2024.

ATTACHMENTS:

- 1) Subdivision Application and applicable waiver date stamped December 11, 2023 Attachment "A".
- 2) Department Comments Attachment "B."
- 3) Peer Review, prepared by Fuss & O'Neill, dated January 4, 2024 Attachment "C"
- 4) Applicant Response to Peer Review, prepared by Keach-Nordstrom Associates, dated March 4. 2024 Attachment "**D**".
- 5) Applicant Response to Town Comments, prepared by Keach-Nordstrom Associates, dated March 5, 2024 Attachment "E".
- 6) Stormwater Management Report, prepared by Keach-Nordstrom Associates, dated December 13, 2023 Attachment "F".
- 7) CAP Fee Worksheet Attachment "G."
- 8) Subdivision plans

REQUESTED WAIVER:

1) §289-37.A – Plan Schedule and Form

APPLICATION TRACKING:

- 1. December 11, 2023 Application received.
- 2. January 4, 2024 Peer review received.
- 3. March 5, 2024 Revisions and responses received.
- 4. March 27, 2024 Public hearing scheduled

COMMENTS & RECOMMENDATIONS:

BACKGROUND

The subject lot is approximately 35.36 acres with approximately 1,160 feet of frontage along Barretts Hill Road. The lot is in the General-One (G-1) zone and directly abuts residential properties on three sides. The site contains no known wetlands and is not located within any flood zone. The site is served by neither municipal water nor sewer. The applicant proposes subdividing Map 151 Lot 059 into thirteen lots sized between 43,785 sq. ft and 77,869 sq. ft. All lots would have no less than 100 linear feet of frontage along the proposed Windsor lane. All of the lots are proposed to be developed with duplexes for a total of 26 units. The plan proposes 754,352 sq. ft. of open space to be preserved within the development, held within a 17.318 acre space that spans from the street frontage to the rear half of the property. A utility easement spans across the property within the rear third of the lot and is located entirely within the proposed open space.

STAFF COMMENTS

The lot meets all subdivision requirements of the town outside of a waiver that has been requested for phasing of construction. The applicant states that due to the smaller size of the project, it would be best suited to be completed within a year as opposed to the standard two years which would result in less disturbance to residential abutters.

PEER REVIEW

Fuss & O'Neill reviewed the subdivision plan on January 4, 2024 (Attachment "C"). The Applicant responded to the Peer Review comments in Attachment "D."

The Applicant has submitted a revised plan set addressing comments provided by Fuss & O'Neill and the Town alongside their response letter.

DEPARTMENT COMMENTS

Engineering provided the following comments:

- 1. Applicant shall provide driveway plan and profile for all proposed lots.
- 2. Applicant shall evaluate further improvement on Barretts Hill related to sight distance downhill site, including Barretts Hill Road relocation.

- 3. Applicant shall provide a bond for onsite and offsite improvements within the existing and proposed Right of Way, prior to recording the plans.
- 4. Applicant shall transfer the ownership of the road to the town and to the satisfaction of the Town prior to receiving the last Certificate of Occupancy of the last lot.

DPW has provided the following comments:

- 1. Station 5+25.00 add additional catch basin to opposite side of road.
- 2. Station 2+00.00 add additional catch basin to opposite side of road.
- 3. DMH # 17 should have a storm grate instead of a manhole cover since it is on the curb line.
- 4. All drainage on Barretts Hill Rd. shall be changed to enclosed drainage with catch basins until it outfalls at the existing drainage easement at 55 Barretts Hill Rd.

Zoning had provided the following comments:

- 1. Are there going to be Decks on the houses?
- 2. Proposed lot 151 lot 59-1 Looks really close to the front setback.
- 3. Are there garages and where are the driveways going to be located?
- 4. 151 lot 59-2 is this going to be a shared driveway with lot 59-8?

Fire has provided the following comments:

- 1. The State Adopted Fire Code, NFPA I requires a water supply of 1000 gallons per minute for one hour for firefighting. A 60,000 gallon cistern is required. Redesign the site plan showing the placement of the 60,000 gallon cistern. Provide a cistern drawing that meets Hudson Fire Department requirements. The cistern shall include 6"metal suction pipe, to 6" female, to 6" to 5" male reducer to be National Hose Thread. There shall also be a cap provided for the 6" to 5" reducer that will be left connected to the riser. Locks shall be provided for all access hatches. The cistern shall have a water level indicator.
- 2. The developer has the option of installing an NFPA 13D sprinkler systems in each dwelling in lieu of the cistern.
- 3. The proposed subdivision falls inside the 1000 foot distance from the center of the Tennessee Gas Pipeline. Advanced notification is required from the gas company per State of NH **RSA** 674:75. The advanced notification shall be sent KMEncroachmentsNorth@kindermorgan.com. This is required prior to any sitework, blasting, or building permits being issued. Once approval has been received this approval shall be sent to Nashua Regional Planning Commission email at outreachnashuarpc.org. All approval documentation shall be submitted with the building permit applications. No permits will be issued without all approvals.

No department comments remain outstanding, and full comments are provided in **Attachment** "B." The applicant provided a response letter on March 5, 2024 (Attachment "E") alongside the revised plan set.

RECOMMENDATIONS

Staff recommends accepting the application and holding a public hearing, followed by deliberation and consideration of the waiver required. Assuming the waiver is granted, staff recommends the board's consideration for approval.

The Applicant has addressed all comments issued by Peer Review and Town Staff, the application meets applicable zoning and land use regulation criteria.

DRAFT MOTIONS:

ACCEPT the subdivision application:

I move to accept the subdivision application for the Barrett Hill Subdivision SB# 08-23, Map 151/Lot 059.

DEFER the subdivision application:

	further consideration of this s 4 Planning Board meeting.	ubdivision application to the specific date of the
Motion by:	Second:	Carried/Failed:
To <u>GRANT</u> a wai	ver:	
1) § 289-37.A – Pla	an schedule and form	
minimum construct Board's discussion,	ion time of two years, wher	A – Plan schedule and form, to not require a e it would typically be required, based on the ant's representative, and in accordance with the est Form for said waiver.
Motion by:	Second:	Carried/Failed:
APPROVE the sub	division application:	

I move to approve the subdivision plan entitled: Barrett Hill Subdivision SB# 08-23, Map 151/Lot 059, 75 Barretts Hill Road, Hudson, NH; prepared by: Keach-Nordstrom Associates, 10 Commerce Park North Suite 3B, Bedford, NH 03110; prepared for: Barrett Hill, LLC, 21 Continental Boulevard, Merrimack NH 03054, consisting of twenty-four sheets, and general notes 1-28 on sheet 1; dated December 6, 2023, last revised March 4, 2024; and:

That the Planning Board finds that this application complies with the Zoning Ordinances, and with the Land Use Regulations; and for the reasons set forth in the written submissions, together with the testimony and factual representations made by the applicant during the public hearing;

Subject to, and revised per, the following stipulations:

- 1. All stipulations of approval shall be incorporated into the Notice of Decision, which shall be recorded at the HCRD, together with the Plan.
- 2. A cost allocation procedure (CAP) amount of \$6,194.00 per single-family residential unit shall be paid prior to the issuance of a Certificate of Occupancy for the new house lots.
- 3. All monumentation shall be set or bonded for prior to Planning Board endorsement of the Planof-Record.
- 4. Prior to the Planning Board endorsement of the Plan, it shall be subject to final administrative review by Town Planner and Town Engineer.
- 5. Construction activities involving the proposed undeveloped lots shall be limited to the hours between 7:00 A.M. and 7:00 P.M., Monday through Saturday. No exterior construction activities shall occur on Sunday.

Motion by:	Casand	Carried/Failed:	
MODOD DV:	Second:	Carried/Falled:	



LETTER OF TRANSMITTAL

DATE: December 11, 2023	
PROJECT NO: <u>23-0414-1</u>	
REFERENCE: Subdivision Application	ı – Barrett Hill, LLC
TO: Town of Hudson – Planning D 12 School Street Hudson, NH 03051	artment
ATTENTION: Mr. Brian Groth, AIG	– Town Planner
WE ARE SENDING YOU THE FOLLOWING ITEMS:	THESE ARE TRANSMITTED AS FOLLOWS:
 □ PLANS □ SPECIFICATIONS □ COPY OF LETTER □ APPLICATION □ CHANGE ORDER □ REPORT □ QUALIFICATIONS □ OTHER 	☐ FOR APPROVAL ☐ FOR YOUR USE ☑ AS REQUESTED ☐ FOR REVIEW & COMMENT ☐ RETURNED FOR CORRECTIONS ☐ APPROVED AS NOTED ☐ APPROVED AS SUBMITTED
One (1) original & one (1)	opy of completed Subdivision Application Package, Fee opy of Narrative, Abutter's List and Two (2) Sets of Plan Sets, Three (3) copies of Stormwater Report, PDF
COPY TO:	SIGNED: Katherine Cooper, Project Engineer
Civil Engineering	Land Surveying Landscape Architecture



December 11, 2023

Town of Hudson Planning Department 12 School Street Hudson, New Hampshire 03051

Subject:

Subdivision Application – Barrett Hill, LLC

Tax Map 151; Lot 59

75 Barretts Hill Road - Hudson, New Hampshire

KNA Project No. 23-0414-1

Dear Chairman and Board Members:

The above referenced project is being submitted for Subdivision approval from the Town of Hudson Planning Board. The property, located at 75 Barretts Hill Road, is approximately 35.36 acres in total area. The parcel, located entirely within the General (G-1) Zoning District, is currently undeveloped consisting mainly of woodlands with a powerline easement crossing the property. The Applicant is proposing a thirteen (13) lot residential open space subdivision and all associated site improvements. The attached documents outline the applicant's request for approval. All required information has been included within the submittal package. Keach Nordstrom Associates, Inc. will be present to further discuss the Subdivision Application at the scheduled hearing.

Enclosed is the following material for your review and approval:

- 1. Application for Subdivision Package (Original & Copy)
- 2. Application Fee: **\$2,837.01**
- 3. Consultant Review Fee: \$2,892.00
- 4. Narrative (Original & Copy)
- 5. Abutter's List & Two (2) Sets of Labels
- 6. Three (3) Full Size Plan Sets
- 7. Three (3) Copies of Stormwater Report
- 8. PDF

Kathine Cooper

If you have any questions or comments, please contact me at (603) 627-2881.

Sincerely,

Katherine Cooper Project Engineer

Keach Nordstrom Associates 10 Commerce Park North, Suite 3

Bedford, NH 03110

Civil Engineering

Land Surveying

Landscape Architecture



Town of Hudson 12 School Street Hudson, NH 03501

SUBDIVISION APPLICATION

Revised July 24, 2023

The following information must be filed with the Planning Department at the time of filing a site plan application:

- 1. One (1) original completed application with original signatures, and one (1) copy.
- 2. Three (3) full plan sets (sheet size: 22" x 34").
- 3. One (1) original copy of the project narrative, and one (1) copy.
- 4. A list of direct abutters and a list of indirect abutters, and two (2) sets of mailing labels for abutter notifications.
- 5. All of the above application materials, including plans, shall also be submitted in electronic form as a PDF.
- 6. All plans shall be folded and all pertinent data shall be attached to the plans with an elastic band or other enclosure.

The following information is required to filed with the Planning Department no later than 10:00 A.M., Tuesday ONE WEEK prior to the scheduled Planning meeting. The purpose of these materials is hardcopy distribution to Planning Board members, not review. Any plan revisions that require staff review must be submitted no later than 10:00A.M., Tuesday TWO WEEKS prior to the scheduled Planning meeting. Depending on the complexity of changes, more time may be required for review. Please contact the Town Planner if you have any questions on this matter.

- 1. Submission of fifteen (15) 11" X 17" plan sets, revised if applicable.
- 2. Submission of two (2) full plan sets (sheet size: 22" x 34"), if revised.
- 3. All of the above application materials, including plans, shall also be submitted in electronic form as a PDF.

Note: Prior to filing an application, it is recommended to schedule an appointment with the Town Planner.

SUBDIVISION APPLICATION

Date of Application: 12/11/23	Tax Map #:151 Lot #:59		
Site Address: 75 Barretts Hill Road			
Name of Project: Subdivision Barrett Hill, LLC			
Zoning District: General (G-1)	General SB#:		
Z.B.A. Action:	(For Town Use Only)		
PROPERTY OWNER:	DEVELOPER:		
Name: Barrett Hill, LLC	9		
Address: 21 Continental Boulevard, Door #4			
Address: Merrimack, NH 03054			
Telephone # 603-320-5123	3		
Email: johngargasz@gmail.com			
PROJECT ENGINEER:	SURVEYOR:		
Name: Keach-Nordstrom Associates, Inc.	Keach-Nordstrom Associates, Inc.		
Address: 10 Commerce Park North, Suite 3	10 Commerce Park North, Suite 3		
Address: Bedford, NH 03110	Bedford, NH 03110		
Telephone #603-627-2881	603-627-2881		
Email: pchisholm@keachnordstrom.com	chickey@keachnordstrom.com		
PURPOSE OF PLAN: The purpose of this plan is subdivision on the property and all associated imp	to depict a thirteen (13) lot open space provements.		
(For Town U	Jse Only)		
Routing Date: Deadline Date:	Meeting Date:		
I have no comments I have	comments (attach to form)		
Title:	Date:		
Department:			
Zoning: Engineering: Assessor: Police:	Fire: DPW: Consultant:		

SUBDIVISION PLAN DATA SHEET

PLAN NAME: _ Barrett Hill, LLC					
PLAN TYPE: Conventional Subdivision Plan of Open Space Development (Circle One)					
LEGAL DESCRIPTION:	MAP_	151	_LOT_	59	
DATE: December 11, 202	3		-		
Address:	75 Barr	etts Hill Road			
Total Area:	S.F	1,540,250	_	Acres:	35.36
Zoning:	Genera	l (G-1)			
Required Lot Area:	566,28	0 SF (43,560 SF ₎	per Lot)		
Required Lot Frontage:	100 FT				
Number of Lots Proposed:	Thirteen (13) Residential Lots & One (1) Non-buildable open space le				
Water and Waste System Proposed:	Individu	al wells and sept	ic systems		
Area in Wetlands:	None				
Existing Buildings To Be Removed:	None				
Flood Zone Reference:	FIRM	Map 33011C0517	7D		
Proposed Linear Feet Of New Roadway:	1,213.1	7 LF			

SUBDIVISION PLAN DATA SHEET

Dates/Case #/Description/ Stipulations of ZBA,		
Conservation Commission, NH Wetlands Board Action:		
NH Weilands Board Action.		
(Attach Stipulations on Separate Sheet)		
List Permits Required: NHD	ES AOT	
NHD	ES Subdivision	
NHD	ES Individual Subsurface Dis	sposal
*Waivers Requested:	Hudson Town Code Reference	Regulation Description
	1.	
	2.	
	3.	
	4.	
	5.	
	6.	
*/I C C 1 C T II \	7.	
*(Left Column for Town Use)		
	(For Town Use Only)	
Data Sheets Checked By:		Date:

P =Pending

Key: Y=Yes

(Continue next page)

This checklist is intended to help the applicant and staff to ensure application completeness. Please refer to the regulations on the exact language of each requirement.

W=Waiver Request

Relevant Regulations: § 276-11.1 General Plan Requirements §§ 289-26 - 289-27 Subdivision Plan Requirements Notes \overline{X} \Box - A list of the names and addresses of the owner(s) of the property, the applicant(s), and all abutters as indicated in the office of the Town Assessor records not more than five (5) days prior to the day of filing [§ 276-11.1.A.] X — Sets of plans and copies as indicated on application. X - Scale no smaller than 50 feet to the inch (1" = 50') [§ 276-11.1.B.(2)] X - Title block in the lower right-hand corner of the plan, containing: [§ 276-11.1.B.(3)] $\overline{\mathbf{X}}$ \square \square - Title, including the term "site plan" or "subdivision plan" 6. X 🗌 🗀 - The name for whom the plan was prepared 7. X 🗆 🗀 - Preparer of the plan - The scale(s) of the plan 9. X 🗆 - Date of the plan 10. \overline{X} - Appropriate revision block 11. X - Approval block (2"x6") located on the lower left corner of each sheet, with the required language and signature lines [§ 276-11.1.B.(4) & § 289-27.A] 12. X - A space (2"x1.5") adjacent to the approval block containing the required statement [§ 276-11.1.B.(5)] [§ 276-11.1.B.(6)] 14. X - Name and address of all abutting property owners [§ 276-11.1.B.(7)] 15. X - A locus plan at one inch equals 1,000 feet (1" = 1,000') [§ 276-11.1.B.(8)]

This checklist is intended to help the applicant and staff to ensure application completeness. Please refer to the regulations on the exact language of each requirement.

Key: Y=Yes P=Pending W=Waiver Request	
<u>Y</u> <u>P</u> <u>W</u>	<u>Notes</u>
16. X — Boundary of the entire parcel held in single ownership with boundary dimensions and bearings [§ 276-11.1.B.(9)]	
17. X — - Error of closure shown and certified by a licensed land surveyor	
18. 🔲 🗌 - North point arrow	
19. X — Zoning classification note of the tract and location of the zoning district boundaries if the property is located in two or more zoning district [§ 276-11.1.B.(10)]	
20. X - The location of all buildings within 50 feet of the tract [§ 276-11.1.B.(15)]	
21. X — - The location of roadways, driveways, travel areas or parking areas within 200 feet of the tract, in accordance with § 276-11.1.B.(16)	
22. X — Existing topography at two-foot contour intervals of that portion of the tract being proposed for development from a topographic survey and contours on the remainder of the tract from a reliable plan source [§ 276-11.1.B.(17)]	
23. X - Proposed topography at two-foot contour intervals [§ 276-11.1.B.(18)]	
24. X — - A note identifying the Tax Map and Lot Number of the tract [§ 276-11.1.B.(19)]	
25. 🗓 🗀 - The location of all existing buildings (including size and height), driveways, sidewalks, parking spaces, loading area, open spaces, large trees, open drainage courses, signs, exterior lighting, service areas, easements landscaping and other pertinent items. [§ 276-11.1.B.(20)]	
26. X - The location of all proposed construction, buildings, structures, pavement, etc. [§ 276-11.1.B.(21)]	
27. X - A green area shown between the right-of-way line and any pavement, gravel or structure meeting the required minimum width [§ 276-11.1.B.(22)]	
N/A 28.	
(Continue next page)	

This checklist is intended to help the applicant and staff to ensure application completeness. Please refer to the regulations on the exact language of each requirement.

Key: Y=Yes	P=Pending	W=Waiver Request	NA=Not Applic	able (please explain)
<u>Y</u> <u>P</u> <u>W</u> 29. ဩ □ □	<u>NA</u> ☐ - Proposed	subdivision name [§ 289	-26.B.(1)]	<u>Notes</u>
30. 🛛 🗌	Names of	owners of record of abu 26.B.(2)]	` '-	
31. 🛛 🗌		of property lines and the ions [§ 289-26.B.(3)]	r approximate	
32. 🗓 🗌	monum control	and description of each pent and benchmark, inch points and reference to a lark [§ 289-27.B.(7)]	iding primary	
33. 🗵 🗌 🗀		and proposed easements stable [§ 289-26.B.(3)]	shown on plan,	
34. 🛛 🗌	rock led	, watercourses, ponds or dges and other essential f 26.B.(3)].		
35. 🛚 🗀	propose providi and sur	vater mains, sewers, culved connections or alternating water supply and disp face / stormwater drainag 26.B.(4) & § 289-27.B.(4)	ive means of osal of sewage	
36. 🛚 🗀	results, field, ea propose	of each percolation test he each proposed septic tand the proposed well and typed on-lot water and sewer 26.B.(4)]	k and drainage pical designs of	
37. 🛛 🗌	existing	name, and right-of-way and proposed streets wiribed in § 289-26.B.(5)		
38. 🗓 🗌		ry designs of any bridges nay be required [§ 289-2		
39. 🛛 🗌	Street cro	ss sections [§ 289-27.B.(3)]	
40. 🗓 🗌		lots, approximate square and setback lines. [§ 28]		
41.	dedicat	of all parcels of land project to public use and the dication [§ 289-26.B.(7)]	conditions of	
(Continue next	page)		,	L

This checklist is intended to help the applicant and staff to ensure application completeness. Please refer to the regulations on the exact language of each requirement.

Key: Y=Yes	P =Pending	W=Waiver Request	NA=Not Applica	able (please explain)
$\underline{\underline{Y}} \ \underline{\underline{P}} \ \underline{\underline{W}}$				<u>Notes</u>
42.	the sub prospec	e plan submitted covers of divider's entire holding, ctive future street system nitted part [§ 289-26.B.(9	a sketch of the of the	
(End of check	list)			

SUBDIVISION PLAN APPLICATION AUTHORIZATION

I hereby apply for Subdivision Plan Review and acknowledge I will comply with all of the Ordinances of the Town of Hudson, New Hampshire State Laws, as well as any stipulations of the Planning Board, in development and construction of this project. I understand that if any of the items listed under the Subdivision Plan specifications or application form are incomplete, the application will be considered rejected.

Pursuant to RSA 674:1-IV, the owner(s) by the filing of this application as indicated above, hereby given permission for any member of the Hudson Planning Board, the Town Planner, the Town Engineer, and such agents or employees of the Town or other persons as the Planning Board may authorize, to enter upon the property which is the subject of this application at all reasonable times for the purpose of such examinations, surveys, tests and inspections as may be appropriate. The owner(s) release(s) any claim to or right he/she (they) may now or hereafter possess against any of the above individuals as a result of any examinations, surveys, tests and/or inspections conducted on his/her (their) property in connection with this applications.

	Signature of Owner:	Date: 12/11/7023
	Print Name of Owner: JOHN GARGASZ	
*	If other than an individual, indicate name of organization and its principal corporate officers.	owner, partners, or
	Signature of Developer:	Date: 17/11/2023
	Print Name of Developer: John GARBASZ	

The developer/individual in charge must have control over all project work and be available to the Code Enforcement Officer/Building Inspector during the construction phase of the project. The individual in charge of the project must notify the Code Enforcement Officer/Building Inspector within two (2) working days of any change.

SCHEDULE OF FEES

A.	REVIEW FEES:	
	1. \$170.00 per proposed lot @13 LOTS	\$_2,210.00
	CONSULTANT REVIEW FEE: (Separate Check)	
	Total 4.82 acres @ \$600.00 per acre, or \$1,250.00, whichever is greater. *Disturbed Acreage	\$_2,892.00
	This is an estimate for cost of consultant review. The fee is expected to cover the amount. A complex project may require additional funds. A simple project may result in a refund.	
	<u>LEGAL FEE:</u>	
	The applicant shall be charged attorney costs billed to the Town review of any application plan set documents.	for the Town's attorney
B.	POSTAGE:	
	Direct Abutters Applicant, Professionals, etc. as required by RSA 676:4.1.d @\$5.01 (or Current Certified Mail Rate	\$ 205.41
		\$6.60
C.	TAX MAP UPDATE FEE	
	2 to 7 lots (# of lots x \$30.00) + \$25.00 (min. \$85.00)	\$
	8 lots or more (min. \$325.00) @13 LOTS	\$ 415.00
	TOTA	\$\frac{\$2,837.01}{\$2,892.00}\$ CHECK #2
	(For Town Use Only)	
AMC	OUNT RECEIVED: \$ DATE RECEIVED:	
REC	CEIPT NO.: RECEIVED BY:	

NOTE: fees below apply only upon plan approval, NOT collected at time of application.

D. RECORDING:

The applicant shall be responsible for the recording of the approved plan, and all documents as required by an approval, at the Hillsborough County Registry of Deeds (HCRD), located at 19 Temple Street, Nashua, NH 03061. Additional fees associated with recording can be found at HCRD.

E. <u>COST ALLOCATION PROCEDURE AMOUNT CONTRIBUTION AND OTHER</u> IMPACT FEE PAYMENTS:

To be determined by the Planning Board at time of plan approval and shall be paid by the applicant at the time of submittal of the Certificate of Occupancy Permit requests.

The applicant shall be responsible for all fees incurred by the town for processing and review of the applicant's application, plan and related materials.



December 11, 2023

Town of Hudson Planning Department 12 School Street Hudson, New Hampshire 03051

Subject:

Subdivision Application - Barrett Hill, LLC

Tax Map 151; Lot 59

75 Barretts Hill Road - Hudson, New Hampshire

KNA Project No. 23-0414-1

PROJECT NARRATIVE

The proposed subdivision application is being submitted for approval for an open space subdivision comprised of thirteen (13) residential lots and one (1) non-buildable open space lot on the property located at 75 Barretts Hill Road in Hudson, New Hampshire. The property, approximately 1,540,250-sf (35.36 acres) in total area, is located entirely within the General (G-1) Zoning District. It is currently undeveloped consisting mainly of woodlands with a powerline easement crossing the property. There are no wetlands onsite.

The proposed lots will have frontage off a proposed cul-de-sac roadway (Windsor Lane), approximately 1,213.17-lf in length. Each lot will be serviced by individual private wells and septic systems. Other site improvements will include stormwater management provisions, comprised of a closed conveyance system which outlets into a large pocket pond, and underground utilities.

Owner Affidavit

I, <u>John Gargasz</u>, authorized representative of Barrett Hill, LLC and owner of the property referenced on Tax Map 151 as Lot 59, located at 75 Barretts Hill Road, Hudson, New Hampshire, hereby verify that I have authorized Keach-Nordstrom Associates, Inc. to submit on my behalf, any and all applicable State and local permit applications as they pertain to improvements on said property.

Additionally, I authorize Keach-Nordstrom Associates, Inc. to aid in the representation of these applications throughout the approval process.

Signature of Owner:	AMM1
Printed Name of Owner:	John Gargasz
Address of Owner:	21 Continental Boulevard, Door #4 Merrimack, NH 03054
Date:	12/11/2023

Abutters List Barrett Hill, LLC Hudson, NH KNA#23-0414-1

Tax Map 151	Lot 59	Owner/Applicant Barrett Hill, LLC 21 Continental Boulevard, Door #4 Merrimack, NH 03054
Tax Map 152	Lot 13-1	Direct Abutters Brian Surette 85A Barretts Hill Road Hudson, NH 03051
152	13-2	David Wilkins 85B Barretts Hill Road Hudson, NH 03051
152	13-3	Shana Marie Beland 87A Barretts Hill Road Hudson, NH 03051
152	13-4	Jennifer Buchanan 87B Barretts Hill Road Hudson, NH 03051
152	9	Jeffrey Rogers 95 Barretts Hill Road Hudson, NH 03051
152	8-1	Ryan Mcallister & Allysa Lamothe 97 Barretts Hill Road Hudson, NH 03051
152	8-2	Stephen Javier Diaz & Brittany Renee Desrocher 99 Barretts Hill Road Hudson, NH 03051
152	7	Donald & Aurelie Connors 101 Barretts Hill Road Hudson, NH 03051

152	5	Karen Brown 107 Barretts Hill Road Hudson, NH 03051
152	4	Conrad & Judith Gauthier 113 Barretts Hill Road Hudson, NH 03051
152	3	Sean Hamel Trust Sean Hamel, Trustee 115 Barretts Hill Road Hudson, NH 03051
152	2	Tulipani Revocable Trust Gail Marie Tulipani, Trustee 23 Mallard Drive Hudson, NH 03051
152	1	Christopher & Alexa Porembski 22 Mallard Drive Hudson, NH 03051
151	41	Thomas & Sharon Fincher 20 Mallard Drive Hudson, NH 03051
151	40	Jesse & Barbara Ciancetta 18 Mallard Drive Hudson, NH 03051
151	39	Beverly & David Salmon 16 Mallard Drive Hudson, NH 03051
151	42	Christine Byrnes PO Box 53 Nashua, NH 03061
151	43-1	Vernon & Sarah Thompson 22A Rangers Drive Hudson, NH 03051
151	43-2	Dominic Jarry & Kay Nash 5 Jarry Way Hudson, NH 03051

151	44-1	Jeremy & Bianca Hicks 20A Rangers Drive Hudson, NH 03051
151	44-2	Stephanie Ruszczyk 20B Rangers Drive Hudson, NH 03051
151	45	Wendy Lorentzen 18A Rangers Drive Hudson, NH 03051
151	46-1	Jasmine & Devon Malburne 16A Rangers Drive Hudson, NH 03051
151	46-2	Elizabeth Rocha 16B Rangers Drive Hudson, NH 03051
151	47-1	Gail Lafond 14A Rangers Drive Hudson, NH 03051
151	47-2	David & Kelly Mastroelli 14B Rangers Drive Hudson, NH 03051
151	48-1	Gary & Ellen-Jean Haugh 12A Rangers Drive Hudson, NH 03051
151	48-2	Jacqueline Smith 12B Rangers Drive Hudson, NH 03051
151	49-1	Dave & Nicole Mathieu 15 Lund Street Litchfield, NH 03052
151	49-2	Christine Mathieu 10 1/2B Rangers Drive Hudson, NH 03051

151	50-1	Susan Scalet 10A Rangers Drive Hudson, NH 03051
151	50-2	Shawn Patterson 10B Rangers Drive Hudson, NH 03051
151	51-1	Daniel & Karen Desmarais 8A Rangers Drive Hudson, NH 03051
151	51-2	Ryan Corkery 8B Rangers Drive Hudson, NH 03051
151	55	William & Kathleen Shea 65 Barretts Hill Road Hudson, NH 03051
151	56	David & Judith Silva 70 Barretts Hill Road Hudson, NH 03051
151	57	John & Kathryn Burke 2 Hilltop Drive Hudson, NH 03051
151	58	Nichole Sullivan 76 Barretts Hill Road Hudson, NH 03051
143	11	23 Land Holding, LLC 23 Woodcrest Drive Hudson, NH 03051
Tax Map 160	Lot 77	Indirect Abutters Kenneth & June Kopka 14 Mallard Drive Hudson, NH 03051
160	76	Robert & Laura Lind 12 Mallard Drive Hudson, NH 03051

151	38-1	Alita Ann Wycoff 24A Rangers Drive Hudson, NH 03051
151	38-2	Sheila Ledoux 24B Rangers Drive Hudson, NH 03051
151	52-1	Lesa Hallbach 6A Rangers Drive Hudson, NH 03051
151	52-2	Kurt & Lisa Perry 6B Rangers Drive Hudson, NH 03051
151	53-1	Webber Revocable Trust of 2006 David Webber, Trustee 8400 Bridgeport Bay Cr Mount Dora, FL 32757
151	53-2	Karen & Sean Merrow 61 Sousa Boulevard Hudson, NH 03051
142	39	Damon & Janna Miller 3 Hilltop Drive Hudson, NH 03051
143 152	13 & 14 14	17 Hudson Associates c/o CPM Inc. 80 Nashua Road Londonderry, NH 03053

Professionals to be notified:

Engineer/Surveyor

Keach-Nordstrom Associates Inc. 10 Commerce Park North, Suite 3B Bedford, NH 03110 March 4, 2024

Mr. Jay Minkarah Acting Town Planner Town of Hudson 12 School Street Hudson, NH 03051

Subject:

Town of Hudson Planning Board Review

Barrett Hill Subdivision Plan

Tax Map 151 Lot 59

KNA Project No. 23-0414-1

The Applicant is requesting a waiver from the following section of the Town of Hudson Site Plan Regulation: Section 289-37(A) Plan Schedule and Form

Hardship reason(s) for granting this waiver:

The applicant would like to construct the proposed roadway and develop the 13 lots as quickly and efficiently as possible and would prefer not to phase the development for two years as required. Adhering to this rule would cause an unnecessary hardship for the applicant and the surrounding residential abutters, as the overall small scale of the project makes it best suited to be developed within a year and disturbance from construction should be limited as such.

Reason(s) for granting this waiver, relative to not being contrary to the spirit and intent of the Land Use Regulations:

Granting this waiver would not be contrary to the spirit and intent of the regulation because requiring phased construction would only increase the amount and time of disturbance to the surrounding residential abutters, which there are many of.

Dubowik, Brooke

From: Dhima, Elvis

Sent: Monday, December 18, 2023 3:26 PM **To:** Dubowik, Brooke; Gradert Benjamin

Cc: Hebert, David; Kirkland, Donald; McElhinney, Steven; Michaud, Jim; Sullivan, Christopher;

Malley, Tim; Twardosky, Jason

Subject: RE: DEPT SIGN OFF - SB# 08-23 Barrett Hill LLC OSD Subdivision Plan

Please see below

- 1. Applicant shall provide driveway plan and profile for all proposed lots
- 2. Applicant shall evaluate further improvement on Barrets Hill related to sight distance downhill site, including Barretts Hill Road relocation.
- 3. Applicant shall provide a bond for onsite and offsite improvements within the existing and proposed Right of Way, prior to recording the plans.
- 4. Applicant shall transfer the ownership of the road to the town and to the satisfaction of the Town prior to receiving the last Certificate of Occupancy of the last lot.

Thanks

Ē

Elvis Dhima, P.E. Town Engineer

12 School Street Hudson, NH 03051 Phone: (603) 886-6008 Mobile: (603) 318-8286



1

Dubowik, Brooke

From: Twardosky, Jason

Sent: Tuesday, December 19, 2023 1:41 PM

To: Dubowik, Brooke; Dhima, Elvis; Gradert Benjamin; Hebert, David; Kirkland, Donald;

McElhinney, Steven; Michaud, Jim; Sullivan, Christopher; Malley, Tim

Subject: RE: DEPT SIGN OFF - SB# 08-23 Barrett Hill LLC OSD Subdivision Plan

1) Station 5+25.00 add additional catch basin to opposite side of road.

- 2) Station 2+00.00 add additional catch basin to opposite side of road.
- 3) DMH # 17 should have a storm grate instead of a manhole cover since it is on the curb line.
- 4) All drainage on Barretts Hill Rd. shall be changed to enclosed drainage with catch basins until it outfalls at the existing drainage easement at 55 Barretts Hill Rd.

SUBDIVISION APPLICATION

Date of Application: 12/11/23	Tax Map #:151 Lot #:59				
Site Address: 75 Barretts Hill Road					
Name of Project: Subdivision Barrett Hill, LLC					
Zoning District: General (G-1)	General SB#: 08-23				
Z.B.A. Action:	(For Town Use Only)				
	DEVELOPER:				
PROPERTY OWNER:	DEVELOTEX.				
Name: Barrett Hill, LLC					
Address: 21 Continental Boulevard, Door #4					
Address: Merrimack, NH 03054					
Telephone # 603-320-5123					
Email: johngargasz@gmail.com					
PROJECT ENGINEER:	SURVEYOR:				
Name: Keach-Nordstrom Associates, Inc.	Keach-Nordstrom Associates, Inc.				
Address: 10 Commerce Park North, Suite 3	10 Commerce Park North, Suite 3				
Address: Bedford, NH 03110	Bedford, NH 03110				
Telephone #603-627-2881	603-627-2881				
Email: _pchisholm@keachnordstrom.com	chickey@keachnordstrom.com				
PURPOSE OF PLAN:					
The purpose of this plan is to depict a thirteen (13) lot open space					
subdivision on the property and all associated improvements.					
(For Town U	**				
Routing Date: 12/18/23 Deadline Date: 12	/26/23 Meeting Date: TBD				
I have no comments X I have comments (attach to form)					
(Initials) Title: Zowi vo Anie Date:					
Department:					
Zoning: X Engineering: Assessor: Police:	Fire: DPW: Consultant:				



TOWN OF HUDSON

Land Use Division





Zoning SB# 08-23 Review

December 20, 2023

- 1. Are there going to be Decks on the houses
- 2. Proposed lot 151 lot 59-1 Looks really close to the front setback
- 3. Are there garages and where are the driveways going to be located.
- 4. 151 lot 59-2 is this going to be a shared driveway with lot 59-8

Chris Sullivan
Zoning Administrator/Code Enforcement Officer
(603) 816-1275
csullivan@hudsonnh.gov

NOTE: this determination may be appealed to the Hudson Zoning Board of Adjustment within 30 days of the receipt of this letter.

SUBDIVISION APPLICATION

Date of Application: 12/11/23	Tax Map #:	151	Lot #:	59	
Site Address: 75 Barretts Hill Road					
Name of Project: Subdivision Barrett Hill, LLC					
Zoning District: General (G-1)	General SB#:				
Z.B.A. Action:			(For Town Us	e Only)	
PROPERTY OWNER:	DEVELOPER:				
Name: Barrett Hill, LLC	8 <u> </u>				
Address: 21 Continental Boulevard, Door #4					
Address: Merrimack, NH 03054					
Telephone # 603-320-5123					
Email: johngargasz@gmail.com					
PROJECT ENGINEER:	SURVEYOR:				
Name: Keach-Nordstrom Associates, Inc.	Keach-Nordstrom Associates, Inc.				
Address: 10 Commerce Park North, Suite 3	10 Commerce Park North, Suite 3				
Address: Bedford, NH 03110	Bedford, NH 03110				
Telephone # 603-627-2881	603-627-2881				
Email: _pchisholm@keachnordstrom.com	chickey@keachnordstrom.com				
PURPOSE OF PLAN: The purpose of this plan is to depict a thirteen (13) lot open space subdivision on the property and all associated improvements.					
(For Town Use Only)					
Routing Date: 12/18/23 Deadline Date: 12/26/23 Meeting Date: TBD					
I have no comments / I have comments (attach to form) ORH Title: Fire Marshal Date: 12/19/23 (Initials)					
Department:	/				
Zoning: Engineering: Assessor: Police: Fire: DPW: Consultant:					



TOWN OF HUDSON

FIRE DEPARTMENT

INSPECTIONAL SERVICES DIVISION



12 SCHOOL STREET, HUDSON, NEW HAMPSHIRE 03051

Emergency Business Fax

911 603-886-6005 603-594-1142 Scott Tice Chief of Department

TO: Town Planner

FR: David Hebert Fire Marshal

DT: December 19, 2023

RE: 75 Barretts Hill Road Residential Subdivision

- The State Adopted Fire Code, NFPA 1 requires a water supply of 1000 gallons per minute for one hour for firefighting. A 60,000 gallon cistern is required. Redesign the site plan showing the placement of the 60,000 gallon cistern. Provide a cistern drawing that meets Hudson Fire Department requirements. The cistern shall include 6"metal suction pipe, to 6" female, to 6" to 5" male reducer to be National Hose Thread. There shall also be a cap provided for the 6" to 5" reducer that will be left connected to the riser. Locks shall be provided for all access hatches. The cistern shall have a water level indicator.
- The developer has the option of installing an NFPA 13D sprinkler systems in each dwelling in lieu of the cistern.
- The proposed subdivision falls inside the 1000 foot distance from the center of the Tennessee Gas Pipeline. Advanced notification is required from the gas company per State of NH RSA 674:75. The advanced notification shall be sent to KMEncroachmentsNorth@kindermorgan.com. This is required prior to any site work, blasting, or building permits being issued. Once approval has been received this approval shall be sent to Nashua Regional Planning Commission email at outreach@nashuarpc.org. All approval documentation shall be submitted with the building permit applications. No permits will be issued without all approvals.

David Hebert Fire Marshal

Dubowik, Brooke

From: Hebert, David

Sent: Tuesday, March 5, 2024 2:17 PM

To: Dubowik, Brooke

Subject: RE: Barrett Hill Subdivision Resubmittal

I am all set with the proposed 30,000 gallon cistern shown on the revised plan. The advanced notification to Tennessee Gas shall be completed and the approval from Tennessee Gas shall be submitted to Nashua Regional Planning Commission and to the building department with permit applications. Last I spoke to Peter @ KNA he stated they sent in the notification. Someone on their end needs to follow thru to get the approval documents.



Town of Hudson | 12 School Street | Hudson, NH 03051 603-886-6005 (Main) | 603-816-1271 (Direct)

Dubowik, Brooke

From: McElhinney, Steven

Sent: Thursday, December 21, 2023 7:07 AM

To: Dubowik, Brooke

Subject: RE: DEPT SIGN OFF - SB# 08-23 Barrett Hill LLC OSD Subdivision Plan

No Comment

SUBDIVISION APPLICATION

Date of Application: 12/11/23	Tax Map #:	151	Lot #:	59	
Site Address: 75 Barretts Hill Road					
Name of Project: Subdivision Barrett Hill, LLC					
Zoning District: General (G-1)	General SB#:		08-23		
Z.B.A. Action:	4		(For Town Us	е Опіу)	
PROPERTY OWNER:	DEVELOPER	<u> </u>			
Name: Barrett Hill, LLC					
Address: 21 Continental Boulevard, Door #4	<u> </u>	7.000		<u> </u>	
Address: Merrimack, NH 03054					
Telephone # 603-320-5123					
Email: johngargasz@gmail.com	Y				
PROJECT ENGINEER:	SURVEYOR:				
Name: Keach-Nordstrom Associates, Inc.	Keach-Nordstrom Associates, Inc.				
Address: 10 Commerce Park North, Suite 3	10 Commerce Park North, Suite 3				
Address: Bedford, NH 03110	Bedford, NH 03110				
Telephone # 603-627-2881	603-627-2881				
Email: pchisholm@keachnordstrom.com	chickey@ke	achnor	dstrom.com		
The purpose of this plan is to depict a thirteen (13) lot open space subdivision on the property and all associated improvements.					
(For Town U	se Only)				
Routing Date: 12/18/23 Deadline Date: 12	/26/23 Me	eting D	ate:TBD)	
I have no comments I have comments (attach to form) Title: ASSESSON Date: 12-18-23 (Initials)					
Department:					
Zoning: Engineering: Assessor: Police: Fire: DPW: Consultant:					



January 4, 2024

Mr. Jay Minkarah Acting Town Planner Town of Hudson 12 School Street Hudson, NH 03051

Re: Town of Hudson Planning Board Review Barrett Hill Subdivision Plan Tax Map 151, Lot 59; Acct. #1350-085 Reference No. 20030249.2330

Dear Mr. Minkarah:

Fuss & O'Neill, Inc. has reviewed the first submission of the materials received on December 15, 2023, related to the above-referenced project. Authorization to proceed was received on December 19, 2023. A list of items reviewed is enclosed. The scope of our review is based on the Subdivision Plan Review Codes, Stormwater Codes, Driveway Review Codes, Sewer Use Ordinance 77, Zoning Regulations, and criteria outlined in the CLD Consulting Engineers Proposal approved September 16, 2003, revised September 20, 2004, June 4, 2007, September 3, 2008, and October 2015.

We have included a copy of Fuss & O'Neill's evaluation of the checklist for your reference. We note that several items could not be verified by Fuss & O'Neill and require action by the Town.

The project appears to consist of subdividing the existing 35.36-acre existing lot to create a thirteen (13)-lot subdivision with open space. A new roadway with a cul-de-sac is proposed as part of the subdivision. The new subdivision lots are proposed to be serviced by private wells and subsurface disposal systems.

The following items are noted:

The Gateway Building 50 Commercial Street Manchester, NH 03101 † 603.668.8223 800.286.2469

www.fando.com

California
Connecticut
Maine
Massachusetts
New Hampshire
Rhode Island

Vermont

1. Administrative and Subdivision Review Codes (HR 276 & HR 289)

- a. HR 276-11.1.B.(13). The applicant has not shown any signs other than traffic signs on the plan set.
- b. HR 276-11.1.B.(14). The applicant has not shown any lighting on the plan set. The applicant should confirm if any lighting is proposed and provide locations and details, or add the required note if lighting is not proposed.
- c. HR 276-11.1.B.(15). The applicant has not shown any buildings within 50 feet of the site.



Mr. Jay Minkarah January 4, 2024 Page 2 of 5

- d. HR 276-11.1.B.(16). The applicant has not shown driveways and parking areas within 200 feet of the site on the plan set.
- e. HR 276-11.1.B.(20). The applicant has not shown any existing buildings or features on the existing site. The applicant should confirm that the site was not previously developed in any way.
- f. HR 289-15. The applicant has noted that the site is not located within a Flood Hazard area.
- g. HR 289-22. The applicant shall coordinate with the Town of Hudson regarding the deeding of open space areas.
- h. HR 289-28.A. The applicant should provide a detail for the proposed granite bounds to be set.
- i. HR 289-37.A. The applicant has not provided any information on phasing of subdivision construction on the plan set. We note that subdivisions with between six and sixteen lots must be developed over two years per the Regulation.

2. Driveway Review Codes (HR 193-10)

- a. HR 193-10. The applicant has shown proposed approximate driveway locations within the Right-of-Way on the plan set, but has not shown any of these further extended into the lots. Due to the extensive grading adjacent to the roadway some driveways may be difficult to construct to meet the Town's maximum slope requirements.
- b. HR 193-10. Several proposed driveway locations are located at property lines (lot 6) or configured in such a way that the driveway may encroach on an adjacent lot (lot 8). Also there doesn't appear to be a proposed driveway location for lot 9.
- c. HR 193-10.E. The applicant has provided sight distance information for the proposed roadway on the plan set. The sight distance looking left (west) from Windsor Lane may be obscured by brush if not maintained and also could be significantly impacted by snowbanks during winter periods. The applicant should clarify how this area is be maintained to provide sufficient sight distance in this direction.

3. Roadway Design

- a. HR 289-18.B.(5). The applicant should add dead-end sign location to the plan set. Sign details have been provided.
- b. The applicant should label the curb curve radii on the plan set.
- c. HR 289-11.1.(21). The applicant has not shown a detectable warning paver at the sidewalk termination at the cul-de-sac.

4. Drainage Design /Stormwater Management (HR 289-20.C./Chapter 290)

- a. HR 289-20.C.(1). The applicant should confirm that all off-site improvements were coordinated with the Town Engineer, included the runoff leaving the site via the rip-rap lined swale along Barretts Hill Road.
- b. HR 290-5.B.2.b. The applicant should provide language in the Stormwater Management Report, stating if and how low impact development (LID) strategies for stormwater runoff were evaluated for this project.



Mr. Jay Minkarah January 4, 2024 Page 3 of 5

- c. HR 290-6.A.8. We note the requirement of the applicant to coordinate a pre-construction meeting with the Town Engineer.
- d. HR 290.6.A.12. The applicant should provide a note for winter stabilization on the erosion control plans.
- e. HR 290-8.A.4 & 5. We note the requirement of the applicant to coordinate the need for a Bond and/or Escrow with the Town Engineer.
- f. In section D. Summary of the project narrative in the Stormwater Management Report the applicant references the Town of Goffstown Stormwater Regulations. This should be revised to the Town of Hudson.
- g. The applicant should clarify how much of the existing 12" CMP is to be replaced at the Barretts Hill Road crossing near pole PSNH 21B/30.
- h. The applicant should clarify if driveway aprons are being replaced as part of the off-site improvements for the drainage swale installation, and if so show this on the plans.
- i. The applicant should forward any review comments received from the NHDES AoT Bureau to the Town for their review and records.
- j. The applicant should confirm that the proposed pocket pond is not considered a dam per NHDES definitions.
- k. The applicant should coordinate with the Town regarding the need for fencing or other means to prevent children, animals, etc. entering the water in the proposed pocket pond.
- 1. ETGTD 930.1. The applicant should review the depth of the drainage within the roadway, it does not maintain the Town minimum depth of 4' of cover for all pipes.
- m. ETGTD 930.4. The applicant shall review the slope of the proposed drainage system with the Town Engineer. Slopes within the proposed Right-of-Way are less than the minimum 2% required.
- n. ETGTD 930.10. We note the requirement for curb inlet drainage structures at all vertical sags. CB19 and CB20 are designed at a vertical sag.
- o. ETGTD 930.11. CB18 and CB20 are shown as double catch basins on the plans. The applicant should review this design with the Town Engineer for approval.
- p. The applicant will be required to comply with all provisions of the Town of Hudson's MS4 permit, including but not limited to annual reporting requirements, construction site stormwater runoff control, and record keeping requirements.
- q. Please note that this review was carried out in accordance with applicable regulations and standards in place in New Hampshire at this time. Note that conditions at the site, including average weather conditions, patterns and trends, and design storm characteristics, may change in the future. In addition, future changes in federal, state or local laws, rules or regulations, or in generally accepted scientific or industry information concerning environmental, atmospheric and geotechnical conditions and developments may affect the information and conclusions set forth in this review. In no way shall Fuss & O'Neill be liable for any of these changed conditions that may impact the review, regardless of the source of or reason for such changed conditions. Other than as described herein, no other investigation or analysis has been requested by the Client or performed by Fuss & O'Neill in preparing this review.



Mr. Jay Minkarah January 4, 2024 Page 4 of 5

5. Zoning (HR 334)

- a. HR 334-14 and HR 276-11.1.B.(20). The applicant has not noted the maximum proposed building heights on the plan set. The applicant should note the maximum building height of 38 feet on the plan set.
- b. HR 334-20. The site is located in the General (G-1) District. The applicant has noted that open space development is the proposed use. The applicant should confirm if the lots are to be single family homes.
- c. HR 334-27. We note that the conventional subdivision design appears to meet the lot size and frontage requirements for the district.
- d. HR 334-35. The applicant has noted that there are no wetlands located on the site.
- e. HR 334-50. The applicant has shown that 13 lots could be accommodated as part of the Conventional Subdivision plan. The applicant has proposed 13 lots reduced the minimum lot areas to 1.0 acre and the frontage to a minimum of 100 feet for each lot.

6. Sewer/Water Design/Conflicts & Utility Design/Conflicts (HR 276-13.E.)

- a. The applicant has not shown any provisions for fire protection (cisterns, etc.). The applicant should coordinate with the Hudson Fire Department to ensure adequate fire protection is provided for the site.
- b. The applicant has shown proposed 4k septic reserve areas and 75-foot protective well radii on the plans for the individual lots. Proposed well radii do not extend onto adjacent lots. They have also provided a typical 4-bedroom residential single family home detail, which shows a water service connection in the rear of the building and a septic tank in the front.

7. Erosion Control/Wetland Impacts

a. The applicant has noted that the Town reserves the right to require additional erosion control measures should they be found necessary

8. State and Local Permits

- a. The applicant has noted the required permits on the plan set.
- b. HR 290-10.B. The applicant has noted the need for a NPDES Notice of Intent and a SWPPP on the plan.
- c. Additional local permitting may be required.

9. Other

a. ETGTD Section 565.1.1. The applicant is reminded that the Town of Hudson has specific requirements for the importing of off-site fill materials for use in constructing this project. We recommended that these requirements be stated on the plans for the Contractors attention.



Mr. Jay Minkarah January 4, 2024 Page 5 of 5

Please feel free to call if you have any questions.

Very truly yours,

Steven W. Reichert, PE

SWR:elc

Enclosure

cc: Town of Hudson Engineering Division – File Keach-Nordstrom Associates, Inc. pchisholm@keachnordstrom.com March 4, 2024

Steven Reichert, PE Fuss & O'Neill 50 Commercial Street, Unit 2S Manchester, NH 03101

Subject:

Town of Hudson Planning Board Review

Barrett Hill Subdivision
Tax Map 151 Lot 59
KNA Project No. 23-0414-1

Dear Mr. Reichert:

Our office is in receipt of a review letter, dated January 4, 2024. Based on the comments, we have made the required modifications and attached revisions for final review. A response to each comment has been provided below.

1. Administrative and Subdivision Review Codes (HR 276 & HR 289)

a. HR 276-11.1.B.(13). The applicant has not shown any signs other than traffic signs on the plan set.

No additional signage is proposed as part of the current application.

b. HR 276-11.1.B.(14). The applicant has not shown any lighting on the plan set. The applicant should confirm if any lighting is proposed and provide locations and details, or add the required note if lighting is not proposed.

No lighting is proposed as part of the current application. Note #23 on Sheet 1 has been added accordingly.

c. HR 276-11.1.B.(15). The applicant has not shown any buildings within 50 feet of the site.

The only building within 50 feet of the site is an existing single-family house on Map 152 Lot 13-1, which is now shown throughout the plan set.

d. HR 276-11.1.B.(16). The applicant has not shown driveways and parking areas within 200 feet of the site on the plan set.

A GIS image showing existing features within 200 feet of the site has been added to the cover sheet to satisfy the requirement.

Civil Engineering

Land Surveying

e. HR 276-11.1.B.(20). The applicant has not shown any existing buildings or features on the existing site. The applicant should confirm that the site was not previously developed in any way.

The site has not been previously developed save for the existing power line easement.

f. HR 289-15. The applicant has noted that the site is not located within a Flood Hazard area.

No comment.

g. HR 289-22. The applicant shall coordinate with the Town of Hudson regarding the deeding of open space areas.

The applicant has noted this information and will continue to work with town staff during the subdivision approval process.

h. HR 289-28.A. The applicant should provide a detail for the proposed granite bounds to be set.

Note #18 on Sheet 1 provides details regarding monumentation to be set.

i. HR 289-37.A. The applicant has not provided any information on phasing of subdivision construction on the plan set. We note that subdivisions with between six and sixteen lots must be developed over two years per the Regulation.

The applicant is requesting a waiver for the phasing regulations (see attached).

2. Driveway Review Codes (HR 193-10)

a. HR 193-10. The applicant has shown proposed approximate driveway locations within the Right-of-Way on the plan set, but has not shown any of these further extended into the lots. Due to the extensive grading adjacent to the roadway some driveways may be difficult to construct to meet the Town's maximum slope requirements.

The plans have been revised to include potential driveway locations and their associated profiles showing proof of concept for each lot. Additionally, Note #6 on Sheet 10 has been modified to include similar information regarding conceptual driveway locations and potential lot development.

b. HR 193-10. Several proposed driveway locations are located at property lines (lot 6) or configured in such a way that the driveway may encroach on an adjacent lot (lot 8). Also there doesn't appear to be a proposed driveway location for lot 9.

Potential driveway locations have been updated and expanded for each lot. A driveway location has been added for Lot 9.

Civil Engineering

Land Surveying

c. HR 193-10.E. The applicant has provided sight distance information for the proposed roadway on the plan set. The sight distance looking left (west) from Windsor Lane may be obscured by brush if not maintained and also could be significantly impacted by snowbanks during winter periods. The applicant should clarify how this area is be maintained to provide sufficient sight distance in this direction.

A sight distance easement has been provided over Lot 59-1 to ensure that sight distance can be adequately maintained.

3. Roadway Design

a. HR 289-18.B.(5). The applicant should add dead-end sign location to the plan set. Sign details have been provided.

A dead-end sign location has been provided on Sheet 8.

b. The applicant should label the curb curve radii on the plan set.

Curb curve radii are now labeled on Sheets 8 and 9.

c. HR 289-11.1.(21). The applicant has not shown a detectable warning paver at the sidewalk termination at the cul-de-sac.

A detectable warning plate has been added as requested (see Sheet 9).

- 4. Drainage Design /Stormwater Management (HR 289-20.C./Chapter 290)
 - a. HR 289-20.C.(1). The applicant should confirm that all off-site improvements were coordinated with the Town Engineer, included the runoff leaving the site via the rip-rap lined swale along Barretts Hill Road.

The design of all proposed off-site improvements shown on Sheet 10 has been coordinated with the Town Engineer accordingly (see attached email correspondence).

b. HR 290-5.B.2.b. The applicant should provide language in the Stormwater Management Report, stating if and how low impact development (LID) strategies for stormwater runoff were evaluated for this project.

The stormwater design minimizes the volume of stormwater discharged from the site. As such, appropriate language regarding LID is now included under the "Post-Development Drainage Conditions" within the Stormwater Management Report narrative (see attached).

c. HR 290-6.A.8. We note the requirement of the applicant to coordinate a pre-construction meeting with the Town Engineer.

The applicant has noted this information.

Civil Engineering

Land Surveying

d. HR 290.6.A.12. The applicant should provide a note for winter stabilization on the erosion control plans.

"Winter Construction Notes" have been added to Sheet 12 as requested.

e. HR 290-8.A.4 & 5. We note the requirement of the applicant to coordinate the need for a Bond and/or Escrow with the Town Engineer.

The applicant has noted this information.

f. In section D. Summary of the project narrative in the Stormwater Management Report the applicant references the Town of Goffstown Stormwater Regulations. This should be revised to the Town of Hudson.

This error has been revised accordingly.

g. The applicant should clarify how much of the existing 12" CMP is to be replaced at the Barretts Hill Road crossing near pole PSNH 21B/30.

The existing 12" CMP that crosses under Barretts Hill Road is not proposed to be replaced. However, the existing 12" CMP, which outlets from the existing catch basin on the south side of the road, will be upgraded to a 15" HDPE as shown on Sheet 11.

h. The applicant should clarify if driveway aprons are being replaced as part of the off-site improvements for the drainage swale installation, and if so show this on the plans.

Driveway aprons are being replaced as part of the drainage improvements. This is now noted on Sheet 11.

i. The applicant should forward any review comments received from the NHDES AoT Bureau to the Town for their review and records.

AoT review comments were forwarded to the town directly from NHDES.

j. The applicant should confirm that the proposed pocket pond is not considered a dam per NHDES definitions.

The proposed pocket pond is not considered a dam as it impounds less than 0.50 acrefeet of water during normal conditions (0.24 acre-feet), has a maximum storage of less than 6 acre-feet (2.91 acre-feet) and is less than 10 feet in height.

k. The applicant should coordinate with the Town regarding the need for fencing or other means to prevent children, animals, etc. entering the water in the proposed pocket pond.

Review letters have been received from the Zoning Administrator, Fire Marshall, Department of Public Works, and Town Engineer. Pond fencing was not an area of

Civil Engineering

Land Surveying

concern for any of these departments, however if the planning board determines that fencing is preferred the applicant will accommodate the request,

1. ETGTD 930.1. The applicant should review the depth of the drainage within the roadway, it does not maintain the Town minimum depth of 4' of cover for all pipes.

Drainage pipes within the roadway have been lowered to accommodate the required four feet of cover.

m. ETGTD 930.4. The applicant shall review the slope of the proposed drainage system with the Town Engineer. Slopes within the proposed Right-of-Way are less than the minimum 2% required.

All drainage pipes within the proposed Right-of-Way have been revised to meet the minimum slope requirement of 2%. The drainage calculations have been updated accordingly to match the revised design (see attached).

n. ETGTD 930.10. We note the requirement for curb inlet drainage structures at all vertical sags. CB19 and CB20 are designed at a vertical sag.

The town engineer was consulted and has accepted the use of double catch basins in lieu of curb inlets with the stipulation that the sidewalk ramp location be shifted to not interfere with the grate location (see attached email correspondence).

o. ETGTD 930.11. CB18 and CB20 are shown as double catch basins on the plans. The applicant should review this design with the Town Engineer for approval.

The town engineer was consulted and has accepted the use of double catch basins in lieu of curb inlets with the stipulation that the sidewalk ramp location be shifted to not interfere with the grate location (see attached email correspondence).

p. The applicant will be required to comply with all provisions of the Town of Hudson's MS4 permit, including but not limited to annual reporting requirements, construction site stormwater runoff control, and record keeping requirements.

The applicant has noted this information. Additionally, Note #21 on Sheet 1 states, "This project meets the 2019 MS4 requirements."

q. Please note that this review was carried out in accordance with applicable regulations and standards in place in New Hampshire at this time. Note that conditions at the site, including average weather conditions, patterns and trends, and design storm characteristics, may change in the future. In addition, future changes in federal, state or local laws, rules or regulations, or in generally accepted scientific or industry information concerning environmental, atmospheric and geotechnical conditions and developments may affect the information and conclusions set forth in this review. In no way shall Fuss & O'Neill be liable for any of these changed conditions that may impact the review,

Civil Engineering

Land Surveying

regardless of the source of or reason for such changed conditions. Other than as described herein, no other investigation or analysis has been requested by the Client or performed by Fuss & O'Neill in preparing this review.

No comment.

5. Zoning (HR 334)

a. HR 334-14 and HR 276-11.1.B.(20). The applicant has not noted the maximum proposed building heights on the plan set. The applicant should note the maximum building height of 38 feet on the plan set.

Note #24 on Sheet 1 has been added as requested.

b. HR 334-20. The site is located in the General (G-1) District. The applicant has noted that open space development is the proposed use. The applicant should confirm if the lots are to be single family homes.

It is the intent of the applicant to construct individual condex units on each lot. Note #25 on Sheet 1 has been added to this effect.

c. HR 334-27. We note that the conventional subdivision design appears to meet the lot size and frontage requirements for the district.

No comment.

d. HR 334-35. The applicant has noted that there are no wetlands located on the site.

No comment.

e. HR 334-50. The applicant has shown that 13 lots could be accommodated as part of the Conventional Subdivision plan. The applicant has proposed 13 lots reduced the minimum lot areas to 1.0 acre and the frontage to a minimum of 100 feet for each lot.

No comment.

6. Sewer/Water Design/Conflicts & Utility Design/Conflicts (HR 276-13.E.)

a. The applicant has not shown any provisions for fire protection (cisterns, etc.). The applicant should coordinate with the Hudson Fire Department to ensure adequate fire protection is provided for the site.

A 30,000 gallon cistern and associated easement are now proposed on Lot 59-2 in accordance with the Fire Department's recommendations. Construction details for the cistern can be found on Sheet 18.

Civil Engineering

Land Surveying

b. The applicant has shown proposed 4k septic reserve areas and 75-foot protective well radii on the plans for the individual lots. Proposed well radii do not extend onto adjacent lots. They have also provided a typical 4-bedroom residential single family home detail, which shows a water service connection in the rear of the building and a septic tank in the front.

The intent is to develop each lot with a condex unit, which will have six total bedrooms for a design flow of 900 GPD per unit. According to Table 1008-4 from Env-Wq, this requires well radii to be increased to 100 feet. The plan has been revised accordingly.

7. Erosion Control/Wetland Impacts

a. The applicant has noted that the Town reserves the right to require additional erosion control measures should they be found necessary.

No comment.

8. State and Local Permits

a. The applicant has noted the required permits on the plan set.

No comment.

b. HR 290-10.B. The applicant has noted the need for a NPDES Notice of Intent and a SWPPP on the plan.

No comment.

c. Additional local permitting may be required.

No comment.

9. Other

a. ETGTD Section 565.1.1. The applicant is reminded that the Town of Hudson has specific requirements for the importing of off-site fill materials for use in constructing this project. We recommended that these requirements be stated on the plans for the Contractors attention.

"Excavation and Embankment Notes" have been added to Sheet 8 as requested.

If you have any questions or comments, please reach out by phone at (603) 627-2881 or by email at pmadsen@keachnordstrom.com.

Civil Engineering

Land Surveying

Respectfully,

Peter Madsen, EIT

Keach Nordstrom Associates, Inc. 10 Commerce Park North, Suite 3

Bedford, NH 03110

Peter Madsen

From: Dhima, Elvis <edhima@hudsonnh.gov>

Sent: Tuesday, February 13, 2024 10:28 AM

To: Peter Madsen

Cc: John Gargasz; Anthony Basso; Twardosky, Jason; Paul Chisholm; Kirkland, Donald

Subject: RE: Barrett Hill Subdivision Offsite Drainage

Caution: This email originated from outside of the organization. Do not click links or open attachments you do not recognize!

Thanks Peter

Ε

Elvis Dhima, P.E. Town Engineer

12 School Street Hudson, NH 03051 Phone: (603) 886-6008 Mobile: (603) 318-8286



From: Peter Madsen <pmadsen@keachnordstrom.com>

Sent: Tuesday, February 13, 2024 10:28 AM **To:** Dhima, Elvis <edhima@hudsonnh.gov>

Cc: John Gargasz < johngargasz@gmail.com >; Anthony Basso <a basso@keachnordstrom.com >; Twardosky, Jason

<jtwardosky@hudsonnh.gov>; Paul Chisholm <pchisholm@keachnordstrom.com>; Kirkland, Donald

<dkirkland@hudsonnh.gov>

Subject: RE: Barrett Hill Subdivision Offsite Drainage

EXTERNAL: Do not open attachments or click links unless you recognize and trust the sender.

Elvis,

Thanks for the review. The 10-year storm event was used for pipe sizing.

Peter Madsen

Project Engineer
Keach-Nordstrom Associates, Inc.
10 Commerce Park North, Ste 3
Bedford, NH 03110
t. (603)-627-2881 | f. (603)-627-2915

pmadsen@keachnordstrom.com www.keachnordstrom.com

From: Dhima, Elvis <edhima@hudsonnh.gov>
Sent: Tuesday, February 13, 2024 10:19 AM

To: Peter Madsen <pmadsen@keachnordstrom.com>

Cc: John Gargasz < johngargasz@gmail.com >; Anthony Basso < abasso@keachnordstrom.com >; Twardosky, Jason

<itwardosky@hudsonnh.gov>; Paul Chisholm <pchisholm@keachnordstrom.com>; Kirkland, Donald

<dkirkland@hudsonnh.gov>

Subject: RE: Barrett Hill Subdivision Offsite Drainage

Caution: This email originated from outside of the organization. Do not click links or open attachments you do not recognize!

Peter

This looks good

What storm event did you use for sizing the 15" pipe? This might come up at PB meeting as well

J, what say you?

Thank you

Ε

Elvis Dhima, P.E. Town Engineer

12 School Street Hudson, NH 03051 Phone: (603) 886-6008 Mobile: (603) 318-8286



From: Peter Madsen <pmadsen@keachnordstrom.com>

Sent: Monday, February 12, 2024 11:07 AM To: Dhima, Elvis <edhima@hudsonnh.gov>

Cc: John Gargasz < johngargasz@gmail.com>; Anthony Basso <a basso@keachnordstrom.com>; Twardosky, Jason

<jtwardosky@hudsonnh.gov>; Paul Chisholm <pchisholm@keachnordstrom.com>

Subject: Barrett Hill Subdivision Offsite Drainage

EXTERNAL: Do not open attachments or click links unless you recognize and trust the sender.

Good morning Elvis,

Please see the attached plan for the proposed drainage improvements along Barrett Hill Road. We are proposing a closed drainage system with drop inlets along the south shoulder of the road to provide an upgrade to the town's drainage while also addressing the outfall from our pond.

A revised plan set with the review engineer and AoT's comments is coming soon, however I wanted to get your opinion on the offsite work prior to submitting that.

Thanks

Peter Madsen

Project Engineer
Keach-Nordstrom Associates, Inc.
10 Commerce Park North, Ste 3
Bedford, NH 03110
t. (603)-627-2881 | f. (603)-627-2915
pmadsen@keachnordstrom.com www.keachnordstrom.com

Peter Madsen

From:

Dhima, Elvis <edhima@hudsonnh.gov>

Sent:

Wednesday, January 24, 2024 2:39 PM

To:

Peter Madsen

Cc:

Paul Chisholm; Twardosky, Jason

Subject:

RE: Barrett Hill Subdivision Granite Curb Inlets

Attachments:

Revisions.pdf

Caution: This email originated from outside of the organization. Do not click links or open attachments you do not recognize!

Peter

Double CB is fine, no issues signing off on the waiver

See attachment and make the revisions on your next submittal

Thanks

F

Elvis Dhima, P.E. Town Engineer

12 School Street Hudson, NH 03051 Phone: (603) 886-6008

Mobile: (603) 318-8286



From: Peter Madsen <pmadsen@keachnordstrom.com>

Sent: Tuesday, January 23, 2024 4:31 PM To: Dhima, Elvis <edhima@hudsonnh.gov>

Cc: Paul Chisholm <pchisholm@keachnordstrom.com>
Subject: Barrett Hill Subdivision Granite Curb Inlets

EXTERNAL: Do not open attachments or click links unless you recognize and trust the sender.

Good afternoon Elvis,

I am currently working through the Fuss & O'Neill review letter for the Barrett Hill Subdivision and would like to get your opinion on the following comment:

n. ETGTD 930.10. We note the requirement for curb inlet drain sags. CB19 and CB20 are designed at a vertical sag.

CB#19 and #20 need to be double catch basins for grate capacity as they take a lot of flow from the hill and cul-de-sac (see PDF). Are you comfortable waiving the curb inlet requirement here or is there a different way to handle this that's been previously used in town?

Appreciate any input.

Thanks

Peter Madsen

Project Engineer
Keach-Nordstrom Associates, Inc.
10 Commerce Park North, Ste 3
Bedford, NH 03110
t. (603)-627-2881 | f. (603)-627-2915
pmadsen@keachnordstrom.com www.keachnordstrom.com

INTRODUCTION

A. Project Description

The project proposes to develop the existing property to accommodate a twelve (13) lot open space residential subdivision with an accompanying 1,200 linear foot public roadway. Site work includes clearing and grubbing the site, construction of the new roadway, individual lot development, and installation of a closed drainage system, pocket pond, and other associated stormwater management provisions.

B. Existing Site Conditions

The subject property consists of one lot, approximately 35.36 acres in total area, and is located at 75 Barretts Hill Road in Hudson's General 1 (G-1) Zoning District. The lot is currently undeveloped. The lot is comprised mostly of woodlands with a utility line easement on the back half of the lot. The subject lot is bordered by Barretts Hill Road to the north and residential lots to the east, west, and south.

A site-specific soil survey, performed onsite by GZA GeoEnvironmental, Inc. on October 4, 2023, listed the following soils as predominant soil types found onsite. Montauk classified as C soils with slopes ranging from 3-25%, Scituate classified as C soils with slopes ranging from 3-25%. The soil types are classified as HSG 'C' and therefore, C soils were used in drainage computations. According to the National Resources Conservation Service (NRCS) soil mapping, the remaining subcatchment areas consist of, Canton fine loamy sand with slopes ranging from 0-15%, Canton fine loamy sand, very stony with slopes ranging from 8-15%, Scarboro stony mucky loamy sand, Scituate fine sandy loam with slopes ranging from 3-8%, and Scituate stony fine sandy loam with slopes ranging from 3-8%. It is important to note that both Scituate and Montauk soils have infiltration rates below 0.50 in/hr, therefore, according to Env-Wq 1508.07(a)(1), infiltration is not permitted on this site.

II. STORM DRAINAGE ANALYSIS & DESIGN

A. Methodology

In accordance with the provisions of the NHDES, the Town of Hudson, and generally accepted engineering practice, the 2-year, 2-year frozen, 10-year, 25-year, and 50-year frequency storms have each been used in the various aspects of analysis and design of stormwater management considerations for the subject site. The closed drainage system has been designed for the 10-year frequency storm and the proposed pocket pond has been designed to not overtop in the 50-year frequency storm.

KNA utilizes HydroCAD version 10.0 to analyze both pre and post-development watershed characteristics. This computer software system is based largely on hydrology techniques (TR-20) developed by the Soil Conservation Service (now the Natural Resources Conservation Service). In addition, the software derives Time of Concentration values using the methodology contained within USDA-S.C.S. publication <u>Urban Hydrology for Small Watersheds Technical Release No. 55 (TR 55).</u>

All design and analysis calculations performed using the referenced methodologies are attached to this report. The minimum time of concentrations used for the analysis is 6 minutes. These calculations document each catchment area, a breakdown of surface type, time of concentration, rainfall intensity, peak discharge volume, Manning's "n" value, peak

velocity, and other descriptive design data for each watershed and pipe segment evaluated. In addition, the "Pre/Post Development Drainage Area Plans" graphically define and illustrate the extent of each watershed or catchment area investigated.

B. Pre-Development Drainage Conditions

In the pre-development scenario, three (3) points of analysis (POA) have been identified as the appropriate points to compare pre vs. post development rates of stormwater discharge. These points of analysis reflect the main discharge points of the site and were analyzed to show the impact from the proposed improvements.

The pre-development drainage model's POA's are further described as follows:

Link A Barretts Hill CrossingLink B Lot 57

Link C Lot 11

The property mainly slopes from southeast to northwest directing runoff to Barretts Hill Road and neighboring lots on the western side of the subject property. Runoff on the northeastern side of the property flows to Barretts Hill Road and is collected by an existing catch basin in the roadway before discharging to Lot 11. Runoff generated on the northwestern portion of the subject property flows to a different existing catch basin on Barretts Hill Road which discharges to Lot 57. Runoff generated from the western portion of the property ultimately flows to an existing drop inlet located at the bottom of Barretts Hill Road. This crossing is identified as Link A and directs runoff under the road towards an existing stream.

C. Post-Development Drainage Conditions:

The same three POA's that were identified in the pre-development scenario have been analyzed in the post-development scenario.

Overall, the design has maintained the drainage patterns to mimic the pre-development conditions. Stormwater will continue to discharge to the same points of analysis identified in the pre-development scenario. The improvements, however, also provide stormwater treatment for the new impervious areas created for the proposed development. These new impervious areas include the roadway, sidewalks, potential future driveways, and a 2,400 sf approximation for residential homes dedicated to each lot.

The proposed stormwater management system utilizes both open and closed drainage practices for the collection, detention, and treatment of runoff. Runoff generated from the entirety of the proposed roadway and the eastern portion of the property is collected by a series of deep sump catch basins and is piped to a proposed pocket pond situated in the northwest corner of the lot. This system is designed to provide stormwater treatment and detention for the site and will not overtop during a 50-year storm event. It is also designed to minimize the volume of stormwater runoff discharged from the site by reducing the volume of stormwater runoff in all modeled storm events, therefore satisfying the town Low-Impact Development requirement. The pond features an overflow structure, which directs outfall into a closed drainage system, that ultimately outlets to the existing drop inlet at the bottom of the hill (Link A). As previously stated, infiltration is not permitted on this site and has not been included in the drainage analysis. Lastly, to adequately account for proposed lot development, conceptual condex locations, driveway locations, and associated site grading are represented on the project plans. The final design and layout of each lot is subject to change however, the overall grading and associated drainage patterns will remain similar to what has been depicted.

The peak stormwater runoff rates and volume discharges for the specific storm frequencies are presented and analyzed in the subsequent summary section of this report (Tables 1 & 2). For a more visual description of the information presented in this section, please refer to the attached "Post-Development Drainage Areas Plan" attached in the appendix of this report.

D. Summary:

The subject site complies with both the Town of Hudson Stormwater Management and Erosion Control Regulations and NHDES Regulations Env-wq 1500 in regard to stormwater treatment and groundwater recharge volume. Proposed stormwater best management practices (BMP) are designed in accordance with the New Hampshire Stormwater Manual Volume 2: Post-Construction Best Management Practices Selection and Design and BMP worksheets provided by the New Hampshire Department of Environmental Services. In addition, stormwater discharges, in terms of peak rate of runoff and total volume, are consistent with the Town of Hudson Stormwater Regulations and NHDES Regulations Env-Wq 1500. The results are reported below in Table 1 and 2.

Table 1: Peak Flow Discharge Rate

Site Pre-Development vs. Post-Development (cfs)											
Description 2-Year			2-Year Frozen		10-Year		25-Year		50-Year		
24-hr Rainfall	2.95 in/hr		2.95 in/hr		4.47 in/hr		5.66 in/hr		6.77 in/hr		
	Pre	Post	Pre	Post	Pre	Post	Pre	Post	Pre	Post	
Α	7.79	5.50	51.93	41.64	25.69	19.22	43.08	32.74	60.85	46.85	
В	2.63	1.27	10.22	3.58	6.88	2.92	10.73	4.35	14.53	5.75	
С	2.29	1.37	8.99	4.76	6.03	3.43	9.42	5.26	12.77	7.07	

Table 2: Volume Comparison

2.53 M N B	Site Pre-Development vs. Post-Development (Storm Volume in Acre-Feet)										
Description	2-Y	'ear	2-Year	Frozen	10-`	Year	25-`	Year	50-	Year	Comments
24-hr Rainfall	2.95 in/hr		2.95 in/hr		4.47 in/hr		5.66 in/hr		6.77 in/hr		
	Pre	Post	Pre	Post	Pre	Post	Pre	Post	Pre	Post	
A	1.43	1.28	7.99	7.34	3.86	3.57	6.19	5.73	8.59	8.58	Complies with NHDES 1507.05, (b), (1), a
В	0.34	0.14	1.29	0.41	0.81	0.31	1.23	0.46	1.66	0.60	Complies with NHDES 1507.05, (b), (1), a & b
С	0.28	0.16	1.06	0.57	0.66	0.38	1.01	0.57	1.36	0.76	Complies with NHDES 1507.05, (b), (1), a & b

III. EROSION & SEDIMENTATION CONTROL PROVISIONS

A. Temporary Erosion Control Measures

As an integral part of the engineering design of this site, an erosion and sedimentation control plan has been developed with the intent of limiting the potential for soil loss and associated receiving water quality degradation, both during and after the construction period. As the project plans indicate, traditional temporary erosion and sedimentation control devices and practices, such as siltation fencing, erosion control blankets, and seeding have been specified for use during the construction period. In preparation of these provisions,

reference was made to the New Hampshire Stormwater Manual; Volume 3: Erosion and Sediment Temporary Controls During Construction. Construction details for each temporary erosion control measure and practice specified have been added to the project plans. These plans also contain a number of erosion control notes, which are offered to the selected contractor in order to supplement the specified measures and practices to the extent practical.

B. Construction Sequence

A site-specific construction sequence sensitive to limiting soil loss due to erosion and associated water quality degradation was prepared specifically for this project and is shown on the project plans. As pointed out in the erosion control notes, it is important for the contractor to recognize that proper judgment in the implementation of work will be essential if erosion is to be limited and protection of completed work is to be realized. Moreover, any specific changes in sequence and/or field conditions affecting the ability of specific erosion control measures to adequately serve their intended purpose should be reported to this office by the contractor. Further, the contractor is encouraged to supplement specified erosion control measures during the construction period where and when in his/ her best judgment additional protection is warranted.

C. Permanent Erosion Control Measures

In the design of this site, consideration was given to limiting the potential for long-term erosion of completed improvements. As a result, several permanent erosion control measures were incorporated into the site design. These provisions include:

- 1) Specification of a turf establishment schedule and seed mixture, utilizing materials and workmanship recognized as appropriate for the site conditions at hand; and
- 2) Construction of rip-rap at the outlet of the stormwater management areas; and
- 3) Design of a pocket pond to reduce runoff and volume.

2304141-POST DEVELOPMENT REV1

Type III 24-hr 10-YEAR Rainfall=4.47"

Prepared by Keach-Nordstrom Associates, Inc.

Revised March 4, 2024 Printed 3/4/2024

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 1

Time span=0.00-24.00 hrs, dt=0.01 hrs, 2401 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Redon redding by byn etc	The motion of totaling by by the me method
Subcatchment 1S: SUBCATCHMENT	Runoff Area=909,943 sf 3.54% Impervious Runoff Depth>1.24" Flow Length=2,172' Tc=37.8 min CN=64.1 Runoff=14.23 cfs 2.16 af
Subcatchment 2S: SUBCATCHMENT	Runoff Area=113,081 sf 12.76% Impervious Runoff Depth>1.08" Flow Length=683' Tc=20.1 min CN=61.4 Runoff=1.94 cfs 0.23 af
Subcatchment 3S: SUBCATCHMENT	Runoff Area=122,736 sf 7.22% Impervious Runoff Depth>1.08" Flow Length=1,189' Tc=23.1 min CN=61.4 Runoff=1.99 cfs 0.25 af
Subcatchment 4S: SUBCATCHMENT	Runoff Area=59,092 sf 16.60% Impervious Runoff Depth>1.43" Flow Length=758' Tc=16.6 min CN=66.9 Runoff=1.55 cfs 0.16 af
Subcatchment 5S: SUBCATCHMENT	Runoff Area=16,441 sf 34.30% Impervious Runoff Depth>1.92" Flow Length=389' Tc=8.7 min CN=73.7 Runoff=0.77 cfs 0.06 af
Subcatchment 6S: SUBCATCHMENT	Runoff Area=79,911 sf 11.18% Impervious Runoff Depth>2.03" Flow Length=667' Tc=20.0 min CN=75.2 Runoff=2.92 cfs 0.31 af
Subcatchment 7S: SUBCATCHMENT	Runoff Area=109,195 sf 5.17% Impervious Runoff Depth>1.81" Flow Length=1,100' Tc=21.3 min CN=72.2 Runoff=3.43 cfs 0.38 af
Subcatchment 8S: SUBCATCHMENT	Runoff Area=117,754 sf 20.59% Impervious Runoff Depth>2.19" Flow Length=766' Tc=20.1 min CN=77.1 Runoff=4.66 cfs 0.49 af
Subcatchment 9S: SUBCATCHMENT	Runoff Area=35,299 sf 21.66% Impervious Runoff Depth>2.20" Flow Length=558' Tc=22.9 min CN=77.3 Runoff=1.34 cfs 0.15 af
Subcatchment 10S: SUBCATCHMENT	Runoff Area=70,637 sf 8.47% Impervious Runoff Depth>1.89" Flow Length=765' Tc=22.8 min CN=73.3 Runoff=2.26 cfs 0.26 af
Subcatchment 11S: SUBCATCHMENT	Runoff Area=9,635 sf 44.10% Impervious Runoff Depth>2.84" Tc=6.0 min CN=84.6 Runoff=0.73 cfs 0.05 af
Subcatchment 12S: SUBCATCHMENT	Runoff Area=143,748 sf 5.17% Impervious Runoff Depth>1.80" Flow Length=812' Tc=17.8 min CN=72.1 Runoff=4.83 cfs 0.49 af
Subcatchment 13S: SUBCATCHMENT	Runoff Area=49,697 sf 12.99% Impervious Runoff Depth>2.05" Flow Length=637' Tc=15.8 min CN=75.3 Runoff=2.01 cfs 0.19 af
Subcatchment 14S: SUBCATCHMENT	Runoff Area=16,268 sf 33.80% Impervious Runoff Depth>2.57" Flow Length=135' Tc=14.0 min CN=81.6 Runoff=0.88 cfs 0.08 af
Subcatchment 15S: SUBCATCHMENT	Runoff Area=8,966 sf 35.77% Impervious Runoff Depth>2.66" Tc=6.0 min CN=82.6 Runoff=0.64 cfs 0.05 af
Subcatchment 16S: SUBCATCHMENT	Runoff Area=14,599 sf 43.85% Impervious Runoff Depth>2.83" Tc=6.0 min CN=84.5 Runoff=1.11 cfs 0.08 af

2304141-POST DEVELOPMENT_REV1 Prepared by Keach-Nordstrom Associates, Inc. HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC Type III 24-hr 10-YEAR Rainfall=4.47" Revised March 4, 2024 Printed 3/4/2024 Page 2
Subcatchment 17S: SUBCATCHMENT Runoff Area=75,110 sf 13.37% Impervious Runoff Depth>2.24" Flow Length=133' Tc=11.2 min CN=77.7 Runoff=3.80 cfs 0.32 af
Subcatchment 18S: SUBCATCHMENT Runoff Area=8,136 sf 46.52% Impervious Runoff Depth>2.89" Tc=6.0 min CN=85.2 Runoff=0.63 cfs 0.05 af
Subcatchment 19S: SUBCATCHMENT Runoff Area=7,842 sf 46.45% Impervious Runoff Depth>2.89" Tc=6.0 min CN=85.1 Runoff=0.61 cfs 0.04 af
Subcatchment 20S: SUBCATCHMENT Runoff Area=1,396 sf 43.41% Impervious Runoff Depth>2.82" Tc=6.0 min CN=84.4 Runoff=0.11 cfs 0.01 af
Subcatchment 21S: SUBCATCHMENT Runoff Area=13,129 sf 15.63% Impervious Runoff Depth>2.06" Flow Length=143' Tc=10.8 min CN=75.5 Runoff=0.62 cfs 0.05 af
Subcatchment 22S: SUBCATCHMENT Runoff Area=3,372 sf 55.90% Impervious Runoff Depth>2.58" Tc=6.0 min CN=81.7 Runoff=0.23 cfs 0.02 af
Subcatchment 23S: SUBCATCHMENT Runoff Area=3,695 sf 57.02% Impervious Runoff Depth>2.62" Tc=6.0 min CN=82.1 Runoff=0.26 cfs 0.02 af
Subcatchment 24S: SUBCATCHMENT Runoff Area=4,421 sf 60.03% Impervious Runoff Depth>2.71" Tc=6.0 min CN=83.2 Runoff=0.32 cfs 0.02 af
Subcatchment 25S: SUBCATCHMENT Runoff Area=2,246 sf 0.00% Impervious Runoff Depth>1.01" Tc=6.0 min CN=60.1 Runoff=0.05 cfs 0.00 af
Reach 1R: EXISTING ROADWAY DITCH
Pond 1P: EXISTING CB Inflow=14.23 cfs 2.16 af Primary=14.23 cfs 2.16 af
Pond 2P: EXISTING DRIVEWAY CULVERT Peak Elev=293.20' Inflow=1.94 cfs 0.23 af 12.0" Round Culvert n=0.013 L=21.0' S=0.0143 '/' Outflow=1.94 cfs 0.23 af
Pond 3P: EXISTING DROP INLET Peak Elev=291.27' Inflow=19.22 cfs 3.57 af 24.0" Round Culvert n=0.011 L=92.1' S=0.0433 '/' Outflow=19.22 cfs 3.57 af
Pond 4P: EXISTING CB Peak Elev=297.98' Inflow=3.46 cfs 0.92 af Primary=3.46 cfs 0.92 af Secondary=0.00 cfs 0.00 af Outflow=3.46 cfs 0.92 af
Pond 5P: EXISTING CB Peak Elev=298.34' Inflow=0.77 cfs 0.06 af Primary=0.77 cfs 0.06 af Secondary=0.00 cfs 0.00 af Outflow=0.77 cfs 0.06 af
Pond 6P: EXISTING CB Peak Elev=364.80' Inflow=2.92 cfs 0.31 af 12.0" Round Culvert n=0.025 L=18.0' S=0.0689 '/' Outflow=2.92 cfs 0.31 af
Pond 7P: EXISTING CB Peak Elev=377.94' Inflow=3.43 cfs 0.38 af 12.0" Round Culvert n=0.011 L=31.9' S=0.0091 '/' Outflow=3.43 cfs 0.38 af

HydroCAD® 10.00-26 s/n 0	dstrom Associates, Inc. Revised March 4, 2024 Printed 3/4/2024 3273 © 2020 HydroCAD Software Solutions LLC Page 3
Pond 8P: CB#20	Peak Elev=350.58' Inflow=0.66 cfs 0.59 af 15.0" Round Culvert n=0.013 L=115.0' S=0.0800 '/' Outflow=0.66 cfs 0.59 af
Pond 9P: DMH#3	Peak Elev=376.48' Inflow=9.60 cfs 1.04 af 18.0" Round Culvert n=0.013 L=20.0' S=0.0075 '/' Outflow=9.60 cfs 1.04 af
Pond 10P: DMH#4	Peak Elev=378.18' Inflow=9.60 cfs 1.04 af 18.0" Round Culvert n=0.013 L=121.7' S=0.0075 '/' Outflow=9.60 cfs 1.04 af
Pond 11P: DMH#5	Peak Elev=380.76' Inflow=9.60 cfs 1.04 af 18.0" Round Culvert n=0.013 L=226.1' S=0.0075 '/' Outflow=9.60 cfs 1.04 af
Pond 12P: CB#6	Peak Elev=382.63' Inflow=9.60 cfs 1.04 af 18.0" Round Culvert n=0.013 L=141.2' S=0.0050 '/' Outflow=9.60 cfs 1.04 af
Pond 13P: CB#7	Peak Elev=383.80' Inflow=9.24 cfs 0.99 af 18.0" Round Culvert n=0.013 L=30.3' S=0.0201 '/' Outflow=9.24 cfs 0.99 af
Pond 14P: CB#8	Peak Elev=387.68' Inflow=8.39 cfs 0.91 af 15.0" Round Culvert n=0.013 L=165.7' S=0.0205 '/' Outflow=8.39 cfs 0.91 af
Pond 15P: CB#80	Peak Elev=387.69' Inflow=0.61 cfs 0.04 af 12.0" Round Culvert n=0.013 L=26.0' S=0.0200 '/' Outflow=0.61 cfs 0.04 af
Pond 16P: CB#10	Peak Elev=394.51' Inflow=6.06 cfs 0.67 af 12.0" Round Culvert n=0.013 L=165.5' S=0.0550 '/' Outflow=6.06 cfs 0.67 af
Pond 17P: CB#100	Peak Elev=394.53' Inflow=1.11 cfs 0.08 af 12.0" Round Culvert n=0.013 L=26.0' S=0.0200 '/' Outflow=1.11 cfs 0.08 af
Pond 18P: CB#11	Peak Elev=401.39' Inflow=1.36 cfs 0.10 af 12.0" Round Culvert n=0.013 L=167.8' S=0.0550 '/' Outflow=1.36 cfs 0.10 af
Pond 19P: CB#110	Peak Elev=402.03' Inflow=0.63 cfs 0.05 af 12.0" Round Culvert n=0.013 L=38.0' S=0.0200 '/' Outflow=0.63 cfs 0.05 af
Pond 20P: DMH#14	Peak Elev=376.89' Inflow=8.24 cfs 0.90 af 18.0" Round Culvert n=0.013 L=15.0' S=0.0800 '/' Outflow=8.24 cfs 0.90 af
Pond 21P: DMH#15	Peak Elev=383.15' Inflow=8.24 cfs 0.90 af 18.0" Round Culvert n=0.013 L=30.7' S=0.0801'/' Outflow=8.24 cfs 0.90 af
Pond 22P: CB#16	Peak Elev=399.31' Inflow=8.24 cfs 0.90 af 18.0" Round Culvert n=0.013 L=44.5' S=0.0800 '/' Outflow=8.24 cfs 0.90 af
Pond 23P: CB#17	Peak Elev=402.68' Inflow=8.20 cfs 0.90 af 18.0" Round Culvert n=0.013 L=164.2' S=0.0200 '/' Outflow=8.20 cfs 0.90 af
Pond 24P: CB#18	Peak Elev=403.69' Inflow=5.97 cfs 0.64 af 15.0" Round Culvert n=0.013 L=36.5' S=0.0200 '/' Outflow=5.97 cfs 0.64 af

2304141-POST DEVELOPMENT_REV1 Type III 24-hr 10-YEAR Rainfall=4.47"

Inflow=2.92 cfs 0.31 af

Inflow=3.43 cfs 0.38 af Primary=3.43 cfs 0.38 af

Primary=2.92 cfs 0.31 af

2304141-POST DEVELO	PMENT_REV1	Type III 24-hr 10-YEAR Rainfall=4.47	-11
Prepared by Keach-Nords	trom Associates, Inc.	Revised March 4, 2024 Printed 3/4/2024	4
	73 © 2020 HydroCAD Software So		
	2	W 18 1	_
Pond 25P: CB#19		Peak Elev=405.19' Inflow=4.66 cfs 0.49 a	ıf
	12.0" Round Culvert n=0.013	3 L=26.0' S=0.0200'/' Outflow=4.66 cfs 0.49 a	ıf
Pond 26P: DI#24		Peak Elev=299.85' Inflow=1.55 cfs 0.70 a	ıf.
	15.0" Round Culvert n=0.013	3 L=50.8' S=0.0419 '/' Outflow=1.55 cfs 0.70 a	ıf
Pond 27P: DI#23		Peak Elev=317.48' Inflow=1.24 cfs 0.68 a	ıf
	15.0" Round Culvert n=0.013	L=220.0' S=0.0800 '/' Outflow=1.24 cfs 0.68 a	
Pond 28P: DI#22		Peak Elev=331.13' Inflow=1.01 cfs 0.66 a	af
	15.0" Round Culvert n=0.013	L=181.4' S=0.0750 '/' Outflow=1.01 cfs 0.66 a	ıf
Pond 29P: DI#21		Peak Elev=340.68' Inflow=0.81 cfs 0.64 a	af
	15.0" Round Culvert n=0.013	L=118.9' S=0.0800 '/' Outflow=0.81 cfs 0.64 a	ıf
Pond PP: POCKET POND	Peak Elev=372.	.01' Storage=85,566 cf Inflow=20.55 cfs 2.26 a	ıf
		ndary=0.00 cfs 0.00 af Outflow=0.66 cfs 0.59 a	
Link A: BARRETTS HILL CF	ROSSING	Inflow=19.22 cfs 3.57 a	af
		Primary=19.22 cfs 3.57 a	

Link B: LOT 57

Link C: LOT 11

Total Runoff Area = 45.830 ac Runoff Volume = 5.93 af Average Runoff Depth = 1.55" 90.81% Pervious = 41.618 ac 9.19% Impervious = 4.212 ac

2304141-POST DEVELOPMENT REV1

Type III 24-hr 10-YEAR Rainfall=4.47"

Revised March 4, 2024 Printed 3/4/2024 Prepared by Keach-Nordstrom Associates, Inc. HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC Page 5

Summary for Subcatchment 1S: SUBCATCHMENT

Runoff 14.23 cfs @ 12.59 hrs, Volume= 2.16 af, Depth> 1.24"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 10-YEAR Rainfall=4.47"

	Α	rea (sf)	CN	Description	n	•					
		7,670	30.0	Woods, G	Good, HSG	A					
	3	63,085	55.0		Woods, Good, HSG B						
	3	77,077	70.0	Woods, Good, HSG C							
		6,684	39.0			Good, HSG A					
		61,618	61.0			Good, HSG B					
		61,554	74.0		ass cover, (Good, HSG C					
*		17,897	98.0	Roofs							
*		14,358	98.0	Paved pa							
		09,943	64.1	Weighted	_						
	8	77,688	62.9	96.46% Pervious Area							
		32,255	98.0	3.54% Impervious Area							
	Тс	Length	Slope	Velocity	Capacity	Description					
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)						
	17.3	100	0.0400	0.10		Sheet Flow,					
						Woods: Light underbrush n= 0.400 P2= 2.84"					
	19.4	1,831	0.0988	1.57		Shallow Concentrated Flow,					
						Woodland Kv= 5.0 fps					
	0.3	30	0.0800	1.98		Shallow Concentrated Flow,					
						Short Grass Pasture Kv= 7.0 fps					
	0.8	211	0.0430	4.21		Shallow Concentrated Flow,					
						Paved Kv= 20.3 fps					
	37.8	2.172	Total								

Summary for Subcatchment 2S: SUBCATCHMENT

Runoff 1.94 cfs @ 12.32 hrs, Volume= 0.23 af, Depth> 1.08"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 10-YEAR Rainfall=4.47"

	Area (sf)	CN	Description
	28,414	30.0	Woods, Good, HSG A
	11,383	55.0	Woods, Good, HSG B
	33,808	70.0	Woods, Good, HSG C
	4,719	39.0	>75% Grass cover, Good, HSG A
	242	61.0	>75% Grass cover, Good, HSG B
	20,087	74.0	>75% Grass cover, Good, HSG C
*	3,849	98.0	Roofs
*	10,579	98.0	Paved parking
	113,081	61.4	Weighted Average
	98,653	56.1	87.24% Pervious Area
	14,428	98.0	12.76% Impervious Area

Page 6

2304141-POST DEVELOPMENT REV1

Type III 24-hr 10-YEAR Rainfall=4.47"

Prepared by Keach-Nordstrom Associates, Inc. HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Revised March 4, 2024 Printed 3/4/2024

					•
Тс	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
12.0	100	0.1000	0.14		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 2.84"
3.9	318	0.0750	1.37		Shallow Concentrated Flow,

Woodland Kv= 5.0 fps 0.7 48 0.0300 Shallow Concentrated Flow, 1.21 Short Grass Pasture Kv= 7.0 fps **Shallow Concentrated Flow,** 3.6 217 0.0410 1.01 Woodland Kv= 5.0 fps 20.1 683 Total

Summary for Subcatchment 3S: SUBCATCHMENT

Runoff

1.99 cfs @ 12.37 hrs, Volume=

0.25 af, Depth> 1.08"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 10-YEAR Rainfall=4.47"

	Α	rea (sf)	CN	Description	n					
		5,732	30.0	Woods, G	Good, HSG	A				
		49,737	55.0	Woods, G	Good, HSG	В				
		30,592	70.0	Woods, G	Good, HSG	C				
		4,799	39.0	>75% Gra	ass cover, (Good, HSG A				
		20,628	61.0	>75% Gra	ass cover, (Good, HSG B				
		2,391	74.0	>75% Gra	ass cover, (Good, HSG C				
*		2,833	98.0	Roofs						
*		6,024	98.0	Paved par	rking					
	1	22,736	61.4	Weighted Average						
	113,879 58.6			92.78% Pervious Area						
		8,857	98.0	7.22% Im	pervious Ai	rea				
	Tc	Length	Slope	Velocity	Capacity	Description				
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
	9.9	100	0.1600	0.17		Sheet Flow,				
						Woods: Light underbrush n= 0.400 P2= 2.84"				
	13.2	1,089	0.0760	1.38		Shallow Concentrated Flow,				
						Woodland Kv= 5.0 fps				
	23.1	1,189	Total							

Summary for Subcatchment 4S: SUBCATCHMENT

Runoff

1.55 cfs @ 12.24 hrs, Volume=

0.16 af, Depth> 1.43"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 10-YEAR Rainfall=4.47"

2304141-POST DEVELOPMENT REV1

Type III 24-hr 10-YEAR Rainfall=4.47"

Prepared by Keach-Nordstrom Associates, Inc.

Revised March 4, 2024 Printed 3/4/2024

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 7

	Α	rea (sf)	CN	Description	n.	Notes and the second se							
		9,614	55.0	Woods, G	ood, HSG	В							
		3,279	70.0	Woods, G	Voods, Good, HSG C								
		35,548	61.0			Good, HSG B							
		841	74.0			Good, HSG C							
*		7,262	98.0	Paved par	king, HSG	B							
*		2,548	98.0	Roofs									
		59,092	66.9	Weighted	Average								
		49,282	60.7	83.40% P	ervious Are	ea							
		9,810	98.0	16.60% In	npervious A	Area							
	Tc	Length	Slope	Velocity	Capacity	Description							
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)								
	11.1	100	0.1200	0.15		Sheet Flow,							
						Woods: Light underbrush n= 0.400 P2= 2.84"							
	0.4	43	0.1400	1.87		Shallow Concentrated Flow,							
						Woodland Kv= 5.0 fps							
	1.3	145	0.0690	1.84		Shallow Concentrated Flow,							
						Short Grass Pasture Kv= 7.0 fps							
	0.1	14	0.0200	2.87		Shallow Concentrated Flow,							
						Paved Kv= 20.3 fps							
	1.9	230	0.0830	2.02		Shallow Concentrated Flow,							
						Short Grass Pasture Kv= 7.0 fps							
	0.2	63	0.0480	4.45		Shallow Concentrated Flow,							
						Paved Kv= 20.3 fps							
	0.9	103	0.0780	1.95		Shallow Concentrated Flow,							
						Short Grass Pasture Kv= 7.0 fps							
	0.7	60	0.0830	1.44		Shallow Concentrated Flow,							
_						Woodland Kv= 5.0 fps							
	16.6	758	Total										

Summary for Subcatchment 5S: SUBCATCHMENT

Runoff =

0.77 cfs @ 12.13 hrs, Volume=

0.06 af, Depth> 1.92"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 10-YEAR Rainfall=4.47"

	Area (sf)	CN	Description
	10,801	61.0	>75% Grass cover, Good, HSG B
*	5,640	98.0	Paved parking
	16,441	73.7	Weighted Average
	10,801	61.0	65.70% Pervious Area
	5,640	98.0	34.30% Impervious Area

2304141-POST DEVELOPMENT REV1

Type III 24-hr 10-YEAR Rainfall=4.47"

Prepared by Keach-Nordstrom Associates, Inc.

Revised March 4, 2024 Printed 3/4/2024

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 8

(n	Tc nin)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0	6.3	100	0.0700	0.26		Sheet Flow,
						Grass: Short n= 0.150 P2= 2.84"
	0.1	16	0.0400	4.06		Shallow Concentrated Flow,
						Paved Kv= 20.3 fps
	1.5	162	0.0680	1.83		Shallow Concentrated Flow,
						Short Grass Pasture Kv= 7.0 fps
	0.1	20	0.0400	4.06		Shallow Concentrated Flow,
						Paved Kv= 20.3 fps
	8.0	91	0.0770	1.94		Shallow Concentrated Flow,
						Short Grass Pasture Kv= 7.0 fps
//	8.7	389	Total			

Summary for Subcatchment 6S: SUBCATCHMENT

Runoff

2.92 cfs @ 12.28 hrs, Volume=

0.31 af, Depth> 2.03"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 10-YEAR Rainfall=4.47"

	Α	rea (sf)	CN	Description	n						
		C									
		38,986	74.0			Good, HSG C					
*		6,498	98.0	Paved par	aved parking						
*		2,434	98.0	Roofs	·						
		217	96.0	Gravel su	ravel surface, HSG C						
		79,911	75.2	Weighted	eighted Average						
		70,979	72.3	88.82% P	3.82% Pervious Area						
		8,932	98.0	11.18% In	.18% Impervious Area						
	Tc	Length	Slope	Velocity	Capacity	Description					
-	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)						
	8.3	65	0.0150	0.13		Sheet Flow,					
						Grass: Short n= 0.150 P2= 2.84"					
	8.6	35	0.0280	0.07		Sheet Flow,					
						Woods: Light underbrush n= 0.400 P2= 2.84"					
	0.5	38	0.0550	1.17		Shallow Concentrated Flow,					
						Woodland Kv= 5.0 fps					
	0.4	62	0.1300	2.52		Shallow Concentrated Flow,					
						Short Grass Pasture Kv= 7.0 fps					
	2.2	467	0.0300	3.52		Shallow Concentrated Flow,					
						Paved Kv= 20.3 fps					
	20.0	667	Total								

2304141-POST DEVELOPMENT REV1

Type III 24-hr 10-YEAR Rainfall=4.47"

Prepared by Keach-Nordstrom Associates, Inc.

Revised March 4, 2024 Printed 3/4/2024

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 9

Summary for Subcatchment 7S: SUBCATCHMENT

Runoff

3.43 cfs @ 12.30 hrs, Volume= 0.38 af, Depth> 1.81"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 10-YEAR Rainfall=4.47"

	Ar	ea (sf)	CN	Description	n	1, 9219					
*/	- 1	85,940	70.0	Woods, G	ood, HSG	С					
		16,846	74.0	>75% Gra	ass cover, (Good, HSG C					
*		5,055	98.0	Paved par	rking						
		764	96.0	Gravel su	rface, HSG	G C					
*		590	98.0	Roofs							
	10	09,195	72.2	Weighted	Weighted Average						
		03,550	70.8		94.83% Pervious Area						
		5,645	98.0	5.17% lm	5.17% Impervious Area						
•	Тс	Length	Slope	Velocity	Capacity	Description					
(mi	in)	(feet)	(ft/ft)	(ft/sec)	(cfs)						
12	2.5	100	0.0900	0.13		Sheet Flow,					
						Woods: Light underbrush n= 0.400 P2= 2.84"					
4	8.4	520	0.1290	1.80		Shallow Concentrated Flow,					
						Woodland Kv= 5.0 fps					
1	.3	125	0.0520	1.60		Shallow Concentrated Flow,					
						Short Grass Pasture Kv= 7.0 fps					
1	.7	127	0.0590	1.21		Shallow Concentrated Flow,					
						Woodland Kv= 5.0 fps					
0).1	13	0.3300	4.02		Shallow Concentrated Flow,					
						Short Grass Pasture Kv= 7.0 fps					
0	9.0	215	0.0375	3.93		Shallow Concentrated Flow,					
						Paved Kv= 20.3 fps					
21	.3	1.100	Total								

1,100 lotal

Summary for Subcatchment 8S: SUBCATCHMENT

Runoff

4.66 cfs @ 12.28 hrs, Volume=

0.49 af, Depth> 2.19"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 10-YEAR Rainfall=4.47"

ō	Area (sf)	CN	Description
	53,938	70.0	Woods, Good, HSG C
	39,569	74.0	>75% Grass cover, Good, HSG C
*	17,005	98.0	Paved parking
0	7,242	98.0	Roofs, HSG C
	117,754	77.1	Weighted Average
	93,507	71.7	79.41% Pervious Area
	24,247	98.0	20.59% Impervious Area

Page 10

2304141-POST DEVELOPMENT REV1

Type III 24-hr 10-YEAR Rainfall=4.47"

Prepared by Keach-Nordstrom Associates, Inc.

Revised M
HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Revised March 4, 2024 Printed 3/4/2024

Velocity Capacity Description Tc Length Slope (min) (feet) (ft/ft) (ft/sec) (cfs) 15.8 100 0.0500 0.11 Sheet Flow. Woods: Light underbrush n= 0.400 P2= 2.84" 1.8 2.25 Shallow Concentrated Flow, 246 0.2030 Woodland Kv= 5.0 fps 0.9 2.77 Shallow Concentrated Flow. 153 0.1570 Short Grass Pasture Kv= 7.0 fps Shallow Concentrated Flow. 0.0 5 0.0200 2.87 Paved Kv= 20.3 fps 0.1 5 0.0400 1.40 **Shallow Concentrated Flow.** Short Grass Pasture Kv= 7.0 fps Shallow Concentrated Flow. 1.5 257 0.0200 2.87 Paved Kv= 20.3 fps 766 Total 20.1

Summary for Subcatchment 9S: SUBCATCHMENT

Runoff = 1.34 cfs @ 12.32 hrs, Volume=

0.15 af, Depth> 2.20"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 10-YEAR Rainfall=4.47"

	A	rea (sf)	CN	Description	n							
	С											
		11,038	74.0	>75% Gra	5% Grass cover, Good, HSG C							
*		5,762	98.0	Paved pa	ved parking							
w		1,882	98.0	Roofs	oofs							
		35,299	77.3	Weighted	eighted Average							
		27,655	71.6	78.34% P	.34% Pervious Area							
		7,644	98.0	21.66% lr	npervious A	Area						
	Tc	Length	Slope	Velocity	Capacity	Description						
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)							
	19.4	100	0.0300	0.09		Sheet Flow,						
						Woods: Light underbrush n= 0.400 P2= 2.84"						
	2.5	350	0.2170	2.33		Shallow Concentrated Flow,						
						Woodland Kv= 5.0 fps						
	0.7	67	0.0520	1.60		Shallow Concentrated Flow,						
						Short Grass Pasture Kv= 7.0 fps						
	0.1	12	0.0200	2.87		Shallow Concentrated Flow,						
						Paved Kv= 20.3 fps						
	0.1	5	0.0400	1.40		Shallow Concentrated Flow,						
						Short Grass Pasture Kv= 7.0 fps						
	0.1	24	0.0200	2.87		Shallow Concentrated Flow,						
_						Paved Kv= 20.3 fps						
	22.9	558	Total									

2304141-POST DEVELOPMENT REV1

Type III 24-hr 10-YEAR Rainfall=4.47"

Prepared by Keach-Nordstrom Associates, Inc.

Revised March 4, 2024 Printed 3/4/2024

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 11

Summary for Subcatchment 10S: SUBCATCHMENT

Runoff = 2.26 cfs @ 12.33 hrs, Volume= 0.26 af, Depth> 1.89"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 10-YEAR Rainfall=4.47"

	P	rea (sf)	CN	Description	n							
		47,862	70.0	Woods, G	oods, Good, HSG C							
		16,792	74.0	>75% Gra	5% Grass cover, Good, HSG C							
*		3,480	98.0	Paved pa	ved parking							
*		2,503	98.0	Roofs	oofs							
		70,637	73.3	Weighted	Average							
		64,654	71.0	91.53% P	53% Pervious Area							
		5,983	98.0	8.47% Im	7% Impervious Area							
	Tc	Length	Slope	Velocity	Capacity	Description						
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)							
	17.3	100	0.0400	0.10		Sheet Flow,						
						Woods: Light underbrush n= 0.400 P2= 2.84"						
	3.1	390	0.1800	2.12		Shallow Concentrated Flow,						
						Woodland Kv= 5.0 fps						
	2.0	170	0.0410	1.42		Shallow Concentrated Flow,						
						Short Grass Pasture Kv= 7.0 fps						
	0.5	105	0.0290	3.46		Shallow Concentrated Flow,						
7						Paved Kv= 20.3 fps						
	22.8	765	Total									

Summary for Subcatchment 11S: SUBCATCHMENT

Runoff = 0.73 cfs @ 12.09 hrs, Volume=

0.05 af, Depth> 2.84"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 10-YEAR Rainfall=4.47"

<i>F</i>	Area (sf)	CN	Description	n							
	5,386	74.0	>75% Gra	5% Grass cover, Good, HSG C							
*	3,066	98.0	Paved par	aved parking							
*	1,183	98.0	Roofs	oofs							
	9,635	84.6	Weighted Average								
	5,386	74.0	55.90% P	ea							
	4,249	98.0	44.10% In	npervious A	Area						
Тс	Length	Slope	Velocity	Capacity	Description						
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	·						
6.0					Divert Entry						

6.0

Direct Entry,

2304141-POST DEVELOPMENT REV1

Type III 24-hr 10-YEAR Rainfall=4.47"

Prepared by Keach-Nordstrom Associates, Inc.

Revised March 4, 2024 Printed 3/4/2024

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 12

Summary for Subcatchment 12S: SUBCATCHMENT

Runoff = 4.83 cfs @ 12.25 hrs, Volume=

0.49 af, Depth> 1.80"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 10-YEAR Rainfall=4.47"

	A	rea (sf)	CN	Description	n	<u> </u>				
8==	1	12,022	70.0	Woods, G	C					
		24,301	74.0	>75% Gra	ass cover, (Good, HSG C				
*		3,649								
*		3,776	98.0	Roofs						
8	1	43,748	72.1	Weighted Average						
	136,323 70.7			94.83% P	94.83% Pervious Area					
		7,425	98.0	5.17% lm	5.17% Impervious Area					
	Tc	Length	Slope	Velocity	Capacity	Description				
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
	12.0	100	0.1000	0.14		Sheet Flow,				
					Woods: Light underbrush n= 0.400 P2= 2.84"					
	3.2	391	0.1660	2.04		Shallow Concentrated Flow,				
						Woodland Kv= 5.0 fps				
	2.2	183	0.0410	1.42		Shallow Concentrated Flow,				
						Short Grass Pasture Kv= 7.0 fps				
	0.5	138	0.0600	4.97		Shallow Concentrated Flow,				
_						Paved Kv= 20.3 fps				
	17.8	812	Total							

Summary for Subcatchment 13S: SUBCATCHMENT

Runoff = 2.01 cfs @ 12.22 hrs, Volume=

0.19 af, Depth> 2.05"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 10-YEAR Rainfall=4.47"

	Area (sf)	CN	Description
	22,024	70.0	Woods, Good, HSG C
	21,217	74.0	>75% Grass cover, Good, HSG C
×	3,956	98.0	Paved parking
*	2,500	98.0	Roofs
	49,697	75.3	Weighted Average
	43,241	72.0	87.01% Pervious Area
	6,456	98.0	12.99% Impervious Area

2304141-POST DEVELOPMENT REV1

Type III 24-hr 10-YEAR Rainfall=4.47"

Prepared by Keach-Nordstrom Associates, Inc.

Revised March 4, 2024 Printed 3/4/2024

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 13

	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description	
	12.0	100	0.1000	0.14	1.7	Sheet Flow,	2.5
						Woods: Light underbrush n= 0.400 P2= 2.84"	
	2.0	250	0.1720	2.07		Shallow Concentrated Flow,	
						Woodland Kv= 5.0 fps	
	1.7	235	0.1100	2.32		Shallow Concentrated Flow,	
						Short Grass Pasture Kv= 7.0 fps	
	0.2	52	0.0600	4.97		Shallow Concentrated Flow,	
						Paved Kv= 20.3 fps	
0.	15.8	637	Total				

Summary for Subcatchment 14S: SUBCATCHMENT

Runoff

0.88 cfs @ 12.19 hrs, Volume=

0.08 af, Depth> 2.57"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 10-YEAR Rainfall=4.47"

		rea (sf)	CN	Description	n								
		1,995	70.0	Woods, G	Good, HSG	C							
		8,774	74.0	>75% Gra	75% Grass cover, Good, HSG C								
*		4,264	98.0	Paved pa	aved parking								
*		1,235	98.0	Roofs	, ,								
		16,268	81.6	Weighted	eighted Average								
		10,769	73.3	66.20% Pervious Area									
		5,499	98.0	33.80% Ir	.80% Impervious Area								
	Tc	Length	Slope	Velocity	Capacity	Description							
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)								
	13.8	100	0.0700	0.12		Sheet Flow,							
						Woods: Light underbrush n= 0.400 P2= 2.84"							
	0.1	24	0.3300	4.02		Shallow Concentrated Flow,							
						Short Grass Pasture Kv= 7.0 fps							
	0.1	11	0.0200	2.87		Shallow Concentrated Flow,							
						Paved Kv= 20.3 fps							
	14.0	135	Total										

Summary for Subcatchment 15S: SUBCATCHMENT

Runoff

0.64 cfs @ 12.09 hrs, Volume=

0.05 af, Depth> 2.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 10-YEAR Rainfall=4.47"

2304141-POST DEVELOPMENT REV1

Type III 24-hr 10-YEAR Rainfall=4.47"

Prepared by Keach-Nordstrom Associates, Inc.

Revised March 4, 2024 Printed 3/4/2024

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 14

	Aı	rea (sf)	CN	Description	n	5 - 1		
		5,759	74.0	>75% Gra	ass cover, (Good, HSG C		
*		3,207	98.0	Paved pa	rking	To the state of th		- 12
		8,966	82.6	Weighted	Average	14.P		
		5,759	74.0	64.23% P	ervious Are	ea - 1 - 1 - 1		
		3,207	98.0	35.77% lr	npervious /	Area		
						* * * * * * * * * * * * * * * * * * *		
	Tc	Length	Slope	Velocity	Capacity	Description		
(n	nin)	(feet)	(ft/ft)	(ft/sec)	(cfs)	n di na n		
	6.0					Direct Entry,		

Summary for Subcatchment 16S: SUBCATCHMENT

Runoff

1.11 cfs @ 12.09 hrs, Volume=

0.08 af, Depth> 2.83"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 10-YEAR Rainfall=4.47"

	A	rea (sf)	CN	Description					
		8,198	74.0	>75% Grass cover, Good, HSG C					
7	r	5,322	98.0	Paved parking					
1		1,079	98.0	Roofs					
- 87		14,599	84.5	Weighted Average					
		8,198	74.0	56.15% Pervious Area					
		6,401	98.0	43.85% Impervious Area					
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	-			
6.0						Direct Entry,			

Summary for Subcatchment 17S: SUBCATCHMENT

Runoff

3.80 cfs @ 12.16 hrs, Volume=

0.32 af, Depth> 2.24"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 10-YEAR Rainfall=4.47"

	Area (sf)	CN	Description			
	9,732	70.0	Woods, Good, HSG C			
	52,648	74.0	>75% Grass cover, Good, HSG C			
*	2,665	98.0	Roofs			
	2,687	96.0	Gravel surface, HSG C			
*	7,378	100.0	Surface Water			
	75,110	77.7	Weighted Average			
	65,067	74.3	86.63% Pervious Area			
	10,043	99.5	13.37% Impervious Area			

2304141-POST DEVELOPMENT REV1

Type III 24-hr 10-YEAR Rainfall=4.47"

Prepared by Keach-Nordstrom Associates, Inc.

Revised March 4, 2024 Printed 3/4/2024

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 15

70	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	10.6	90	0.1110	0.14	1.1	Sheet Flow,
						Woods: Light underbrush n= 0.400 P2= 2.84"
	0.5	10	0.3300	0.31	· .	Sheet Flow,
						Grass: Short n= 0.150 P2= 2.84"
	0.1	33	0.3300	4.02		Shallow Concentrated Flow,
						Short Grass Pasture Kv= 7.0 fps
	11.2	133	Total			

Summary for Subcatchment 18S: SUBCATCHMENT

Runoff

0.63 cfs @ 12.09 hrs, Volume=

0.05 af, Depth> 2.89"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 10-YEAR Rainfall=4.47"

	Aı	rea (sf)	CN	Description						
		4,351	74.0	>75% Grass cover, Good, HSG C						
*		2,848	98.0	Paved pa	Paved parking					
*		937	98.0	Roofs						
		8,136	85.2	Weighted Average						
		4,351	74.0	53.48% Pervious Area						
		3,785	98.0	46.52% Impervious Area						
	_									
		Length	Slope	Velocity	Capacity	Description				
(n	nin)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
	6.0					Direct Entry,				

Summary for Subcatchment 19S: SUBCATCHMENT

Runoff

0.61 cfs @ 12.09 hrs, Volume=

0.04 af, Depth> 2.89"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 10-YEAR Rainfall=4.47"

	Α	rea (sf)	CN	Description					
		4,199	74.0	>75% Grass cover, Good, HSG C					
•	k	3,084	98.0	Paved parking					
1	k	559	98.0	Roofs					
		7,842	85.1	Weighted Average					
		4,199	74.0	53.55% Pervious Area					
		3,643	98.0	46.45% Impervious Area					
	Тс	Length	Slope	Velocity	Capacity	Description			
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
	6.0					Direct Entry			

6.0

Direct Entry,

2304141-POST DEVELOPMENT REV1

Type III 24-hr 10-YEAR Rainfall=4.47"

Prepared by Keach-Nordstrom Associates, Inc.

Revised March 4, 2024 Printed 3/4/2024

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 16

Summary for Subcatchment 20S: SUBCATCHMENT

Runoff

0.11 cfs @ 12.09 hrs, Volume= 0.01 af, Depth> 2.82"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 10-YEAR Rainfall=4.47"

	A	rea (sf)	· CN	Description	n	D. D. C.				
		790	74.0	>75% Gra	ass cover, (Good, HSG C				
*		499	98.0	Paved pa	rking					
*		107	98.0	Roofs	oofs					
		1,396	84.4	Weighted	Veighted Average					
		790	74.0	56.59% Pervious Area				;		
		606	98.0	43.41% Ir	43.41% Impervious Area					
		1	01	V/-14-	0	Description				
	Tc	Length	Slope	Velocity	Capacity	Description				
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
	6.0					Direct Entry,				

Direct Entry,

Summary for Subcatchment 21S: SUBCATCHMENT

Runoff

0.62 cfs @ 12.15 hrs, Volume=

0.05 af, Depth> 2.06"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 10-YEAR Rainfall=4.47"

_	A	rea (sf)	CN	Description	n					
		860	61.0	>75% Gra	>75% Grass cover, Good, HSG B					
		6,744 74.0 >75% Grass cover, Good, HSG C								
		270	55.0	Woods, G	Good, HSG	В				
		3,203	70.0	Woods, G	Good, HSG	C				
*		2,052	98.0	Paved pa	rking					
		13,129	75.5	Weighted	Average					
		11,077	71.4	84.37% P	ervious Are	ea				
		2,052	98.0	15.63% In	npervious A	Area				
					•					
	Tc	Length	Slope	Velocity	Capacity	Description				
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
	10.5	100	0.1400	0.16		Sheet Flow,				
						Woods: Light underbrush n= 0.400 P2= 2.84"				
	0.1	17	0.1800	2.12		Shallow Concentrated Flow,				
						Woodland Kv= 5.0 fps				
	0.2	26	0.0960	2.17		Shallow Concentrated Flow,				
						Short Grass Pasture Kv= 7.0 fps				
	10.8	143	Total							

Type III 24-hr 10-YEAR Rainfall=4.47"

Prepared by Keach-Nordstrom Associates, Inc.

Revised March 4, 2024 Printed 3/4/2024

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 17

Summary for Subcatchment 22S: SUBCATCHMENT

Runoff

0.23 cfs @ 12.09 hrs, Volume= 0.02 af, Depth> 2.58"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 10-YEAR Rainfall=4.47"

	A	rea (sf)	CN	Description	n	3			
		1,487	61.0	>75% Gra	ass cover, (Good, HSG B			
*		1,885	98.0	Paved pa	rking				
		3,372	81.7	Weighted	Average				
		1,487	61.0	44.10% P	44.10% Pervious Area				
		1,885	98.0	55.90% In	npervious A	Area			
	Тс	Length	Slope	Velocity	Capacity	Description			
(r	min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	·			
	6.0		***			Direct Entry.			

Summary for Subcatchment 23S: SUBCATCHMENT

Runoff

0.26 cfs @ 12.09 hrs, Volume=

0.02 af, Depth> 2.62"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 10-YEAR Rainfall=4.47"

	Area (sf)	CN	Description	Description				
(a.	1,588	61.0	>75% Gra	75% Grass cover, Good, HSG B				
*	2,107	98.0	Paved pa	Paved parking				
	3,695	82.1	Weighted	Veighted Average				
	1,588	61.0	42.98% P	42.98% Pervious Area				
	2,107	98.0	57.02% lr	57.02% Impervious Area				
To (min	N N O N	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	•			
6.0)				Direct Entry,			

Summary for Subcatchment 24S: SUBCATCHMENT

Runoff

0.32 cfs @ 12.09 hrs, Volume=

0.02 af, Depth> 2.71"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 10-YEAR Rainfall=4.47"

	Area (sf)	CN	Description
	1,725	61.0	>75% Grass cover, Good, HSG B
	42	55.0	Woods, Good, HSG B
*	2,654	98.0	Paved parking
	4,421	83.2	Weighted Average
	1,767	60.9	39.97% Pervious Area
	2,654	98.0	60.03% Impervious Area

2304141-POST DEVELOPMENT REV1

Type III 24-hr 10-YEAR Rainfall=4.47"

Prepared by Keach-Nordstrom Associates. Inc.

Revised March 4, 2024 Printed 3/4/2024

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 18

Tc (min)		Velocity (ft/sec)		Description	
6.0			2	Direct Entry,	

Summary for Subcatchment 25S: SUBCATCHMENT

Runoff

0.05 cfs @ 12.10 hrs, Volume=

0.00 af. Depth> 1.01"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 10-YEAR Rainfall=4.47"

Α	rea (sf)	CN	Description	scription					
	1,907	61.0	>75% Gra	5% Grass cover, Good, HSG B					
	339	55.0	Woods, G	oods, Good, HSG B					
	2,246	60.1	Weighted	eighted Average					
	2,246	60.1	100.00%	100.00% Pervious Area					
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description				
6.0					Direct Entry,				

Summary for Reach 1R: EXISTING ROADWAY DITCH

Inflow Area =

15.186 ac, 17.13% Impervious, Inflow Depth > 0.73" for 10-YEAR event

Inflow

3.46 cfs @ 12.15 hrs, Volume=

0.92 af

Outflow

3.44 cfs @ 12.17 hrs, Volume=

0.92 af, Atten= 0%, Lag= 0.9 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Max. Velocity= 3.67 fps, Min. Travel Time= 1.0 min Avg. Velocity = 2.17 fps, Avg. Travel Time= 1.8 min

Peak Storage= 216 cf @ 12.17 hrs Average Depth at Peak Storage= 0.56'

Bank-Full Depth= 1.00' Flow Area= 3.0 sf, Capacity= 16.21 cfs

0.00' x 1.00' deep channel, n= 0.022 Earth, clean & straight

Side Slope Z-value= 3.0 '/' Top Width= 6.00'

Length= 230.0' Slope= 0.0173 '/'

inlet Invert= 295.64', Outlet Invert= 291.66'



Type III 24-hr 10-YEAR Rainfall=4.47"

Prepared by Keach-Nordstrom Associates, Inc.

Revised March 4, 2024 Printed 3/4/2024

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 19

Summary for Pond 1P: EXISTING CB

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 20.889 ac, 3.54% Impervious, Inflow Depth > 1.24" for 10-YEAR event

Inflow = 14.23 cfs @ 12.59 hrs, Volume= 2.16 af

Primary = 14.23 cfs @ 12.59 hrs, Volume= 2.16 af, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Summary for Pond 2P: EXISTING DRIVEWAY CULVERT

Inflow Area = 2.596 ac. 12.76% Impervious, Inflow Depth > 1.08" for 10-YEAR event

Inflow = 1.94 cfs @ 12.32 hrs, Volume= 0.23 af

Outflow = 1.94 cfs @ 12.32 hrs, Volume= 0.23 af, Atten= 0%, Lag= 0.0 min

Primary = 1.94 cfs @ 12.32 hrs, Volume= 0.23 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 293.20' @ 12.32 hrs

Flood Elev= 294.00'

Device Routing Invert Outlet Devices

#1 Primary 292.40' **12.0" Round Culvert**L= 21.0' CPP, square edge headwall, Ke= 0.500

Inlet / Outlet Invert= 292.40' / 292.10' S= 0.0143 '/' Cc= 0.900

n= 0.013, Flow Area= 0.79 sf

Primary OutFlow Max=1.94 cfs @ 12.32 hrs HW=293.20' TW=290.79' (Dynamic Tailwater) 1=Culvert (Barrel Controls 1.94 cfs @ 3.94 fps)

Summary for Pond 3P: EXISTING DROP INLET

Inflow Area = 41.489 ac, 9.34% Impervious, Inflow Depth > 1.03" for 10-YEAR event

Inflow = 19.22 cfs @ 12.51 hrs, Volume= 3.57 af

Outflow = 19.22 cfs @ 12.51 hrs, Volume= 3.57 af, Atten= 0%, Lag= 0.0 min

Primary = 19.22 cfs @ 12.51 hrs, Volume= 3.57 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 291.27' @ 12.51 hrs

Flood Elev= 293.75'

Device	Routing	Invert	Outlet Devices
#1	Primary	288.66'	24.0" Round Culvert
			L= 92.1' RCP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 288.66' / 284.67' S= 0.0433 '/' Cc= 0.900
			n= 0.011 Concrete pipe, straight & clean, Flow Area= 3.14 sf

Primary OutFlow Max=19.21 cfs @ 12.51 hrs HW=291.27' TW=0.00' (Dynamic Tailwater) 1=Culvert (Inlet Controls 19.21 cfs @ 6.12 fps)

Type III 24-hr 10-YEAR Rainfall=4.47"

Prepared by Keach-Nordstrom Associates, Inc.

Revised March 4, 2024 Printed 3/4/2024

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 20

Summary for Pond 4P: EXISTING CB

Inflow Area = 15.186 ac, 17.13% Impervious, Inflow Depth > 0.73" for 10-YEAR event 3.46 cfs @ 12.15 hrs, Volume= 0.92 af 0.92 af, Atten= 0%, Lag= 0.0 min 3.46 cfs @ 12.15 hrs, Volume= 0.92 af 0.00 cfs @ 0.00 hrs, Volume= 0.00 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 297.98' @ 12.15 hrs

Flood Elev= 300.17'

Device	Routing	Invert	Outlet Devices
#1	Primary	297.00'	15.0" Round Culvert
			L= 49.1' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 297.00' / 295.65' S= 0.0275 '/' Cc= 0.900
			n= 0.013, Flow Area= 1.23 sf
#2	Secondary	300.17'	24.0" x 24.0" Horiz. Orifice/Grate C= 0.600
			Limited to weir flow at low heads

Primary OutFlow Max=3.45 cfs @ 12.15 hrs HW=297.98' TW=296.20' (Dynamic Tailwater) 1=Culvert (Inlet Controls 3.45 cfs @ 3.36 fps)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=297.00' TW=295.64' (Dynamic Tailwater) 2=Orifice/Grate (Controls 0.00 cfs)

Summary for Pond 5P: EXISTING CB

Inflow Area =	0.377 ac, 34.30% Impervious, Inflow	Depth > 1.92" for 10-YEAR event
Inflow =	0.77 cfs @ 12.13 hrs, Volume=	0.06 af
Outflow =	0.77 cfs @ 12.13 hrs, Volume=	0.06 af, Atten= 0%, Lag= 0.0 min
Primary =	0.77 cfs @ 12.13 hrs, Volume=	0.06 af
Secondary =	0.00 cfs @ 0.00 hrs, Volume=	0.00 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 298.34' @ 12.14 hrs

Flood Elev= 301.16'

Device	Routing	Invert	Outlet Devices
#1	Primary	297.70'	12.0" Round Culvert
	•		L= 34.1' CMP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 297.70' / 297.30' S= 0.0117 '/' Cc= 0.900
			n= 0.025 Corrugated metal, Flow Area= 0.79 sf
#2	Secondary	301.16'	24.0" x 24.0" Horiz. Orifice/Grate C= 0.600
			Limited to weir flow at low heads

Primary OutFlow Max=0.75 cfs @ 12.13 hrs HW=298.33' TW=297.96' (Dynamic Tailwater) 1=Culvert (Outlet Controls 0.75 cfs @ 2.05 fps)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=297.70' TW=0.00' (Dynamic Tailwater) 2=Orifice/Grate (Controls 0.00 cfs)

Type III 24-hr 10-YEAR Rainfall=4.47"

Prepared by Keach-Nordstrom Associates. Inc.

Revised March 4, 2024 Printed 3/4/2024

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 21

Summary for Pond 6P: EXISTING CB

Inflow Area =

1.835 ac, 11.18% Impervious, Inflow Depth > 2.03" for 10-YEAR event

Inflow

2.92 cfs @ 12.28 hrs. Volume=

Outflow = 2.92 cfs @ 12.28 hrs. Volume=

0.31 af, Atten= 0%, Lag= 0.0 min

Primary

2.92 cfs @ 12.28 hrs, Volume=

0.31 af

0.31 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 364.80' @ 12.28 hrs

Flood Elev= 366.35'

Device Routing Invert Outlet Devices

#1 Primary 363.70'

12.0" Round Culvert

L= 18.0' CMP, square edge headwall, Ke= 0.500

Inlet / Outlet Invert= 363.70' / 362.46' S= 0.0689 '/' Cc= 0.900

n= 0.025 Corrugated metal, Flow Area= 0.79 sf

Primary OutFlow Max=2.92 cfs @ 12.28 hrs HW=364.80' TW=0.00' (Dynamic Tailwater) 1=Culvert (Inlet Controls 2.92 cfs @ 3.72 fps)

Summary for Pond 7P: EXISTING CB

Inflow Area =

2.507 ac, 5.17% Impervious, Inflow Depth > 1.81" for 10-YEAR event

Inflow Outflow = 3.43 cfs @ 12.30 hrs, Volume=

0.38 af

3.43 cfs @ 12.30 hrs, Volume=

0.38 af, Atten= 0%, Lag= 0.0 min

Primary

3.43 cfs @ 12.30 hrs, Volume=

0.38 af

Routing by Dyn-Stor-Ind method. Time Span= 0.00-24.00 hrs. dt= 0.01 hrs

Peak Elev= 377.94' @ 12.30 hrs

Flood Elev= 380.49'

=

Device Routing

Invert Outlet Devices

#1 Primary 376.57' 12.0" Round Culvert

L= 31.9' RCP, square edge headwall, Ke= 0.500

Inlet / Outlet Invert= 376.57' / 376.28' S= 0.0091 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf

Primary OutFlow Max=3.43 cfs @ 12.30 hrs HW=377.94' TW=0.00' (Dynamic Tailwater) 1=Culvert (Barrel Controls 3.43 cfs @ 4.36 fps)

Summary for Pond 8P: CB#20

Inflow Area =

12.886 ac, 15.89% Impervious, Inflow Depth > 0.55" for 10-YEAR event

Inflow

0.66 cfs @ 19.10 hrs, Volume=

0.59 af

Outflow

0.66 cfs @ 19.10 hrs, Volume=

0.59 af, Atten= 0%, Lag= 0.1 min

Primary

0.66 cfs @ 19.10 hrs, Volume=

0.59 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 350.58' @ 19.10 hrs

Flood Elev= 360.00'

=

2304141-POST DEVELOPMENT REV1

Type III 24-hr 10-YEAR Rainfall=4.47"

Prepared by Keach-Nordstrom Associates, Inc.

Revised March 4, 2024 Printed 3/4/2024

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 22

Invert: Outlet Devices Device Routing #1 Primary 350.20' 15.0" Round Culvert

L= 115.0' CPP, square edge headwall, Ke= 0.500

Inlet / Outlet Invert= 350.20' / 341.00' S= 0.0800 '/' Cc= 0.900

n= 0.013. Flow Area= 1.23 sf

Primary OutFlow Max=0.66 cfs @ 19.10 hrs HW=350.58' TW=340.64' (Dynamic Tailwater) 1=Culvert (Inlet Controls 0.66 cfs @ 2.10 fps)

Summary for Pond 9P: DMH#3

Inflow Area =

5.943 ac, 15.71% Impervious, Inflow Depth > 2.09" for 10-YEAR event

Inflow

9.60 cfs @ 12.21 hrs, Volume=

1.04 af 1.04 af, Atten= 0%, Lag= 0.0 min

Outflow = Primary = 9.60 cfs @ 12.21 hrs, Volume= 9.60 cfs @ 12.21 hrs. Volume=

1.04 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 376.48' @ 12.21 hrs

Flood Elev= 377.79'

Device Routing **Outlet Devices** Invert 18.0" Round Culvert #1 Primary 374.27'

L= 20.0' CPP, square edge headwall, Ke= 0.500

Inlet / Outlet Invert= 374.27' / 374.12' S= 0.0075 '/' Cc= 0.900

n= 0.013. Flow Area= 1.77 sf

Primary OutFlow Max=9.60 cfs @ 12.21 hrs HW=376.47' TW=368.49' (Dynamic Tailwater) -1=Culvert (Barrel Controls 9.60 cfs @ 5.43 fps)

Summary for Pond 10P: DMH#4

Inflow Area =

5.943 ac, 15.71% Impervious, Inflow Depth > 2.09" for 10-YEAR event

Inflow

9.60 cfs @ 12.21 hrs, Volume=

1.04 af

Outflow = 9.60 cfs @ 12.21 hrs, Volume=

1.04 af. Atten= 0%. Lag= 0.0 min

Primary

9.60 cfs @ 12.21 hrs, Volume=

1.04 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs. dt= 0.01 hrs

Peak Elev= 378.18' @ 12.21 hrs

Flood Elev= 381.66'

Device Routing Invert **Outlet Devices** #1 18.0" Round Culvert Primary 375.28'

L= 121.7' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 375.28' / 374.37' S= 0.0075 '/' Cc= 0.900

n= 0.013, Flow Area= 1.77 sf

Primary OutFlow Max=9.59 cfs @ 12.21 hrs HW=378.18' TW=376.47' (Dynamic Tailwater) 1=Culvert (Outlet Controls 9.59 cfs @ 5.43 fps)

Type III 24-hr 10-YEAR Rainfall=4.47"

Prepared by Keach-Nordstrom Associates, Inc.

Revised March 4, 2024 Printed 3/4/2024

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 23

Summary for Pond 11P: DMH#5

Inflow Area = 5.943 ac, 15.71% Impervious, Inflow Depth > 2.09" for 10-YEAR event

Inflow = 9.60 cfs @ 12.21 hrs, Volume= 1.04 af

Outflow = 9.60 cfs @ 12.21 hrs, Volume= 1.04 af, Atten= 0%, Lag= 0.0 min

Primary = 9.60 cfs @ 12.21 hrs, Volume= 1.04 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 380.76' @ 12.21 hrs

Flood Elev= 384.00'

Primary OutFlow Max=9.58 cfs @ 12.21 hrs HW=380.75' TW=378.18' (Dynamic Tailwater) 1=Culvert (Outlet Controls 9.58 cfs @ 5.42 fps)

Summary for Pond 12P: CB#6

Inflow Area = 5.943 ac, 15.71% Impervious, Inflow Depth > 2.09" for 10-YEAR event

Inflow = 9.60 cfs @ 12.21 hrs, Volume= 1.04 af

Outflow = 9.60 cfs @ 12.21 hrs, Volume= 1.04 af, Atten= 0%, Lag= 0.0 min

Primary = 9.60 cfs @ 12.21 hrs, Volume= 1.04 af

Routing by Dyn-Stor-Ind method. Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 382.63' @ 12.22 hrs

Flood Elev= 384.30'

Device	Routing	Invert	Outlet Devices
#1	Primary	377.89'	18.0" Round Culvert
			L= 141.2' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 377.89' / 377.18' S= 0.0050 '/' Cc= 0.900
			n= 0.013, Flow Area= 1.77 sf

Primary OutFlow Max=9.54 cfs @ 12.21 hrs HW=382.60' TW=380.75' (Dynamic Tailwater)
1=Culvert (Outlet Controls 9.54 cfs @ 5.40 fps)

Summary for Pond 13P: CB#7

Inflow Area = 5.737 ac, 14.99% Impervious, Inflow Depth > 2.07" for 10-YEAR event

Inflow = 9.24 cfs @ 12.22 hrs, Volume= 0.99 af

Outflow = 9.24 cfs @ 12.22 hrs, Volume= 0.99 af, Atten= 0%, Lag= 0.0 min

Primary = 9.24 cfs @ 12.22 hrs, Volume= 0.99 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 383.80' @ 12.23 hrs

Flood Elev= 384.30'

2304141-POST DEVELOPMENT REV1

Type III 24-hr 10-YEAR Rainfall=4.47"

Prepared by Keach-Nordstrom Associates, Inc.

Revised March 4, 2024 Printed 3/4/2024

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 24

Primary OutFlow Max=9.17 cfs @ 12.22 hrs HW=383.78' TW=382.62' (Dynamic Tailwater)
1=Culvert (Inlet Controls 9.17 cfs @ 5.19 fps)

Summary for Pond 14P: CB#8

[80] Warning: Exceeded Pond 15P by 0.76' @ 12.13 hrs (3.29 cfs 0.03 af)

Inflow Area = 5.364 ac, 13.68% Impervious, Inflow Depth > 2.03" for 10-YEAR event

Inflow = 8.39 cfs @ 12.23 hrs, Volume= 0.91 af

Outflow = 8.39 cfs @ 12.23 hrs, Volume= 0.91 af, Atten= 0%, Lag= 0.0 min

Primary = 8.39 cfs @ 12.23 hrs, Volume= 0.91 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 387.68' @ 12.23 hrs

Flood Elev= 388.51'

| Device | Routing | Invert | Outlet Devices | | 382.24' | | 15.0" | Round Culvert | L= 165.7' | CPP, square edge headwall, Ke= 0.500 | Inlet / Outlet Invert= 382.24' / 378.85' | S= 0.0205 '/' | Cc= 0.900 | n= 0.013, Flow Area= 1.23 sf

Primary OutFlow Max=8.36 cfs @ 12.23 hrs HW=387.67' TW=383.80' (Dynamic Tailwater) 1=Culvert (Outlet Controls 8.36 cfs @ 6.81 fps)

Summary for Pond 15P: CB#80

Inflow Area = 0.180 ac, 46.45% Impervious, Inflow Depth > 2.89" for 10-YEAR event

Inflow = 0.61 cfs @ 12.09 hrs, Volume= 0.04 af

Outflow = 0.61 cfs @ 12.09 hrs, Volume= 0.04 af, Atten= 0%, Lag= 0.0 min

Primary = 0.61 cfs @ 12.09 hrs, Volume= 0.04 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 387.69' @ 12.24 hrs

Flood Elev= 388.51'

| Device Routing | Invert Outlet Devices | | 382.86' | 12.0" | Round Culvert | | L= 26.0' | CPP, square edge headwall, Ke= 0.500 | Inlet / Outlet Invert= 382.86' / 382.34' | S= 0.0200 '/' | Cc= 0.900 | n= 0.013, Flow Area= 0.79 sf

Primary OutFlow Max=0.00 cfs @ 12.09 hrs HW=384.19' TW=384.27' (Dynamic Tailwater)
1=Culvert (Controls 0.00 cfs)

Type III 24-hr 10-YEAR Rainfall=4.47"

Prepared by Keach-Nordstrom Associates, Inc.

Revised March 4, 2024 Printed 3/4/2024

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 25

Summary for Pond 16P: CB#10

[80] Warning: Exceeded Pond 17P by 0.09' @ 12.05 hrs (1.11 cfs 0.01 af)

Inflow Area = 4.043 ac, 12.41% Impervious, Inflow Depth > 1.99" for 10-YEAR event

Inflow = 6.06 cfs @ 12.23 hrs, Volume= 0.67 af

Outflow = 6.06 cfs @ 12.23 hrs, Volume= 0.67 af, Atten= 0%, Lag= 0.0 min

Primary = 6.06 cfs @ 12.23 hrs, Volume= 0.67 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 394.51' @ 12.23 hrs

Flood Elev= 397.31'

Device	Routing	Invert	Outlet Devices
#1	Primary		12.0" Round Culvert L= 165.5' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 391.44' / 382.34' S= 0.0550 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

Primary OutFlow Max=6.06 cfs @ 12.23 hrs HW=394.51' TW=387.68' (Dynamic Tailwater)
—1=Culvert (Inlet Controls 6.06 cfs @ 7.71 fps)

Summary for Pond 17P: CB#100

Inflow Area = 0.335 ac, 43.85% Impervious, Inflow Depth > 2.83" for 10-YEAR event

Inflow = 1.11 cfs @ 12.09 hrs, Volume= 0.08 af

Outflow = 1.11 cfs @ 12.09 hrs, Volume= 0.08 af, Atten= 0%, Lag= 0.0 min

Primary = 1.11 cfs @ 12.09 hrs, Volume= 0.08 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 394.53' @ 12.24 hrs

Flood Elev= 397.31'

Device	Routing	Invert	Outlet Devices
#1	Primary	392.06'	12.0" Round Culvert
			L= 26.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 392.06' / 391.54' S= 0.0200 '/' Cc= 0.900
			n= 0.013, Flow Area= 0.79 sf

Primary OutFlow Max=0.00 cfs @ 12.09 hrs HW=393.75' TW=393.79' (Dynamic Tailwater) 1=Culvert (Controls 0.00 cfs)

Summary for Pond 18P: CB#11

Inflow Area = 0.408 ac. 45.21% Impervious, Inflow Depth > 2.86" for 10-YEAR event

Inflow = 1.36 cfs @ 12.09 hrs, Volume= 0.10 af

Outflow = 1.36 cfs @ 12.09 hrs, Volume= 0.10 af, Atten= 0%, Lag= 0.0 min

Primary = 1.36 cfs @ 12.09 hrs, Volume= 0.10 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

2304141-POST DEVELOPMENT REV1

Type III 24-hr 10-YEAR Rainfall=4.47"

Prepared by Keach-Nordstrom Associates, Inc.

Revised March 4, 2024 Printed 3/4/2024

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 26

Peak Elev= 401.39' @ 12.09 hrs

Flood Elev= 406.88'

Primary OutFlow Max=1.36 cfs @ 12.09 hrs HW=401.39' TW=393.79' (Dynamic Tailwater) 1=Culvert (Inlet Controls 1.36 cfs @ 2.68 fps)

Summary for Pond 19P: CB#110

Inflow Area = 0.187 ac, 46.52% Impervious, Inflow Depth > 2.89" for 10-YEAR event

Inflow = 0.63 cfs @ 12.09 hrs, Volume= 0.05 af

Outflow = 0.63 cfs @ 12.09 hrs, Volume= 0.05 af, Atten= 0%, Lag= 0.0 min

Primary = 0.63 cfs @ 12.09 hrs, Volume= 0.05 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 402.03' @ 12.09 hrs

Flood Elev= 406.64'

| Device Routing | Invert Outlet Devices | 401.63' | 12.0" Round Culvert | L= 38.0' CPP, square edge headwall, Ke= 0.500 | Inlet / Outlet Invert= 401.63' / 400.87' S= 0.0200 '/' Cc= 0.900 | n= 0.013, Flow Area= 0.79 sf

Primary OutFlow Max=0.63 cfs @ 12.09 hrs HW=402.03' TW=401.39' (Dynamic Tailwater) 1=Culvert (Inlet Controls 0.63 cfs @ 2.15 fps)

Summary for Pond 20P: DMH#14

Inflow Area = 5.167 ac, 17.10% Impervious, Inflow Depth > 2.10" for 10-YEAR event

Inflow = 8.24 cfs @ 12.30 hrs, Volume= 0.90 af

Outflow = 8.24 cfs @ 12.30 hrs, Volume= 0.90 af, Atten= 0%, Lag= 0.0 min

Primary = 8.24 cfs @ 12.30 hrs, Volume= 0.90 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 376.89' @ 12.30 hrs

Flood Elev= 383.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	375.20'	18.0" Round Culvert
			L= 15.0' CPP, square edge headwall, Ke= 0.500
			inlet / Outlet Invert= 375.20' / 374.00' S= 0.0800 '/' Cc= 0.900
			n= 0.013. Flow Area= 1.77 sf

Primary OutFlow Max=8.24 cfs @ 12.30 hrs HW=376.89' TW=369.08' (Dynamic Tailwater)
1=Culvert (Inlet Controls 8.24 cfs @ 4.66 fps)

Type III 24-hr 10-YEAR Rainfall=4.47"

Prepared by Keach-Nordstrom Associates, Inc.

Revised March 4, 2024 Printed 3/4/2024

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 27

Summary for Pond 21P: DMH#15

Inflow Area = 5.167 ac, 17.10% Impervious, Inflow Depth > 2.10" for 10-YEAR event

Inflow = 8.24 cfs @ 12.30 hrs, Volume= 0.90 af

Outflow = 8.24 cfs @ 12.30 hrs, Volume= 0.90 af, Atten= 0%, Lag= 0.0 min

Primary = 8.24 cfs @ 12.30 hrs, Volume= 0.90 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 383.15' @ 12.30 hrs

Flood Elev= 387.85'

Device	Routing	Invert	Outlet Devices
#1	Primary	381.46'	18.0" Round Culvert L= 30.7' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 381.46' / 379.00' S= 0.0801 '/' Cc= 0.900 n= 0.013, Flow Area= 1.77 sf

Primary OutFlow Max=8.24 cfs @ 12.30 hrs HW=383.15' TW=376.89' (Dynamic Tailwater) 1=Culvert (Inlet Controls 8.24 cfs @ 4.66 fps)

Summary for Pond 22P: CB#16

Inflow Area = 5.167 ac, 17.10% Impervious, Inflow Depth > 2.10" for 10-YEAR event

Inflow = 8.24 cfs @ 12.30 hrs, Volume= 0.90 af

Outflow = 8.24 cfs @ 12.30 hrs, Volume= 0.90 af, Atten= 0%, Lag= 0.0 min

Primary = 8.24 cfs @ 12.30 hrs, Volume= 0.90 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 399.31' @ 12.30 hrs

Flood Elev= 409.82'

Device	Routing	Invert	Outlet Devices
#1	Primary	397.62'	18.0" Round Culvert
			L= 44.5' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 397.62' / 394.06' S= 0.0800 '/' Cc= 0.900
			n= 0.013. Flow Area= 1.77 sf

Primary OutFlow Max=8.24 cfs @ 12.30 hrs HW=399.31' TW=383.15' (Dynamic Tailwater) 1=Culvert (Inlet Controls 8.24 cfs @ 4.66 fps)

Summary for Pond 23P: CB#17

Inflow Area = 5.135 ac, 16.93% Impervious, Inflow Depth > 2.09" for 10-YEAR event

Inflow = 8.20 cfs @ 12.30 hrs, Volume= 0.90 af

Outflow = 8.20 cfs @ 12.30 hrs, Volume= 0.90 af, Atten= 0%, Lag= 0.0 min

Primary = 8.20 cfs @ 12.30 hrs, Volume= 0.90 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 402.68' @ 12.30 hrs

Flood Elev= 407.95'

2304141-POST DEVELOPMENT REV1

Type III 24-hr 10-YEAR Rainfall=4.47"

Prepared by Keach-Nordstrom Associates, Inc.

Revised March 4, 2024 Printed 3/4/2024

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 28

Primary OutFlow Max=8.20 cfs @ 12.30 hrs HW=402.68' TW=399.31' (Dynamic Tailwater)
—1=Culvert (Inlet Controls 8.20 cfs @ 4.64 fps)

Summary for Pond 24P: CB#18

Inflow Area = 3.514 ac, 20.84% Impervious, Inflow Depth > 2.19" for 10-YEAR event

Inflow = 5.97 cfs @ 12.29 hrs, Volume= 0.64 af

Outflow = 5.97 cfs @ 12.29 hrs, Volume= 0.64 af, Atten= 0%, Lag= 0.0 min

Primary = 5.97 cfs @ 12.29 hrs, Volume= 0.64 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 403.69' @ 12.30 hrs

Flood Elev= 408.04'

| Device | Routing | Invert | Outlet Devices | 401.98' | 401.98' | 401.98' | 15.0" | Round Culvert | L= 36.5' | CPP, square edge headwall, Ke= 0.500 | Inlet / Outlet Invert= 401.98' / 401.25' | S= 0.0200 '/' | Cc= 0.900 | n= 0.013. Flow Area= 1.23 sf

Primary OutFlow Max=5.95 cfs @ 12.29 hrs HW=403.69' TW=402.67' (Dynamic Tailwater) 1=Culvert (Inlet Controls 5.95 cfs @ 4.85 fps)

Summary for Pond 25P: CB#19

Inflow Area = 2.703 ac, 20.59% Impervious, Inflow Depth > 2.19" for 10-YEAR event

Inflow = 4.66 cfs @ 12.28 hrs, Volume= 0.49 af

Outflow = 4.66 cfs @ 12.28 hrs, Volume= 0.49 af, Atten= 0%, Lag= 0.0 min

Primary = 4.66 cfs @ 12.28 hrs, Volume= 0.49 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 405.19' @ 12.30 hrs

Flood Elev= 407.78'

| Device | Routing | Invert | Outlet Devices | 402.75' | 12.0" | Round Culvert | L= 26.0' | CPP, square edge headwall, Ke= 0.500 | Inlet / Outlet Invert= 402.75' / 402.23' | S= 0.0200 '/' | Cc= 0.900 | n= 0.013, Flow Area= 0.79 sf

Primary OutFlow Max=4.62 cfs @ 12.28 hrs HW=405.17' TW=403.68' (Dynamic Tailwater) 1=Culvert (Inlet Controls 4.62 cfs @ 5.88 fps)

2304141-POST DEVELOPMENT REV1

Type III 24-hr 10-YEAR Rainfall=4.47"

Prepared by Keach-Nordstrom Associates, Inc.

Revised March 4, 2024 Printed 3/4/2024

HvdroCAD® 10.00-26 s/n 03273 © 2020 HvdroCAD Software Solutions LLC

Page 29

Summary for Pond 26P: DI#24

Inflow Area =

13.452 ac. 16.71% Impervious. Inflow Depth > 0.63" for 10-YEAR event

Inflow

1.55 cfs @ 12.11 hrs, Volume=

0.70 af. Atten= 0%. Lag= 0.0 min

1.55 cfs @ 12.11 hrs, Volume=

Primary

Outflow-

1.55 cfs @ 12.11 hrs, Volume=

0.70 af

0.70 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 299.85' @ 12.11 hrs

Flood Elev= 303.26'

Device Routing #1 Primary

Invert Outlet Devices

299.25'

15.0" Round Culvert

L= 50.8' CPP, square edge headwall, Ke= 0.500

Inlet / Outlet Invert= 299.25' / 297.12' S= 0.0419 '/' Cc= 0.900

n= 0.013, Flow Area= 1.23 sf

Primary OutFlow Max=1.55 cfs @ 12.11 hrs HW=299.85' TW=297.94' (Dynamic Tailwater)

-1=Culvert (Inlet Controls 1.55 cfs @ 2.64 fps)

Summary for Pond 27P: DI#23

Inflow Area =

13.350 ac, 16.38% Impervious, Inflow Depth > 0.61" for 10-YEAR event

Inflow

1.24 cfs @ 12.12 hrs, Volume= 0.68 af

0.68 af, Atten= 0%, Lag= 0.0 min

Outflow =

Primary

1.24 cfs @ 12.12 hrs, Volume= 1.24 cfs @ 12.12 hrs, Volume=

0.68 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 317.48' @ 12.12 hrs

Flood Elev= 320.62'

Device Routing

Invert Outlet Devices

#1 **Primary** 316.95' 15.0" Round Culvert

L= 220.0' CPP, square edge headwall, Ke= 0.500

Inlet / Outlet Invert= 316.95' / 299.35' S= 0.0800 '/' Cc= 0.900

n= 0.013, Flow Area= 1.23 sf

Primary OutFlow Max=1.24 cfs @ 12.12 hrs HW=317.48' TW=299.85' (Dynamic Tailwater)

1=Culvert (Inlet Controls 1.24 cfs @ 2.49 fps)

Summary for Pond 28P: DI#22

Inflow Area =

13.265 ac, 16.12% Impervious, Inflow Depth > 0.60" for 10-YEAR event

Inflow

1.01 cfs @ 12.13 hrs, Volume=

0.66 af 0.66 af, Atten= 0%, Lag= 0.0 min

Outflow

1.01 cfs @ 12.13 hrs, Volume=

Primary

1.01 cfs @ 12.13 hrs, Volume=

0.66 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 331.13' @ 12.13 hrs

Flood Elev= 334.44'

=

2304141-POST DEVELOPMENT REV1

Type III 24-hr 10-YEAR Rainfall=4.47"

Prepared by Keach-Nordstrom Associates, Inc.

Revised March 4, 2024 Printed 3/4/2024

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 30

Device	Routing	Invert	Outlet Devices.	
#1	Primary	330.65'	15.0" Round Culvert	
			L= 181.4' CPP, square edge headwall, Ke= 0.500	
			Inlet / Outlet Invert= 330.65' / 317.05' S= 0.0750 '/' Cc= 0.900	
			n= 0.013, Flow Area= 1.23 sf	

Primary OutFlow Max=1.01 cfs @ 12.13 hrs HW=331.13' TW=317.48' (Dynamic Tailwater) 1=Culvert (Inlet Controls 1.01 cfs @ 2.35 fps)

Summary for Pond 29P: DI#21

Inflow Area = 13.188 ac, 15.88% Impervious, Inflow Depth > 0.59" for 10-YEAR event

0.81 cfs @ 12.15 hrs. Volume= Inflow 0.64 af

0.81 cfs @ 12.15 hrs, Volume= Outflow = 0.64 af. Atten= 0%. Lag= 0.0 min

0.81 cfs @ 12.15 hrs. Volume= Primary 0.64 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 340.68' @ 12.15 hrs

Flood Elev= 344.55'

Device	Routing	Invert	Outlet Devices				
#1	Primary	340.26'	5.0" Round Culvert				
			L= 118.9' CPP, square edge headwall, Ke= 0.500				
			Inlet / Outlet Invert= 340.26' / 330.75' S= 0.0800 '/' Cc= 0.900				
			n= 0.013, Flow Area= 1.23 sf				

Primary OutFlow Max=0.81 cfs @ 12.15 hrs HW=340.68' TW=331.12' (Dynamic Tailwater) -1=Culvert (Inlet Controls 0.81 cfs @ 2.22 fps)

Summary for Pond PP: POCKET POND

Inflow Area =	12.835 ac, 15.95% Impervious, Inflow Depth > 2.11" for 10-YEAR event
Inflow =	20.55 cfs @ 12.23 hrs, Volume= 2.26 af
Outflow =	0.66 cfs @ 19.16 hrs, Volume= 0.59 af, Atten= 97%, Lag= 415.5 min
Primary =	0.66 cfs @ 19.16 hrs, Volume= 0.59 af
Secondary =	0.00 cfs @ 0.00 hrs, Volume= 0.00 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Starting Elev= 366.00' Surf.Area= 7,378 sf Storage= 10,599 cf

Peak Elev= 372.01' @ 19.16 hrs Surf.Area= 19,519 sf Storage= 85,566 cf (74,967 cf above start)

Flood Elev= 374.00' Surf.Area= 23,689 sf Storage= 127,049 cf (116,450 cf above start)

Plug-Flow detention time= 556.2 min calculated for 0.34 af (15% of inflow)

Center-of-Mass det. time= 261.2 min (1,104.7 - 843.5)

Volume	Invert	Avail.Storage	Storage Description			
#1	363.00'	127,049 cf	Custom Stage Data (Prismatic) Listed below (Recalc)			
#2	#2 369.00' 0 cf		Sediment Forebay (No Storage) (Prismatic) Listed below (Recalc)			
			870 cf Overall x 0.0% Voids			
		127 0/0 cf	Total Available Storage			

127,049 cf Total Available Storage

Type III 24-hr 10-YEAR Rainfall=4.47"

Prepared by Keach-Nordstrom Associates, Inc.

Revised March 4, 2024 Printed 3/4/2024

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 31

Elevation	Surf.Area	Inc.Store	Inc.Store Cum.Store			
(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)			
363.00	520	0	sal a sa O			
364.00	2,109	1,315	1,315			
365.00	4,541	3,325	4,640			
366.00	7,378	5,960	10,599			
368.00	10,543	17,921	28,520			
370.00	14,007	24,550	53,070			
371.00	15,844	14,926	67,996			
372.00	18,773	17,309	85,304			
374.00	22,972	41,745	127,049			
Elevation	Surf.Area	Inc.Store	Cum.Store			
(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)			
369.00	153	0	0			
371.00	717	870	870			

Device	Routing	Invert	Outlet Devices
#1	Primary	364.57'	15.0" Round Culvert
	,		L= 106.9' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 364.57' / 356.50' S= 0.0755 '/' Cc= 0.900
			n= 0.013, Flow Area= 1.23 sf
#2	Device 1	366.00'	2.0" Vert. Orifice C= 0.600
#3	Device 1	369.45'	2.5" Vert. Orifice C= 0.600
#4	Device 1	371.50'	3.0" Vert. Orifice C= 0.600
#5	Device 1	373.70'	48.0" x 48.0" Horiz. Grate C= 0.600
			Limited to weir flow at low heads
#6	Secondary	373.90'	5.0' long x 95.0' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60
			Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63

Primary OutFlow Max=0.66 cfs @ 19.16 hrs HW=372.01' TW=350.58' (Dynamic Tailwater)
1=Culvert (Passes 0.66 cfs of 15.43 cfs potential flow)

2=Orifice (Orifice Controls 0.26 cfs @ 11.73 fps)

-3=Orifice (Orifice Controls 0.26 cfs @ 7.55 fps)

-4=Orifice (Orifice Controls 0.15 cfs @ 3.00 fps)

-5=Grate (Controls 0.00 cfs)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=366.00' TW=350.20' (Dynamic Tailwater) 6=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Link A: BARRETTS HILL CROSSING

Inflow Area = 41.489 ac, 9.34% Impervious, Inflow Depth > 1.03" for 10-YEAR event

Inflow = 19.22 cfs @ 12.51 hrs, Volume= 3.57 af

Primary = 19.22 cfs @ 12.51 hrs, Volume= 3.57 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

2304141-POST DEVELOPMENT_REV1

Type III 24-hr 10-YEAR Rainfall=4.47"

Prepared by Keach-Nordstrom Associates. Inc.

Revised March 4, 2024 Printed 3/4/2024

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 32

Summary for Link B: LOT 57

Inflow Area =

1.835 ac, 11.18% Impervious, Inflow Depth > 2.03" for 10-YEAR event

Inflow

2.92 cfs @ 12.28 hrs, Volume=

0.31 af

Primary

2.92 cfs @ 12.28 hrs, Volume=

0.31 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Summary for Link C: LOT 11

Inflow Area =

2.507 ac,

5.17% Impervious, Inflow Depth > 1.81" for 10-YEAR event 0.38 af

Inflow **Primary**

3.43 cfs @ 12.30 hrs, Volume= 3.43 cfs @ 12.30 hrs, Volume=

0.38 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

March 5, 2024

Mr. Jay Minkarah Acting Town Planner Town of Hudson 12 School Street Hudson, NH 03051

Subject:

Town of Hudson Planning Board Review

Barrett Hill Subdivision Plan

Tax Map 151 Lot 59

KNA Project No. 23-0414-1

Dear Mr. Minkarah:

Our office is in receipt of four review letters, dated December 11, 18 & 19 of 2023. Based on the comments from town staff, we have made the required modifications and attached revisions for final review. A response to each comment has been provided below.

1. Engineering

a. Applicant shall provide driveway plan and profile for all proposed lots.

The plans have been revised to include potential lot development including driveway and condex locations as well as driveway and lot grading. The layout shown on the Grading & Drainage Plan is likely what will be constructed; however it is still subject to change slightly due to conditions present in the field. Note #6 on Sheet 10 has been modified accordingly.

b. Applicant shall evaluate further improvement on Barretts Hill related to sight distance downhill site, including Barretts Hill Road relocation.

A sight distance easement has been provided over proposed Lot 59-1 to ensure adequate sight distance can be maintained at all times.

c. Applicant shall provide a bond for onsite and offsite improvements within the existing and proposed Right of Way, prior to recording the plans.

Note #26 on Sheet 1 has been added to this effect.

d. Applicant shall transfer the ownership of the road to the town and to the satisfaction of the Town prior to receiving the last Certificate of Occupancy of the last lot.

Note #27 on Sheet 1 has been added to this effect.

2. DPW

a. Station 5+25.00 add additional catch basin to opposite side of road.

An additional catch basin has been added as requested.

b. Station 2+00.00 add additional catch basin to opposite side of road.

An additional catch basin has been added as requested.

c. DMH #17 should have a storm grate instead of a manhole cover since it is on the curb line.

DMH#17 is located 20 feet from the high point of the roadway and is solely designed to provide a direction change for the drain pipe. A typical Type B frame and grate is not required in this location as the structure would receive almost no flow due to its vicinity to the high point of the roadway. The newly added CB #110, at Station 5+25, is in a much better location to intake stormwater runoff from said high point.

d. All drainage on Barretts Hill Rd. shall be changed to enclosed drainage with catch basins until it outfalls at the existing drainage easement at 55 Barretts Hill Rd.

The drainage design on Barretts Hill Road has been modified as requested according to recommendations from the Town Engineer and the DPW.

3. Zoning

a. Are there going to be Decks on the houses.

Patios/decks are proposed for each condex unit. All patios or decks will fall within the required setbacks.

b. Proposed lot 151 lot 59-1 Looks really close to the front setback.

The proposed lot adheres to all zoning requirements.

c. Are there garages and where are the driveways going to be located.

Each condex unit will have two garages either at grade or drive-under. The Grading & Drainage Plan has been revised to include future potential lot development.

d. 151 lot 59-2 is this going to be a shared driveway with lot 59—8.

The driveway locations are now shown on the plan to provide proof of concept. All

Civil Engineering

Land Surveying

Landscape Architecture

lots will be accessed by their own individual driveways.

4. Fire

a. The State Adopted Fire Code, NFPA 1 requires a water supply of 1000 gallons per minute for one hour for firefighting. A 60,000 gallon cistern is required. Redesign the site plan showing the placement of the 60,000 gallon cistern. Provide a cistern drawing that meets Hudson Fire Department requirements. The cistern shall include 6"metal suction pipe, to 6" female, to 6" to 5" male reducer to be National Hose Thread. There shall also be a cap provided for the 6" to 5" reducer that will be left connected to the riser. Locks shall be provided for all access hatches. The cistern shall have a water level indicator.

A 30,000-gallon cistern is now proposed on Lot 59-2 with an associated easement perdirection of the Hudson Fire Department. Construction details are located on Sheet 18.

b. The developer has the option of installing an NFPA 130 sprinkler systems in each dwelling in lieu of the cistern.

Sprinklers are not proposed as a cistern has been provided.

c. The proposed subdivision falls inside the 1000 foot distance from the center of the Tennessee Gas Pipeline. Advanced notification is required from the gas company per State of NH RSA 674:75. The advanced notification shall be sent to KMEncroachmentsNorth@kindermorgan.com. This is required prior to any site work, blasting, or building permits being issued. Once approval has been received this approval shall be sent to Nashua Regional Planning Commission email at outreach@nashuarpc.org. All approval documentation shall be submitted with the building permit applications. No permits will be issued without all approvals.

The applicant has noted this information. The gas company has been notified.

If you have any questions or comments, please reach out by phone at (603) 627-2881 or by email at pmadsen@keachnordstrom.com.

Respectfully,

Peter Madsen, EIT

Keach Nordstrom Associates, Inc. 10 Commerce Park North, Suite 3

Bedford, NH 03110

Civil Engineering

Land Surveying

Landscape Architecture

Alteration of Terrain Application

Barrett Hill Subdivision

Tax Map 151, Lot 59 75 Barretts Hill Road Hudson, New Hampshire

12/13/2023

KNA Project No. 23-0414-1



Prepared For:

Barrett Hill, LLC

21 Continental Blvd. Door #4 Merrimack, NH 03110

Prepared By:

Keach-Nordstrom Associates, Inc. 10 Commerce Park North, Suite 3 Bedford, New Hampshire 03110

(603) 627-2881 (603) 627-2915 (fax)



Table of Contents

- 1. SIGNED OWNER/APPLICANT AFFIDAVIT
- 2. AOT APPLICATION
- 3. AOT APPLICATION CHECKLIST
- 4. COPY OF AOT APPLICATION CHECK
- 5. MUNICIPAL SUBMISSION: TOWN OF HUDSON
- 6. USGS LOCATION MAP
- 7. PROJECT NARRATIVE
- 8. SURFACE WATER IMPAIRMENTS
- 9. SCREENING LAYERS
- 10. NEW HAMPSHIRE NATURAL HERITAGE INVENTORY LETTER
- 11. WEB SOIL SURVEY
- 12. AERIAL
- 13. SITE PHOTOGRAPHS
- 14. EXTREME PRECIPITATION TABLE
- 15. BMP WORKSHEETS
- 16. HYDROCAD DRAINAGE ANALYSIS
 PRE-DEVELOPMENT MODEL
 POST-DEVELOPMENT MODEL
- 17. RIP RAP APRON CALCULATIONS
- 18. SITE SPECIFIC SOIL SURVEY REPORT
- 19. OPERATION and MAINTENANCE PLAN with CHECKLIST
- 20. APPENDICES
 RESIDENTIAL SUBDIVISION PLAN (22"x34" Colorless)
 PRE-DEVELOPMENT DRAIN AREA PLAN (22"x34" Colorless)
 POST-DEVELOPMENT DRAIN AREA PLAN (22"x34" Colorless)

PRE-DEVELOPMENT DRAIN AREA PLAN (22"x34" – with Color) POST-DEVELOPMENT DRAIN AREA PLAN (22"x34" – with Color) 1. SIGNED OWNER/APPLICANT AFFIDAVIT

Owner Affidavit

I, <u>John Gargasz</u>, authorized representative of Barrett Hill, LLC and owner of the property referenced on Tax Map 151 as Lot 59, located at 75 Barretts Hill Road, Hudson, New Hampshire, hereby verify that I have authorized Keach-Nordstrom Associates, Inc. to submit on my behalf, any and all applicable State and local permit applications as they pertain to improvements on said property.

Additionally, I authorize Keach-Nordstrom Associates, Inc. to aid in the representation of these applications throughout the approval process.

Printed Name of Owner:

John Gargasz

Address of Owner:

21 Continental Boulevard, Door #4

Merrimack, NH 03054

Date:

12/11/2023

2. AOT APPLICATION



ALTERATION OF TERRAIN PERMIT APPLICATION

Attachment "F"

Water Division/ Alteration of Terrain Bureau/ Land Resources Management Check the Status of your Application: www.des.nh.gov/onestop

RSA/ Rule: RSA 485-A:17, Env-Wq 1500

				file Norr	Number:	
Administrative Use	Administrative	Administrat	ive E	Check N	6. 1 10 10 17 1	
Only:	Use Only	Use Only		Amount		
				Initials.		
1. APPLICANT INFORMATION (INTE	NDED PERMIT HOLDER)					
Applicant Name: Barrett Hill, LLC		Contact Name: John	Gargasz			
Email: johngargasz@gmail.com		Daytime Telephone:	(603) 320-512	23		
Mailing Address: 21 Continental Blv	rd. Door #4					
Town/City: Merrimack			State: NH		Zip Code: 03054	
2. APPLICANT'S AGENT INFORMATI	ON If none, check here:	\boxtimes				
Business Name:		Contact Name:				
Email:		Daytime Telephone:	e Telephone:			
Address:		ıA				
Town/City:			State:		Zip Code:	
3. PROPERTY OWNER INFORMATIO	N (IF DIFFERENT FROM APPLIC	CANT)	*		,	
Applicant Name:		Contact Name:				
Email:		Daytime Telephone:	ephone:			
Mailing Address:		A				
Town/City:			State:		Zip Code:	
4. PROPERTY OWNER'S AGENT INFO	DRMATION If none, ch	eck here: 🔀			1	
Business Name:		Contact Name:				
Email:		Daytime Telephone:	e:			
Address:						
Town/City:			State:		Zip Code:	
5. CONSULTANT INFORMATION	If none, check here:					
Engineering Firm: Keach-Nordstrom	Associates, Inc.	Contact Name: Mitcl	: Mitchell Heidler			
Email: mheidler@keachnordstrom.c	Daytime Telephone:	none: (603) 627-2881				
Address: 10 Commerce Park North,	Suite 3B					
Town/City: Bedford			State: NH Zip Code: 03110			

NHDES-W-01-003					1 11711
6. PROJECT TYPE				Atta	nchment "F"
Excavation Only Residential] Commercial	Golf Co	ourse Scho	ol Municipal
Agricultural Land	l Conversion	Other	:		
7. PROJECT LOCATION INFORMATIO	N				
Project Name: Barrett Hill Subdivision	<u> </u>				
Street/Road Address: 75 Barretts Hill	Road				
Town/City: Hudson		Co	unty: Hillsboi	ough	
Tax Map: 151	Block:		Lot Numbe	er: 59	Unit:
Location Coordinates: 42.78803N, 71.	39329W	☐ Latitude/L	ongitude	□ итм	State Plane
Post-development, will the proposed pr	oject withdraw fro	om or directly disc	harge to any o	of the following? If yes	, identify the purpose.
1. Stream or Wetland			Yes	☐ Withdrawa	l Discharge
Purpose:			⊠ No		
2. Man-made pond created by impour	nding a stream or	wetland	Yes	☐ Withdrawa	l Discharge
Purpose:			⊠ No		
3. Unlined pond dug into the water ta	ble		Yes	☐ Withdrawa	Discharge
Purpose:			⊠ No		
Post-development, will the proposed pr	oject discharge to	:	-		
A surface water impaired for phosphorus		en? 🛮 No 🔲 Y	es - include in	formation to demonst	rate that project will not
 cause net increase in phosphorus a A Class A surface water or Outstanding 		P I⊠ No I	Ves - includ	e information to dem	onstrate that project will not
cause net increase in phosphorus a	-	. 23 140		e information to dem	onstrate that project will not
A lake or pond not covered previously		Yes - include in	formation to d	emonstrate that proje	ect will not cause net increase
in phosphorus in the lake or pond					
Is the project a High Load area? You If yes, specify the type of high load	_	y:			
Is the project within a Water Supply Inta	ake Protection Are	a (WSIPA)?	Yes	⊠ No	
Is the project within a Groundwater Pro	tection Area (GPA)?	Yes	⊠ No	
Will the well setbacks identified in E	nv-Wq 1508.02 be	e met?	X Yes	☐ No	
Note: Guidance document titled " <u>Using</u>				<u>eas</u> " is available online	. For more details on the
restrictions in these areas, read Chapter			No No		
Is any part of the property within the 10 If yes: Cut volume: cubic			⊠ NO		
Fill volume: cubic					
Project IS within ¼ mile of a design		Name of River:			
Project is NOT within ¼ mile of a design		Name of River.			
Project IS within a Coastal/Great Bay Region community - include info required by Env-Wq 1503.08(I) if applicable					
Project is NOT within a Coastal/Gr				,, ,	approance
8. BRIEF PROJECT DESCRIPTION (PLEA			:D")		
The project purpose is to develop the site to accommadate 13 single-family residential lots.					
		o 10 0B.c 10,	Testa et tela i te		
O IF ADDITION TO BE SEED TO THE SEED TO TH	DI OTA DEPARE				
9. IF APPLICABLE, DESCRIBE ANY WO	KK STARTED PRIC	JK TO RECEIVING	PERMIT		
N/A					

10. ADDITIONAL REQUIRED INFORMATION				Attachment "F"	
Date a copy of the application was sent to (Attach proof of delivery)	o the municipality as requi	red by Env-W	/q 1503.05	5(e) ¹ : <u>12/14/2023.</u>	
B. Date a copy of the application was sent to (Attach proof of delivery)	o the local river advisory co	ommittee if r	equired by	y Env-Wq 1503.05(e) ² :/	
C. Type of plan required: Land Conversion Detailed Development Excavation, Grading & Reclamation Steep Slope					
D. Additional plans required: X Stormwate	er Drainage & Hydrologic S	oil Groups	Source	Control Chloride Management	
E. Total area of disturbance: 209,800 squar	e feet				
 F. Additional impervious cover as a result of coverage). Total final impervious cover: 40,697 square 		e feet (use tl	he "-" sym	bol to indicate a net reduction in impervious	
G. Total undisturbed cover: 1,330,450 squa	re feet				
H. Number of lots proposed: 13					
I. Total length of roadway: 1,213 linear fee	t				
J. Name(s) of receiving water(s):					
				n application has been filed and is pending, or proval letter number, as applicable.	
K. Identify all other NHDES permits required the required approval has been issued pro	ovide the permit number, i	registration o			
K. Identify all other NHDES permits required		registration o		proval letter number, as applicable.	
K. Identify all other NHDES permits required the required approval has been issued pro	Application File	registration o	date, or ap	proval letter number, as applicable. Status	
K. Identify all other NHDES permits required the required approval has been issued pro	Application File Yes No	ed?	date, or ap	proval letter number, as applicable. Status If Issued:	
K. Identify all other NHDES permits required the required approval has been issued protection. Type of Approval 1. Water Supply Approval	Application File Yes No Yes No	ed?	date, or ap	proval letter number, as applicable. Status If Issued: Permit number:	
 K. Identify all other NHDES permits required the required approval has been issued provided by the required approval. Type of Approval. 1. Water Supply Approval. 2. Wetlands Permit. 	Application File Yes No Yes No Yes No	ed? N/A	date, or ap	Permit number: Permit number:	
K. Identify all other NHDES permits required the required approval has been issued protection. Type of Approval 1. Water Supply Approval 2. Wetlands Permit 3. Shoreland Permit	Application File Yes No Yes No Yes No Yes No Yes No Yes No	ed? N/A N/A N/A	date, or ap	Permit number: Permit number: Permit number: Permit number:	
K. Identify all other NHDES permits required the required approval has been issued provided by the required approval. 1. Water Supply Approval. 2. Wetlands Permit. 3. Shoreland Permit. 4. UIC Registration.	Application File Yes No	ed? N/A N/A N/A N/A	Pending	Permit number: Permit number: Permit number: Permit number: Permit number: Registration date:	
K. Identify all other NHDES permits required the required approval has been issued provided by the required approval. 1. Water Supply Approval. 2. Wetlands Permit. 3. Shoreland Permit. 4. UIC Registration. 5. Large/Small Community Well Approval.	Application File Yes No	ed? N/A N/A N/A N/A N/A	Pending	Permit number: Permit number: Permit number: Permit number: Permit number: Approval letter date:	
K. Identify all other NHDES permits required the required approval has been issued protection. Type of Approval 1. Water Supply Approval 2. Wetlands Permit 3. Shoreland Permit 4. UIC Registration 5. Large/Small Community Well Approval 6. Large Groundwater Withdrawal Permit	Application File Yes No	registration of ed? N/A N/A N/A N/A N/A N/A N/A	Pending	Permit number: Registration date: Approval letter date: Permit number: Permit number:	

O. Will blasting of bedrock be required? Yes No If yes, estimated quantity of blast rock: _____ cubic yards If yes, standard blasting BMP notes must be placed on the plans, available at:

http://des.nh.gov/organization/commissioner/pip/publications/wd/documents/wd-10-12.pdf

NOTE: If greater than 5,000 cubic yards of blast rock will be generated, a groundwater monitoring program must be developed and submitted to NHDES. Contact AOT staff for additional detail.

N. Did the applicant/applicant's agent have a pre-application meeting with AOT staff?

Yes No
If yes, name of staff member:

¹ Env-Wq 1503.05(c)(6), requires proof that a completed application form, checklist, plans and specifications, and all other supporting materials have been sent or delivered to the governing body of each municipality in which the project is proposed.

² Env-Wq 1503.05(c)(6), requires proof that a completed application form, checklist, plans and specifications, and all other supporting materials have been sent or delivered to the Local River Advisory Committee, if the project is within ¼ mile of a designated river.

One set of design plans on 34 - 36" by 22 - 24" white paper (see Application Checklist for details)

Pre & post-development color coded soil plans on 11" x 17" (see Application Checklist for details)

Pre & post-development drainage area plans on 34 - 36" by 22 - 24" white paper (see Application Checklist for details)

100-YEAR FLOODPLAIN REPORT:

All information required in Env-Wq 1503.09, submitted as a separate report.

ADDITIONAL INFORMATION RE: NUTRIENTS, CLIMATE

See Checklist for Details

REVIEW APPLICATION FOR COMPLETENESS & CONFIRM INFORMATION LISTED ON THE APPLICATION IS INCLUDED WITH SUBMITTAL.

12. REQUIRED SIGNATURES

Attachment "F"

By initialing here, I acknowledge that I am required by Env-Wq 1503.20(e) to submit a copy of all approved documents to the department in PDF format on a CD within one week after permit approval.

By signing below, I certify that:

- The information contained in or otherwise submitted with this application is true, complete, and not misleading to the best of my knowledge and belief;
- I understand that the submission of false, incomplete, or misleading information constitutes grounds for the department to deny the application, revoke any permit that is granted based on the information, and/or refer the matter to the board of professional engineers established by RSA 310-A:3 if I am a professional engineer; and

• I understand that I am subject to the penalties specified in New Hampshire law for falsification in official matters, currently RSA 641.					

3. AOT APPLICATION CHECKLIST

ATTACHMENT A:

ALTERATION OF TERRAIN PERMIT APPLICATION CHECKLIST

Check the box to indicate the item has been provided or provide an explanation why the item does not apply.

DESIGN PLANS
Plans printed on 34 - 36" by 22 - 24" white paper
☑ PE stamp
Wetland delineation
☐ Temporary erosion control measures
Treatment for all stormwater runoff from impervious surfaces such as roadways (including gravel roadways), parking areas, and non-residential roof runoff. Guidance on treatment BMPs can be found in Volume 2, Chapter 4 of the NH Stormwater Management Manual.
Pre-existing 2-foot contours
Proposed 2-foot contours
☐ Drainage easements protecting the drainage/treatment structures
Compliance with the Wetlands Bureau, RSA 482- A http://des.nh.gov/organization/divisions/water/wetlands/index.htm . Note that artificial detention in wetlands is not allowed.
Compliance with the Comprehensive Shoreland Protection Act, RSA 483-B. http://des.nh.gov/organization/divisions/water/wetlands/cspa
Benches. Benching is needed if you have more than 20 feet change in elevation on a 2:1 slope, 30 feet change in elevation on a 3:1 slope, 40 feet change in elevation on a 4:1 slope.
Check to see if any proposed ponds need state Dam permits. http://des.nh.gov/organization/divisions/water/dam/documents/damdef.pdf
DETAILS
☑ Typical roadway x-section
☑ Detention basin with inverts noted on the outlet structure
Stone berm level spreader
Outlet protection – riprap aprons
🔀 A general installation detail for an erosion control blanket
☑ Silt fences or mulch berm
Storm drain inlet protection. Note that since hay bales must be embedded 4 inches into the ground, they are not to be used on hard surfaces such as pavement.
Hay bale barriers
Stone check dams
Gravel construction exit
☑ Temporary sediment trap
The treatment BMP's proposed
Any innovative RMP's proposed

NHDES-W-01-003

CONSTRUCTION SEQUENCE/EROSION CONTROL

Attachment "F"

- Note that the project is to be managed in a manner that meets the requirements and intent of RSA 430:53 and Chapter Agr 3800 relative to invasive species. Note that perimeter controls shall be installed prior to earth moving operations. Note that temporary water diversion (swales, basins, etc) must be used as necessary until areas are stabilized. Note that ponds and swales shall be installed early on in the construction sequence (before rough grading the site). Note that all ditches and swales shall be stabilized prior to directing runoff to them. igwedge Note that all roadways and parking lots shall be stabilized within 72 hours of achieving finished grade. Note that all cut and fill slopes shall be seeded/loamed within 72 hours of achieving finished grade Note that all erosion controls shall be inspected weekly AND after every half-inch of rainfall. Note the limits on the open area allowed, see Env-Wg 1505.02 for detailed information. Example note: The smallest practical area shall be disturbed during construction, but in no case shall exceed 5 acres at any one time before disturbed areas are stabilized. Note the definition of the word "stable" Example note: An area shall be considered stable if one of the following has occurred: Base course gravels have been installed in areas to be paved. A minimum of 85 percent vegetated growth has been established. A minimum of 3 inches of non-erosive material such stone or riprap has been installed. Or, erosion control blankets have been properly installed.
- Note the limit of time an area may be exposed

Example note: All areas shall be stabilized within 45 days of initial disturbance.

- Provide temporary and permanent seeding specifications. (Reed canary grass is listed in the Green Book; however, this is a problematic species according to the Wetlands Bureau and therefore should not be specified)
- Provide winter construction notes that meet or exceed our standards.

Standard Winter Notes:

- All proposed vegetated areas that do not exhibit a minimum of 85 percent vegetative growth by October 15, or which are disturbed after October 15, shall be stabilized by seeding and installing erosion control blankets on slopes greater than 3:1, and seeding and placing 3 to 4 tons of mulch per acre, secured with anchored netting, elsewhere. The installation of erosion control blankets or mulch and netting shall not occur over accumulated snow or on frozen ground and shall be completed in advance of thaw or spring melt events.
- All ditches or swales which do not exhibit a minimum of 85 percent vegetative growth by October 15, or which are disturbed after October 15, shall be stabilized temporarily with stone or erosion control blankets appropriate for the design flow conditions.
- After October 15, incomplete road or parking surfaces, where work has stopped for the winter season, shall be protected with a minimum of 3 inches of crushed gravel per NHDOT item 304.3.
- Note at the end of the construction sequence that "Lot disturbance, other than that shown on the approved plans, shall not commence until after the roadway has the base course to design elevation and the associated drainage is complete and stable." – This note is applicable to single/duplex family subdivisions, when lot development is not part of the permit.

DRAINAGE ANALYSES

Please double-side 8 $\frac{1}{2}$ " × 11" sheets where possible but, do not reduce the text such that more than one Attachment . "F"
□ PE stamp
Rainfall amount obtained from the Northeast Regional Climate Center- http://precip.eas.cornell.edu/ . Include extreme precipitation table as obtained from the above referenced website.
☑ Drainage analyses, in the following order:
Pre-development analysis: Drainage diagram.
Pre-development analysis: Area Listing and Soil Listing.
Pre-development analysis: Node listing 1-year (if applicable), 2-year, 10-year and 50-year.
Pre-development analysis: Full summary of the 10-year storm.
Post-development analysis: Drainage diagram.
Post-development analysis: Area Listing and Soil Listing.
Post-development analysis: Node listing for the 2-year, 10-year and 50-year.
Post-development analysis: Full summary of the 10-year storm.
Review the Area Listing and Soil Listing reports
Hydrologic soil groups (HSG) match the HSGs on the soil maps provided.
There is the same or less HSG A soil area after development (check for each HSG).
There is the same or less "woods" cover in the post-development.
Undeveloped land was assumed to be in "good" condition.
The amount of impervious cover in the analyses is correct.
Note: A good check is to subtract the total impervious area used in the pre analysis from the total impervious area used in the post-analysi For residential projects without demolition occurring, a good check is to take this change in impervious area, subtract out the roadway and divide the remaining by the number of houses/units proposed. Do these numbers make sense?
Check the storage input used to model the ponds.
Check to see if the artificial berms pass the 50-year storm, i.e., make sure the constructed berms on ponds are not overtopped.
Check the outlet structure proposed and make sure it matches that modeled.
Check to see if the total areas in the pre and post analyses are same.
Confirm the correct NRCS storm type was modeled (Coos, Carroll & Grafton counties are Type II, all others Type III).
. PRE- AND POST-DEVELOPMENT DRAINAGE AREA PLANS
Plans printed on 34 - 36" by 22 - 24" on white paper.
Submit these plans separate from the soil plans.
🔀 A north arrow.
☐ A scale.
☐ Labeled subcatchments, reaches and ponds.
☑ Tc lines.
A clear delineation of the subcatchment boundaries.
☐ Roadway station numbers.
☐ Culverts and other conveyance structures.

PRE AND POST-DEVELOPMENT COLOR-CODED SOIL PLANS

NHDES-W-01-003

NHDES-W-01-003 11" × 17"sheets suitable, as long as it is readable.	Attachment "F"
Submit these plans separate from the drainage area plans.	
🔀 A north arrow.	
☑ A scale.	
Name of the soil scientist who performed the survey and date the soil survey took place.	
2-foot contours (5-foot contours if application is for a gravel pit) as well as other surveyed	features.
Delineation of the soil boundaries and wetland boundaries.	
Delineation of the subcatchment boundaries.	
Soil series symbols (e.g., 26).	
igtie A key or legend which identifies each soil series symbol and its associated soil series name	(e.g., 26 = Windsor).
The hydrologic soil group color coding (A = Green, B = yellow, C= orange, D=red, Water=bloom	ue, & Impervious = gray).
Please note that excavation projects (e.g., gravel pits) have similar requirements to that abo exceptions/additions:	ove, however the following are common
☐ Drainage report is not needed if site does not have off-site flow.	
5 foot contours allowed rather than 2 foot.	
No PE stamp needed on the plans.	
Add a note to the plans that the applicant must submit to the Department of Environment and revised plans documenting the project status every five years from the date of the Alto	
Add reclamation notes.	
See NRCS publication titled: Vegetating New Hampshire Sand and Gravel Pits for a good resounttp://des.nh.gov/organization/divisions/water/aot/categories/publications.	rce, it is posted online at:
ADDITIONAL INFORMATION RE: NUTRIENTS, CLIMATE	
If project will discharge stormwater to a surface water impaired for phosphorus and/or nit that project will not cause net increase in phosphorus and/or nitrogen.	rogen, include information to demonstrate
If project will discharge stormwater to a Class A surface water or Outstanding Resource War project will not cause net increase in phosphorus and/or nitrogen.	ater, include information to demonstrate that
If project will discharge stormwater to a lake or pond not covered previously, include infor cause net increase in phosphorus in the lake or pond.	mation to demonstrate that project will not

☐ If project is within a Coastal/Great Bay Region community, include info required by Env-Wq 1503.08(I) if applicable.

4. COPY OF AOT APPLICATION CHECK

MENT HAS A COLORED BACKGROUND ON WHITE PAPER AND ORIGINAL DOCUMENT SECURITY SCREEN ON BACK WITH PADLOCK SECURITY ICON. 1004 Barrett Hill, LLC **Bank of America** 21 Continental Blvd Merrimack, NH 03054 54-49/114 12/11/2023 PAY TO THE Treasurer, State of NH **4,375.00 ORDER OF Security features included. DOLLARS Treasurer, State of NH a MEMO **AOT APPLICATION FEE** #001004# #011400495# 388004130685#

Original Amt.

4,375.00

Balance Due

4,375.00

Barrett Hill, LLC

Date

12/11/2023 Bill

Treasurer, State of NH

Type Reference

1004

Payment

4,375.00

4,375.00

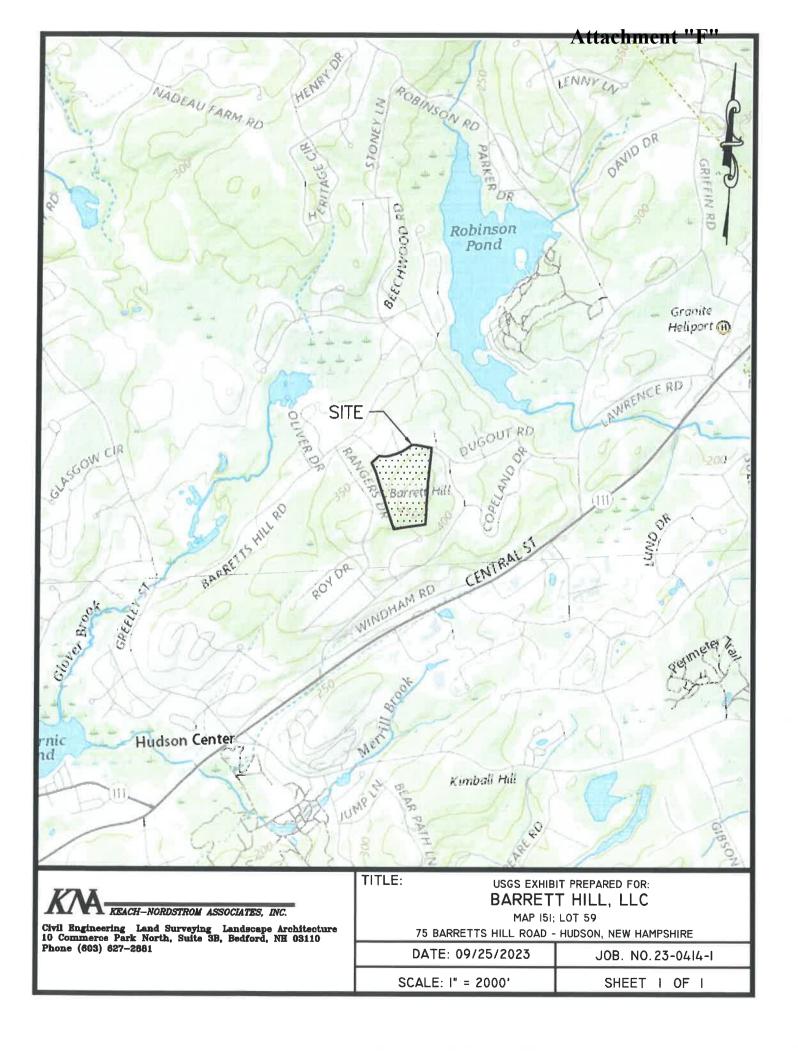
12/11/2023

Check Amount

Discount

5. MUNICIPAL SUBMISSION: TOWN OF HUDSON

6. USGS LOCATION MAP



7. PROJECT NARRATIVE

. INTRODUCTION

A. Project Description

The project proposes to develop the existing property to accommodate a twelve (13) lot open space residential subdivision with an accompanying 1,200 linear foot public roadway. Site work includes clearing and grubbing the site, construction of the new roadway, individual lot development, and installation of a closed drainage system, pocket pond, and other associated stormwater management provisions.

B. Existing Site Conditions

The subject property consists of one lot, approximately 35.36 acres in total area, and is located at 75 Barretts Hill Road in Hudson's General 1 (G-1) Zoning District. The lot is currently undeveloped. The lot is comprised mostly of woodlands with a utility line easement on the back half of the lot. The subject lot is bordered by Barretts Hill Road to the north and residential lots to the east, west, and south.

A site-specific soil survey, performed onsite by GZA GeoEnvironmental, Inc. on October 4, 2023, listed the following soils as predominant soil types found onsite. Montauk classified as C soils with slopes ranging from 3-25%, Scituate classified as C soils with slopes ranging from 3-25%. The soil types are classified as HSG 'C' and therefore, C soils were used in drainage computations. According to the National Resources Conservation Service (NRCS) soil mapping, the remaining subcatchment areas consist of, Canton fine loamy sand with slopes ranging from 0-15%, Canton fine loamy sand, very stony with slopes ranging from 8-15%, Scarboro stony mucky loamy sand, Scituate fine sandy loam with slopes ranging from 3-8%, and Scituate stony fine sandy loam with slopes ranging from 3-8%. It is important to note that both Scituate and Montauk soils have infiltration rates below 0.50 in/hr, therefore, according to Env-Wq 1508.07(a)(1), infiltration is not permitted on this site.

II. STORM DRAINAGE ANALYSIS & DESIGN

A. Methodology

In accordance with the provisions of the NHDES, the Town of Hudson, and generally accepted engineering practice, the 2-year, 2-year frozen, 10-year, 25-year, and 50-year frequency storms have each been used in the various aspects of analysis and design of stormwater management considerations for the subject site. The closed drainage system has been designed for the 10-year frequency storm and the proposed pocket pond has been designed to not overtop in the 50-year frequency storm.

KNA utilizes HydroCAD version 10.0 to analyze both pre and post-development watershed characteristics. This computer software system is based largely on hydrology techniques (TR-20) developed by the Soil Conservation Service (now the Natural Resources Conservation Service). In addition, the software derives Time of Concentration values using the methodology contained within USDA-S.C.S. publication <u>Urban Hydrology for Small</u> Watersheds Technical Release No. 55 (TR 55).

All design and analysis calculations performed using the referenced methodologies are attached to this report. The minimum time of concentrations used for the analysis is 6 minutes. These calculations document each catchment area, a breakdown of surface type, time of concentration, rainfall intensity, peak discharge volume, Manning's "n" value, peak

velocity, and other descriptive design data for each watershed and pipe segment evaluated. In addition, the "Pre/Post Development Drainage Area Plans" graphically define and illustrate the extent of each watershed or catchment area investigated.

B. Pre-Development Drainage Conditions

In the pre-development scenario, three (3) points of analysis (POA) have been identified as the appropriate points to compare pre vs. post development rates of stormwater discharge. These points of analysis reflect the main discharge points of the site and were analyzed to show the impact from the proposed improvements.

The pre-development drainage model's POA's are further described as follows:

Link A	Barretts Hill Crossing
Link B	Lot 57
Link C	Lot 11

The property mainly slopes from southeast to northwest directing runoff to Barretts Hill Road and neighboring lots on the western side of the subject property. Runoff on the northeastern side of the property flows to Barretts Hill Road and is collected by an existing catch basin in the roadway before discharging to Lot 11. Runoff generated on the northwestern portion of the subject property flows to a different existing catch basin on Barretts Hill Road which discharges to Lot 57. Runoff generated from the western portion of the property ultimately flows to an existing drop inlet located at the bottom of Barretts Hill Road. This crossing is identified as Link A and directs runoff under the road towards an existing stream.

C. Post-Development Drainage Conditions:

The same three POA's that were identified in the pre-development scenario have been analyzed in the post-development scenario.

Overall, the design has maintained the drainage patterns to mimic the pre-development conditions. Stormwater will continue to discharge to the same points of analysis identified in the pre-development scenario. The improvements, however, also provide stormwater treatment for the new impervious areas created for the proposed development. These new impervious areas include the roadway, sidewalks, potential future driveways, and a 2,400 sf approximation for residential homes dedicated to each lot.

The proposed stormwater management system utilizes both open and closed drainage practices for the collection, detention, and treatment of runoff. Runoff generated from the entirety of the proposed roadway and the eastern portion of the property is collected by a series of deep sump catch basins and is piped to a proposed pocket pond situated in the northwest corner of the lot. This system is designed to provide stormwater treatment and detention for the site and will not overtop during a 50-year storm event. The pond features an overflow structure which directs flow to a proposed rip rap lined roadside swale along Barretts Hill Road. Runoff is conveyed down this swale and through two separate driveway culverts before it is collected by the existing drop inlet at the bottom of the hill and ultimately discharged to Link A. As previously stated, infiltration is not permitted on this site and has not been included in the drainage analysis.

The peak stormwater runoff rates and volume discharges for the specific storm frequencies are presented and analyzed in the subsequent summary section of this report (Tables 1 & 2). For a more visual description of the information presented in this section, please refer to the attached "Post-Development Drainage Areas Plan" attached in the appendix of this report.

D. Summary:

The subject site complies with both the Town of Hudson Stormwater Management and Erosion Control Regulations and NHDES Regulations Env-wq 1500 in regard to stormwater treatment and groundwater recharge volume. Proposed stormwater best management practices (BMP) are designed in accordance with the New Hampshire Stormwater Manual Volume 2: Post-Construction Best Management Practices Selection and Design and BMP worksheets provided by the New Hampshire Department of Environmental Services. In addition, stormwater discharges, in terms of peak rate of runoff and total volume, are consistent with the Town of Goffstown Stormwater Regulations and NHDES Regulations Env-Wq 1500. The results are reported below in Table 1 and 2.

Table 1: Peak Flow Discharge Rate

	Sit	e Pre-D	evelopn	nent vs. I	Post-De	evelopi	nent (c	fs)			
Description		'ear		Frozen		Year		Year	50-	Year	
24-hr Rainfall	2.95	in/hr	2.95	in/hr	4.47	in/hr	5.66	in/hr	6.77 in/hr		
	Pre	Post	Pre	Post	Pre	Post	Pre	Post	Pre	Post	
Α	7.79	5.83	51.93	42.48	25.69	19.97	43.08	33.99	60.85	48.31	
В	2.63	1.52	10.22	4.14	6.88	3.45	10.73	5.12	14.53	6.74	
С	2.29	1.57	8.99	4.99	6.03	3.80	9.42	5.76	12.77	7.67	

Table 1: Volume Comparison (2-Year Volume)

E PARENT	Site	Pre-De	velopme	nt vs. Po	st-Dev	elopme	ent (St	orm Vo	lume i	n Acre	-Feet)
Description	2-1	'ear		Frozen		Year		Year		Year	Comments
24-hr Rainfall	2.95	in/hr	2.95	in/hr	4.47	in/hr	5.66 in/hr		6.77 in/hr		
	Pre	Post	Pre	Post	Pre	Post	Pre	Post	Pre	Post	
Α	1.43	1.39	7.99	7.78	3.86	3.82	6.19	6.15	8.59	8.59	Complies with NHDES 1507.05, (b), (1), a
В	0.34	0.17	1.29	0.47	0.81	0.36	1.23	0.53	1.66	0.70	Complies with NHDES 1507.05, (b), (1), a & b
С	0.28	0.18	1.06	0.59	0.66	0.41	1.01	0.61	1.36	0.82	Complies with NHDES 1507.05, (b), (1), a & b

III. EROSION & SEDIMENTATION CONTROL PROVISIONS

A. Temporary Erosion Control Measures

As an integral part of the engineering design of this site, an erosion and sedimentation control plan has been developed with the intent of limiting the potential for soil loss and associated receiving water quality degradation, both during and after the construction period. As the project plans indicate, traditional temporary erosion and sedimentation control devices and practices, such as siltation fencing, erosion control blankets, and seeding have been specified for use during the construction period. In preparation of these provisions, reference was made to the New Hampshire Stormwater Manual; Volume 3: Erosion and Sediment Temporary Controls During Construction. Construction details for each temporary erosion control measure and practice specified have been added to the project plans. These plans also contain a number of erosion control notes, which are offered to the selected contractor in order to supplement the specified measures and practices to the extent practical.

B. Construction Sequence

A site-specific construction sequence sensitive to limiting soil loss due to erosion and associated water quality degradation was prepared specifically for this project and is shown on the project plans. As pointed out in the erosion control notes, it is important for the contractor to recognize that proper judgment in the implementation of work will be essential if erosion is to be limited and protection of completed work is to be realized. Moreover, any specific changes in sequence and/or field conditions affecting the ability of specific erosion control measures to adequately serve their intended purpose should be reported to this office by the contractor. Further, the contractor is encouraged to supplement specified erosion control measures during the construction period where and when in his/ her best judgment additional protection is warranted.

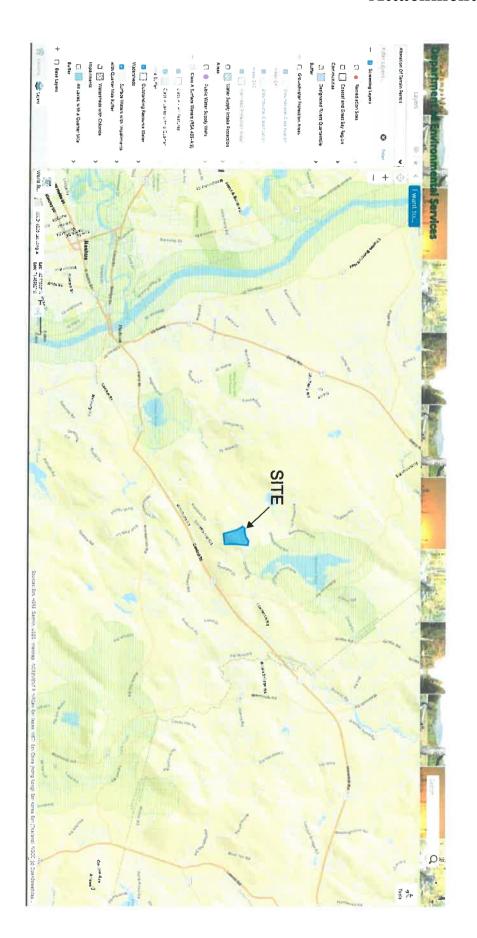
C. Permanent Erosion Control Measures

In the design of this site, consideration was given to limiting the potential for long-term erosion of completed improvements. As a result, several permanent erosion control measures were incorporated into the site design. These provisions include:

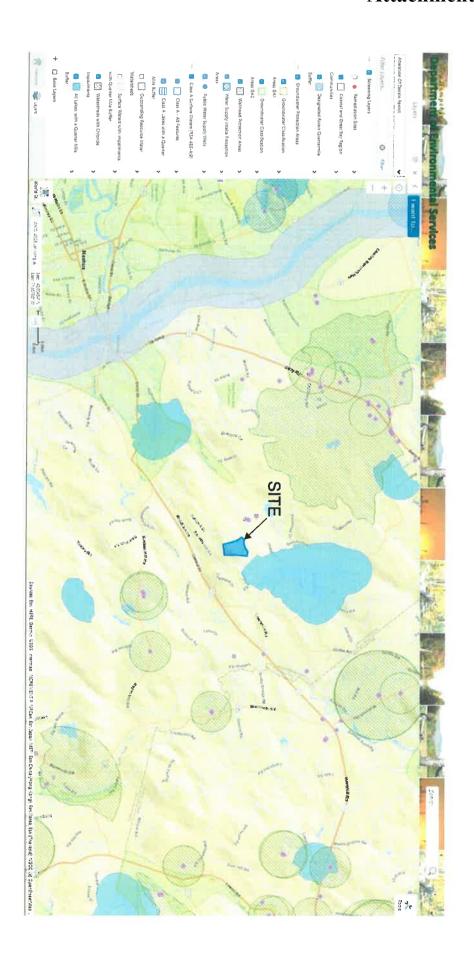
- Specification of a turf establishment schedule and seed mixture, utilizing materials and workmanship recognized as appropriate for the site conditions at hand; and
- 2) Construction of rip-rap at the outlet of the stormwater management areas; and
- 3) Design of a pocket pond to reduce runoff and volume.

8. SURFACE WATER IMPAIRMENTS

Surface Water Impairments:



9. SCREENING LAYERS



10. NEW HAMPSHIRE NATURAL HERITAGE INVENTORY LETTER



NHB DataCheck Results Letter

NH Natural Heritage Bureau

Please note: maps and NHB record pages are confidential and shall be redacted from public documents.

To: Mitchell Heidler, Keach-Nordstrom Associates Inc.

10 Commerce Park North Suite 3

Bedford, NH 03110

mheidler@keachnordstrom.com

From: NHB Review

NH Natural Heritage Bureau

Main Contact: Ashley Litwinenko - nhbreview@dncr.nh.gov

cc: NHFG Review

Date: 11/16/2023 (valid until 11/16/2024)

Re: DataCheck Review by NH Natural Heritage Bureau and NH Fish & Game

Permits: MUNICIPAL POR - Hudson, NHDES - Alteration of Terrain Permit, USEPA - Stormwater Pollution Prevention

NHB ID: NHB23-3253

Town: Hudson

Location: 75 Barretts Hill Road

Project Description: The purpose of this project is to develop a 12 lot subdivision, a singular access roadway with a cul-de-sac with closed drainage and a Stormwater Best Management Practice. The extent of the subdivision areas reach prior to the power line easement which runs across the back half of the lot.

Next Steps for Applicant:

NHB's database has been searched for records of rare species and exemplary natural communities. Please carefully read the comments and consultation requirements below.

NHB Comments: No comments at this time.

NHFG Comments: Please refer to NHFG consultation requirements below.

NHB Consultation

If this NHB DataCheck letter includes records of rare plants and/or natural communities/systems, please contact NHB and provide any requested supplementary materials by emailing nhbreview@dncr.nh.gov.

If this NHB DataCheck letter DOES NOT include any records of rare plants and/or natural communities/systems, no further consultation with NHB is required.



NHB DataCheck Results Letter

NH Natural Heritage Bureau

Please note: maps and NHB record pages are confidential and shall be redacted from public documents.

NH Fish and Game Department Consultation

If this NHB DataCheck letter DOES NOT include <u>ANY</u> wildlife species records, then, based on the information submitted, no further consultation with the NH Fish and Game Department pursuant to Fis 1004 is required.

If this NHB DataCheck letter includes a record for a threatened (T) or endangered (E) wildlife species, consultation with the New Hampshire Fish and Game Department under Fis 1004 may be required. To review the Fis 1000 rules (effective February 3, 2022), please go to https://www.wildlife.nh.gov/wildlife-and-habitat/nongame-and-endangered-species/environmental-review. All requests for consultation and submittals should be sent via email to NHFGreview@wildlife.nh.gov or can be sent by mail, and must include the NHB DataCheck results letter number and "Fis 1004 consultation request" in the subject line.

If the NHB DataCheck response letter does not include a threatened or endangered wildlife species but includes other wildlife species (e.g., Species of Special Concern), consultation under Fis 1004 is not required; however, some species are protected under other state laws or rules, so coordination with NH Fish & Game is highly recommended or may be required for certain permits. While some permitting processes are exempt from required consultation under Fis 1004 (e.g., statutory permit by notification, permit by rule, permit by notification, routine roadway registration, docking structure registration, or conditional authorization by rule), coordination with NH Fish & Game may still be required under the rules governing those specific permitting processes, and it is recommended you contact the applicable permitting agency. For projects not requiring consultation under Fis 1004, but where additional coordination with NH Fish and Game is requested, please email NHFGreview@wildlife.nh.gov, and include the NHB DataCheck results letter number and "review request" in the email subject line.

Contact NH Fish & Game at (603) 271-0467 with questions.



NHB DataCheck Results Letter

NH Natural Heritage Bureau

Please note: maps and NHB record pages are confidential and shall be redacted from public documents.

NHB Database Records:

The following record(s) have been documented in the vicinity of the proposed project. Please see the map and detailed information about the record(s) on the following pages.

Vertebrate species	State ¹	Federal	Notes
Blanding's Turtle (Emydoidea	Ε		Contact the NH Fish & Game Dept (see below).
blandingii)			

¹Codes: "E" = Endangered, "T" = Threatened, "SC" = Special Concern, "--" = an exemplary natural community, or a rare species tracked by NH Natural Heritage that has not yet been added to the official state list.

An asterisk (*) indicates that the most recent report for that occurrence was 20 or more years ago.

For all animal reviews, refer to 'IMPORTANT: NHFG Consultation' section above.

<u>Disclaimer</u>: NHB's database can only tell you of <u>known</u> occurrences that have been reported to NHFG/NHB. Known occurrences are based on information gathered by qualified biologists or members of the public, reported to our offices, and verified by NHB/NHFG.

However, many areas have never been surveyed, or have only been surveyed for certain species.

NHB recommends surveys to determine what species/natural communities are present onsite.



NHB DataCheck Results Letter

NH Natural Heritage Bureau

Please note: maps and NHB record pages are confidential and shall be redacted from public documents.

NHB23-3253 llanding's Turtle 0.4 0.8 Blanding's Turtle 1 Miles Blanding's Turtle Plant Site bounds Animal System Community

NHB DataCheck Results Letter

NH Natural Heritage Bureau

Please note: maps and NHB record pages are confidential and shall be redacted from public documents.

NHB23-3253

EOCODE:

ARAAD04010*745*NH

New Hampshire Natural Heritage Bureau - Animal Record

Blanding's Turtle (Emydoidea blandingii)

Legal Status

Conservation Status

Federal: Not listed

Global: Apparently secure but with cause for concern

State:

Listed Endangered

State: Critically imperiled due to rarity or vulnerability

Description at this Location

Conservation Rank:

Not ranked

Comments on Rank: --

Detailed Description: 2018: Area 13123M: 1 adult female observed, laying eggs. Eggs hatched on 9/4 or 9/5.

2011: Area 13123M: 1 adult female observed, laying 8-9 eggs.

General Area:

2018: Area 13123M: Property abuts a large marshy area and small stream. Quarry

beyond the marsh. 2011: Area 13123M: Nesting in residential yard.

General Comments: --

Management

Comments:

Location

Survey Site Name: Robinson Pond

Managed By:

County:

Hillsborough Town(s): Hudson

Size:

1.9 acres

Elevation:

Precision:

Within (but not necessarily restricted to) the area indicated on the map.

Directions:

2011: Area 13123M: 28 Pinewood Road, Hudson.

Dates documented

First reported:

2011-06-20

Last reported:

2018-06-13

The New Hampshire Fish & Game Department has jurisdiction over rare wildlife in New Hampshire. Please contact them at 11 Hazen Drive, Concord, NH 03301 or at (603) 271-2461.

NHB DataCheck Results Letter

NH Natural Heritage Bureau

Please note: maps and NHB record pages are confidential and shall be redacted from public documents.

NHB23-3253

EOCODE:

ARAAD04010*877*NH

New Hampshire Natural Heritage Bureau - Animal Record

Blanding's Turtle (Emydoidea blandingii)

Legal Status

Conservation Status

Federal: Not listed

Global: Apparently secure but with cause for concern State: Critically imperiled due to rarity or vulnerability

State:

Listed Endangered

Description at this Location

Conservation Rank: Not ranked

Comments on Rank: --

General Area:

Detailed Description: 2014: Area 13571: 1 adult male observed. 2014: Area 13571: Marshy area leading to small stream. Roads surrounding the marshy

General Comments: --

Management Comments:

Location

Survey Site Name: Robinson Pond

Managed By:

County:

Hillsborough Town(s): Hudson

Size:

.4 acres

Precision:

Within (but not necessarily restricted to) the area indicated on the map.

Directions:

2014: Area 13571: Greeley Street, Hudson, at crossing of Glover Brook.

Dates documented

First reported:

2014-05-02

Last reported:

Elevation:

2014-05-02

The New Hampshire Fish & Game Department has jurisdiction over rare wildlife in New Hampshire. Please contact them at 11 Hazen Drive, Concord, NH 03301 or at (603) 271-2461.

NHB DataCheck Results Letter

NH Natural Heritage Bureau

Please note: maps and NHB record pages are confidential and shall be redacted from public documents.

NHB23-3253

EOCODE:

ARAAD04010*082*NH

New Hampshire Natural Heritage Bureau - Animal Record

Blanding's Turtle (Emydoidea blandingii)

Legal Status Conservation Status

Federal: Not listed Global: Apparently secure but with cause for concern State: Listed Endangered State: Critically imperiled due to rarity or vulnerability

Description at this Location

Conservation Rank: Fair quality, condition and/or landscape context ('C' on a scale of A-D).

Comments on Rank: --

Detailed Description: 1999: Area 1636: 1 turtle. 8.5 in. hinge across plastron.

General Area: 1999: Area 1636: Swampy wetland.

General Comments: 1999: Area 1636: Hissed whenever I picked it up. Confirmed identification using

Conant's Field Guide Rep & Amphibs. Light spots on carapace not easily seen-very dark

bright yellow on chin & throat.

Management

-

Comments:

Location

Survey Site Name: Robinson Pond

Managed By:

County: Hillsborough Town(s): Hudson

Size: .0 acres Elevation:

Precision: Within (but not necessarily restricted to) the area indicated on the map.

Directions: 1999: Area 1636: Crossing Robinson Road, just north of intersection with Griffin Rd, under

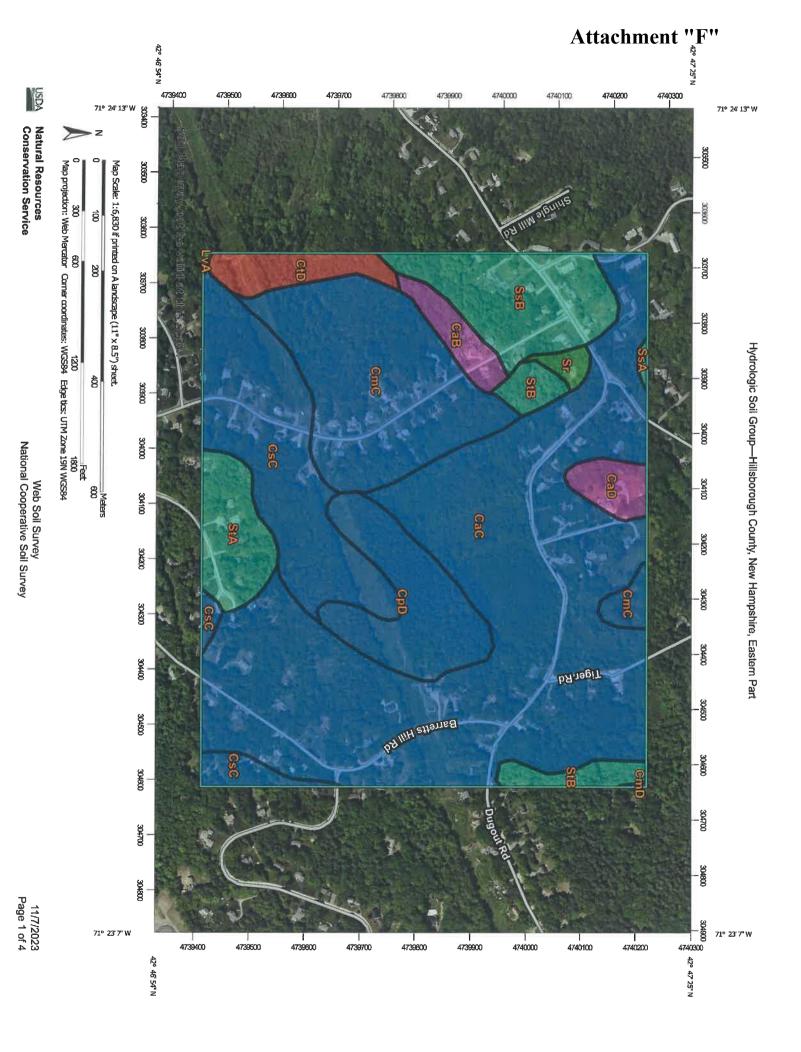
power lines.

Dates documented

First reported: 1999-09-29 Last reported: 1999-09-29

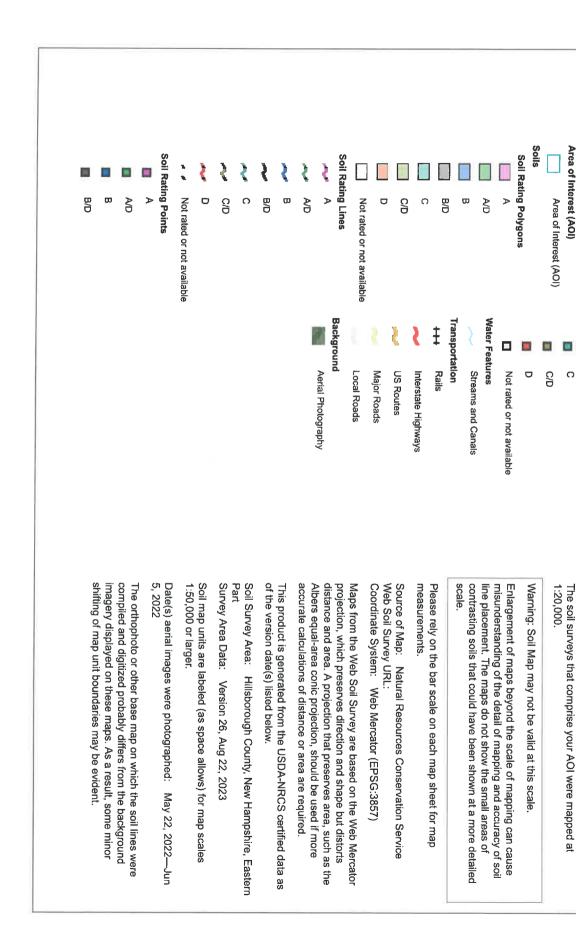
The New Hampshire Fish & Game Department has jurisdiction over rare wildlife in New Hampshire. Please contact them at 11 Hazen Drive, Concord, NH 03301 or at (603) 271-2461.

11. WEB SOIL SURVEY



MAP LEGEND

MAP INFORMATION



National Cooperative Soil Survey Web Soil Survey

Hydrologic Soil Group

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
СаВ	Canton fine sandy loam, 0 to 8 percent slopes	A	3.4	1.8%
CaC	Canton fine sandy loam, 8 to 15 percent slopes	В	88.5	45.7%
CaD	Canton fine sandy loam, 15 to 25 percent slopes	Α	3.1	1.6%
CmC	Canton fine sandy loam, 8 to 15 percent slopes, very stony	В	28.0	14.5%
CmD	Canton fine sandy loam, 15 to 25 percent slopes, very stony	В	0.1	0.0%
CpD	Chatfield-Hollis-Canton complex, 15 to 25 percent slopes, very rocky	В	13.2	6.8%
CsC	Chatfield-Hollis complex, 8 to 15 percent slopes, rocky	В	25.3	13.1%
CtD	Chatfield-Hollis-Rock outcrop complex, 15 to 35 percent slopes	D	4.7	2.4%
LvA	Leicester-Walpole complex stony, 0 to 3 percent slopes	A/D	0.1	0.0%
Sr	Scarboro stony mucky loamy sand	A/D	1.2	0.6%
SsA	Scituate fine sandy loam, 0 to 3 percent slopes	С	0.3	0.2%
SsB	Scituate fine sandy loam, 3 to 8 percent slopes	С	13.6	7.0%
StA	Scituate stony fine sandy loam, 0 to 3 percent slopes	C	7.4	3.8%
StB	Scituate stony fine sandy loam, 3 to 8 percent slopes	С	4.7	2.4%
Totals for Area of Inter	est		193.5	100.0%

Description

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

Rating Options

Aggregation Method: Dominant Condition

Component Percent Cutoff: None Specified

Tie-break Rule: Higher

12. AERIAL

Attachment "F" Granite State Roofing Veterans of Foreign Wars Hudson SITE Suppa's Pizza Hudson Town Fore Smith Farm Bill Cahill's Super Subs enson Park TITLE: AERIAL EXHIBIT PREPARED FOR: BARRETT HILL, LLC KEACH-NORDSTROM ASSOCIATES, INC. MAP 151; LOT 59 Civil Engineering Land Surveying Landscape Architecture 10 Commerce Park North, Suite 3B, Bedford, NH 03110 Phone (603) 627-2881 75 BARRETTS HILL ROAD - HUDSON, NEW HAMPSHIRE DATE: 09/25/2023 JOB. NO. 23-0414-1

SCALE: I" = 2,000"

SHEET I OF I

13. SITE PHOTOGRAPHS

KNA Project No. 23-0414-1

Photo No. 1: Looking south onto the subject property from Barretts Hill Road.



Photo No. 2: Looking northwest down Barretts Hill Road from existing gravel access.



Photo No. 3: Looking southeast down Barretts Hill from existing gravel access.



Photo No. 4: Looking at existing foliage located on the subject property.



Photo No. 5: Looking at existing pathway and foliage located on the subject property.



Photo No. 6: Looking southeast at an existing drop inlet located on the side of the road near the Barretts Hill Road crossing. This structure is located next to lots 55 and 61.



Civil Engineering

Land Surveying

Landscape Architecture

14. EXTREME PRECIPITATION TABLE

Extreme Precipitation Tables

Northeast Regional Climate Center

Data represents point estimates calculated from partial duration series. All precipitation amounts are displayed in inches.

Metadata for Point

Smoothing

State

Location

Latitude 42.786 degrees North 71.393 degrees West

Yes

Elevation 130 feet

Date/Time Mon Sep 25 2023 10:32:33 GMT-0400 (Eastern Daylight

Time)

Extreme Precipitation Estimates

	5min	10min	15min	30min	60min	120min		1hr	2hr	3hr	6hr	12hr	24hr	48hr		1day	2day	4day	7day	10day	
1yr	0.27	0.42	0.52	0.68	0.85	1.07	1yr	0.73	1.01	1.23	1.55	1.96	2.47	2.70	1yr	2.19	2.60	3.03	3.71	4.32	1yr
2yr	0.33	0.51	0.63	0.84	1.05	1.32	2yr	0.91	1.21	1.53	1.90	2.37	2.95	3.28	2yr	2.61	3.15	3.66	4.37	4.97	2yr
5yr	0.39	0.61	0.76	1.02	1.31	1.66	5yr	1.13	1.51	1.93	2.41	3.00	3.74	4.17	5yr	3.31	4.01	4.64	5.50	6.22	5yr
10yr	0.44	0.69	0.88	1.19	1.55	1.98	10yr	1.34	1.79	2.31	2.90	3.60	4.47	5.01	10yr	3.95	4.82	5.56	6.53	7.37	10yr
25yr	0.52	0.83	1.06	1.46	1.93	2.50	25yr	1.67	2.24	2.92	3.68	4.58	5.66	6.39	25yr	5.01	6.15	7.05	8.21	9.23	25yr
50yr	0.59	0.94	1.21	1.70	2.29	3.00	50yr	1.98	2.66	3.51	4.42	5.50	6.77	7.69	50yr	5.99	7.39	8.46	9.77	10.94	50yr
100yr	0.68	1.09	1.41	2.00	2.72	3.57	100yr	2.35	3.15	4.20	5.30	6.59	8.11	9.25	100yr	7.17	8.89	10.14	11.63	12.98	100yr
200yr	0.77	1.26	1.63	2.34	3.23	4.27	200yr	2.79	3.74	5.03	6.36	7.90	9.71	11.13	200yr	8.59	10.70	12.16	13.84	15.41	200yr
500yr	0.92	1.52	1.99	2.89	4.05	5.40	500yr	3.50	4.69	6.38	8.08	10.05	12.33	14.23	500yr	10.91	13.68	15.48	17.44	19.33	500yr

Lower Confidence Limits

	5min	10min	15min	30min	60min	120min		1hr	2hr	3hr	6hr	12hr	24hr	48hr		1day	2day	4day	7day	10day	
1yr	0.23	0.35	0.43	0.58	0.71	0.81	1yr	0.61	0.79	1.07	1.31	1.67	2.21	2.54	1yr	1.96	2.44	2.69	3.02	3.79	1yr
2yr	0.31	0.49	0.60	0.81	1.00	1.19	2yr	0.86	1.17	1.37	1.79	2.30	2.85	3.17	2yr	2.52	3.05	3.54	4.24	4.82	2yr
5yr	0.36	0.55	0.69	0.94	1.20	1.42	5yr	1.03	1.39	1.62	2.10	2.69	3.50	3.80	5yr	3.10	3.65	4.22	5.06	5.73	5yr
10yr	0.39	0.60	0.75	1.05	1.35	1.61	10yr	1.17	1.57	1.83	2.38	3.03	4.04	4.34	10yr	3.58	4.18	4.83	5.78	6.50	10yr
25yr	0.45	0.68	0.85	1.21	1.59	1.88	25yr	1.37	1.84	2.16	2.80	3.52	4.89	5.21	25yr	4.33	5.01	5.77	6.90	7.62	25yr
50yr	0.49	0.74	0.92	1.33	1.78	2.14	50yr	1.54	2.10	2.45	3.18	3.95	5.66	5.99	50yr	5.01	5.76	6.63	7.91	8.58	50yr
100yr	0.53	0.81	1.01	1.46	2.01	2.42	100yr	1.73	2.37	2.78	3.58	4.45	5.96	6.91	100yr	5.27	6.64	7.65	9.08	9.64	100yr
200yr	0.59	0.89	1.12	1.63	2.27	2.75	200yr	1.96	2.69	3.13	4.08	5.04	6.78	7.98	200yr	6.00	7.68	8.83	10.45	10.85	200yr
500yr	0.67	1.00	1.29	1.87	2.66	3.27	500yr	2.29	3.20	3.70	4.85	5.95	8.05	9.74	500yr	7.12	9.37	10.69	12.60	12.67	500yr

Upper Confidence Limits

	5min	10min	15min	30min	60min	120min		1hr	2hr	3hr	6hr	12hr	24hr	48hr		1day	2day	4day	7day	10day	
1yr	0.31	0.47	0.58	0.78	0.96	1.11	1yr	0.82	1.09	1.26	1.64	2.08	2.69	2.86	1yr	2.38	2.75	3.42	4.17	4.81	1yr
2yr	0.35	0.54	0.66	0.90	1.11	1.31	2yr	0.96	1.28	1.48	1.92	2.46	3.09	3.45	2yr	2.74	3.32	3.81	4.54	5.22	2yr
5yr	0.44	0.67	0.83	1.14	1.45	1.66	5yr	1.25	1.62	1.89	2.42	3.04	4.03	4.62	5yr	3.56	4.44	5.06	5.98	6.73	5yr
10yr	0.53	0.81	1.00	1.40	1.81	2.02	10yr	1.56	1.98	2.28	2.90	3.61	4.99	5.78	10yr	4.41	5.56	6.30	7.38	8.26	10yr
25yr	0.68	1.04	1.29	1.84	2.42	2.62	25yr	2.09	2.56	2.94	3.67	4.50	6.62	7.79	25yr	5.86	7.49	8.39	9.72	10.87	25yr
50yr	0.83	1.26	1.57	2.25	3.03	3.19	50yr	2.62	3.12	3.56	4.40	5.33	8.22	9.77	50yr	7.27	9.40	10.42	11.96	13.38	50yr
100yr	1.01	1.53	1.92	2.77	3.79	3.89	100yr	3.27	3.80	4.33	5.29	6.30	11.05	12.24	100yr	9.78	11.77	12.93	14.75	16.49	100yr
200yr	1.23	1.86	2.35	3.41	4.75	4.73	200yr	4.10	4.63	5.24	6.34	7.47	13.88	15.31	200yr	12.28	14.72	16.02	18.17	20.33	200yr
500yr	1.62	2.41	3.10	4.50	6.40	6.13	500yr	5.52	6.00	6.78	8.04	9.33	18.79	20.58	500yr	16.63	19.79	21.30	23.94	26.84	500yr



15. BMP WORKSHEETS

New HAMPSHIRE DEPARTMENT OF Environmental Services

Attachment "F"

STORMWATER POND DESIGN CRITERIA

Env-Wq 1508.03

Type/Node Name: Pocket Pond

Enter the type of stormwater pond (e.g., Wet Pond) and the node name in the drainage analysis, if applicable.

44.0		, , , , , , , , , , , , , , , , , , ,	ramage analysis, il applicable.
11.2	/_ac 5 ac	A = Area draining to the practice	
		A _I = Impervious area draining to the practice	
	2 decimal	I = Percent impervious area draining to the practice, in decimal form	
	unitless	Rv = Runoff coefficient = 0.05 + (0.9 x I)	
	3 ac-in	WQV= 1" x Rv x A	
6,464		WQV conversion (ac-in x 43,560 sf/ac x 1ft/12")	
	cf cf	10% x WQV (check calc for sediment forebay and micropool volume)	
3,232		50% x WQV (check calc for extended detention volume)	
1,678	cf —	V _{SED} = Sediment forebay volume	≥ 10%WQV
7,477	cf	V_{PP} = Permanent pool volume (volume below the lowest invert of the o	utlet structure) Attach
7,477	CI	stage-storage table.	, , , , , , , , , , , , , , , , , , , ,
no	cf	Extended Detention? ¹	≤ 50% WQV
		V_{ED} = Volume of extended detention (if "yes" is given in box above)	
		E _{ED} = Elevation of WQV if "yes" is given in box above ⁴	
=	cfs	$2Q_{avg} = 2*V_{ED} / 24 \text{ hrs } * (1\text{hr} / 3600 \text{ sec}) \text{ (used to check against } Q_{EDmax} \text{ b}$	elow)
	cfs	Q_{EDmax} = Discharge at the E_{ED} (attach stage-discharge table)	< 2Q _{avg}
Mar Talli	hours	T_{ED} = Drawdown time of extended detention = $2V_{ED}/Q_{EDmax}$	≥ 24-nrs
3.00) :1	Pond side slopes	≥3:1
370.17	¹ ft	Elevation of seasonal high water table	
367.00) ft	Elevation of lowest pond outlet	
364.00) ft	Max floor = Maximum elevation of pond bottom (ft)	
359.00) ft	Minimum floor (to maintain depth at less than 8')	< 8 ft
364.00	4		≤ Max floor and > Min
304.00	-	Elevation of pond floor ³	floor
205.00	ft	Length of the flow path between the inlet and outlet at mid-depth	
66.00	ft	Average width ([average of the top width + average bottom width]/2)	1
3.11	:1	Length to average width ratio	≥ 3:1
Yes	Yes/No	Is the perimeter curvilinear.	← Yes
Yes	Yes/No	Are the inlet and outlet located as far apart as possible.	← Yes
No	Yes/No	Is there a manually-controlled drain to dewater the pond over a 24hr pe	
If no	state why:	Pond will be manually drained if maintenance is required	- 1
		What mechanism is proposed to prevent the outlet structure from clogg	ing (applicable for
	Rack	orifices/weirs with a dimension of <6")?	- 05 10 1 10 1 10 1
373.79		Peak elevation of the 50-year storm event	
374.00	ft	Berm elevation of the pond	
YES		50 peak elevation ≤ the berm elevation?	←yes
1 16 46			

- 1. If the entire WQV is stored in the perm. pool, there is no extended det., and the following five lines do not apply.
- 2. This is the elevation of WQV if the hydrologic analysis is set up to include the permanent pool storage in the node description.
- 3. If the pond floor elevation is above the max floor elev., a hydrologic budget must be submitted to demonstrate that a minimum depth of 3 feet can be maintained. (First check whether a revised "lowest pond outlet" elev. will resolve the issue.)

Designer's Notes:

Pond will be lined with 30 mil PVC impervious liner

Type III 24-hr 50-YEAR Rainfall=6.77"

Prepared by Keach-Nordstrom Associates, Inc.

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Printed 12/6/2023

Stage-Area-Storage for Pond PP: POCKET POND

Elevation (feet)	Storage (cubic-feet)	Elevation (feet)	Storage (cubic-feet)	Elevation (feet)	Storage
364.00	Ó	365.04	621		(cubic-feet)
364.02	3	365.06		366.08	2,201
364.04	6		642	366.10	2,252
364.06	10	365.08	663	366.12	2,305
364.08		365.10	685	366.14	2,362
364.10	14	365.12	707	366.16	2,421
364.12	19	365.14	730	366.18	2,482
	24	365.16	753	366.20	2,547
364.14	29	365.18	776	366.22	2,615
364.16	34	365.20	800	366.24	2,685
364.18	40	365.22	824	366.26	2,759
364.20	46	365.24	848	366.28	2,835
364.22	53	365.26	873	366.30	2,914
364.24	60	365.28	897	366.32	2,996
364.26	67	365.30	923	366.34	3,080
364.28	75	365.32	948	366.36	3,168
364.30	83	365.34	974	366.38	3,258
364.32	91	365.36	1,001	366.40	3,352
364.34	100	365.38	1,028	366.42	3,448
364.36	109	365.40	1,055	366.44	3,547
364.38	118	365.42	1,082	366.46	
364.40	128	365.44	1,110	366.48	3,649
364.42	138	365.46	1,138	366.50	3,753
364.44	148	365.48	1,166	366.52	3,861
364.46	159	365.50	1,195	366.54	3,971
364.48	170	365.52	1,224		4,085
364.50	181	365.54	1,254	366.56	4,201
364.52	193	365.56		366.58	4,320
364.54	205	365.58	1,284	366.60	4,442
364.56	217	365.60	1,314	366.62	4,566
364.58	230	365.62	1,344	366.64	4,694
364.60	243	365.64	1,375	366.66	4,824
364.62	257		1,406	366.68	4,957
364.64	271	365.66	1,438	366.70	5,093
364.66	285	365.68	1,470	366.72	5,232
364.68	300	365.70	1,502	366.74	5,374
364.70	314	365.72	1,535	366.76	5,519
364.72		365.74	1,568	366.78	5,666
364.74	330 345	365.76	1,601	366.80	5,817
364.76		365.78	1,635	366.82	5,970
364.78	361	365.80	1,669	366.84	6,126
364.80	377	365.82	1,703	366.86	6,285
	394	365.84	1,738	366.88	6,447
364.82 364.84	411	365.86	1,773	366.90	6,611
	428	365.88	1,808	366.92	6,779
364.86	446	365.90	1,844	366.94	6,949
364.88	464	365.92	1,880	366.96	7,122
364.90	482	365.94	1,917	366.98	7,298
364.92	501	365.96	1,953	367.00	7,477
364.94	520	365.98	1,991	367.02	7,658
364.96	540	366.00	2,028	367.04	7,839
364.98	559	366.02	2,067	367.06	8,021
365.00	580	366.04	2,109	367.08	8,203
365.02	600	366.06	2,154	367.10	8,386
	1				,

Type III 24-hr 50-YEAR Rainfall=6.77"

Prepared by Keach-Nordstrom Associates, Inc.

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Printed 12/6/2023

Stage-Area-Storage for Pond PP: POCKET POND (continued)

		a I		W.	
Elevation	Storage	Elevation	Storage	Elevation	Storage
(feet)	(cubic-feet)	(feet)	(cubic-feet)	(feet)	(cubic-feet)
367.12	8,570	368.16	18,977	369.20	31,126
367.14	8,754	368.18	19,194	369.22	31,377
367.16	8,939	368.20	19,411	369.24	31,629
367.18	9,125	368.22	19,630	369.26	31,882
367.20	9,311	368.24	19,848	369.28	32,135
367.22	9,498	368.26	20,068	369.30	32,390
367.24	9,685	368.28	20,288	369.32	32,644
367.26	9,873	368.30	20,509	369.34	32,900
367.28	10,062	368.32	20,730	369.36	33,156
367.30	10,251	368.34	20,952	369.38	33,413
367.32	10,441	368.36	21,175	369.40	33,671
367.34	10,632	368.38	21,398	369.42	33,929
367.36	10,823	368.40	21,623	369.44	34,188
367.38	11,014	368.42	21,847	369.46	34,448
367.40	11,207	368.44	22,073	369.48	34,708
367.42	11,400	368.46	22,299	369.50	34,969
367.44	11,593	368.48	22,525	369.52	35,231
367.46	11,788	368.50	22,753	369.54	35,494
367.48	11,983	368.52	22,981	369.56	35,757
367.50	12,178	368.54	23,209	369.58	36,021
367.52	12,374	368.56	23,439	369.60	36,285
367.54	12,571	368.58	23,669	369.62	36,550
367.56	12,768	368.60	23,899	369.64	36,816
367.58	12,966	368.62	24,131	369.66	37,083
367.60	13,165	368.64	24,363	369.68	37,350
367.62	13,364	368.66	24,595	369.70	37,618
367.64	13,564	368.68	24,828	369.72	37,887
367.66	13,764	368.70	25,062	369.74	38,156
367.68	13,965	368.72	25,297	369.76	38,427
367.70	14,167	368.74	25,532	369.78	38,697
367.72	14,369	368.76	25,768	369.80	38,969
367.74	14,572	368.78	26,005	369.82	39,241
367.76	14,775	368.80	26,242	369.84	39,514
367.78	14,980	368.82	26,480	369.86	39,788
367.80	15,184	368.84	26,718	369.88	40,062
367.82	15,390	368.86	26,957	369.90	40,337
367.84	15,596	368.88	27,197	369.92	40,612
367.86	15,802	368.90	27,438	369.94	40,889
367.88	16,010	368.92	27,679	369.96	41,166
367.90	16,217	368.94	27,921	369.98	41,444
367.92	16,426	368.96	28,163	370.00	41,722
367.94	16,635	368.98	28,406	370.02	42,001
367.96	16,845	369.00	28,650	370.04	42,282
367.98	17,055	369.02	28,894	370.06	42,564
368.00	17,266	369.04	29,140	370.08	42,848
368.02	17,478	369.06	29,385	370.10	43,133
368.04	17,690	369.08	29,632	370.12	43,419
368.06	17,903	369.10	29,879	370.14	43,706
368.08	18,116	369.12	30,127	370.16	43,995
368.10	18,331	369.14	30,376	370.18	44,285
368.12	18,545	369.16	30,625	370.20	44,577
368.14	18,761	369.18	30,875	370.22	44,869
	J		I		,,,,,

Type III 24-hr 50-YEAR Rainfall=6.77"

Prepared by Keach-Nordstrom Associates, Inc.
HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Printed 12/6/2023

Stage-Area-Storage for Pond PP: POCKET POND (continued)

	_		_		··· (oominac
Elevation	Storage	Elevation	Storage	Elevation	Storage
(feet)	(cubic-feet)	(feet)	(cubic-feet)	(feet)	(cubic-feet)
370.24	45,164	371.28	62,250	372.32	81,949
370.26	45,459	371.30	62,607	372.34	82,351
370.28	45,756	371.32	62,966	372.36	82,753
370.30	46,054	371.34	63,325	372.38	83,157
370.32	46,353	371.36	63,685	372.40	83,561
370.34	46,654	371.38	64,046	372.42	83,966
370.36	46,956	371.40	64,407	372.44	84,372
370.38	47,260	371.42	64,770	372.46	84,779
370.40	47,565	371.44	65,133	372.48	85,186
370.42	47,871	371.46	65,498	372.50	85,595
370.44	48,178	371.48	65,863	372.52	86,004
370.46	48,487	371.50	66,229	372.54	86,415
370.48	48,797	371.52	66,595	372.56	86,826
370.50	49,109	371.54	66,963	372.58	87,238
370.52	49,422	371.56	67,331	372.60	87,651
370.54	49,736	371.58	67,700	372.62	88,065
370.56	50,051	371.60	68,070	372.64	88,480
370.58	50,368	371.62	68,441	372.66	88,896
370.60	50,686	371.64	68,813	372.68	89,312
370.62	51,006	371.66	69,185	372.70	89,729
370.64	51,327	371.68	69,559	372.72	
370.66	51,649	371.70	69,933	372.74	90,148 90,567
370.68	51,972	371.72	70,308	372.76	
370.70	52,297	371.74	70,684	372.78	90,987 91,408
370.72	52,624	371.76	71,060	372.80	91,830
370.74	52,951	371.78	71,438	372.82	92,252
370.76	53,280	371.80	71,816	372.84	92,232
370.78	53,610	371.82	72,195	372.86	93,101
370.80	53,942	371.84	72,575	372.88	93,526
370.82	54,275	371.86	72,956	372.90	93,952
370.84	54,609	371.88	73,337	372.92	
370.86	54,944	371.90	73,720	372.94	94,379
370.88	55,281	371.92	74,103	372.96	94,807
370.90	55,619	371.94	74,487	372.98	95,236
370.92	55,959	371.96	74,872	373.00	95,666
370.94	56,300	371.98	75,258	373.02	96,097
370.96	56,642	372.00	75,645	373.04	96,528
370.98	56,986	372.02	76,032	373.06	96,960 97,394
371.00	57,331	372.04	76,420	373.08	
371.02	57,676	372.06	76,809	373.10	97,828
371.04	58,023	372.08	77,200	373.10	98,263
371.06	58,371	372.10	77,591	373.14	98,699
371.08	58,719	372.12	77,982	373.14	99,136
371.10	59,069	372.14	78,375	373.18	99,573
371.12	59,419	372.16	78,769	373.10	100,012
371.14	59,770	372.18	79,163	373.22	100,451
371.16	60,122	372.20	79,559	373.24	100,892
371.18	60,474	372.22	79,955	373.24 373.26	101,333
371.20	60,828	372.24	80,352		101,775
371.22	61,182	372.26	80,750	373.28 373.30	102,218
371.24	61,537	372.28	81,149		102,662
371.26	61,893	372.30	81,549	373.32	103,107
	0.,000	012.00	01,048	373.34	103,552
	W.		1		

Type III 24-hr 50-YEAR Rainfall=6.77"

Prepared by Keach-Nordstrom Associates, Inc.

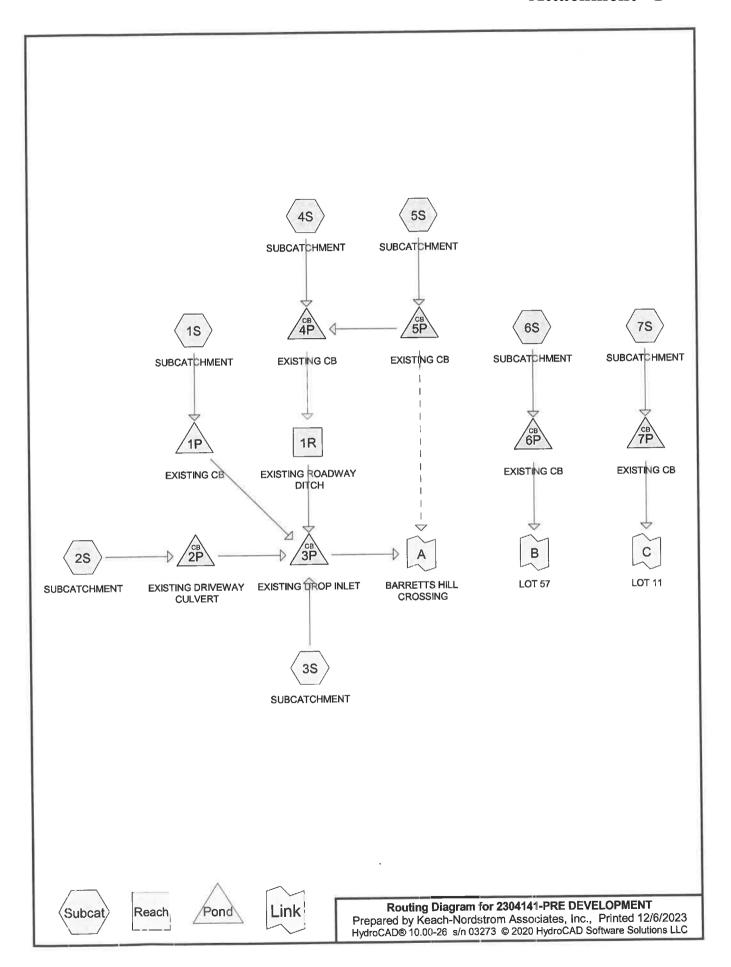
HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Printed 12/6/2023

Stage-Area-Storage for Pond PP: POCKET POND (continued)

F 1	0.
Elevation	Storage
(feet)	(cubic-feet)
373.36	103,999
373.38	104,446
373.40	104,894
373.42	105,344
373.44	105,794
373.46	106,245
373.48	106,696
373.50	107,149
373.52	107,603
373.54	108,057
373.56	108,512
373.58	108,969
373.60	109,426
373.62	109,884
373.64	110,342
373.66	110,802
373.68	111,263
373.70	111,724
373.72	112,187
373.74	112,650
373.76	113,114
373.78	113,579
373.80	114,045
373.82	114,512
373.84	114,979
373.86	115,448
373.88	115,917
373.90	116,388
373.92	116,859
373.94	117,331
373.96	117,804
373.98	118,278
374.00	118,753

16. HYDROCAD DRAINAGE ANALYSIS



2304141-PRE DEVELOPMENT

Prepared by Keach-Nordstrom Associates, Inc.

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Printed 12/6/2023 Page 2

Area Listing (all nodes)

Area (acres)	CN	Description (subcatchment-numbers)
0.372 3.099 1.111 0.018 1.364 0.099 0.543	39.0 61.0 74.0 96.0 98.0 98.0 98.0	>75% Grass cover, Good, HSG A (1S, 2S, 3S) >75% Grass cover, Good, HSG B (1S, 2S, 3S, 4S, 5S) >75% Grass cover, Good, HSG C (1S, 2S, 6S, 7S) Gravel surface, HSG C (7S) Paved parking (1S, 2S, 3S, 4S, 5S, 6S) Paved parking, HSG C (7S) Roofs (1S, 2S, 3S, 4S)
0.960 10.021 28.244 45.830	30.0 55.0 70.0 66.4	Woods, Good, HSG A (1S, 2S, 3S) Woods, Good, HSG B (1S, 2S, 3S, 4S) Woods, Good, HSG C (1S, 2S, 3S, 4S, 6S, 7S) TOTAL AREA

2304141-PRE DEVELOPMENT

Prepared by Keach-Nordstrom Associates, Inc.

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Printed 12/6/2023 Page 3

Soil Listing (all nodes)

Area	Soil	Subcatchment
(acres)	Group	Numbers
1.332	HSG A	1S, 2S, 3S
13.120	HSG B	1S, 2S, 3S, 4S, 5S
29.471	HSG C	1S, 2S, 3S, 4S, 6S, 7S
0.000	HSG D	
1.907	Other	1S, 2S, 3S, 4S, 5S, 6S
45.830		TOTAL AREA

2304141-PRE DEVELOPMENT

Link A: BARRETTS HILL CROSSING

Type III 24-hr 2-YEAR Rainfall=2.95"

Prepared by Keach-Nordstrom Associates, Inc.
HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Printed 12/6/2023

Inflow=7.79 cfs 1.43 af Primary=7.79 cfs 1.43 af

Page 4

Time span=0.00-24.00 hrs, dt=0.01 hrs, 2401 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Reach routing by Dyn-St	or-Ind method - Pond routing by Dyn-Stor-Ind method				
Subcatchment 1S: SUBCATCHMENT	Runoff Area=1,062,579 sf 2.71% Impervious Runoff Depth>0.47" Flow Length=2,203' Tc=38.1 min CN=64.7 Runoff=5.06 cfs 0.95 af				
Subcatchment 2S: SUBCATCHMENT	Runoff Area=113,081 sf 12.76% Impervious Runoff Depth>0.36" Flow Length=683' Tc=20.1 min CN=61.4 Runoff=0.44 cfs 0.08 af				
Subcatchment 3S: SUBCATCHMENT	Runoff Area=128,229 sf 6.91% Impervious Runoff Depth>0.37" Flow Length=1,189' Tc=23.1 min CN=61.7 Runoff=0.51 cfs 0.09 af				
Subcatchment 4S: SUBCATCHMENT	Runoff Area=223,873 sf 8.43% Impervious Runoff Depth>0.67" Flow Length=1,889' Tc=31.1 min CN=69.9 Runoff=1.97 cfs 0.29 af				
Subcatchment 5S: SUBCATCHMENT	Runoff Area=16,441 sf 34.30% Impervious Runoff Depth>0.86" Flow Length=389' Tc=8.7 min CN=73.7 Runoff=0.32 cfs 0.03 af				
Subcatchment 6S: SUBCATCHMENT	Runoff Area=248,437 sf 2.62% Impervious Runoff Depth>0.72" Flow Length=1,105' Tc=24.4 min CN=70.8 Runoff=2.63 cfs 0.34 af				
Subcatchment 7S: SUBCATCHMENT	Runoff Area=203,705 sf 2.12% Impervious Runoff Depth>0.72" Flow Length=1,021' Tc=20.7 min CN=70.8 Runoff=2.29 cfs 0.28 af				
Reach 1R: EXISTING ROADWAY DITO n=0.022	CH Avg. Flow Depth=0.46' Max Vel=3.24 fps Inflow=2.09 cfs 0.32 af L=230.0' S=0.0173 '/' Capacity=16.21 cfs Outflow=2.09 cfs 0.32 af				
Pond 1P: EXISTING CB	Inflow=5.06 cfs 0.95 af Primary=5.06 cfs 0.95 af				
Pond 2P: EXISTING DRIVEWAY CULVERT Peak Elev=292.73' Inflow=0.44 cfs 0.08 af 12.0" Round Culvert n=0.013 L=21.0' S=0.0143 '/' Outflow=0.44 cfs 0.08 af					
Pond 3P: EXISTING DROP INLET 24.0"	Peak Elev=289.90' Inflow=7.79 cfs 1.43 af Round Culvert n=0.011 L=92.1' S=0.0433 '/' Outflow=7.79 cfs 1.43 af				
Pond 4P: EXISTING CB Primary=2	Peak Elev=297.84' Inflow=2.09 cfs 0.32 af 2.09 cfs 0.32 af Secondary=0.00 cfs 0.00 af Outflow=2.09 cfs 0.32 af				
Pond 5P: EXISTING CB Primary=0	Peak Elev=298.06' Inflow=0.32 cfs 0.03 af 0.32 cfs 0.03 af Outflow=0.32 cfs 0.03 af				
Pond 6P: EXISTING CB	Peak Elev=364.68' Inflow=2.63 cfs 0.34 af Round Culvert n=0.025 L=18.0' S=0.0689 '/' Outflow=2.63 cfs 0.34 af				
Pond 7P: EXISTING CB	Peak Elev=377.48' Inflow=2.29 cfs 0.28 af Round Culvert n=0.011 L=31.9' S=0.0091 '/' Outflow=2.29 cfs 0.28 af				

2304141-PRE DEVELOPMENT

Type III 24-hr 2-YEAR Rainfall=2.95"

Prepared by Keach-Nordstrom Associates, Inc.

Printed 12/6/2023

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 5

Link B: LOT 57

Inflow=2.63 cfs 0.34 af

Primary=2.63 cfs 0.34 af

Link C: LOT 11

Inflow=2.29 cfs 0.28 af Primary=2.29 cfs 0.28 af

Total Runoff Area = 45.830 ac Runoff Volume = 2.05 af Average Runoff Depth = 0.54" 95.62% Pervious = 43.824 ac 4.38% Impervious = 2.006 ac

2304141-PRE DEVELOPMENT

Type III 24-hr 2-YEAR Rainfall=2.95"

Prepared by Keach-Nordstrom Associates, Inc.
HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Printed 12/6/2023

Page 6

Summary for Subcatchment 1S: SUBCATCHMENT

Runoff = 5.06 cfs @ 12.66 hrs, Volume=

0.95 af, Depth> 0.47"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 2-YEAR Rainfall=2.95"

	A	rea (sf)	CN	Description	on					
		7,670	30.0	Woods, C	Voods, Good, HSG A					
	3	63,085	55.0		Good, HSG					
	5	74,693	70.0	Woods, G	Good, HSG	C				
		6,684	39.0	>75% Gra	75% Grass cover, Good, HSG A					
		61,618	61.0		75% Grass cover, Good, HSG B					
		20,067	74.0			Good, HSG C				
*		14,404	98.0	Roofs						
*		14,358	98.0	Paved pa	rking					
	1,0	62,579	64.7	Weighted	Average					
	1,0	33,817	63.8		ervious Are	ea				
		28,762	98.0	2.71% lm	pervious A	rea				
	Tc	Length	Slope	Velocity	Capacity	Description				
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
	17.3	100	0.0400	0.10		Sheet Flow,				
						Woods: Light underbrush n= 0.400 P2= 2.84"				
	19.7	1,862	0.0988	1.57		Shallow Concentrated Flow,				
						Woodland Kv= 5.0 fps				
	0.3	30	0.0800	1.98		Shallow Concentrated Flow,				
						Short Grass Pasture Kv= 7.0 fps				
	0.8	211	0.0430	4.21		Shallow Concentrated Flow,				
_						Paved Kv= 20.3 fps				
	38.1	2,203	Total							

Summary for Subcatchment 2S: SUBCATCHMENT

Runoff = 0.44 cfs @ 12.43 hrs, Volume=

0.08 af, Depth> 0.36"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 2-YEAR Rainfall=2.95"

	Area (sf)	CN	Description				
	28,414	30.0	Woods, Good, HSG A				
	11,383	55.0	Woods, Good, HSG B				
	33,808	70.0	Woods, Good, HSG C				
	4,719	39.0	>75% Grass cover, Good, HSG A				
	242	61.0	75% Grass cover, Good, HSG B				
	20,087	74.0	75% Grass cover, Good, HSG C				
*	3,849	98.0	Roofs				
*	10,579	98.0	Paved parking				
	113,081	61.4	Weighted Average				
	98,653	56.1	87.24% Pervious Area				
	14,428	98.0	12.76% Impervious Area				

2304141-PRE DEVELOPMENT

Type III 24-hr 2-YEAR Rainfall=2.95"

Prepared by Keach-Nordstrom Associates, Inc.

Printed 12/6/2023

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 7

Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
12.0	100	0.1000	0.14		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 2.84"
3.9	318	0.0750	1.37		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
0.7	48	0.0300	1.21		Shallow Concentrated Flow,
					Short Grass Pasture Kv= 7.0 fps
3.6	217	0.0410	1.01		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
20.1	683	Total			

Summary for Subcatchment 3S: SUBCATCHMENT

Runoff = 0.51 cfs @ 12.49 hrs, Volume=

0.09 af, Depth> 0.37"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 2-YEAR Rainfall=2.95"

	Α	rea (sf)	CN	Descriptio	n						
-		5,732	30.0	Woods, G	Voods, Good, HSG A						
		49,737	55.0	Woods, G	Voods, Good, HSG B						
		38,476	70.0	Woods, G	lood, HSG	С					
		4,799	39.0	>75% Gra	iss cover, (Good, HSG A					
		20,628	61.0	>75% Gra	ss cover, C	Good, HSG B					
*		2,833	98.0	Roofs							
*		6,024	98.0	Paved par	rking						
	1	128,229 61.7 Weighted Average									
		19,372	59.0 93.09% Pervious Are			ea					
		8,857	98.0	6.91% lm	pervious Ar	rea					
		•									
	Tc	Length	Slope	Velocity	Capacity	Description					
(1	min)	(feet)	(ft/ft)	(ft/sec)	(cfs)						
	9.9	100	0.1600	0.17		Sheet Flow,					
						Woods: Light underbrush n= 0.400	P2= 2.84"				
	13.2	1,089	0.0760	1.38		Shallow Concentrated Flow,					
						Woodland Kv= 5.0 fps					
	23.1	1,189	Total								

Summary for Subcatchment 4S: SUBCATCHMENT

Runoff = 1.97 cfs @ 12.51 hrs, Volume=

0.29 af, Depth> 0.67"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN. Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 2-YEAR Rainfall=2.95"

2304141-PRE DEVELOPMENT

Type III 24-hr 2-YEAR Rainfall=2.95"

Prepared by Keach-Nordstrom Associates, Inc.

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Printed 12/6/2023 Page 8

_	A	rea (sf)	CN	Description	on						
		12,309	55.0	Woods, G	Voods, Good, HSG B						
	1	50,990	70.0		oods, Good, HSG C						
		41,699	61.0	>75% Gra	% Grass cover, Good, HSG B						
*		2,548	98.0	Roofs	·						
*		16,327	98.0	Paved pa	rking						
	2	23,873	69.9	Weighted	Average						
		04,998	67.3	91.57% P	ervious Are	ea					
		18,875	98.0	8.43% Im	pervious A	rea					
	Тс	Length	Slope	Velocity	Capacity	Description					
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)						
	15.8	100	0.0500	0.11		Sheet Flow,					
		4				Woods: Light underbrush n= 0.400 P2= 2.84"					
	10.6	1,127	0.1250	1.77		Shallow Concentrated Flow,					
	0.5					Woodland Kv= 5.0 fps					
	0.5	55	0.0730	1.89		Shallow Concentrated Flow,					
	0.0	00	0.0500	4.04		Short Grass Pasture Kv= 7.0 fps					
	0.9	68	0.0590	1.21		Shallow Concentrated Flow,					
	0.2	30	0.4000	0.04		Woodland Kv= 5.0 fps					
	0.2	30	0.1000	2.21		Shallow Concentrated Flow,					
	0.1	16	0.0200	2.87		Short Grass Pasture Kv= 7.0 fps					
	0.1	10	0.0200	2.07		Shallow Concentrated Flow,					
	2.1	240	0.0750	1.92		Paved Kv= 20.3 fps					
		2-10	0.0700	1.52		Shallow Concentrated Flow,					
	0.3	73	0.0410	4.11		Short Grass Pasture Kv= 7.0 fps Shallow Concentrated Flow,					
			0.0110	7.11		Paved Kv= 20.3 fps					
	0.6	180	0.0720	5.45		Shallow Concentrated Flow,					
				0,10		Paved Kv= 20.3 fps					
	31.1	1,889	Total			1 41 40 10 1p3					
		.,000	. 0.01								

Summary for Subcatchment 5S: SUBCATCHMENT

Runoff = 0.32 cfs @ 12.13 hrs, Volume=

0.03 af, Depth> 0.86"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 2-YEAR Rainfall=2.95"

-	Area (sf)	CN	Description
	10,801	61.0	>75% Grass cover, Good, HSG B
*	5,640		Paved parking
	16,441	73.7	Weighted Average
	10,801		65.70% Pervious Area
	5,640	98.0	34.30% Impervious Area

2304141-PRE DEVELOPMENT

Type III 24-hr 2-YEAR Rainfall=2.95"

Prepared by Keach-Nordstrom Associates, Inc.

Printed 12/6/2023

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 9

	Тс	Length	Slope	Velocity	Capacity	Description
(1	min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	6.3	100	0.0700	0.26		Sheet Flow,
						Grass: Short n= 0.150 P2= 2.84"
	0.1	16	0.0400	4.06		Shallow Concentrated Flow,
						Paved Kv= 20.3 fps
	1.5	162	0.0680	1.83		Shallow Concentrated Flow,
						Short Grass Pasture Kv= 7.0 fps
	0.1	20	0.0400	4.06		Shallow Concentrated Flow,
						Paved Kv= 20.3 fps
	0.8	91	0.0770	1.94		Shallow Concentrated Flow,
						Short Grass Pasture Kv= 7.0 fps
	8.7	389	Total			

Summary for Subcatchment 6S: SUBCATCHMENT

Runoff =

2.63 cfs @ 12.40 hrs, Volume=

0.34 af, Depth> 0.72"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 2-YEAR Rainfall=2.95"

	Aı	rea (sf)	CN	Description	n		
	237,230 70.0		Woods, Good, HSG C				
*		4,709	74.0	>75% Gra	ss cover, (Good, HSG C	
*		6,498	98.0	Paved par			
	2	48,437	70.8	Weighted	Average		
		41,939	70.1	_	ervious Are	ea	
		6,498	98.0	2.62% lm	pervious Ai	rea	
	Тс	Length	Slope	Velocity	Capacity	Description	
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
	15.8	100	0.0500	0.11		Sheet Flow,	
						Woods: Light underbrush n= 0.400 P2= 2.84"	
	8.1	856	0.1250	1.77		Shallow Concentrated Flow,	
						Woodland Kv= 5.0 fps	
	0.0	10	0.3000	3.83		Shallow Concentrated Flow,	
						Short Grass Pasture Kv= 7.0 fps	
	0.5	139	0.0500	4.54		Shallow Concentrated Flow,	
						Paved Kv= 20.3 fps	
-	24.4	1 105	Total				

24.4 1,105 Total

Summary for Subcatchment 7S: SUBCATCHMENT

Runoff =

2.29 cfs @ 12.33 hrs, Volume=

0.28 af, Depth> 0.72"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs. Type III 24-hr 2-YEAR Rainfall=2.95"

2304141-PRE DEVELOPMENT

Type III 24-hr 2-YEAR Rainfall=2.95"

Prepared by Keach-Nordstrom Associates, Inc.

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Printed 12/6/2023 Page 10

A	rea (sf)	CN	Description	on				
1	195,095 70.0			Woods, Good, HSG C				
	3,533	74.0			Good, HSG C			
	4,313	98.0	Paved pa	rking, HSG	i C			
	764	96.0		rface, HSC				
2	03,705	70.8	Weighted		- -			
1	99,392	70.2		ervious Are	ea			
	4,313	98.0		pervious A				
			'					
Tc	Length	Slope	Velocity	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
12.5	100	0.0900	0.13		Sheet Flow,			
					Woods: Light underbrush n= 0.400 P2= 2.84"			
7.5	755	0.1130	1.68		Shallow Concentrated Flow,			
					Woodland Kv= 5.0 fps			
0.0	6	0.3000	3.83		Shallow Concentrated Flow,			
					Short Grass Pasture Kv= 7.0 fps			
0.7	160	0.0375	3.93		Shallow Concentrated Flow,			
					Paved Kv= 20.3 fps			
20.7	1,021	Total						

Summary for Reach 1R: EXISTING ROADWAY DITCH

Inflow Area = 5.517 ac, 10.20% Impervious, Inflow Depth > 0.69" for 2-YEAR event

Inflow = 2.09 cfs @ 12.48 hrs, Volume= 0.32 af

Outflow = 2.09 cfs @ 12.50 hrs, Volume= 0.32 af, Atten= 0%, Lag= 1.2 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Max. Velocity= 3.24 fps, Min. Travel Time= 1.2 min Avg. Velocity = 1.78 fps, Avg. Travel Time= 2.2 min

Peak Storage= 148 cf @ 12.50 hrs Average Depth at Peak Storage= 0.46'

Bank-Full Depth= 1.00' Flow Area= 3.0 sf, Capacity= 16.21 cfs

0.00' x 1.00' deep channel, n= 0.022 Earth, clean & straight

Side Slope Z-value= 3.0 '/' Top Width= 6.00' Length= 230.0' Slope= 0.0173 '/'

Inlet Invert= 295.64', Outlet Invert= 291.66'



Type III 24-hr 2-YEAR Rainfall=2.95"

Prepared by Keach-Nordstrom Associates, Inc.

Printed 12/6/2023

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 11

Summary for Pond 1P: EXISTING CB

Inflow Area = 24.393 ac, 2.71% Impervious, Inflow Depth > 0.47" for 2-YEAR event

Inflow = 5.06 cfs @ 12.66 hrs, Volume= 0.95 af

Primary = 5.06 cfs @ 12.66 hrs, Volume= 0.95 af, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Summary for Pond 2P: EXISTING DRIVEWAY CULVERT

Inflow Area = 2.596 ac, 12.76% Impervious, Inflow Depth > 0.36" for 2-YEAR event

Inflow = 0.44 cfs @ 12.43 hrs, Volume= 0.08 af

Outflow = 0.44 cfs @ 12.43 hrs, Volume= 0.08 af, Atten= 0%, Lag= 0.0 min

Primary = 0.44 cfs @ 12.43 hrs, Volume= 0.08 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 292.73' @ 12.43 hrs

Flood Elev= 294.00'

Primary OutFlow Max=0.44 cfs @ 12.43 hrs HW=292.73' TW=289.79' (Dynamic Tailwater) 1=Culvert (Inlet Controls 0.44 cfs @ 1.96 fps)

Summary for Pond 3P: EXISTING DROP INLET

Inflow Area = 35.450 ac, 4.96% Impervious, Inflow Depth > 0.48" for 2-YEAR event

inflow = 7.79 cfs @ 12.60 hrs, Volume= 1.43 af

Outflow = 7.79 cfs @ 12.60 hrs, Volume= 1.43 af, Atten= 0%, Lag= 0.0 min

Primary = 7.79 cfs @ 12.60 hrs, Volume= 1.43 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 289.90' @ 12.60 hrs

Flood Elev= 293.75'

Device	Routing	Invert	Outlet Devices
#1	Primary	288.66'	24.0" Round Culvert
	-		L= 92.1' RCP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 288.66' / 284.67' S= 0.0433 '/' Cc= 0.900
			n= 0.011 Concrete pipe, straight & clean, Flow Area= 3.14 sf

Primary OutFlow Max=7.79 cfs @ 12.60 hrs HW=289.90' TW=0.00' (Dynamic Tailwater)
1=Culvert (Inlet Controls 7.79 cfs @ 3.80 fps)

Type III 24-hr 2-YEAR Rainfall=2.95"

Prepared by Keach-Nordstrom Associates, Inc.
HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Printed 12/6/2023 Page 12

Summary for Pond 4P: EXISTING CB

Inflow Area = 5.517 ac, 10.20% Impervious, Inflow Depth > 0.69" for 2-YEAR event lnflow = 2.09 cfs @ 12.48 hrs, Volume= 0.32 af Outflow = 2.09 cfs @ 12.48 hrs, Volume= 0.32 af, Atten= 0%, Lag= 0.0 min Primary = 2.09 cfs @ 12.48 hrs, Volume= 0.32 af Secondary = 0.00 cfs @ 0.00 hrs, Volume= 0.00 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 297.84' @ 12.48 hrs

Flood Elev= 300.17'

Device	Routing	Invert	Outlet Devices
#1	Primary	297.00'	12.0" Round Culvert
			L= 49.1' CMP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 297.00' / 295.64' S= 0.0277 '/' Cc= 0.900
//0			n= 0.025 Corrugated metal, Flow Area= 0.79 sf
#2	Secondary	300.17'	24.0" x 24.0" Horiz. Orifice/Grate C= 0.600
			Limited to weir flow at low heads

Primary OutFlow Max=2.09 cfs @ 12.48 hrs HW=297.84' TW=296.10' (Dynamic Tailwater)
1=Culvert (Barrel Controls 2.09 cfs @ 4.01 fps)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=297.00' TW=295.64' (Dynamic Tailwater) 2=Orifice/Grate (Controls 0.00 cfs)

Summary for Pond 5P: EXISTING CB

Inflow Area =	0.377 ac, 34.30% Impervious, Inflow Depth > 0.86" for	or 2-YFAR event
Inflow =	0.32 cfs @ 12.13 hrs, Volume= 0.03 af	
	0.32 cfs @ 12.13 hrs, Volume= 0.03 af, Atten=	0%, Lag= 0.0 min
Primary =	0.32 cfs @ 12.13 hrs, Volume= 0.03 af	
Secondary =	0.00 cfs @ 0.00 hrs, Volume= 0.00 af	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 298.06' @ 12.13 hrs

Flood Elev= 301.16'

Device	Routing	Invert	Outlet Devices
#1	Primary	297.70'	12.0" Round Culvert
			L= 34.1' CMP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 297.70' / 297.30' S= 0.0117 '/' Cc= 0.900
			n= 0.025 Corrugated metal, Flow Area= 0.79 sf
#2	Secondary	301.16'	
			Limited to weir flow at low heads

Primary OutFlow Max=0.32 cfs @ 12.13 hrs HW=298.06' TW=297.49' (Dynamic Tailwater) 1=Culvert (Barrel Controls 0.32 cfs @ 1.88 fps)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=297.70' TW=0.00' (Dynamic Tailwater)

-2=Orifice/Grate (Controls 0.00 cfs)

Type III 24-hr 2-YEAR Rainfall=2.95"

Prepared by Keach-Nordstrom Associates, Inc.

Printed 12/6/2023

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 13

Summary for Pond 6P: EXISTING CB

Inflow Area = 5.703 ac, 2.62% Impervious, Inflow Depth > 0.72" for 2-YEAR event

Inflow = 2.63 cfs @ 12.40 hrs, Volume= 0.34 af

Outflow = 2.63 cfs @ 12.40 hrs, Volume= 0.34 af, Atten= 0%, Lag= 0.0 min

Primary = 2.63 cfs @ 12.40 hrs, Volume= 0.34 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 364.68' @ 12.40 hrs

Flood Elev= 366.35'

Device	Routing	Invert	Outlet Devices
#1	Primary	363.70'	12.0" Round Culvert
			L= 18.0' CMP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 363.70' / 362.46' S= 0.0689 '/' Cc= 0.900
			n= 0.025 Corrugated metal, Flow Area= 0.79 sf

Primary OutFlow Max=2.63 cfs @ 12.40 hrs HW=364.68' TW=0.00' (Dynamic Tailwater)
—1=Culvert (Inlet Controls 2.63 cfs @ 3.36 fps)

Summary for Pond 7P: EXISTING CB

Inflow Area = 4.676 ac, 2.12% Impervious, Inflow Depth > 0.72" for 2-YEAR event

Inflow = 2.29 cfs @ 12.33 hrs, Volume= 0.28 af

Outflow = 2.29 cfs @ 12.33 hrs, Volume= 0.28 af, Atten= 0%, Lag= 0.0 min

Primary = 2.29 cfs @ 12.33 hrs, Volume= 0.28 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 377.48' @ 12.33 hrs

Flood Elev= 380.49'

Device	Routing	Invert	Outlet Devices
#1	Primary	376.57'	12.0" Round Culvert L= 31.9' RCP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 376.57' / 376.28' S= 0.0091 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf

Primary OutFlow Max=2.29 cfs @ 12.33 hrs HW=377.48' TW=0.00' (Dynamic Tailwater) 1=Culvert (Barrel Controls 2.29 cfs @ 4.01 fps)

Summary for Link A: BARRETTS HILL CROSSING

Inflow Area = 35.450 ac, 4.96% Impervious, Inflow Depth > 0.48" for 2-YEAR event

Inflow = 7.79 cfs @ 12.60 hrs, Volume= 1.43 af

Primary = 7.79 cfs @ 12.60 hrs, Volume= 1.43 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

2304141-PRE DEVELOPMENT

Type III 24-hr 2-YEAR Rainfall=2.95"

Prepared by Keach-Nordstrom Associates, Inc.

Printed 12/6/2023

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 14

Summary for Link B: LOT 57

Inflow Area = 5.703 ac, 2.62% Impervious, Inflow Depth > 0.72" for 2-YEAR event

Inflow = 2.63 cfs @ 12.40 hrs, Volume= 0.34 af

Primary = 2.63 cfs @ 12.40 hrs, Volume= 0.34 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Summary for Link C: LOT 11

Inflow Area = 4.676 ac, 2.12% Impervious, Inflow Depth > 0.72" for 2-YEAR event

Inflow = 2.29 cfs @ 12.33 hrs, Volume= 0.28 af

Primary = 2.29 cfs @ 12.33 hrs, Volume= 0.28 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

2304141-PRE DEVELOPMENT

Type III 24-hr 10-YEAR Rainfall=4.47"

Prepared by Keach-Nordstrom Associates, Inc.
HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Printed 12/6/2023

Page 15

Time span=0.00-24.00 hrs, dt=0.01 hrs, 2401 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Runoff Area=1,062,579 sf 2.71% Impervious Runoff Depth>1.28" **Subcatchment 1S: SUBCATCHMENT** Flow Length=2.203' Tc=38.1 min CN=64.7 Runoff=17.21 cfs 2.60 af Runoff Area=113,081 sf 12.76% Impervious Runoff Depth>1.08" Subcatchment 2S: SUBCATCHMENT Flow Length=683' Tc=20.1 min CN=61.4 Runoff=1.94 cfs 0.23 af Runoff Area=128,229 sf 6.91% Impervious Runoff Depth>1.10" Subcatchment 3S: SUBCATCHMENT Flow Length=1.189' Tc=23.1 min CN=61.7 Runoff=2.12 cfs 0.27 af Runoff Area=223,873 sf 8.43% Impervious Runoff Depth>1.63" Subcatchment 4S: SUBCATCHMENT Flow Length=1,889' Tc=31.1 min CN=69.9 Runoff=5.32 cfs 0.70 af Runoff Area=16,441 sf 34.30% Impervious Runoff Depth>1.92" Subcatchment 5S: SUBCATCHMENT Flow Length=389' Tc=8.7 min CN=73.7 Runoff=0.77 cfs 0.06 af Runoff Area=248,437 sf 2.62% Impervious Runoff Depth>1.70" Subcatchment 6S: SUBCATCHMENT Flow Length=1.105' Tc=24.4 min CN=70.8 Runoff=6.88 cfs 0.81 af Runoff Area=203,705 sf 2.12% Impervious Runoff Depth>1.70" Subcatchment 7S: SUBCATCHMENT Flow Length=1,021' Tc=20.7 min CN=70.8 Runoff=6.03 cfs 0.66 af Reach 1R: EXISTING ROADWAY DITCH Avg. Flow Depth=0.67' Max Vel=4.14 fps Inflow=5.61 cfs 0.76 af n=0.022 L=230.0' S=0.0173 '/' Capacity=16.21 cfs Outflow=5.60 cfs 0.76 af Inflow=17.21 cfs 2.60 af Pond 1P: EXISTING CB Primary=17.21 cfs 2.60 af Peak Elev=293.20' Inflow=1.94 cfs 0.23 af Pond 2P: EXISTING DRIVEWAY CULVERT 12.0" Round Culvert n=0.013 L=21.0' S=0.0143 '/' Outflow=1.94 cfs 0.23 af Peak Elev=292.54' Inflow=25.69 cfs 3.86 af Pond 3P: EXISTING DROP INLET 24.0" Round Culvert n=0.011 L=92.1' S=0.0433'/' Outflow=25.69 cfs 3.86 af Pond 4P: EXISTING CB Peak Elev=300,29' Inflow=5.61 cfs 0.76 af Primary=4.49 cfs 0.73 af Secondary=1.12 cfs 0.02 af Outflow=5.61 cfs 0.76 af Peak Elev=300.30' Inflow=0.77 cfs 0.06 af Pond 5P: EXISTING CB Primary=0.77 cfs 0.06 af Secondary=0.00 cfs 0.00 af Outflow=0.77 cfs 0.06 af Peak Elev=367.74' Inflow=6.88 cfs 0.81 af Pond 6P: EXISTING CB 12.0" Round Culvert n=0.025 L=18.0' S=0.0689 '/' Outflow=6.88 cfs 0.81 af Pond 7P: EXISTING CE Peak Elev=379.62' Inflow=6.03 cfs 0.66 af 12.0" Round Culvert n=0.011 L=31.9' S=0.0091 '/' Outflow=6.03 cfs 0.66 af

Link A: BARRETTS HILL CROSSING

Inflow=25.69 cfs 3.86 af Primary=25.69 cfs 3.86 af

2304141-PRE DEVELOPMENT

Type III 24-hr 10-YEAR Rainfall=4.47"

Prepared by Keach-Nordstrom Associates, Inc.

Printed 12/6/2023

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 16

Link B: LOT 57

Inflow=6.88 cfs 0.81 af

Primary=6.88 cfs 0.81 af

Link C: LOT 11

Inflow=6.03 cfs 0.66 af

Primary=6.03 cfs 0.66 af

Total Runoff Area = 45.830 ac Runoff Volume = 5.33 af Average Runoff Depth = 1.40" 95.62% Pervious = 43.824 ac 4.38% Impervious = 2.006 ac

Type III 24-hr 10-YEAR Rainfall=4.47"

Prepared by Keach-Nordstrom Associates, Inc.

Printed 12/6/2023

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 17

Summary for Subcatchment 1S: SUBCATCHMENT

17.21 cfs @ 12.58 hrs, Volume= 2.60 af, Depth> 1.28" Runoff

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 10-YEAR Rainfall=4.47"

	A	rea (sf)	CN	Description	n				
		7,670	30.0	Woods, Good, HSG A					
	3	63,085	55.0	Woods, G	Good, HSG	В			
	574,693 70.0			Woods, G	Good, HSG	C			
		6,684	39.0	>75% Gra	>75% Grass cover, Good, HSG A				
		61,618	61.0	>75% Gra	ass cover, (Good, HSG B			
		20,067	74.0	>75% Gra	ass cover, (Good, HSG C			
*		14,404	98.0	Roofs					
*		14,358	98.0	Paved pa	rking				
	1,0	62,579	64.7	Weighted	Average				
	1,0	33,817	63.8	97.29% P	ervious Are	ea			
	•	28,762	98.0	2.71% lm	pervious A	rea			
	Tc	Length	Slope	Velocity	Capacity	Description			
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
	17.3	100	0.0400	0.10		Sheet Flow,			
						Woods: Light underbrush n= 0.400 P2= 2.84"			
	19.7	1,862	0.0988	1.57		Shallow Concentrated Flow,			
						Woodland Kv= 5.0 fps			
	0.3	30	0.0800	1.98		Shallow Concentrated Flow,			
						Short Grass Pasture Kv= 7.0 fps			
	8.0	211	0.0430	4.21		Shallow Concentrated Flow,			
_						Paved Kv= 20.3 fps			
	38.1	2.203	Total						

Summary for Subcatchment 2S: SUBCATCHMENT

0.23 af, Depth> 1.08" Runoff 1.94 cfs @ 12.32 hrs, Volume=

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 10-YEAR Rainfall=4.47"

	Area (sf)	CN	Description
	28,414	30.0	Woods, Good, HSG A
	11,383	55.0	Woods, Good, HSG B
	33,808	70.0	Woods, Good, HSG C
	4,719	39.0	>75% Grass cover, Good, HSG A
	242	61.0	>75% Grass cover, Good, HSG B
	20,087	74.0	>75% Grass cover, Good, HSG C
*	3,849	98.0	Roofs
*	10,579	98.0	Paved parking
	113,081	61.4	Weighted Average
	98,653	56.1	87.24% Pervious Area
	14,428	98.0	12.76% Impervious Area

2304141-PRE DEVELOPMENT

Type III 24-hr 10-YEAR Rainfall=4.47"

Prepared by Keach-Nordstrom Associates, Inc.
HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Printed 12/6/2023

Page 18

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
12.0	100	0.1000	0.14		Sheet Flow,
3.9	318	0.0750	1.37		Woods: Light underbrush n= 0.400 P2= 2.84" Shallow Concentrated Flow, Woodland Kv= 5.0 fps
0.7	48	0.0300	1.21		Shallow Concentrated Flow,
3.6	217	0.0410	1.01		Short Grass Pasture Kv= 7.0 fps Shallow Concentrated Flow, Woodland Kv= 5.0 fps
20.1	683	Total			

Summary for Subcatchment 3S: SUBCATCHMENT

Runoff = 2.12 cfs @ 12.37 hrs, Volume=

0.27 af, Depth> 1.10"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 10-YEAR Rainfall=4.47"

	A	rea (sf)	CN	Description	on			
		5,732	30.0	Woods, C	Woods, Good, HSG A			
		49,737	55.0	Woods, Good, HSG B				
		38,476	70.0		Good, HSG			
		4,799	39.0		>75% Grass cover, Good, HSG A			
		20,628	61.0			Good, HSG B		
*		2,833	98.0	Roofs	,			
* 6,024 98.0 Paved parking								
	128,229 61.7			Weighted Average				
			59.0	93.09% Pervious Area				
	-		98.0	6.91% Impervious Area				
		•			, , , , , , ,			
	Tc	Length	Slope	Velocity	Capacity	Description		
(r	min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	2 doon place		
	9.9	100	0.1600	0.17	1	Sheet Flow,		
				• • • • • • • • • • • • • • • • • • • •		Woods: Light underbrush n= 0.400 P2= 2.84"		
1	13.2	1,089	0.0760	1.38		Shallow Concentrated Flow,		
		·				Woodland Kv= 5.0 fps		
2	23.1	1,189	Total			Totalsila III Olo Ipo		

Summary for Subcatchment 4S: SUBCATCHMENT

Runoff = 5.32 cfs @ 12.47 hrs, Volume=

0.70 af, Depth> 1.63"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 10-YEAR Rainfall=4.47"

Page 19

2304141-PRE DEVELOPMENT

Type III 24-hr 10-YEAR Rainfall=4.47"

Printed 12/6/2023 Prepared by Keach-Nordstrom Associates, Inc. HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

	A	rea (sf)	CN	Description	n				
		12,309	55.0	В					
	1	50,990	70.0	Woods, G	ood, HSG	C			
		41,699	61.0	>75% Gra	iss cover, (Good, HSG B			
*		2,548	98.0	Roofs					
*		16,327	98.0	Paved par	Paved parking				
()	2	23,873	69.9	Weighted	Average				
	2	04,998	67.3	91.57% P	ervious Are	ea			
		18,875	98.0	8.43% lm	pervious A	rea			
	Tc	Length	Slope	Velocity	Capacity	Description			
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
	15.8	100	0.0500	0.11		Sheet Flow,			
						Woods: Light underbrush n= 0.400 P2= 2.84"			
	10.6	1,127	0.1250	1.77		Shallow Concentrated Flow,			
				4.00		Woodland Kv= 5.0 fps			
	0.5	55	0.0730	1.89		Shallow Concentrated Flow,			
		00	0.0500	4.04		Short Grass Pasture Kv= 7.0 fps			
	0.9	68	0.0590	1.21		Shallow Concentrated Flow, Woodland Kv= 5.0 fps			
	0.0	20	0.4000	2.21		Shallow Concentrated Flow,			
	0.2	30	0.1000	2.21		Short Grass Pasture Kv= 7.0 fps			
	0.1	16	0.0200	2.87		Shallow Concentrated Flow,			
	0.1	10	0.0200	2.07		Paved Kv= 20.3 fps			
	2.1	240	0.0750	1.92		Shallow Concentrated Flow,			
	2. 1	240	0.0700	1.02		Short Grass Pasture Kv= 7.0 fps			
	0.3	73	0.0410	4.11		Shallow Concentrated Flow,			
	0.0	70	0.0110			Paved Kv= 20.3 fps			
	0.6	180	0.0720	5.45		Shallow Concentrated Flow,			
	0.0		3.0.20			Paved Kv= 20.3 fps			
	31.1	1,889	Total						

Summary for Subcatchment 5S: SUBCATCHMENT

0.77 cfs @ 12.13 hrs, Volume= 0.06 af, Depth> 1.92" Runoff

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 10-YEAR Rainfall=4.47"

	Area (sf)	CN	Description
	10,801	61.0	>75% Grass cover, Good, HSG B
*	5,640	98.0	Paved parking
	16,441	73.7	Weighted Average
	10,801	61.0	65.70% Pervious Area
	5,640	98.0	34.30% Impervious Area

2304141-PRE DEVELOPMENT

Type III 24-hr 10-YEAR Rainfall=4.47"

Prepared by Keach-Nordstrom Associates, Inc.

Printed 12/6/2023

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 20

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.3	100	0.0700	0.26		Sheet Flow,
0.4					Grass: Short n= 0.150 P2= 2.84"
0.1	16	0.0400	4.06		Shallow Concentrated Flow,
4.5	400	0.0000			Paved Kv= 20.3 fps
1.5	162	0.0680	1.83		Shallow Concentrated Flow,
					Short Grass Pasture Kv= 7.0 fps
0.1	20	0.0400	4.06		Shallow Concentrated Flow,
					Paved Kv= 20.3 fps
0.8	91	0.0770	1.94		Shallow Concentrated Flow,
					Short Grass Pasture Kv= 7.0 fps
8.7	389	Total			

Summary for Subcatchment 6S: SUBCATCHMENT

Runoff = 6.88 cfs @ 12.35 hrs, Volume=

0.81 af, Depth> 1.70"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 10-YEAR Rainfall=4.47"

	A	rea (sf)	CN	Description	n				
	2	37,230	70.0	Woods, G	/oods, Good, HSG C				
*		4,709	74.0	>75% Gra	ass cover, (Good, HSG C			
*		6,498	98.0		Paved parking				
	2	48,437	70.8	Weighted	Weighted Average				
	2	41,939	70.1	97.38% P	ervious Are	ea			
		6,498	98.0		2.62% Impervious Area				
				'					
	Тс	Length	Slope	Velocity	Capacity	Description			
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
	15.8	100	0.0500	0.11		Sheet Flow,			
						Woods: Light underbrush n= 0.400 P2= 2.84"			
	8.1	856	0.1250	1.77		Shallow Concentrated Flow,			
						Woodland Kv= 5.0 fps			
	0.0	10	0.3000	3.83		Shallow Concentrated Flow,			
						Short Grass Pasture Kv= 7.0 fps			
	0.5	139	0.0500	4.54		Shallow Concentrated Flow,			
_						Paved Kv= 20.3 fps			
	24.4	1,105	Total						

Summary for Subcatchment 7S: SUBCATCHMENT

Runoff = 6.03 cfs @ 12.30 hrs, Volume=

0.66 af, Depth> 1.70"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 10-YEAR Rainfall=4.47"

2304141-PRE DEVELOPMENT

Type III 24-hr 10-YEAR Rainfall=4.47"

Prepared by Keach-Nordstrom Associates, Inc. HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC Printed 12/6/2023

Page 21

	Aı	rea (sf)	CN	Description	n				
	195,095 70.0			Woods, G	Woods, Good, HSG C				
		3,533	74.0	>75% Gra	iss cover, (Good, HSG C			
		4,313	98.0	Paved par	rking, HSG	C			
190		764	96.0	Gravel su	Gravel surface, HSG C				
	2	03,705	70.8	Weighted	Average				
	1	99,392	70.2	97.88% P	ervious Are	ea			
		4,313	98.0	2.12% lm	pervious Ai	rea			
	Тс	Length	Slope		Capacity	Description			
09	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
	12.5	100	0.0900	0.13		Sheet Flow,			
						Woods: Light underbrush n= 0.400 P2= 2.84"			
	7.5	755	0.1130	1.68		Shallow Concentrated Flow,			
						Woodland Kv= 5.0 fps			
	0.0	6	0.3000	3.83		Shallow Concentrated Flow,			
						Short Grass Pasture Kv= 7.0 fps			
	0.7	160	0.0375	3.93		Shallow Concentrated Flow,			
						Paved Kv= 20.3 fps			
	20.7	1.021	Total						

Summary for Reach 1R: EXISTING ROADWAY DITCH

Inflow Area = 5.517 ac, 10.20% Impervious, Inflow Depth > 1.65" for 10-YEAR event

Inflow = 5.61 cfs @ 12.44 hrs, Volume= 0.76 af

Outflow = 5.60 cfs @ 12.46 hrs, Volume= 0.76 af, Atten= 0%, Lag= 1.2 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Max. Velocity= 4.14 fps, Min. Travel Time= 0.9 min Avg. Velocity = 2.07 fps, Avg. Travel Time= 1.8 min

Peak Storage= 311 cf @ 12.46 hrs Average Depth at Peak Storage= 0.67'

Bank-Full Depth= 1.00' Flow Area= 3.0 sf, Capacity= 16.21 cfs

0.00' x 1.00' deep channel, n= 0.022 Earth, clean & straight

Side Slope Z-value= 3.0 '/' Top Width= 6.00'

Length= 230.0' Slope= 0.0173 '/'

Inlet Invert= 295.64', Outlet Invert= 291.66'



2304141-PRE DEVELOPMENT

Type III 24-hr 10-YEAR Rainfall=4.47"

Prepared by Keach-Nordstrom Associates, Inc. HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Printed 12/6/2023

Page 22

Summary for Pond 1P: EXISTING CB

Inflow Area = 24.393 ac, 2.71% Impervious, Inflow Depth > 1.28" for 10-YEAR event

Inflow = 17.21 cfs @ 12.58 hrs, Volume= 2.60 af

Primary = 17.21 cfs @ 12.58 hrs, Volume= 2.60 af, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Summary for Pond 2P: EXISTING DRIVEWAY CULVERT

Inflow Area = 2.596 ac, 12.76% Impervious, Inflow Depth > 1.08" for 10-YEAR event

Inflow = 1.94 cfs @ 12.32 hrs, Volume= 0.23 af

Outflow = 1.94 cfs @ 12.32 hrs, Volume= 0.23 af, Atten= 0%, Lag= 0.0 min

Primary = 1.94 cfs @ 12.32 hrs, Volume= 0.23 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 293.20' @ 12.32 hrs

Flood Elev= 294.00'

Primary OutFlow Max=1.94 cfs @ 12.32 hrs HW=293.20' TW=291.44' (Dynamic Tailwater) 1=Culvert (Barrel Controls 1.94 cfs @ 3.94 fps)

Summary for Pond 3P: EXISTING DROP INLET

Inflow Area = 35.450 ac, 4.96% Impervious, Inflow Depth > 1.31" for 10-YEAR event

Inflow = 25.69 cfs @ 12.53 hrs, Volume= 3.86 af

Outflow = 25.69 cfs @ 12.53 hrs, Volume= 3.86 af, Atten= 0%, Lag= 0.0 min

Primary = 25.69 cfs @ 12.53 hrs, Volume= 3.86 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 292.54' @ 12.53 hrs

Flood Elev= 293.75'

Device	Routing	Invert	Outlet Devices
#1	Primary	288.66'	24.0" Round Culvert
			L= 92.1' RCP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 288.66' / 284.67' S= 0.0433 '/' Cc= 0.900
			n= 0.011 .Concrete pipe, straight & clean, Flow Area= 3.14 sf

Primary OutFlow Max=25.69 cfs @ 12.53 hrs HW=292.54' TW=0.00' (Dynamic Tailwater) 1=Culvert (Inlet Controls 25.69 cfs @ 8.18 fps)

Type III 24-hr 10-YEAR Rainfall=4.47"

Prepared by Keach-Nordstrom Associates, Inc.

Printed 12/6/2023

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 23

Summary for Pond 4P: EXISTING CB

Inflow Area = 5.517 ac, 10.20% Impervious, Inflow Depth > 1.65" for 10-YEAR event Inflow = 5.61 cfs @ 12.44 hrs, Volume= 0.76 af Outflow = 5.61 cfs @ 12.44 hrs, Volume= 0.76 af, Atten= 0%, Lag= 0.0 min Primary = 4.49 cfs @ 12.44 hrs, Volume= 0.73 af Secondary = 1.12 cfs @ 12.44 hrs, Volume= 0.02 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 300.29' @ 12.44 hrs

Flood Elev= 300.17'

Device	Routing	Invert	Outlet Devices
#1	Primary	297.00'	12.0" Round Culvert
	•		L= 49.1' CMP, square edge headwall, Ke= 0.500
			Inlet / Outlet invert= 297.00' / 295.64' S= 0.0277 '/' Cc= 0.900
			n= 0.025 Corrugated metal, Flow Area= 0.79 sf
#2	Secondary	300.17'	24.0" x 24.0" Horiz. Orifice/Grate C= 0.600
	•		Limited to weir flow at low heads

Primary OutFlow Max=4.49 cfs @ 12.44 hrs HW=300.29' TW=296.31' (Dynamic Tailwater)
1=Culvert (Barrel Controls 4.49 cfs @ 5.71 fps)

Secondary OutFlow Max=1.12 cfs @ 12.44 hrs HW=300.29' TW=296.31' (Dynamic Tailwater)

—2=Orifice/Grate (Weir Controls 1.12 cfs @ 1.14 fps)

Summary for Pond 5P: EXISTING CB

Inflow Area =	0.377 ac, 34.30% Impervious, Inflow D	Depth > 1.92" for 10-YEAR event
Inflow =	0.77 cfs @ 12.13 hrs, Volume=	0.06 af
Outflow =	0.77 cfs @ 12.13 hrs, Volume=	0.06 af, Atten= 0%, Lag= 0.0 min
Primary =	0.77 cfs @ 12.13 hrs, Volume=	0.06 af
Secondary =	0.00 cfs @ 0.00 hrs, Volume=	0.00 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 300.30' @ 12.44 hrs

Flood Elev= 301.16'

Device	Routing	Invert	Outlet Devices
#1	Primary	297.70'	12.0" Round Culvert
	,		L= 34.1' CMP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 297.70' / 297.30' S= 0.0117 '/' Cc= 0.900
			n= 0.025 Corrugated metal, Flow Area= 0.79 sf
#2	Secondary	301.16'	24.0" x 24.0" Horiz. Orifice/Grate C= 0.600
	•		Limited to weir flow at low heads

Primary OutFlow Max=0.72 cfs @ 12.13 hrs HW=298.36' TW=298.06' (Dynamic Tailwater) 1=Culvert (Outlet Controls 0.72 cfs @ 1.87 fps)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=297.70' TW=0.00' (Dynamic Tailwater) 2=Orifice/Grate (Controls 0.00 cfs)

Type III 24-hr 10-YEAR Rainfall=4.47"

Prepared by Keach-Nordstrom Associates, Inc.

Printed 12/6/2023

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 24

Summary for Pond 6P: EXISTING CB

Inflow Area = 5.703 ac, 2.62% Impervious, Inflow Depth > 1.70" for 10-YEAR event

Inflow = 6.88 cfs @ 12.35 hrs, Volume= 0.81 af

Outflow = 6.88 cfs @ 12.35 hrs, Volume= 0.81 af, Atten= 0%, Lag= 0.0 min

Primary = 6.88 cfs @ 12.35 hrs, Volume= 0.81 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 367.74' @ 12.35 hrs

Flood Elev= 366.35'

Device Routing Invert Outlet Devices

#1 Primary 363.70' 12.0" Round Culvert

L= 18.0' CMP, square edge headwall, Ke= 0.500
Inlet / Outlet Invert= 363.70' / 362.46' S= 0.0689 '/' Cc= 0.900
n= 0.025 Corrugated metal, Flow Area= 0.79 sf

Primary OutFlow Max=6.88 cfs @ 12.35 hrs HW=367.74' TW=0.00' (Dynamic Tailwater) 1=Culvert (Barrel Controls 6.88 cfs @ 8.76 fps)

Summary for Pond 7P: EXISTING CB

Inflow Area = 4.676 ac, 2.12% Impervious, Inflow Depth > 1.70" for 10-YEAR event

Inflow = 6.03 cfs @ 12.30 hrs, Volume= 0.66 af

Outflow = 6.03 cfs @ 12.30 hrs, Volume= 0.66 af, Atten= 0%, Lag= 0.0 min

Primary = 6.03 cfs @ 12.30 hrs, Volume= 0.66 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 379.62' @ 12.30 hrs

Flood Elev= 380.49'

Device	Routing	Invert	Outlet Devices
#1	Primary	376.57'	12.0" Round Culvert
			L= 31.9' RCP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 376.57' / 376.28' S= 0.0091 '/' Cc= 0.900
			n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf

Primary OutFlow Max=6.03 cfs @ 12.30 hrs HW=379.61' TW=0.00' (Dynamic Tailwater) 1=Culvert (Inlet Controls 6.03 cfs @ 7.68 fps)

Summary for Link A: BARRETTS HILL CROSSING

Inflow Area = 35.450 ac, 4.96% Impervious, Inflow Depth > 1.31" for 10-YEAR event

Inflow = 25.69 cfs @ 12.53 hrs, Volume= 3.86 af

Prirnary = 25.69 cfs @ 12.53 hrs, Volume= 3.86 af, Atten= 0%, Lag= 0.0 min

Prirnary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

2304141-PRE DEVELOPMENT

Type III 24-hr 10-YEAR Rainfall=4.47"

Prepared by Keach-Nordstrom Associates, Inc.

Printed 12/6/2023

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 25

Summary for Link B: LOT 57

Inflow Area = 5.703 ac, 2.62% Impervious, Inflow Depth > 1.70" for 10-YEAR event

Inflow = 6.88 cfs @ 12.35 hrs, Volume= 0.81 af

Primary = 6.88 cfs @ 12.35 hrs, Volume= 0.81 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Summary for Link C: LOT 11

Inflow Area = 4.676 ac, 2.12% Impervious, Inflow Depth > 1.70" for 10-YEAR event

Inflow = 6.03 cfs @ 12.30 hrs, Volume= 0.66 af

Primary = 6.03 cfs @ 12.30 hrs, Volume= 0.66 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

2304141-PRE DEVELOPMENT

Type III 24-hr 25-YEAR Rainfall=5.66"

Prepared by Keach-Nordstrom Associates, Inc. HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Printed 12/6/2023

Page 26

Time span=0.00-24.00 hrs, dt=0.01 hrs, 2401 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment 1S: SUBCATCHMENT Runoff Area=1,062,579 sf 2.71% Impervious Runoff Depth>2.06" Flow Length=2,203' Tc=38.1 min CN=64.7 Runoff=29.03 cfs 4.19 af

Subcatchment 2S: SUBCATCHMENT

Runoff Area=113,081 sf 12.76% Impervious Runoff Depth>1.80"
Flow Length=683' Tc=20.1 min CN=61.4 Runoff=3.49 cfs 0.39 af

Subcatchment 3S: SUBCATCHMENT

Runoff Area=128,229 sf 6.91% Impervious Runoff Depth>1.83"

Flow Length=1,189' Tc=23.1 min CN=61.7 Runoff=3.80 cfs 0.45 af

Subcatchment 4S: SUBCATCHMENT

Runoff Area=223,873 sf 8.43% Impervious Runoff Depth>2.51"

Flow Length=1,889' Tc=31.1 min CN=69.9 Runoff=8.36 cfs 1.07 af

Subcatchment 5S: SUBCATCHMENT

Runoff Area=16,441 sf 34.30% Impervious Runoff Depth>2.87"

Flow Length=389' Tc=8.7 min CN=73.7 Runoff=1.16 cfs 0.09 af

Subcatchment 6S: SUBCATCHMENT

Runoff Area=248,437 sf 2.62% Impervious Runoff Depth>2.60"

Flow Length=1,105' Tc=24.4 min CN=70.8 Runoff=10.73 cfs 1.23 af

Subcatchment 7S: SUBCATCHMENT

Runoff Area=203,705 sf 2.12% Impervious Runoff Depth>2.59"

Flow Length=1,021' Tc=20.7 min CN=70.8 Runoff=9.42 cfs 1.01 af

Reach 1R: EXISTING ROADWAY DITCH Avg. Flow Depth=0.80' Max Vel=4.64 fps Inflow=8.80 cfs 1.16 af n=0.022 L=230.0' S=0.0173 '/' Capacity=16.21 cfs Outflow=8.80 cfs 1.16 af

Pond 1P: EXISTING CB Inflow=29.03 cfs 4.19 af Primary=29.03 cfs 4.19 af

Pond 2P: EXISTING DRIVEWAY CULVERT Peak Elev=298.26' Inflow=3.49 cfs 0.39 af 12.0" Round Culvert n=0.013 L=21.0' S=0.0143 '/' Outflow=3.49 cfs 0.39 af

Pond 3P: EXISTING DROP INLET

Peak Elev=297.77' Inflow=43.08 cfs 6.19 af 24.0" Round Culvert n=0.011 L=92.1' S=0.0433 '/' Outflow=43.08 cfs 6.19 af

Pond 4P: EXISTING CB

Peak Elev=300 47' Inflow=8 80 cfs 1 16 a

Pond 4P: EXISTING CB
Peak Elev=300.47' Inflow=8.80 cfs 1.16 af
Primary=4.59 cfs 1.01 af Secondary=4.21 cfs 0.15 af Outflow=8.80 cfs 1.16 af

Pond 5P: EXISTING CB

Peak Elev=300.50' Inflow=1.16 cfs 0.09 af

Primary=1.16 cfs 0.09 af Secondary=0.00 cfs 0.00 af Outflow=1.16 cfs 0.09 af

,

Pond 6P: EXISTING CB Peak Elev=373.86' Inflow=10.73 cfs 1.23 af 12.0" Round Culvert n=0.025 L=18.0' S=0.0689 '/' Outflow=10.73 cfs 1.23 af

Pond 7P: EXISTING CB

Peak Elev=383.28' Inflow=9.42 cfs 1.01 af

12.0" Round Culvert n=0.011 L=31.9' S=0.0091 '/' Outflow=9.42 cfs 1.01 af

Link A: BARRETTS HILL CROSSING

Inflow=43.08 cfs 6.19 af
Primary=43.08 cfs 6.19 af

2304141-PRE DEVELOPMENT

Type III 24-hr 25-YEAR Rainfall=5.66"

Prepared by Keach-Nordstrom Associates, Inc.

Printed 12/6/2023

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 27

Link B: LOT 57 Inflow=10.73 cfs 1.23 af

Primary=10.73 cfs 1.23 af

Link C: LOT 11 Inflow=9.42 cfs 1.01 af

Primary=9.42 cfs 1.01 af

Total Runoff Area = 45.830 ac Runoff Volume = 8.44 af Average Runoff Depth = 2.21" 95.62% Pervious = 43.824 ac 4.38% Impervious = 2.006 ac

2304141-PRE DEVELOPMENT

Type III 24-hr 25-YEAR Rainfall=5.66"

Prepared by Keach-Nordstrom Associates, Inc.

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Printed 12/6/2023

Page 28

Summary for Subcatchment 1S: SUBCATCHMENT

Runoff = 29.03 cfs @ 12.57 hrs, Volume=

4.19 af, Depth> 2.06"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 25-YEAR Rainfall=5.66"

_	A	rea (sf)	CN	Description	on	
	7,670 30.0 Woods, Good, HSG A					A
	363,085 55.0 Woods, Good, HSG E					В
	574,693 70.0 Woods, Good, HSG (
	6,684 39.0 >75% Grass cover, G					
		61,618	61.0			Good, HSG B
		20,067	74.0	>75% Gra	ass cover, (Good, HSG C
*		14,404	98.0	Roofs		
*		14,358	98.0	Paved pa	rking	
	1,0	62,579	64.7	Weighted	Average	
	1,0	33,817	63.8	97.29% P	ervious Are	ea
		28,762 98.0 2.71% Impervious Ar				rea
	_					
	Tc	Length	Slope	Velocity	Capacity	Description
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	17.3	100	0.0400	0.10		Sheet Flow,
						Woods: Light underbrush n= 0.400 P2= 2.84"
	19.7	1,862	0.0988	1.57		Shallow Concentrated Flow,
						Woodland Kv= 5.0 fps
	0.3	30	0.0800	1.98		Shallow Concentrated Flow,
			_			Short Grass Pasture Kv= 7.0 fps
	8.0	211	0.0430	4.21		Shallow Concentrated Flow,
-						Paved Kv= 20.3 fps
	38.1	2,203	Total			

Summary for Subcatchment 2S: SUBCATCHMENT

Runoff = 3.49 cfs @ 12.29 hrs, Volume=

0.39 af, Depth> 1.80"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 25-YEAR Rainfall=5.66"

	Area (sf)	CN	Description
	28,414	30.0	Woods, Good, HSG A
	11,383	55.0	Woods, Good, HSG B
	33,808	70.0	Woods, Good, HSG C
	4,719	39.0	>75% Grass cover, Good, HSG A
	242	61.0	>75% Grass cover, Good, HSG B
	20,087	74.0	>75% Grass cover, Good, HSG C
*	3,849	98.0	Roofs
*	10,579	98.0	Paved parking
	113,081	61.4	Weighted Average
	98,653	56.1	87.24% Pervious Area
	14,428	98.0	12.76% Impervious Area

2304141-PRE DEVELOPMENT

Type III 24-hr 25-YEAR Rainfall=5.66"

Prepared by Keach-Nordstrom Associates, Inc.

Printed 12/6/2023

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 29

	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	12.0	100	0.1000	0.14		Sheet Flow,
						Woods: Light underbrush n= 0.400 P2= 2.84"
	3.9	318	0.0750	1.37		Shallow Concentrated Flow,
						Woodland Kv= 5.0 fps
	0.7	48	0.0300	1.21		Shallow Concentrated Flow,
						Short Grass Pasture Kv= 7.0 fps
	3.6	217	0.0410	1.01		Shallow Concentrated Flow,
						Woodland Kv= 5.0 fps
_	20.1	683	Total			

Summary for Subcatchment 3S: SUBCATCHMENT

Runoff = 3.80 cfs @ 12.34 hrs, Volume=

0.45 af, Depth> 1.83"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 25-YEAR Rainfall=5.66"

	Aı	rea (sf)	CN	Description	n			
		5,732	30.0	Woods, G	Good, HSG	A		
		49,737	55.0	Woods, G	Good, HSG	В		
		38,476	70.0	Woods, G	Good, HSG	C		
4,799 39.0 >75% Grass cover, Good, HSG A						Good, HSG A		
20,628 61.0 >75% Grass cover, Good, HSG B						Good, HSG B		
*		2,833	98.0	Roofs				
* 6,024 98.0 Paved parking								
-	128,229 61.7 Weighted Average							
	119,372 59.0			93.09% Pervious Area				
	8,857 98.0			6.91% Impervious Area				
	Тс	Length	Slope	Velocity	Capacity	Description		
(m	in)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
	9.9	100	0.1600	0.17		Sheet Flow,		
						Woods: Light underbrush n= 0.400 P2= 2.84"		
13	3.2	1,089	0.0760	1.38		Shallow Concentrated Flow,		
		•				Woodland Kv= 5.0 fps		
2	3.1	1,189	Total					

Summary for Subcatchment 4S: SUBCATCHMENT

Runoff = 8.36 cfs @ 12.44 hrs, Volume=

1.07 af, Depth> 2.51"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 25-YEAR Rainfall=5.66"

2304141-PRE DEVELOPMENT

Type III 24-hr 25-YEAR Rainfall=5.66"

Prepared by Keach-Nordstrom Associates, Inc.

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Printed 12/6/2023 Page 30

	٨	rea (sf)	CN	Doporintia				
-			Description					
	*		Woods, Good, HSG B					
	*		Woods, Good, HSG C					
*	41,699 61.0 2,548 98.0		>75% Grass cover, Good, HSG B Roofs					
*								
* 16,327 98.0 Paved parking								
		23,873	69.9		Weighted Average			
		04,998	67.3		ervious Are			
		18,875	98.0	8.43% lm	pervious A	rea		
	То	ما المسمدة	01	V/-1 10	0 "			
	Tc (min)	Length	Slope	Velocity	Capacity	Description		
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
	15.8	100	0.0500	0.11		Sheet Flow,		
	10.0	4 407	0.4050	4		Woods: Light underbrush n= 0.400 P2= 2.84"		
	10.6	1,127	0.1250	1.77		Shallow Concentrated Flow,		
	0.5		0.0700	4.00		Woodland Kv= 5.0 fps		
	0.5	55	0.0730	1.89		Shallow Concentrated Flow,		
	0.9	68	0.0590	4.04		Short Grass Pasture Kv= 7.0 fps		
	0.9	00	0.0590	1.21		Shallow Concentrated Flow,		
	0.2	30	0.1000	2.24		Woodland Kv= 5.0 fps		
	0.2	30	0.1000	2.21		Shallow Concentrated Flow,		
	0.1	16	0.0200	2.87		Short Grass Pasture Kv= 7.0 fps		
	0.1	10	0.0200	2.01		Shallow Concentrated Flow,		
	2.1	240	0.0750	1.92		Paved Kv= 20.3 fps		
	۷. ۱	240	0.0730	1.92		Shallow Concentrated Flow,		
	0.3	73	0.0410	4.11		Short Grass Pasture Kv= 7.0 fps		
	0.0	7.5	0.0410	4.11		Shallow Concentrated Flow,		
	0.6	180	0.0720	5.45		Paved Kv= 20.3 fps		
	0.0	100	0.0120	0.70		Shallow Concentrated Flow, Paved Kv= 20.3 fps		
	31.1	1,889	Total			raveu IV- 20.3 Ips		
	51.1	1,009	iUlai					

Summary for Subcatchment 5S: SUBCATCHMENT

Runoff = 1.16 cfs @ 12.12 hrs, Volume=

0.09 af, Depth> 2.87"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 25-YEAR Rainfall=5.66"

	Area (sf)	CN	Description
	10,801	61.0	>75% Grass cover, Good, HSG B
*	5,640		Paved parking
16,441 73.		73.7	Weighted Average
	10,801	61.0	65.70% Pervious Area
	5,640	98.0	34.30% Impervious Area

2304141-PRE DEVELOPMENT

Type III 24-hr 25-YEAR Rainfall=5.66"

Prepared by Keach-Nordstrom Associates, Inc.

Printed 12/6/2023

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 31

	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	6.3	100	0.0700	0.26		Sheet Flow,
						Grass: Short n= 0.150 P2= 2.84"
	0.1	16	0.0400	4.06		Shallow Concentrated Flow,
						Paved Kv= 20.3 fps
	1.5	162	0.0680	1.83		Shallow Concentrated Flow,
						Short Grass Pasture Kv= 7.0 fps
	0.1	20	0.0400	4.06		Shallow Concentrated Flow,
						Paved Kv= 20.3 fps
	0.8	91	0.0770	1.94		Shallow Concentrated Flow,
						Short Grass Pasture Kv= 7.0 fps
-	8.7	389	Total			

Summary for Subcatchment 6S: SUBCATCHMENT

10.73 cfs @ 12.35 hrs, Volume= Runoff

1.23 af, Depth> 2.60"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 25-YEAR Rainfall=5.66"

	A	rea (sf)	CN	Description	n					
	2	37,230	70.0	Woods, G	ood, HSG	С				
*		4,709	74.0	>75% Gra	>75% Grass cover, Good, HSG C					
*		6,498	98.0	Paved par	Paved parking					
_	2	48,437	70.8	Weighted	Average					
		41,939	70.1	•	ervious Are	ea				
		6,498	98.0	2.62% lm	pervious Ai	rea				
,										
	Tc	Length	Slope	Velocity	Capacity	Description				
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
	15.8	100	0.0500	0.11		Sheet Flow,				
						Woods: Light underbrush n= 0.400 P2= 2.84"				
	8.1	856	0.1250	1.77		Shallow Concentrated Flow,				
						Woodland Kv= 5.0 fps				
	0.0	10	0.3000	3.83		Shallow Concentrated Flow,				
						Short Grass Pasture Kv= 7.0 fps				
	0.5	139	0.0500	4.54		Shallow Concentrated Flow,				
						Paved Kv= 20.3 fps				
	24.4	1,105	Total							

1,105 Total

Summary for Subcatchment 7S: SUBCATCHMENT

9.42 cfs @ 12.29 hrs, Volume= Runoff

1.01 af, Depth > 2.59"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 25-YEAR Rainfall=5.66"

2304141-PRE DEVELOPMENT

Type III 24-hr 25-YEAR Rainfall=5.66"

Prepared by Keach-Nordstrom Associates, Inc.
HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Printed 12/6/2023 Page 32

	A	rea (sf)	CN	Description	Description						
	1	95,095	70.0	Woods, G	Good, HSG	С					
		3,533	74.0	>75% Gra	ass cover. (Good, HSG C					
		4,313	98.0		rking, HSG						
		764	96.0		rface, HSG						
-	2	03,705	70.8	Weighted	Average						
	1	99,392	70.2		ervious Are	ea					
		4,313	98.0	2.12% lm	pervious Ai	rea					
	Tc	Length	Slope	Velocity	Capacity	Description					
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	•					
	12.5	100	0.0900	0.13		Sheet Flow,					
						Woods: Light underbrush n= 0.400 P2= 2.84"					
	7.5	755	0.1130	1.68		Shallow Concentrated Flow,					
						Woodland Kv= 5.0 fps					
	0.0	6	0.3000	3.83		Shallow Concentrated Flow,					
						Short Grass Pasture Kv= 7.0 fps					
	0.7	160	0.0375	3.93		Shallow Concentrated Flow,					
						Paved Kv= 20.3 fps					
	20.7	1,021	Total								

Summary for Reach 1R: EXISTING ROADWAY DITCH

Inflow Area = 5.517 ac, 10.20% Impervious, Inflow Depth > 2.53" for 25-YEAR event

Inflow = 8.80 cfs @ 12.44 hrs, Volume= 1.16 af

Outflow = 8.80 cfs @ 12.44 hrs, Volume= 1.16 af, Atten= 0%, Lag= 0.4 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Max. Velocity= 4.64 fps, Min. Travel Time= 0.8 min Avg. Velocity = 2.25 fps, Avg. Travel Time= 1.7 min

Peak Storage= 436 cf @ 12.44 hrs Average Depth at Peak Storage= 0.80'

Bank-Full Depth= 1.00' Flow Area= 3.0 sf, Capacity= 16.21 cfs

0.00' x 1.00' deep channel, n= 0.022 Earth, clean & straight

Side Slope Z-value= 3.0 '/' Top Width= 6.00'

Length= 230.0' Slope= 0.0173 '/'

Inlet Invert= 295.64', Outlet Invert= 291.66'



Type III 24-hr 25-YEAR Rainfall=5.66"

Prepared by Keach-Nordstrom Associates, Inc.

Printed 12/6/2023

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 33

Summary for Pond 1P: EXISTING CB

Inflow Area = 24.393 ac, 2.71% Impervious, Inflow Depth > 2.06" for 25-YEAR event

Inflow = 29.03 cfs @ 12.57 hrs, Volume= 4.19 af

Primary = 29.03 cfs @ 12.57 hrs, Volume= 4.19 af, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Summary for Pond 2P: EXISTING DRIVEWAY CULVERT

Inflow Area = 2.596 ac, 12.76% Impervious, Inflow Depth > 1.80" for 25-YEAR event

Inflow = 3.49 cfs @ 12.29 hrs, Volume= 0.39 af

Outflow = 3.49 cfs @ 12.29 hrs, Volume= 0.39 af, Atten= 0%, Lag= 0.0 min

Primary = 3.49 cfs @ 12.29 hrs, Volume= 0.39 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 298.26' @ 12.50 hrs

Flood Elev= 294.00'

Primary OutFlow Max=2.90 cfs @ 12.29 hrs HW=295.16' TW=294.58' (Dynamic Tailwater)
1=Culvert (Inlet Controls 2.90 cfs @ 3.69 fps)

Summary for Pond 3P: EXISTING DROP INLET

Inflow Area = 35.450 ac, 4.96% Impervious, Inflow Depth > 2.10" for 25-YEAR event

Inflow = 43.08 cfs @ 12.49 hrs, Volume= 6.19 af

Outflow = 43.08 cfs @ 12.49 hrs, Volume= 6.19 af, Atten= 0%, Lag= 0.0 min

Primary = 43.08 cfs @ 12.49 hrs, Volume= 6.19 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 297.77' @ 12.49 hrs

Flood Elev= 293.75'

Device	Routing	Invert	Outlet Devices
#1	Primary	288.66'	24.0" Round Culvert
	·		L= 92.1' RCP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 288.66' / 284.67' S= 0.0433 '/' Cc= 0.900
			n= 0.011 Concrete pipe, straight & clean, Flow Area= 3.14 sf

Primary CutFlow Max=43.06 cfs @ 12.49 hrs HW=297.76' TW=0.00' (Dynamic Tailwater) —1=Culvert (Inlet Controls 43.06 cfs @ 13.71 fps)

Type III 24-hr 25-YEAR Rainfall=5.66"

Prepared by Keach-Nordstrom Associates, Inc. HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Printed 12/6/2023

Page 34

Summary for Pond 4P: EXISTING CB

Inflow Area = 5.517 ac, 10.20% Impervious, Inflow Depth > 2.53" for 25-YEAR event Inflow = 8.80 cfs @ 12.44 hrs, Volume= 1.16 af Outflow = 8.80 cfs @ 12.44 hrs, Volume= 1.16 af, Atten= 0%, Lag= 0.0 min 4.59 cfs @ 12.44 hrs, Volume= 1.01 af Secondary = 4.21 cfs @ 12.44 hrs, Volume= 0.15 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 300.47' @ 12.44 hrs

Flood Elev= 300.17'

Primary OutFlow Max=4.59 cfs @ 12.44 hrs HW=300.47' TW=296.43' (Dynamic Tailwater) 1=Culvert (Barrel Controls 4.59 cfs @ 5.85 fps)

Secondary OutFlow Max=4.21 cfs @ 12.44 hrs HW=300.47' TW=296.43' (Dynamic Tailwater) 2=Orifice/Grate (Weir Controls 4.21 cfs @ 1.78 fps)

Summary for Pond 5P: EXISTING CB

Inflow Area = 0.377 ac, 34.30% Impervious, Inflow Depth > 2.87" for 25-YEAR event Inflow = 1.16 cfs @ 12.12 hrs, Volume= 0.09 af Outflow = 1.16 cfs @ 12.12 hrs, Volume= 0.09 af, Atten= 0%, Lag= 0.0 min 1.16 cfs @ 12.12 hrs, Volume= 0.09 af Secondary = 0.00 cfs @ 0.00 hrs, Volume= 0.00 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 300.50' @ 12.41 hrs

Flood Elev= 301.16'

Device	Routing	Invert	Outlet Devices
#1	Primary	297.70'	12.0" Round Culvert
			L= 34.1' CMP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 297.70' / 297.30' S= 0.0117 '/' Cc= 0.900
			n= 0.025 Corrugated metal, Flow Area= 0.79 sf
#2	Secondary	301.16'	24.0" x 24.0" Horiz. Orifice/Grate C= 0.600
			Limited to weir flow at low heads

Primary OutFlow Max=1.08 ofs @ 12.12 hrs HW=300.38' TW=300.22' (Dynamic Tailwater)

1=Culvert (Outlet Controls 1.08 ofs @ 1.37 fps)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=297.70' TW=0.00' (Dynamic Tailwater) 2=Orifice/Grate (Controls 0.00 cfs)

Type III 24-hr 25-YEAR Rainfall=5.66"

Prepared by Keach-Nordstrom Associates, Inc.

Printed 12/6/2023

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 35

Summary for Pond 6P: EXISTING CB

Inflow Area = 5.703 ac, 2.62% Impervious, Inflow Depth > 2.60" for 25-YEAR event

Inflow = 10.73 cfs @ 12.35 hrs, Volume= 1.23 af

Outflow = 10.73 cfs @ 12.35 hrs, Volume= 1.23 af, Atten= 0%, Lag= 0.0 min

Primary = 10.73 cfs @ 12.35 hrs, Volume= 1.23 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 373.86' @ 12.35 hrs

Flood Elev= 366.35'

Device	Routing	Invert	Outlet Devices
#1	Primary	363.70'	12.0" Round Culvert L= 18.0' CMP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 363.70' / 362.46' S= 0.0689 '/' Cc= 0.900 n= 0.025 Corrugated metal, Flow Area= 0.79 sf

Primary OutFlow Max=10.72 cfs @ 12.35 hrs HW=373.85' TW=0.00' (Dynamic Tailwater) 1=Culvert (Barrel Controls 10.72 cfs @ 13.65 fps)

Summary for Pond 7P: EXISTING CB

Inflow Area = 4.676 ac, 2.12% Impervious, Inflow Depth > 2.59" for 25-YEAR event

Inflow = 9.42 cfs @ 12.29 hrs, Volume= 1.01 af

Outflow = 9.42 cfs @ 12.29 hrs, Volume= 1.01 af, Atten= 0%, Lag= 0.0 min

Primary = 9.42 cfs @ 12.29 hrs, Volume= 1.01 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 383.28' @ 12.29 hrs

Flood Elev= 380.49'

Device	Routing	Invert	Outlet Devices
#1	Primary	376.57'	12.0" Round Culvert
	-		L= 31.9' RCP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 376.57' / 376.28' S= 0.0091 '/' Cc= 0.900
			n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf

Primary OutFlow Max=9.42 cfs @ 12.29 hrs HW=383.27' TW=0.00' (Dynamic Tailwater)
—1=Culvert (Inlet Controls 9.42 cfs @ 11.99 fps)

Summary for Link A: BARRETTS HILL CROSSING

Inflow Area = 35.450 ac, 4.96% Impervious, Inflow Depth > 2.10" for 25-YEAR event

Inflow = 43.08 cfs @ 12.49 hrs, Volume≔ 6.19 af

Primary = 43.08 cfs @ 12.49 hrs, Volume= 6.19 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

2304141-PRE DEVELOPMENT

Type III 24-hr 25-YEAR Rainfall=5.66"

Prepared by Keach-Nordstrom Associates, Inc.

Printed 12/6/2023

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 36

Summary for Link B: LOT 57

Inflow Area =

5.703 ac, 2.62% Impervious, Inflow Depth > 2.60" for 25-YEAR event

Inflow

Primary

10.73 cfs @ 12.35 hrs, Volume= 10.73 cfs @ 12.35 hrs, Volume=

1.23 af

1.23 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Summary for Link C: LOT 11

Inflow Area =

9.42 cfs @ 12.29 hrs, Volume=

1.01 af

4.676 ac, 2.12% Impervious, Inflow Depth > 2.59" for 25-YEAR event

Inflow Primary

9.42 cfs @ 12.29 hrs, Volume=

1.01 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

2304141-PRE DEVELOPMENT

Type III 24-hr 50-YEAR Rainfall=6.77"

Prepared by Keach-Nordstrom Associates, Inc.

Printed 12/6/2023

Primary=60.85 cfs 8.59 af

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 37

Time span=0.00-24.00 hrs, dt=0.01 hrs, 2401 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment 1S: SUBCATCHMENT Runoff Area=1,062,579 sf 2.71% Impervious Runoff Depth>2.87" Flow Length=2.203' Tc=38.1 min CN=64.7 Runoff=41.09 cfs 5.83 af

Subcatchment 2S: SUBCATCHMENT

Runoff Area=113,081 sf 12.76% Impervious Runoff Depth>2.56"

Flow Length=683' Tc=20.1 min CN=61.4 Runoff=5.12 cfs 0.55 af

Subcatchment 3S: SUBCATCHMENT

Runoff Area=128,229 sf 6.91% Impervious Runoff Depth>2.59"

Flow Length=1,189' Tc=23.1 min CN=61.7 Runoff=5.55 cfs 0.64 af

Subcatchment 4S: SUBCATCHMENT Runoff Area=223,873 sf 8.43% Impervious Runoff Depth>3.39" Flow Length=1,889' Tc=31.1 min CN=69.9 Runoff=11.40 cfs 1.45 af

Subcatchment 5S: SUBCATCHMENT

Runoff Area=16,441 sf 34.30% Impervious Runoff Depth>3.80"

Flow Length=389' Tc=8.7 min CN=73.7 Runoff=1.54 cfs 0.12 af

Subcatchment 6S: SUBCATCHMENT

Runoff Area=248,437 sf 2.62% Impervious Runoff Depth>3.49"

Flow Length=1,105' Tc=24.4 min CN=70.8 Runoff=14.53 cfs 1.66 af

Subcatchment 7S: SUBCATCHMENT Runoff Area=203,705 sf 2.12% Impervious Runoff Depth>3.49" Flow Length=1,021' Tc=20.7 min CN=70.8 Runoff=12.77 cfs 1.36 af

Reach 1R: EXISTING ROADWAY DITCH Avg. Flow Depth=0.89' Max Vel=5.01 fps Inflow=11.98 cfs 1.57 af n=0.022 L=230.0' S=0.0173'/' Capacity=16.21 cfs Outflow=11.98 cfs 1.57 af

Pond 1P: EXISTING CB Inflow=41.09 cfs 5.83 af
Primary=41.09 cfs 5.83 af

Pond 2P: EXISTING DRIVEWAY CULVERT Peak Elev=306.84' Inflow=5.12 cfs 0.55 af 12.0" Round Culvert n=0.013 L=21.0' S=0.0143 '/' Outflow=5.12 cfs 0.55 af

Pond 3P: EXISTING DROP INLET

Peak Elev=305.84' Inflow=60.85 cfs 8.59 af 24.0" Round Culvert n=0.011 L=92.1' S=0.0433'/' Outflow=60.85 cfs 8.59 af

Pond 4P: EXISTING CB Peak Elev=300.60' Inflow=11.98 cfs 1.57 af

Primary=4.67 cfs 1.25 af Secondary=7.31 cfs 0.32 af Outflow=11.98 cfs 1.57 af

Pond 5P: EXISTING CB Peak Elev=300.68' Inflow=1.54 cfs 0.12 af Primary=1.54 cfs 0.12 af Secondary=0.00 cfs 0.00 af Outflow=1.54 cfs 0.12 af

Pond 6P: EXISTING CB Peak Elev=382.54' Inflow=14.53 cfs 1.66 af

12.0" Round Culvert n=0.025 L=18.0' S=0.0689 '/' Outflow=14.53 cfs 1.66 af

Pond 7P: EXISTING CB

Peak Elev=388.48' Inflow=12.77 cfs 1.36 af 12.0" Round Culvert n=0.011 L=31.9' S=0.0091 '/' Outflow=12.77 cfs 1.36 af

Link A: BARRETTS HILL CROSSING Inflow=60.85 cfs 8.59 af

2304141-PRE DEVELOPMENT

Type III 24-hr 50-YEAR Rainfall=6.77"

Prepared by Keach-Nordstrom Associates, Inc.

Printed 12/6/2023

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 38

Link B: LOT 57

Inflow=14.53 cfs 1.66 af

Primary=14.53 cfs 1.66 af

Link C: LOT 11

Inflow=12.77 cfs 1.36 af

Primary=12.77 cfs 1.36 af

Total Runoff Area = 45.830 ac Runoff Volume = 11.62 af Average Runoff Depth = 3.04" 95.62% Pervious = 43.824 ac 4.38% Impervious = 2.006 ac

Type III 24-hr 50-YEAR Rainfall=6.77"

Prepared by Keach-Nordstrom Associates, Inc.

Printed 12/6/2023

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 39

Summary for Subcatchment 1S: SUBCATCHMENT

Runoff = 41.09 cfs @ 12.57 hrs, Volume=

5.83 af, Depth> 2.87"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 50-YEAR Rainfall=6.77"

	Aı	rea (sf)	CN	Description	n			
	7,670 30.0 Woods, Good, HSG A					A		
	363,085 55.0 Woods, Good, HSG B					В		
	5	74,693	70.0	Woods, G	lood, HSG	C		
		6,684	39.0	>75% Gra	iss cover, (Good, HSG A		
		61,618	61.0	>75% Gra	iss cover, (Good, HSG B		
		20,067	74.0	>75% Gra	ass cover, (Good, HSG C		
*		14,404	98.0	Roofs				
*		14,358	98.0	Paved pa	rking			
	1,0	62,579	64.7	Weighted	Average			
	1,0	33,817	63.8	97.29% Pervious Area				
		28,762	98.0	2.71% Impervious Area				
					_			
	Tc	Length	Slope	Velocity	Capacity	Description		
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
	17.3	100	0.0400	0.10		Sheet Flow,		
						Woods: Light underbrush n= 0.400 P2= 2.84"		
	19.7	1,862	0.0988	1.57		Shallow Concentrated Flow,		
						Woodland Kv= 5.0 fps		
	0.3	30	0.0800	1.98		Shallow Concentrated Flow,		
						Short Grass Pasture Kv= 7.0 fps		
	8.0	211	0.0430	4.21		Shallow Concentrated Flow,		
						Paved Kv= 20.3 fps		
	38.1	2,203	Total					

Summary for Subcatchment 2S: SUBCATCHMENT

Runoff = 5.12 cfs @ 12.29 hrs, Volume=

0.55 af, Depth> 2.56"

	Area (sf)	CN	Description
	28,414	30.0	Woods, Good, HSG A
	11,383	55.0	Woods, Good, HSG B
	33,808	70.0	Woods, Good, HSG C
	4,719	39.0	>75% Grass cover, Good, HSG A
	242	61.0	>75% Grass cover, Good, HSG B
	20,087	74.0	>75% Grass cover, Good, HSG C
*	3,849	98.0	Roofs
*	10,579	98.0	Paved parking
	113,081	61.4	Weighted Average
	98,653	56.1	87.24% Pervious Area
	14,428	98.0	12.76% Impervious Area

2304141-PRE DEVELOPMENT

Type III 24-hr 50-YEAR Rainfall=6.77"

Prepared by Keach-Nordstrom Associates, Inc. HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Printed 12/6/2023

Page 40

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
12.0	100	0.1000	0.14		Sheet Flow,
3.9	318	0.0750	1.37		Woods: Light underbrush n= 0.400 P2= 2.84" Shallow Concentrated Flow, Woodland Kv= 5.0 fps
0.7	48	0.0300	1.21		Shallow Concentrated Flow,
3.6	217	0.0410	1.01		Short Grass Pasture Kv= 7.0 fps Shallow Concentrated Flow, Woodland Kv= 5.0 fps
20.1	683	Total			

Summary for Subcatchment 3S: SUBCATCHMENT

Runoff = 5.55 cfs @ 12.34 hrs, Volume=

0.64 af, Depth> 2.59"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 50-YEAR Rainfall=6.77"

		rea (sf)	CN	Description	on						
		5,732	30.0	Woods, C	Woods, Good, HSG A						
		49,737	55.0		Woods, Good, HSG B						
		38,476	70.0	Woods, C	Good, HSG	C					
		4,799	39.0			Good, HSG A					
		20,628	61.0			Good, HSG B					
*		2,833	98.0	Roofs	,						
*		6,024	98.0	Paved pa	rking						
		128,229	61.7	Weighted Average							
	•	119,372	59.0	93.09% Pervious Area							
		8,857	98.0	6.91% Impervious Area							
					,						
	Tc	Length	Slope	Velocity	Capacity	Description					
((min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	2 000.101.011					
	9.9	100	0.1600	0.17	, , , , ,	Sheet Flow,					
				0		Woods: Light underbrush n= 0.400 P2= 2.84"					
	13.2	1,089	0.0760	1.38		Shallow Concentrated Flow,					
						Woodland Kv= 5.0 fps					
	23.1	1,189	Total			osalana 117 olo ipo					

Summary for Subcatchment 4S: SUBCATCHMENT

Runoff = 11.40 cfs @ 12.44 hrs, Volume= 1.4

1.45 af, Depth> 3.39"

2304141-PRE DEVELOPMENT

Type III 24-hr 50-YEAR Rainfall=6.77"

Prepared by Keach-Nordstrom Associates, Inc. HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Printed 12/6/2023 Page 41

	٨	rea (sf)	CN	Description	in :			
_			55.0	Woods, Good, HSG B				
	12,309 55.0 150,990 70.0				iood, HSG			
		•						
*	41,699 61.0 >75% Grass cover, Good, HSG B							
*	2,548 98.0 Roofs 16,327 98.0 Paved parking							
_								
		23,873	69.9	Weighted	Average ervious Are			
		04,998	67.3 98.0					
		18,875	90.0	0.43%	pervious A	rea		
	Тс	Length	Slope	Velocity	Capacity	Description		
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	•		
	15.8	100	0.0500	0.11		Sheet Flow,		
						Woods: Light underbrush n= 0.400 P2= 2.84"		
	10.6	1,127	0.1250	1.77		Shallow Concentrated Flow,		
						Woodland Kv= 5.0 fps		
	0.5	55	0.0730	1.89		Shallow Concentrated Flow,		
						Short Grass Pasture Kv= 7.0 fps		
	0.9	68	0.0590	1.21		Shallow Concentrated Flow,		
						Woodland Kv= 5.0 fps		
	0.2	30	0.1000	2.21		Shallow Concentrated Flow,		
	0.4	40	0.0000	0.07		Short Grass Pasture Kv= 7.0 fps		
	0.1	16	0.0200	2.87		Shallow Concentrated Flow,		
	2.1	240	0.0750	1.92		Paved Kv= 20.3 fps Shallow Concentrated Flow.		
	2.1	240	0.0750	1.92		Short Grass Pasture Kv= 7.0 fps		
	0.3	73	0.0410	4.11		Shallow Concentrated Flow.		
	0.5	73	0.0410	7.11		Paved Kv= 20.3 fps		
	0.6	180	0.0720	5.45		Shallow Concentrated Flow.		
	0.0	, 00	3.0.20	0.70		Paved Kv= 20.3 fps		
	31.1	1,889	Total					

Summary for Subcatchment 5S: SUBCATCHMENT

Runoff = 1.54 cfs @ 12.12 hrs, Volume=

0.12 af, Depth> 3.80"

	Area (sf)	CN	Description		
	10,801	61.0	75% Grass cover, Good, HSG B		
*	5,640	98.0	Paved parking		
	16,441	73.7	Weighted Average		
	10,801	61.0	65.70% Pervious Area		
	5,640	98.0	34.30% Impervious Area		

2304141-PRE DEVELOPMENT

389 Total

8.7

Type III 24-hr 50-YEAR Rainfall=6.77"

Prepared by Keach-Nordstrom Associates, Inc.
HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Printed 12/6/2023

Page 42

T (mii	c Length	0 0 10	Velocity (ft/sec)	Capacity (cfs)	Description
6.	3 100	0.0700	0.26		Sheet Flow,
					Grass: Short n= 0.150 P2= 2.84"
0.	.1 16	0.0400	4.06		Shallow Concentrated Flow,
					Paved Kv= 20.3 fps
1.	5 162	0.0680	1.83		Shallow Concentrated Flow,
					Short Grass Pasture Kv= 7.0 fps
0.	1 20	0.0400	4.06		Shallow Concentrated Flow,
					Paved Kv= 20.3 fps
0.	8 91	0.0770	1.94		Shallow Concentrated Flow,
					Short Grass Pasture Kv= 7.0 fps

Summary for Subcatchment 6S: SUBCATCHMENT

Runoff = 14.53 cfs @ 12.35 hrs, Volume=

1.66 af, Depth> 3.49"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 50-YEAR Rainfall=6.77"

	Area (sf) CN Description				n		
	2	37,230	70.0	Woods, Good, HSG C			
*				>75% Gra	5% Grass cover, Good, HSG C		
*				Paved parking			
	248,437 70.8		Weighted Average				
	2	41,939	70.1		ervious Are	ea	
		6,498	98.0	2.62% lm	pervious A	rea	
	Tc	Length	Slope	Velocity	Capacity	Description	
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	•	
	15.8	100	0.0500	0.11		Sheet Flow,	
						Woods: Light underbrush n= 0.400 P2= 2.84"	
	8.1	856	0.1250	1.77		Shallow Concentrated Flow,	
						Woodland Kv= 5.0 fps	
	0.0	10	0.3000	3.83		Shallow Concentrated Flow,	
						Short Grass Pasture Kv= 7.0 fps	
	0.5	139	0.0500	4.54		Shallow Concentrated Flow,	
_						Paved Kv= 20.3 fps	
	24.4	1,105	Total				

Summary for Subcatchment 7S: SUBCATCHMENT

Runoff = 12.77 cfs @ 12.29 hrs, Volume=

1.36 af, Depth> 3.49"

Type III 24-hr 50-YEAR Rainfall=6.77"

Prepared by Keach-Nordstrom Associates, Inc.
HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Printed 12/6/2023

Page 43

Δι	rea (sf)	CN	Description	n			
					0		
1	195,095 70.0		Woods, Good, HSG C				
	3,533	74.0	>75% Grass cover, Good, HSG C				
	4,313 98.0		Paved parking, HSG C				
	764 96.0		Gravel surface, HSG C				
2	03,705	70.8	Weighted	Weighted Average			
1	99,392	70.2	97.88% P	ervious Are	a		
	4,313	98.0	2.12% lm	pervious Ai	rea		
	.,						
Tc	Length	Slope	Velocity	Capacity	Description		
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
12.5	100	0.0900	0.13		Sheet Flow,		
,			0		Woods: Light underbrush n= 0.400 P2= 2.84"		
7.5	755	0.1130	1.68		Shallow Concentrated Flow,		
7.5	700	0.1150	1.00		Woodland Kv= 5.0 fps		
0.0	0	0.0000	0.00		·		
0.0	6	0.3000	3.83		Shallow Concentrated Flow,		
					Short Grass Pasture Kv= 7.0 fps		
0.7	160	0.0375	3.93		Shallow Concentrated Flow,		
					Paved Kv= 20.3 fps		
20.7	1,021	Total					

Summary for Reach 1R: EXISTING ROADWAY DITCH

Inflow Area = 5.517 ac, 10.20% Impervious, Inflow Depth > 3.42" for 50-YEAR event

Inflow = 11.98 cfs @ 12.41 hrs, Volume= 1.57 af

Outflow = 11.98 cfs @ 12.43 hrs, Volume= 1.57 af, Atten= 0%, Lag= 1.4 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Max. Velocity= 5.01 fps, Min. Travel Time= 0.8 min Avg. Velocity = 2.37 fps, Avg. Travel Time= 1.6 min

Peak Storage= 550 cf @ 12.43 hrs Average Depth at Peak Storage= 0.89'

Bank-Full Depth= 1.00' Flow Area= 3.0 sf, Capacity= 16.21 cfs

0.00' x 1.00' deep channel, n= 0.022 Earth, clean & straight

Side Slope Z-value= 3.0 '/' Top Width= 6.00'

Length= 230.0' Slope= 0.0173 '/'

Inlet Invert= 295.64', Outlet Invert= 291.66'



2304141-PRE DEVELOPMENT

Type III 24-hr 50-YEAR Rainfall=6.77"

Prepared by Keach-Nordstrom Associates, Inc.

Printed 12/6/2023

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 44

Summary for Pond 1P: EXISTING CB

Inflow Area = 24.393 ac, 2.71% Impervious, Inflow Depth > 2.87" for 50-YEAR event

Inflow = 41.09 cfs @ 12.57 hrs, Volume= 5.83 af

Primary = 41.09 cfs @ 12.57 hrs, Volume= 5.83 af, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Summary for Pond 2P: EXISTING DRIVEWAY CULVERT

Inflow Area = 2.596 ac, 12.76% Impervious, Inflow Depth > 2.56" for 50-YEAR event

Inflow = 5.12 cfs @ 12.29 hrs, Volume= 0.55 af

Outflow = 5.12 cfs @ 12.29 hrs, Volume= 0.55 af, Atten= 0%, Lag= 0.0 min

Primary = 5.12 cfs @ 12.29 hrs, Volume= 0.55 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 306.84' @ 12.50 hrs

Flood Elev= 294.00'

Primary OutFlow Max=4.32 cfs @ 12.29 hrs HW=301.22' TW=299.91' (Dynamic Tailwater) 1=Culvert (Inlet Controls 4.32 cfs @ 5.51 fps)

Summary for Pond 3P: EXISTING DROP INLET

Inflow Area = 35.450 ac, 4.96% Impervious, Inflow Depth > 2.91" for 50-YEAR event

Inflow = 60.85 cfs @ 12.49 hrs, Volume= 8.59 af

Outflow = 60.85 cfs @ 12.49 hrs, Volume= 8.59 af, Atten= 0%, Lag= 0.0 min

Primary = 60.85 cfs @ 12.49 hrs, Volume= 8.59 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 305.84' @ 12.49 hrs

Flood Elev= 293.75'

Device	Routing	Invert	Outlet Devices
#1	Primary	288.66'	24.0" Round Culvert
			L= 92.1' RCP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 288.66' / 284.67' S= 0.0433 '/' Cc= 0.900
			n= 0.011 Concrete pipe, straight & clean, Flow Area= 3.14 sf

Primary OutFlow Max=60.85 cfs @ 12.49 hrs HW=305.84' TW=0.00' (Dynamic Tailwater) 1=Culvert (Inlet Controls 60.85 cfs @ 19.37 fps)

Type III 24-hr 50-YEAR Rainfall=6.77"

Prepared by Keach-Nordstrom Associates, Inc.

Printed 12/6/2023

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 45

Summary for Pond 4P: EXISTING CB

Inflow Area = 5.517 ac, 10.20% Impervious, Inflow Depth > 3.42" for 50-YEAR event 11.98 cfs @ 12.41 hrs, Volume= 1.57 af
Outflow = 11.98 cfs @ 12.41 hrs, Volume= 1.57 af, Atten= 0%, Lag= 0.0 min 4.67 cfs @ 12.41 hrs, Volume= 1.25 af
Secondary = 7.31 cfs @ 12.41 hrs, Volume= 0.32 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 300.60' @ 12.41 hrs

Flood Elev= 300.17'

Device	Routing	Invert	Outlet Devices
#1	Primary	297.00'	12.0" Round Culvert
	•		L= 49.1' CMP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 297.00' / 295.64' S= 0.0277 '/' Cc= 0.900
			n= 0.025 Corrugated metal, Flow Area= 0.79 sf
#2	Secondary	300.17'	24.0" x 24.0" Horiz. Orifice/Grate C= 0.600
	-		I imited to weir flow at low heads

Primary OutFlow Max=4.67 cfs @ 12.41 hrs HW=300.60' TW=296.53' (Dynamic Tailwater)
—1=Culvert (Barrel Controls 4.67 cfs @ 5.95 fps)

Secondary OutFlow Max=7.31 cfs @ 12.41 hrs HW=300.60' TW=296.53' (Dynamic Tailwater) 2=Orifice/Grate (Weir Controls 7.31 cfs @ 2.14 fps)

Summary for Pond 5P: EXISTING CB

Inflow Area = 0.377 ac, 34.30% Impervious, Inflow Depth > 3.80" for 50-YEAR event 1.54 cfs @ 12.12 hrs, Volume= 0.12 af 0.12 af, Atten= 0%, Lag= 0.0 min Primary = 1.54 cfs @ 12.12 hrs, Volume= 0.12 af 0.00 cfs @ 0.00 hrs, Volume= 0.00 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 300.68' @ 12.14 hrs

Flood Elev= 301.16'

Device	Routing	Invert	Outlet Devices
#1	Primary	297.70'	12.0" Round Culvert
	•		L= 34.1' CMP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 297.70' / 297.30' S= 0.0117 '/' Cc= 0.900
			n= 0.025 Corrugated metal, Flow Area= 0.79 sf
#2	Secondary	301.16'	24.0" x 24.0" Horiz. Orifice/Grate C= 0.600
	•		Limited to weir flow at low heads

Primary OutFlow Max=1.50 cfs @ 12.12 hrs HW=300.66' TW=300.35' (Dynamic Tailwater)
1=Culvert (Outlet Controls 1.50 cfs @ 1.91 fps)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=297.70' TW=0.00' (Dynamic Tailwater) 2=Orifice/Grate (Controls 0.00 cfs)

2304141-PRE DEVELOPMENT

Type III 24-hr 50-YEAR Rainfall=6.77"

Prepared by Keach-Nordstrom Associates, Inc.

Printed 12/6/2023

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 46

Summary for Pond 6P: EXISTING CB

Inflow Area = 5.703 ac, 2.62% Impervious, Inflow Depth > 3.49" for 50-YEAR event

Inflow = 14.53 cfs @ 12.35 hrs, Volume= 1.66 af

Outflow = 14.53 cfs @ 12.35 hrs, Volume= 1.66 af, Atten= 0%, Lag= 0.0 min

Primary = 14.53 cfs @ 12.35 hrs, Volume= 1.66 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs. dt= 0.01 hrs

Peak Elev= 382.54' @ 12.35 hrs

Flood Elev= 366.35'

Device Routing Invert Outlet Devices

#1 Primary 363.70' 12.0" Round Culvert

L= 18.0' CMP, square edge headwall, Ke= 0.500
Inlet / Outlet Invert= 363.70' / 362.46' S= 0.0689 '/' Cc= 0.900
n= 0.025 Corrugated metal, Flow Area= 0.79 sf

Primary OutFlow Max=14.52 cfs @ 12.35 hrs HW=382.52' TW=0.00' (Dynamic Tailwater) 1=Culvert (Barrel Controls 14.52 cfs @ 18.49 fps)

Summary for Pond 7P: EXISTING CB

Inflow Area = 4.676 ac, 2.12% Impervious, Inflow Depth > 3.49" for 50-YEAR event

Inflow = 12.77 cfs @ 12.29 hrs, Volume= 1.36 af

Outflow = 12.77 cfs @ 12.29 hrs, Volume= 1.36 af, Atten= 0%, Lag= 0.0 min

Primary = 12.77 cfs @ 12.29 hrs, Volume= 1.36 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 388.48' @ 12.29 hrs

Flood Elev= 380.49'

Device Routing Invert Outlet Devices

#1 Primary 376.57' 12.0" Round Culvert

L= 31.9' RCP, square edge headwall, Ke= 0.500
Inlet / Outlet Invert= 376.57' / 376.28' S= 0.0091 '/' Cc= 0.900

n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf

Primary OutFlow Max=12.77 cfs @ 12.29 hrs HW=388.47' TW=0.00' (Dynamic Tailwater)

1=Culvert (Inlet Controls 12.77 cfs @ 16.26 fps)

Summary for Link A: BARRETTS HILL CROSSING

Inflow Area = 35.450 ac, 4.96% Impervious, Inflow Depth > 2.91" for 50-YEAR event

Inflow = 60.85 cfs @ 12.49 hrs, Volume= 8.59 af

Primary = 60.85 cfs @ 12.49 hrs, Volume= 8.59 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

2304141-PRE DEVELOPMENT

Type III 24-hr 50-YEAR Rainfall=6.77"

Prepared by Keach-Nordstrom Associates, Inc.

Printed 12/6/2023

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 47

Summary for Link B: LOT 57

Inflow Area = 5.703 ac, 2.62% Impervious, Inflow Depth > 3.49" for 50-YEAR event

Inflow = 14.53 cfs @ 12.35 hrs, Volume= 1.66 af

Primary = 14.53 cfs @ 12.35 hrs, Volume= 1.66 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

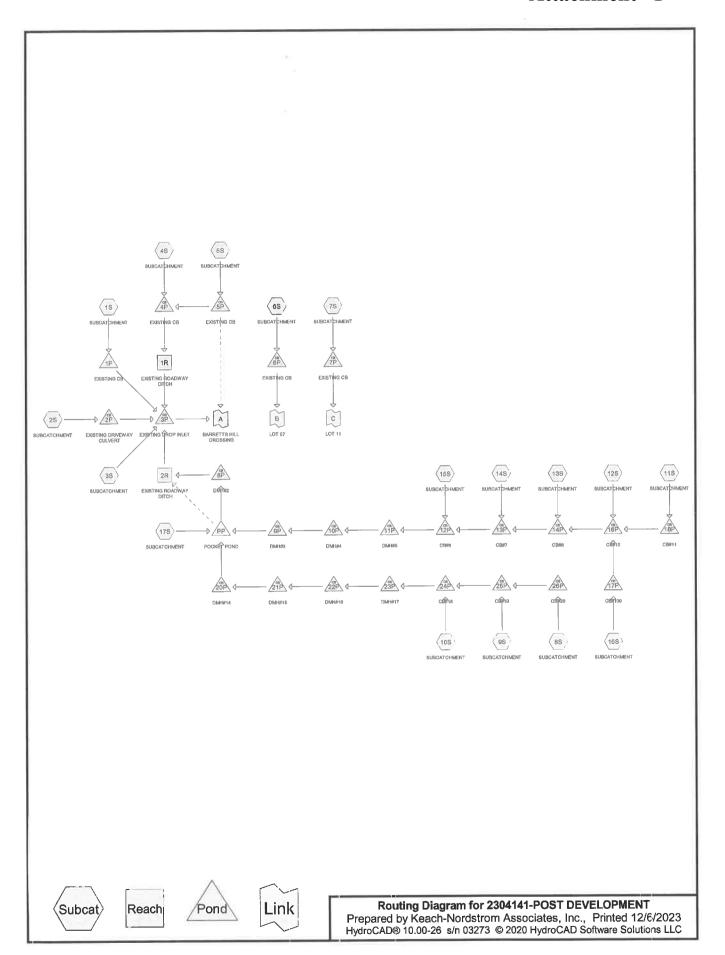
Summary for Link C: LOT 11

Inflow Area = 4.676 ac, 2.12% Impervious, Inflow Depth > 3.49" for 50-YEAR event

Inflow = 12.77 cfs @ 12.29 hrs, Volume= 1.36 af

Primary = 12.77 cfs @ 12.29 hrs, Volume= 1.36 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs



2304141-POST DEVELOPMENT

Prepared by Keach-Nordstrom Associates, Inc.

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Printed 12/6/2023 Page 2

Area Listing (all nodes)

Area	CN	Description
(acres)		(subcatchment-numbers)
0.372	39.0	>75% Grass cover, Good, HSG A (1S, 2S, 3S)
3.066	61.0	>75% Grass cover, Good, HSG B (1S, 2S, 3S, 4S, 5S)
6.687	74.0	>75% Grass cover, Good, HSG C (1S, 2S, 3S, 4S, 6S, 7S, 8S, 9S, 10S, 11S,
		12S, 13S, 14S, 15S, 16S, 17S)
0.058	96.0	Gravel surface, HSG C (6S, 7S, 17S)
2.320	98.0	Paved parking (1S, 2S, 3S, 5S, 6S, 7S, 8S, 9S, 10S, 11S, 12S, 13S, 14S, 15S,
		16S, 17S)
0.375	98.0	Paved parking, HSG C (4S)
1.204	98.0	Roofs (1S, 2S, 3S, 4S, 6S, 7S, 9S, 10S, 12S, 17S)
0.960	30.0	Woods, Good, HSG A (1S, 2S, 3S)
9.984	55.0	Woods, Good, HSG B (1S, 2S, 3S, 4S)
20.805	70.0	Woods, Good, HSG C (1S, 2S, 3S, 4S, 6S, 7S, 8S, 9S, 10S, 11S, 12S, 13S,
		14S, 17S)
45.830	68.0	TOTAL AREA

2304141-POST DEVELOPMENT

Prepared by Keach-Nordstrom Associates, Inc. HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC Printed 12/6/2023 Page 3

Soil Listing (all nodes)

Ar	rea Soil	Subcatchment
(acre	es) Group	Numbers
1.3	32 HSG	A 1S, 2S, 3S
13.0	50 HSG	B 1S, 2S, 3S, 4S, 5S
27.9	24 HSG	C 1S, 2S, 3S, 4S, 6S, 7S, 8S, 9S, 10S, 11S, 12S, 13S, 14S, 15S, 16S, 17S
0.0	00 HSG	D
3.5	24 Other	1S, 2S, 3S, 4S, 5S, 6S, 7S, 8S, 9S, 10S, 11S, 12S, 13S, 14S, 15S, 16S, 17S
45.8	30	TOTAL AREA

2304141-POST DEVELOPMENT

Type III 24-hr 2-YEAR Rainfall=2.95" Printed 12/6/2023

Prepared by Keach-Nordstrom Associates, Inc.

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 4

Time span=0.00-24.00 hrs, dt=0.01 hrs, 2401 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment 1S: SUBCATCHMENT	Runoff Area=961,091 sf 4.08% Impervious Runoff Depth>0.46" Flow Length=2,203' Tc=38.1 min CN=64.6 Runoff=4.52 cfs 0.85 af
Subcatchment 2S: SUBCATCHMENT	Runoff Area=113,081 sf 12.76% Impervious Runoff Depth>0.36" Flow Length=683' Tc=20.1 min CN=61.4 Runoff=0.44 cfs 0.08 af
Subcatchment 3S: SUBCATCHMENT	Runoff Area=124,041 sf 7.14% Impervious Runoff Depth>0.36" Flow Length=1,189' Tc=23.1 min CN=61.5 Runoff=0.48 cfs 0.09 af
Subcatchment 4S: SUBCATCHMENT	Runoff Area=86,429 sf 21.84% Impervious Runoff Depth>0.71" Tc=6.0 min CN=70.6 Runoff=1.45 cfs 0.12 af
Subcatchment 5S: SUBCATCHMENT	Runoff Area=16,441 sf 34.30% Impervious Runoff Depth>0.86" Flow Length=389' Tc=8.7 min CN=73.7 Runoff=0.32 cfs 0.03 af
Subcatchment 6S: SUBCATCHMENT	Runoff Area=91,362 sf 14.39% Impervious Runoff Depth>0.96" Flow Length=692' Tc=19.5 min CN=75.7 Runoff=1.52 cfs 0.17 af
Subcatchment 7S: SUBCATCHMENT	Runoff Area=112,988 sf 9.49% Impervious Runoff Depth>0.85" Flow Length=1,021' Tc=20.7 min CN=73.5 Runoff=1.57 cfs 0.18 af
Subcatchment 8S: SUBCATCHMENT	Runoff Area=30,577 sf 34.65% Impervious Runoff Depth>1.24" Flow Length=719' Tc=20.3 min CN=80.6 Runoff=0.68 cfs 0.07 af
Subcatchment 9S: SUBCATCHMENT	Runoff Area=67,824 sf 18.27% Impervious Runoff Depth>0.99" Flow Length=1,293' Tc=26.9 min CN=76.4 Runoff=1.03 cfs 0.13 af
Subcatchment 9S: SUBCATCHMENT Subcatchment 10S: SUBCATCHMENT	Flow Length=1,293' Tc=26.9 min CN=76.4 Runoff=1.03 cfs 0.13 af
	Flow Length=1,293' Tc=26.9 min CN=76.4 Runoff=1.03 cfs 0.13 af Runoff Area=68,434 sf 8.00% Impervious Runoff Depth>0.82" Flow Length=676' Tc=23.0 min CN=73.0 Runoff=0.88 cfs 0.11 af
Subcatchment 10S: SUBCATCHMENT	Flow Length=1,293' Tc=26.9 min CN=76.4 Runoff=1.03 cfs 0.13 af Runoff Area=68,434 sf 8.00% Impervious Runoff Depth>0.82" Flow Length=676' Tc=23.0 min CN=73.0 Runoff=0.88 cfs 0.11 af Runoff Area=66,396 sf 2.68% Impervious Runoff Depth>0.74" Flow Length=690' Tc=20.7 min CN=71.4 Runoff=0.79 cfs 0.09 af
Subcatchment 10S: SUBCATCHMENT Subcatchment 11S: SUBCATCHMENT	Flow Length=1,293' Tc=26.9 min CN=76.4 Runoff=1.03 cfs 0.13 af Runoff Area=68,434 sf 8.00% Impervious Runoff Depth>0.82" Flow Length=676' Tc=23.0 min CN=73.0 Runoff=0.88 cfs 0.11 af Runoff Area=66,396 sf 2.68% Impervious Runoff Depth>0.74" Flow Length=690' Tc=20.7 min CN=71.4 Runoff=0.79 cfs 0.09 af Runoff Area=59,686 sf 10.36% Impervious Runoff Depth>0.87"
Subcatchment 10S: SUBCATCHMENT Subcatchment 11S: SUBCATCHMENT Subcatchment 12S: SUBCATCHMENT	Flow Length=1,293' Tc=26.9 min CN=76.4 Runoff=1.03 cfs 0.13 af Runoff Area=68,434 sf 8.00% Impervious Runoff Depth>0.82" Flow Length=676' Tc=23.0 min CN=73.0 Runoff=0.88 cfs 0.11 af Runoff Area=66,396 sf 2.68% Impervious Runoff Depth>0.74" Flow Length=690' Tc=20.7 min CN=71.4 Runoff=0.79 cfs 0.09 af Runoff Area=59,686 sf 10.36% Impervious Runoff Depth>0.87" Flow Length=753' Tc=18.7 min CN=73.9 Runoff=0.89 cfs 0.10 af Runoff Area=54,469 sf 6.63% Impervious Runoff Depth>0.81" Flow Length=680' Tc=16.3 min CN=72.6 Runoff=0.78 cfs 0.08 af
Subcatchment 10S: SUBCATCHMENT Subcatchment 11S: SUBCATCHMENT Subcatchment 12S: SUBCATCHMENT Subcatchment 13S: SUBCATCHMENT	Flow Length=1,293' Tc=26.9 min CN=76.4 Runoff=1.03 cfs 0.13 af Runoff Area=68,434 sf 8.00% Impervious Runoff Depth>0.82" Flow Length=676' Tc=23.0 min CN=73.0 Runoff=0.88 cfs 0.11 af Runoff Area=66,396 sf 2.68% Impervious Runoff Depth>0.74" Flow Length=690' Tc=20.7 min CN=71.4 Runoff=0.79 cfs 0.09 af Runoff Area=59,686 sf 10.36% Impervious Runoff Depth>0.87" Flow Length=753' Tc=18.7 min CN=73.9 Runoff=0.89 cfs 0.10 af Runoff Area=54,469 sf 6.63% Impervious Runoff Depth>0.81" Flow Length=680' Tc=16.3 min CN=72.6 Runoff=0.78 cfs 0.08 af Runoff Area=38,021 sf 9.74% Impervious Runoff Depth>0.86"

Type III 24-hr 2-YEAR Rainfall=2.95"

2304141-POST DEVELOR	**
Prepared by Keach-Nordstr	
HydroCAD® 10.00-26 s/n 0327.	3 © 2020 HydroCAD Software Solutions LLC Page 5
Subcatchment 17S: SUBCAT	CHMENT Runoff Area=82,926 sf 4.39% Impervious Runoff Depth>0.88" Flow Length=230' Tc=13.2 min CN=74.2 Runoff=1.45 cfs 0.14 af
Reach 1R: EXISTING ROADV	VAY DITCH Avg. Flow Depth=0.43' Max Vel=3.08 fps Inflow=1.75 cfs 0.14 af n=0.022 L=230.0' S=0.0173 '/' Capacity=16.21 cfs Outflow=1.72 cfs 0.14 af
Reach 2R: EXISTING ROADV	VAY DITCH Avg. Flow Depth=0.19' Max Vel=1.29 fps Inflow=0.26 cfs 0.24 af n=0.069 L=915.0' S=0.0621 '/' Capacity=12.04 cfs Outflow=0.26 cfs 0.24 af
Pond 1P: EXISTING CB	Inflow=4.52 cfs 0.85 af Primary=4.52 cfs 0.85 af
Pond 2P: EXISTING DRIVEW	AY CULVERT Peak Elev=292.73' Inflow=0.44 cfs 0.08 af 12.0" Round Culvert n=0.013 L=21.0' S=0.0143 '/' Outflow=0.44 cfs 0.08 af
Pond 3P: EXISTING DROP IN	Peak Elev=289.71' Inflow=5.83 cfs 1.39 af 24.0" Round Culvert n=0.011 L=92.1' S=0.0433 '/' Outflow=5.83 cfs 1.39 af
Pond 4P: EXISTING CB	Peak Elev=297.75' Inflow=1.75 cfs 0.14 af Primary=1.75 cfs 0.14 af Secondary=0.00 cfs 0.00 af Outflow=1.75 cfs 0.14 af
Pond 5P: EXISTING CB	Peak Elev=298.09' Inflow=0.32 cfs 0.03 af Primary=0.32 cfs 0.03 af Secondary=0.00 cfs 0.00 af Outflow=0.32 cfs 0.03 af
Pond 6P: EXISTING CB	Peak Elev=364.36' Inflow=1.52 cfs 0.17 af 12.0" Round Culvert n=0.025 L=18.0' S=0.0689 '/' Outflow=1.52 cfs 0.17 af
Pond 7P: EXISTING CB	Peak Elev=377.28' Inflow=1.57 cfs 0.18 af 12.0" Round Culvert n=0.011 L=31.9' S=0.0091 '/' Outflow=1.57 cfs 0.18 af
Pond 8P: DMH#2	Peak Elev=349.93' Inflow=0.26 cfs 0.24 af 15.0" Round Culvert n=0.013 L=15.0' S=0.0800 '/' Outflow=0.26 cfs 0.24 af
Pond 9P: DMH#3	Peak Elev=375.27' Inflow=3.47 cfs 0.41 af 18.0" Round Culvert n=0.013 L=30.0' S=0.0073 '/' Outflow=3.47 cfs 0.41 af
Pond 10P: DMH#4	Peak Elev=376.49' Inflow=3.47 cfs 0.41 af 18.0" Round Culvert n=0.013 L=121.7' S=0.0100 '/' Outflow=3.47 cfs 0.41 af
Pond 11P: DMH#5	Peak Elev=378.82' Inflow=3.47 cfs 0.41 af 18.0" Round Culvert n=0.013 L=226.1' S=0.0100 '/' Outflow=3.47 cfs 0.41 af
Pond 12P: CB#6	Peak Elev=380.33' Inflow=3.47 cfs 0.41 af 18.0" Round Culvert n=0.013 L=141.2' S=0.0100 '/' Outflow=3.47 cfs 0.41 af
Pond 13P: CB#7	Peak Elev=381.26' Inflow=3.20 cfs 0.37 af 15.0" Round Culvert n=0.013 L=30.3' S=0.0099 '/' Outflow=3.20 cfs 0.37 af
Pond 14P: CB#8	Peak Elev=385.31' Inflow=2.62 cfs 0.31 af 15.0" Round Culvert n=0.013 L=165.7' S=0.0250 '/' Outflow=2.62 cfs 0.31 af

2304141-POST DEVELOPMENT

2304141-POST DEVELOPrepared by Keach-Nords HydroCAD® 10.00-26 s/n 03	
Pond 16P: CB#10	Peak Elev=392.86' Inflow=1.85 cfs 0.22 af 12.0" Round Culvert n=0.013 L=165.5' S=0.0500'/' Outflow=1.85 cfs 0.22 af
Pond 17P: CB#100	Peak Elev=393.57' Inflow=0.43 cfs 0.03 af 12.0" Round Culvert n=0.013 L=26.0' S=0.0200 '/' Outflow=0.43 cfs 0.03 af
Pond 18P: CB#11	Peak Elev=403.25' Inflow=0.79 cfs 0.09 af 12.0" Round Culvert n=0.013 L=167.8' S=0.0600 '/' Outflow=0.79 cfs 0.09 af
Pond 20P: DMH#14	Peak Elev=375.94' Inflow=2.54 cfs 0.31 af 18.0" Round Culvert n=0.013 L=15.0' S=0.0800 '/' Outflow=2.54 cfs 0.31 af
Pond 21P: DMH#15	Peak Elev=382.20' Inflow=2.54 cfs 0.31 af 18.0" Round Culvert n=0.013 L=30.7' S=0.0801 '/' Outflow=2.54 cfs 0.31 af
Pond 22P: DMH#16	Peak Elev=393.66' Inflow=2.54 cfs 0.31 af 18.0" Round Culvert n=0.013 L=111.5' S=0.0800 '/' Outflow=2.54 cfs 0.31 af
Pond 23P: DMH#17	Peak Elev=401.87' Inflow=2.54 cfs 0.31 af 18.0" Round Culvert n=0.013 L=45.0' S=0.0800 '/' Outflow=2.54 cfs 0.31 af
Pond 24P: CB#18	Peak Elev=403.61' Inflow=2.54 cfs 0.31 af 18.0" Round Culvert n=0.013 L=164.2' S=0.0100 '/' Outflow=2.54 cfs 0.31 af
Pond 25P: CB#19	Peak Elev=404.05' Inflow=1.66 cfs 0.20 af 15.0" Round Culvert n=0.013 L=36.5' S=0.0101 '/' Outflow=1.66 cfs 0.20 af
Pond 26P: CB#20	Peak Elev=404.24' Inflow=0.68 cfs 0.07 af 12.0" Round Culvert n=0.013 L=26.0' S=0.0100 '/' Outflow=0.68 cfs 0.07 af
Pond PP: POCKET POND	Peak Elev=369.49' Storage=34,886 cf Inflow=7.14 cfs 0.86 af Primary=0.26 cfs 0.24 af Secondary=0.00 cfs 0.00 af Outflow=0.26 cfs 0.24 af
Link A: BARRETTS HILL CF	ROSSING Inflow=5.83 cfs 1.39 af Primary=5.83 cfs 1.39 af
Link B: LOT 57	Inflow=1.52 cfs 0.17 af Primary=1.52 cfs 0.17 af
Link C: LOT 11	Inflow=1.57 cfs 0.18 af Primary=1.57 cfs 0.18 af

Total Runoff Area = 45.830 ac Runoff Volume = 2.37 af Average Runoff Depth = 0.62" 91.49% Pervious = 41.932 ac 8.51% Impervious = 3.898 ac

2304141-POST DEVELOPMENT

Type III 24-hr 2-YEAR Rainfall=2.95"

Prepared by Keach-Nordstrom Associates, Inc.

Printed 12/6/2023

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 7

Summary for Subcatchment 1S: SUBCATCHMENT

Runoff

4.52 cfs @ 12.66 hrs, Volume=

0.85 af, Depth> 0.46"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 2-YEAR Rainfall=2.95"

	Α	rea (sf)	CN	Description	n			
	7,670 30.0			Woods, G	Good, HSG	A		
	363,085 55.0			Woods, G	Good, HSG	В		
	427,890 70.0			Woods, Good, HSG C				
	6,684 39.0			>75% Grass cover, Good, HSG A				
	61,618 61.0			>75% Grass cover, Good, HSG B				
		54,927	74.0	>75% Gra	ass cover, (Good, HSG C		
*		21,604	98.0	Roofs				
*		17,613	98.0	Paved pa	rking			
	9	61,091	64.6	Weighted	Average			
	9	21,874	63.2	95.92% Pervious Area				
	39,217 98.0		98.0	4.08% Impervious Area				
	Тс	Length	Slope	Velocity	Capacity	Description		
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
	17.3	100	0.0400	0.10		Sheet Flow,		
						Woods: Light underbrush n= 0.400 P2= 2.84"		
	19.7	1,862	0.0988	1.57		Shallow Concentrated Flow,		
						Woodland Kv= 5.0 fps		
	0.3	30	0.0800	1.98		Shallow Concentrated Flow,		
						Short Grass Pasture Kv= 7.0 fps		
	8.0	211	0.0430	4.21		Shallow Concentrated Flow,		
						Paved Kv= 20.3 fps		
	38.1	2.203	Total					

Summary for Subcatchment 2S: SUBCATCHMENT

Runoff

0.44 cfs @ 12.43 hrs, Volume=

0.08 af, Depth> 0.36"

	Area (sf)	CN	Description
	28,414 30.0 Woods, Good, HSG A		Woods, Good, HSG A
	11,383	55.0	Woods, Good, HSG B
	33,808	70.0	Woods, Good, HSG C
	4,719	39.0	>75% Grass cover, Good, HSG A
	242	61.0	>75% Grass cover, Good, HSG B
	20,087	74.0	>75% Grass cover, Good, HSG C
*	3,849	98.0	Roofs
*	10,579	98.0	Paved parking
	113,081	61.4	Weighted Average
	98,653	56.1	87.24% Pervious Area
	14,428	98.0	12.76% Impervious Area

2304141-POST DEVELOPMENT

Type III 24-hr 2-YEAR Rainfall=2.95"

Prepared by Keach-Nordstrom Associates, Inc.

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Printed 12/6/2023

Page 8

Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
12.0	100	0.1000	0.14		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 2.84"
3.9	318	0.0750	1.37		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
0.7	48	0.0300	1.21		Shallow Concentrated Flow,
					Short Grass Pasture Kv= 7.0 fps
3.6	217	0.0410	1.01		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
20.1	683	Total			

Summary for Subcatchment 3S: SUBCATCHMENT

Runoff = 0.48 cfs @ 12.49 hrs, Volume=

0.09 af, Depth> 0.36"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 2-YEAR Rainfall=2.95"

	rea (sf)	CN	Description	n				
	5,732	30.0	Woods, G	Good, HSG	A			
	49,946	55.0	Woods, Good, HSG B					
	30,651	70.0	Woods, C	Good, HSG	C			
	4,799	39.0	>75% Gra	ass cover, (Good, HSG A			
	20,628	61.0	>75% Grass cover, Good, HSG B					
	3,428	74.0	>75% Gra	ass cover, (Good, HSG C			
*	2,833	98.0	Roofs					
*	* 6,024 98.0 Paved p							
1	124,041 61.5			Weighted Average				
1	115,184 58.7			ervious Are	ea			
	8,857	98.0	7.14% lm	pervious Ai	rea			
Tc	Length	Slope	Velocity	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
9.9	100	0.1600	0.17		Sheet Flow,			
					Woods: Light underbrush n= 0.400 P2= 2.84"			
13.2	1,089	0.0760	1.38		Shallow Concentrated Flow,			
					Woodland Kv= 5.0 fps			
23.1	1,189	Total						

Summary for Subcatchment 4S: SUBCATCHMENT

Runoff = 1.45 cfs @ 12.10 hrs, Volume=

0.12 af, Depth> 0.71"

2304141-POST DEVELOPMENT

CNI

Aran (nf)

Description

Type III 24-hr 2-YEAR Rainfall=2.95"

Prepared by Keach-Nordstrom Associates, Inc.

Printed 12/6/2023

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC Page 9

Α	rea (sf)	CN	Description						
	10,510	55.0	Woods, Good, HSG B						
	6,756	70.0	Woods, Good, HSG C						
	40,259	61.0	>75% Grass cover, Good, HSG B						
	10,029	74.0	>75% Grass cover, Good, HSG C						
	16,327	98.0	Paved parking, HSG C						
*	2,548	98.0	Roofs						
	86,429	70.6	Weighted Average						
	67,554	62.9	78.16% Pervious Area						
	18,875	98.0	21.84% Impervious Area						
Tc	Length	Slope	Velocity Capacity Description						
(min)	(feet)	(ft/ft)	(ft/sec) (cfs)						
6.0			Direct Entry,						

Summary for Subcatchment 5S: SUBCATCHMENT

Runoff = 0.32 cfs @ 12.13 hrs, Volume= 0.03 af, Depth> 0.86"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 2-YEAR Rainfall=2.95"

	A	rea (st)	CN	Description	n		
		10,801	61.0	>75% Gra	ass cover, (Good, HSG B	
*		5,640	98.0	Paved pa	rking		
		16,441	73.7	Weighted	Average		
		10,801	61.0	65.70% P	ervious Are	ea	
		5,640	98.0	34.30% Ir	npervious A	Area	
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description	
	6.3	100	0.0700	0.26		Sheet Flow,	
						Grass: Short n= 0.150 P2= 2.84"	
	0.1	16	0.0400	4.06		Shallow Concentrated Flow,	
						Paved Kv= 20.3 fps	
	1.5	162	0.0680	1.83		Shallow Concentrated Flow,	
						Short Grass Pasture Kv= 7.0 fps	
	0.1	20	0.0400	4.06		Shallow Concentrated Flow,	
						Paved Kv= 20.3 fps	
	0.8	91	0.0770	1.94		Shallow Concentrated Flow,	
-						Short Grass Pasture Kv= 7.0 fps	
	8.7	389	Total				

Summary for Subcatchment 6S: SUBCATCHMENT

Runoff = 1.52 cfs @ 12.29 hrs, Volume= 0.17

0.17 af, Depth> 0.96"

2304141-POST DEVELOPMENT

Type III 24-hr 2-YEAR Rainfall=2.95"

Prepared by Keach-Nordstrom Associates, Inc.

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Printed 12/6/2023

Page 10

		2.0	011			
_	A	rea (sf)	CN	Description		
		42,274	70.0		Good, HSG	
		35,721	74.0	>75% Gra	ass cover, (Good, HSG C
*		8,350	98.0	Paved pa	rking	
*		4,800	98.0	Roofs		
		217	96.0	Gravel su	rface, HSG	B C
		91,362	75.7	Weighted		
		78,212	71.9		ervious Are	ea
		13,150	98.0		npervious /	
		,				
	Tc	Length	Slope	Velocity	Capacity	Description
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	15.8	100	0.0500	0.11		Sheet Flow,
						Woods: Light underbrush n= 0.400 P2= 2.84"
	1.0	80	0.0750	1.37		Shallow Concentrated Flow,
						Woodland Kv= 5.0 fps
	0.2	18	0.0400	1.40		Shallow Concentrated Flow,
						Short Grass Pasture Kv= 7.0 fps
	0.2	12	0.0400	1.00		Shallow Concentrated Flow,
						Woodland Kv= 5.0 fps
	0.1	15	0.3000	3.83		Shallow Concentrated Flow,
						Short Grass Pasture Kv= 7.0 fps
	2.2	467	0.0300	3.52		Shallow Concentrated Flow,
						Paved Kv= 20.3 fps
	19.5	692	Total			The state of the s

Summary for Subcatchment 7S: SUBCATCHMENT

Runoff = 1.57 cfs @ 12.32 hrs, Volume=

0.18 af, Depth> 0.85"

-	Area (sf)	CN	Description
	83,250	70.0	Woods, Good, HSG C
	18,246	74.0	>75% Grass cover, Good, HSG C
*	5,928	98.0	Paved parking
	764	96.0	Gravel surface, HSG C
*	4,800	98.0	Roofs
	112,988	73.5	Weighted Average
	102,260	70.9	90.51% Pervious Area
	10,728	98.0	9.49% Impervious Area

2304141-POST DEVELOPMENT

Type III 24-hr 2-YEAR Rainfall=2.95"

Prepared by Keach-Nordstrom Associates, Inc.

Printed 12/6/2023

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 11

	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
-	12.5	100	0.0900	0.13		Sheet Flow,
						Woods: Light underbrush n= 0.400 P2= 2.84"
	7.5	755	0.1130	1.68		Shallow Concentrated Flow,
						Woodland Kv= 5.0 fps
	0.0	6	0.3000	3.83		Shallow Concentrated Flow,
						Short Grass Pasture Kv= 7.0 fps
	0.7	160	0.0375	3.93		Shallow Concentrated Flow,
12						Paved Kv= 20.3 fps
	20.7	1,021	Total			

Summary for Subcatchment 8S: SUBCATCHMENT

Runoff = 0.68 cfs @ 12.28 hrs, Volume=

0.07 af, Depth> 1.24"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 2-YEAR Rainfall=2.95"

	A	rea (sf)	CN	Description	n	
		13,046	70.0	Woods, G	Good, HSG	C
		6,935	74.0	>75% Gra	ass cover, (Good, HSG C
*		10,596	98.0	Paved pa	rking	
		30,577	80.6	Weighted	Average	
		19,981	71.4		ervious Are	ea ea
		10,596	98.0	34.65% Ir	npervious A	Area
	Tc	Length	Slope	Velocity	Capacity	Description
(1	min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	15.8	100	0.0500	0.11		Sheet Flow,
						Woods: Light underbrush n= 0.400 P2= 2.84"
	2.9	329	0.1480	1.92		Shallow Concentrated Flow,
						Woodland Kv= 5.0 fps
	0.1	33	0.3330	4.04		Shallow Concentrated Flow,
						Short Grass Pasture Kv= 7.0 fps
	1.5	257	0.0200	2.87		Shallow Concentrated Flow,
						Paved Kv= 20.3 fps
1	20.3	719	Total			

Summary for Subcatchment 9S: SUBCATCHMENT

Runoff = 1.03 cfs @ 12.40 hrs, Volume=

0.13 af, Depth> 0.99"

2304141-POST DEVELOPMENT

Type III 24-hr 2-YEAR Rainfall=2.95"

Prepared by Keach-Nordstrom Associates, Inc.

Printed 12/6/2023

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 12

		rea (sf)	CN	Description	n	
		34,147	70.0	Woods, G	Good, HSG	C
		21,287	74.0	>75% Gra	ass cover, (Good, HSG C
4	•	7,590	98.0	Paved pa	•	,
4	•	4,800	98.0	Roofs		
		67,824	76.4	Weighted	Average	
		55,434	71.5		ervious Are	ea
		12,390	98.0	18.27% Ir	npervious A	Area
					•	
	Тс	Length	Slope	Velocity	Capacity	Description
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	19.4	100	0.0300	0.09		Sheet Flow,
						Woods: Light underbrush n= 0.400 P2= 2.84"
	2.6	350	0.1940	2.20		Shallow Concentrated Flow,
						Woodland Kv= 5.0 fps
	0.2	43	0.3330	4.04		Shallow Concentrated Flow,
						Short Grass Pasture Kv= 7.0 fps
	4.6	800	0.0200	2.87		Shallow Concentrated Flow,
_						Paved Kv= 20.3 fps
	26.9	1,293	Total			

Summary for Subcatchment 10S: SUBCATCHMENT

Runoff = 0.88 cfs @ 12.36 hrs, Volume=

0.11 af, Depth> 0.82"

	A	rea (sf)	CN	Description	n	
		49,642	70.0	Woods, G	Good, HSG	C
		13,315	74.0		•	Good, HSG C
*		3,077	98.0	Paved pa	rking	,
*		2,400	98.0	Roofs		
		68,434	73.0	Weighted	Average	
		62,957	70.8		ervious Are	ea
		5,477	98.0	8.00% lm	pervious A	rea
					•	
	Tc	Length	Slope	Velocity	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	17.3	100	0.0400	0.10		Sheet Flow,
						Woods: Light underbrush n= 0.400 P2= 2.84"
	4.0	479	0.1610	2.01		Shallow Concentrated Flow,
						Woodland Kv= 5.0 fps
	1.5	65	0.0100	0.70		Shallow Concentrated Flow,
						Short Grass Pasture Kv= 7.0 fps
	0.2	32	0.0200	2.87		Shallow Concentrated Flow,
						Paved Kv= 20.3 fps
	23.0	676	Total			

2304141-POST DEVELOPMENT

Type III 24-hr 2-YEAR Rainfall=2.95"

Prepared by Keach-Nordstrom Associates, Inc. HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Printed 12/6/2023

Page 13

Summary for Subcatchment 11S: SUBCATCHMENT

Runoff = 0.79 cfs @ 12.33 hrs, Volume=

0.09 af, Depth> 0.74"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 2-YEAR Rainfall=2.95"

	A	rea (sf)	CN	Description	n	
		54,267	70.0	Woods, G	ood, HSG	C
		10,350	74.0	>75% Gra	iss cover, (Good, HSG C
	*	1,779	98.0	Paved par	rking	
66,396 71.4 Weighted Average						
		64,617	70.6	97.32% P	ervious Are	ea
		1,779	98.0	2.68% lm	pervious Ai	rea
	Tc	Length	Slope	Velocity	Capacity	Description
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	15.8	100	0.0500	0.11		Sheet Flow,
						Woods: Light underbrush n= 0.400 P2= 2.84"
	4.2	485	0.1510	1.94		Shallow Concentrated Flow,
						Woodland Kv= 5.0 fps
	0.6	73	0.0800	1.98		Shallow Concentrated Flow,
						Short Grass Pasture Kv= 7.0 fps
	0.1	32	0.0600	4.97		Shallow Concentrated Flow,
						Paved Kv= 20.3 fps
	20.7	690	Total			

Summary for Subcatchment 12S: SUBCATCHMENT

Runoff = 0.89 cfs @ 12.28 hrs, Volume=

0.10 af, Depth> 0.87"

	Area (sf)	CN	Description
	38,779	70.0	Woods, Good, HSG C
	14,722	74.0	>75% Grass cover, Good, HSG C
*	3,785	98.0	Paved parking
*	2,400	98.0	Roofs
	59,686	73.9	Weighted Average
	53,501	71.1	89.64% Pervious Area
	6,185	98.0	10.36% Impervious Area

2304141-POST DEVELOPMENT

Type III 24-hr 2-YEAR Rainfall=2.95"

Prepared by Keach-Nordstrom Associates, Inc.

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Printed 12/6/2023

LC Page 14

	Τc	Length	Slope	Velocity	Capacity	Description
(r	nin)	(feet)	(ft/ft)	(ft/sec)	(cfs)	<u> </u>
1	3.8	100	0.0700	0.12		Sheet Flow,
						Woods: Light underbrush n= 0.400 P2= 2.84"
	4.0	462	0.1470	1.92		Shallow Concentrated Flow,
						Woodland Kv= 5.0 fps
	0.4	55	0.1100	2.32		Shallow Concentrated Flow,
						Short Grass Pasture Kv= 7.0 fps
	0.5	136	0.0600	4.97		Shallow Concentrated Flow,
_						Paved Kv= 20.3 fps
1	8.7	753	Total			

Summary for Subcatchment 13S: SUBCATCHMENT

Runoff = 0.78 cfs @ 12.24 hrs, Volume=

0.08 af, Depth> 0.81"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 2-YEAR Rainfall=2.95"

	A	rea (sf)	CN	Description	n	
		40,261	70.0	Woods, G	Good, HSG	C
		10,598	74.0	>75% Gra	ass cover, (Good, HSG C
	*	3,610	98.0	Paved pa		,
		54,469	72.6	Weighted	Average	
		50,859	70.8		ervious Are	ea
		3,610	98.0		pervious A	
	Tc	Length	Slope	Velocity	Capacity	Description
0	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	12.0	100	0.1000	0.14		Sheet Flow,
						Woods: Light underbrush n= 0.400 P2= 2.84"
	3.8	425	0.1410	1.88		Shallow Concentrated Flow,
						Woodland Kv= 5.0 fps
	0.3	61	0.3300	4.02		Shallow Concentrated Flow,
						Short Grass Pasture Kv= 7.0 fps
	0.3	94	0.0600	4.97		Shallow Concentrated Flow,
100						Paved Kv= 20.3 fps
	16.3	680	Total			

Summary for Subcatchment 14S: SUBCATCHMENT

Runoff = 0.58 cfs @ 12.26 hrs, Volume=

0.06 af, Depth> 0.86"

2304141-POST DEVELOPMENT

Type III 24-hr 2-YEAR Rainfall=2.95"

Prepared by Keach-Nordstrom Associates, Inc.

Printed 12/6/2023

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 15

	Α	rea (sf)	CN	Description	n	
		25,189	70.0	Woods, G	ood, HSG	С
		9,128	74.0	>75% Gra	iss cover, (Good, HSG C
*		3,704	98.0	Paved par	rking	
		38,021	73.7	Weighted	Average	
		34,317	71.1	90.26% P	ervious Are	ea
		3,704	98.0	9.74% lm	pervious Ar	rea
	Tc	Length	Slope	Velocity	Capacity	Description
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	11.1	100	0.1200	0.15		Sheet Flow,
						Woods: Light underbrush n= 0.400 P2= 2.84"
	5.5	529	0.1020	1.60		Shallow Concentrated Flow,
						Woodland Kv= 5.0 fps
	0.2	46	0.3300	4.02		Shallow Concentrated Flow,
						Short Grass Pasture Kv= 7.0 fps
	0.3	56	0.0200	2.87		Shallow Concentrated Flow,
						Paved Kv= 20.3 fps
S-	17.2	731	Total			

Summary for Subcatchment 15S: SUBCATCHMENT

Runoff = 0.57 cfs @ 12.09 hrs, Volume=

0.04 af, Depth> 1.46"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 2-YEAR Rainfall=2.95"

	Α	rea (sf)	CN	Description	n		
		8,678	74.0	>75% Gra	ss cover, (Good, HSG C	
*		5,925	98.0	Paved par	rking		
		14,603	83.7	Weighted	Average		
		8,678	74.0	59.43% P	ervious Are	rea	
		5,925	98.0	40.57% In	npervious A	Area	
U	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	, ·	
	6.0					Direct Entry,	

Summary for Subcatchment 16S: SUBCATCHMENT

Runoff = 0.43 cfs @ 12.09 hrs, Volume=

0.03 af, Depth> 2.01"

2304141-POST DEVELOPMENT

Type III 24-hr 2-YEAR Rainfall=2.95"

Prepared by Keach-Nordstrom Associates, Inc.

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Printed 12/6/2023

Solutions LLC Page 16

	Area (sf)	CN	Description	
*	2,375	74.0	>75% Grass cover, Good, HSG C	
	5,613	98.0	Paved parking	
	7,988 2,375 5,613	90.9 74.0 98.0	Weighted Average 29.73% Pervious Area 70.27% Impervious Area	
(m	Tc Length in) (feet)	Slope (ft/ft)	y and and a conference	
(6.0		Direct Entry,	

Summary for Subcatchment 17S: SUBCATCHMENT

Runoff = 1.45 cfs @ 12.20 hrs, Volume= 0.14 af, Depth> 0.88"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 2-YEAR Rainfall=2.95"

	Area (sf)	CN	Description	on	
	26,296	70.0	Woods, C	Good, HSG	C
	51,457	74.0			Good, HSG C
*	1,238	98.0	Paved pa		,
*	2,400	98.0	Roofs	Ü	
	1,535	96.0	Gravel su	ırface, HSG	B C
	82,926	74.2	Weighted	Average	
	79,288	73.1		ervious Are	ea
	3,638	98.0	4.39% lm	pervious A	rea
T		Slope	Velocity	Capacity	Description
(min) (feet)	(ft/ft)	(ft/sec)	(cfs)	
12.0	100	0.1000	0.14		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 2.84"
1.0	90	0.0860	1.47		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
0.2	2 40	0.3300	4.02		Shallow Concentrated Flow,
					Short Grass Pasture Kv= 7.0 fps
13.2	2 230	Total			

Summary for Reach 1R: EXISTING ROADWAY DITCH

Inflow Area = 2.362 ac, 23.83% Impervious, Inflow Depth > 0.73" for 2-YEAR event

Inflow = 1.75 cfs @ 12.11 hrs, Volume= 0.14 af

Outflow = 1.72 cfs @ 12.12 hrs, Volume= 0.14 af, Atten= 2%, Lag= 1.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Max. Velocity:= 3.08 fps, Min. Travel Time= 1.2 min

Avg. Velocity = 1.46 fps, Avg. Travel Time= 2.6 min

2304141-POST DEVELOPMENT

Type III 24-hr 2-YEAR Rainfall=2.95"

Prepared by Keach-Nordstrom Associates, Inc.

Printed 12/6/2023

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 17

Peak Storage= 128 cf @ 12.12 hrs
Average Depth at Peak Storage= 0.43'

Bank Full Dooth= 1.00', Flow Area= 3.0 sf. C

Bank-Full Depth= 1.00' Flow Area= 3.0 sf, Capacity= 16.21 cfs

0.00' x 1.00' deep channel, n= 0.022 Earth, clean & straight

Side Slope Z-value= 3.0 '/' Top Width= 6.00'

Length= 230.0' Slope= 0.0173 '/'

Inlet Invert= 295.64', Outlet Invert= 291.66'



Summary for Reach 2R: EXISTING ROADWAY DITCH

Inflow Area = 11.270 ac, 12.00% Impervious, Inflow Depth > 0.26" for 2-YEAR event

Inflow = 0.26 cfs @ 20.94 hrs, Volume= 0.24 af

Outflow = 0.26 cfs @ 21.08 hrs, Volume= 0.24 af, Atten= 0%, Lag= 8.3 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Max. Velocity= 1.29 fps, Min. Travel Time= 11.8 min Avg. Velocity = 1.10 fps, Avg. Travel Time= 13.9 min

Peak Storage= 182 cf @ 21.08 hrs Average Depth at Peak Storage= 0.19'

Bank-Full Depth= 1.00' Flow Area= 3.5 sf, Capacity= 12.04 cfs

0.50' x 1.00' deep channel, n= 0.069 Riprap, 6-inch Side Slope Z-value= 3.0 '/' Top Width= 6.50'

Length= 915.0' Slope= 0.0621 '/'

Inlet Invert= 348.50', Outlet Invert= 291.66'



Summary for Pond 1P: EXISTING CB

Inflow Area = 22.064 ac, 4.08% Impervious, Inflow Depth > 0.46" for 2-YEAR event

Inflow = 4.52 cfs @ 12.66 hrs, Volume=: 0.85 af

Primary = 4.52 cfs @ 12.66 hrs, Volume: 0.85 af, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method. Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

2304141-POST DEVELOPMENT

Type III 24-hr 2-YEAR Rainfall=2.95"

Prepared by Keach-Nordstrom Associates, Inc. HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Printed 12/6/2023

Page 18

Summary for Pond 2P: EXISTING DRIVEWAY CULVERT

Inflow Area = 2.596 ac, 12.76% Impervious, Inflow Depth > 0.36" for 2-YEAR event

Inflow = 0.44 cfs @ 12.43 hrs, Volume= 0.08 af

Outflow = 0.44 cfs @ 12.43 hrs, Volume= 0.08 af, Atten= 0%, Lag= 0.0 min

Primary = 0.44 cfs @ 12.43 hrs, Volume= 0.08 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 292.73' @ 12.43 hrs

Flood Elev= 294.00'

Primary OutFlow Max=0.44 cfs @ 12.43 hrs HW=292.73' TW=289.63' (Dynamic Tailwater) 1=Culvert (Inlet Controls 0.44 cfs @ 1.96 fps)

Summary for Pond 3P: EXISTING DROP INLET

Inflow Area = 41.139 ac, 8.14% Impervious, Inflow Depth > 0.41" for 2-YEAR event

Inflow = 5.83 cfs @ 12.62 hrs, Volume= 1.39 af

Outflow = 5.83 cfs @ 12.62 hrs, Volume= 1.39 af, Atten= 0%, Lag= 0.0 min

Primary = 5.83 cfs @ 12.62 hrs, Volume= 1.39 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 289.71' @ 12.62 hrs

Flood Elev= 293.75'

Device	Routing	Invert	Outlet Devices
#1	Primary	288.66'	24.0" Round Culvert
			L= 92.1' RCP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 288.66' / 284.67' S= 0.0433 '/' Cc= 0.900
			n= 0.011 Concrete pipe, straight & clean, Flow Area= 3.14 sf

Primary OutFlow Max=5.83 cfs @ 12.62 hrs HW=289.71' TW=0.00' (Dynamic Tailwater) 1=Culvert (Inlet Controls 5.83 cfs @ 3.49 fps)

Summary for Pond 4P: EXISTING CB

Inflow Area =	2.362 ac, 23.83% Impervious, Inflow	Depth > 0.73" for 2-YEAR event
Inflow =	1.75 cfs @ 12.11 hrs, Volume=	0.14 af
Outflow =	1.75 cfs @ 12.11 hrs, Volume=	0.14 af, Atten= 0%, Lag= 0.0 min
Primary =	1.75 cfs @ 12.11 hrs, Volume=	0.14 af
Secondary =	0.00 cfs @ 0.00 hrs, Volume=	0.00 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

2304141-POST DEVELOPMENT

Type III 24-hr 2-YEAR Rainfall=2.95"

Prepared by Keach-Nordstrom Associates, Inc.

Printed 12/6/2023

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 19

Peak Elev= 297.75' @ 12.11 hrs

Flood Elev= 300.17'

Device	Routing	Invert	Outlet Devices
#1	Primary	297.00'	12.0" Round Culvert
	·		L= 49.1' CMP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 297.00' / 295.64' S= 0.0277 '/' Cc= 0.900
			n= 0.025 Corrugated metal, Flow Area= 0.79 sf
#2	Secondary	300.17'	24.0" x 24.0" Horiz. Orifice/Grate C= 0.600
	•		Limited to weir flow at low heads

Primary OutFlow Max=1.75 cfs @ 12.11 hrs HW=297.75' TW=296.07' (Dynamic Tailwater) 1=Culvert (Barrel Controls 1.75 cfs @ 3.86 fps)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=297.00' TW=295.64' (Dynamic Tailwater) 2=Orifice/Grate (Controls 0.00 cfs)

Summary for Pond 5P: EXISTING CB

Inflow Area =	0.377 ac, 34.30% Impervious, Inflow	Depth > 0.86" for 2-YEAR event
Inflow =	0.32 cfs @ 12.13 hrs, Volume=	0.03 af
Outflow =	0.32 cfs @ 12.13 hrs, Volume=	0.03 af, Atten= 0%, Lag= 0.0 min
Primary =	0.32 cfs @ 12.13 hrs, Volume=	0.03 af
Secondary =	0.00 cfs @ 0.00 hrs, Volume=	0.00 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 298.09' @ 12.13 hrs Flood Elev= 301.16'

Device	Routing	Invert	Outlet Devices
#1	Primary	297.70'	12.0" Round Culvert
	•		L= 34.1' CMP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 297.70' / 297.30' S= 0.0117 '/' Cc= 0.900
			n= 0.025 Corrugated metal, Flow Area= 0.79 sf
#2	Secondary	301.16'	24.0" x 24.0" Horiz. Orifice/Grate C= 0.600
	•		Limited to weir flow at low heads

Primary OutFlow Max=0.33 cfs @ 12.13 hrs HW=298.09' TW=297.72' (Dynamic Tailwater) 1=Culvert (Outlet Controls 0.33 cfs @ 1.69 fps)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=297.70' TW=0.00' (Dynamic Tailwater) 2=Orifice/Grate (Controls 0.00 cfs)

Summary for Pond 6P: EXISTING CB

Inflow Are	a =	2.097 ac, 14.39% Impe vious, Inflow Depth > 0.96" for 2-YEAR ev	/ent
Inflow	=	1.52 cfs @ 12.29 hrs, Volume= 0.17 af	
Outflow	=	1.52 cfs @ 12.29 hrs, Volume= 0.17 af, Atten= 0%, Lag= 0.0) min
Primary	=	1.52 cfs @ 12.29 hrs, Volume= 0.17 af	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

2304141-POST DEVELOPMENT

Type III 24-hr 2-YEAR Rainfall=2.95"

Prepared by Keach-Nordstrom Associates, Inc.

Printed 12/6/2023

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 20

Peak Elev= 364.36' @ 12.29 hrs

Flood Elev= 366.35'

Device	Routing	Invert	Outlet Devices
#1	Primary	363.70'	12.0" Round Culvert
			L= 18.0' CMP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 363.70' / 362.46' S= 0.0689 '/' Cc= 0.900
			n= 0.025 Corrugated metal. Flow Area= 0.79 sf

Primary OutFlow Max=1.52 cfs @ 12.29 hrs HW=364.36' TW=0.00' (Dynamic Tailwater) 1=Culvert (Inlet Controls 1.52 cfs @ 2.76 fps)

Summary for Pond 7P: EXISTING CB

Inflow Area = 2.594 ac, 9.49% Impervious, Inflow Depth > 0.85" for 2-YEAR event

Inflow = 1.57 cfs @ 12.32 hrs, Volume= 0.18 af

Outflow = 1.57 cfs @ 12.32 hrs, Volume= 0.18 af, Atten= 0%, Lag= 0.0 min

Primary = 1.57 cfs @ 12.32 hrs, Volume= 0.18 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 377.28' @ 12.32 hrs

Flood Elev= 380.49'

Device	Routing	Invert	Outlet Devices
#1	Primary	376.57'	12.0" Round Culvert
			L= 31.9' RCP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 376.57' / 376.28' S= 0.0091 '/' Cc= 0.900
			n= 0.011 Concrete pipe, straight & clean Flow Area= 0.79 sf

Primary OutFlow Max=1.57 cfs @ 12.32 hrs HW=377.28' TW=0.00' (Dynamic Tailwater)
1=Culvert (Barrel Controls 1.57 cfs @ 3.71 fps)

Summary for Pond 8P: DMH#2

Inflow Area = 11.270 ac, 12.00% Impervious, Inflow Depth > 0.26" for 2-YEAR event

Inflow = 0.26 cfs @ 20.93 hrs, Volume= 0.24 af

Outflow = 0.26 cfs @ 20.94 hrs, Volume= 0.24 af, Atten= 0%, Lag= 0.4 min

Primary = 0.26 cfs @ 20.94 hrs, Volume= 0.24 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 349.93' @ 20.94 hrs

Flood Elev= 353.05'

Device	Routing	Invert	Outlet Devices
#1	Primary	349.70'	15.0" Round Culvert L= 15.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 349.70' / 348.50' S= 0.0800 '/' Cc= 0.900 n= 0.013, Flow Area= 1.23 sf

Primary OutFlow Max=0.26 cfs @ 20.94 hrs HW=349.93' TW=348.69' (Dynamic Tailwater)
1=Culvert (Inlet Controls 0.26 cfs @ 1.64 fps)

Type III 24-hr 2-YEAR Rainfall=2.95"

Prepared by Keach-Nordstrom Associates, Inc.

Printed 12/6/2023

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 21

Summary for Pond 9P: DMH#3

Inflow Area = 5.536 ac, 11.12% Impervious, Inflow Depth > 0.89" for 2-YEAR event

Inflow = 3.47 cfs @ 12.27 hrs, Volume= 0.41 af

Outflow = 3.47 cfs @ 12.27 hrs, Volume= 0.41 af, Atten= 0%, Lag= 0.0 min

Primary = 3.47 cfs @ 12.27 hrs, Volume= 0.41 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 375,27' @ 12.27 hrs

Flood Elev= 377.79'

Device	Routing	Invert	Outlet Devices
#1	Primary	374.26'	18.0" Round Culvert L= 30.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 374.26' / 374.04' S= 0.0073 '/' Cc= 0.900 n= 0.013, Flow Area= 1.77 sf

Primary OutFlow Max=3.47 cfs @ 12.27 hrs HW=375.27' TW=367.71' (Dynamic Tailwater) 1=Culvert (Barrel Controls 3.47 cfs @ 3.86 fps)

Summary for Pond 10P: DMH#4

Inflow Area = 5.536 ac, 11.12% Impervious, Inflow Depth > 0.89" for 2-YEAR event

Inflow = 3.47 cfs @ 12.27 hrs, Volume= 0.41 af

Outflow = 3.47 cfs @ 12.27 hrs, Volume= 0.41 af, Atten= 0%, Lag= 0.0 min

Primary = 3.47 cfs @ 12.27 hrs, Volume= 0.41 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 376.49' @ 12.27 hrs

Flood Elev= 381.66'

Device	Routing	Invert	Outlet Devices
#1	Primary	375.58'	18.0" Round Culvert
	·		L= 121.7' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 375.58' / 374.36' S= 0.0100 '/' Cc= 0.900
			n= 0.013. Flow Area= 1.77 sf

Primary OutFlow Max=3.47 cfs @ 12.27 hrs HW=376.49' TW=375.27' (Dynamic Tailwater) 1=Culvert (Outlet Controls 3.47 cfs @ 4.46 fps)

Summary for Pond 11P: DMH#5

Inflow Area = 5.536 ac, 11.12% Impervious, Inflow Depth > 0.89" for 2-YEAR event

Inflow = 3.47 cfs @ 12.27 hrs, Volume= 0.41 af

Outflow = 3.47 cfs @ 12.27 hrs, Volume= 0.41 af, Atten= 0%, Lag= 0.0 min

Primary = 3.47 cfs @ 12.27 hrs, Volume= 0.41 af

Routing by Dyn-Stor-Ind method, Tirne Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 378.82' @ 12.27 hrs

Flood Elev= 384.00'

2304141-POST DEVELOPMENT

Type III 24-hr 2-YEAR Rainfall=2.95"

Prepared by Keach-Nordstrom Associates, Inc.
HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Printed 12/6/2023

Page 22

Device	Routing	Invert	Outlet Devices
#1	Primary	377.94'	18.0" Round Culvert L= 226.1' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 377.94' / 375.68' S= 0.0100 '/' Cc= 0.900 n= 0.013, Flow Area= 1.77 sf

Primary OutFlow Max=3.47 cfs @ 12.27 hrs HW=378.82' TW=376.49' (Dynamic Tailwater) 1=Culvert (Inlet Controls 3.47 cfs @ 3.20 fps)

Summary for Pond 12P: CB#6

Inflow Area = 5.536 ac, 11.12% Impervious, Inflow Depth > 0.89" for 2-YEAR event

Inflow = 3.47 cfs @ 12.27 hrs, Volume= 0.41 af

Outflow = 3.47 cfs @ 12.27 hrs, Volume= 0.41 af, Atten= 0%, Lag= 0.0 min

Primary = 3.47 cfs @ 12.27 hrs, Volume= 0.41 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 380.33' @ 12.27 hrs

Flood Elev= 384.30'

Primary OutFlow Max=3.47 cfs @ 12.27 hrs HW=380.33' TW=378.82' (Dynamic Tailwater) 1=Culvert (Inlet Controls 3.47 cfs @ 3.20 fps)

Summary for Pond 13P: CB#7

Inflow Area = 5.201 ac, 9.22% Impervious, Inflow Depth > 0.85" for 2-YEAR event

Inflow = 3.20 cfs @ 12.27 hrs, Volume= 0.37 af

Outflow = 3.20 cfs @ 12.27 hrs, Volume= 0.37 af, Atten= 0%, Lag= 0.0 min

Primary = 3.20 cfs @ 12.27 hrs, Volume= 0.37 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 381.26' @ 12.27 hrs

Flood Elev= 384.30'

Device	Routing	Invert	Outlet Devices
#1	Primary	380.25'	15.0" Round Culvert
			L= 30.3' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 380.25' / 379.95' S= 0.0099 '/' Cc= 0.900
			n= 0.013, Flow Area= 1.23 sf

Primary OutFlow Max=3.19 cfs @ 12.27 hrs HW=381.26' TW=380.33' (Dynamic Tailwater) 1=Culvert (Barrel Controls 3.19 cfs @ 4.11 fps)

Type III 24-hr 2-YEAR Rainfall=2.95"

Prepared by Keach-Nordstrom Associates, Inc.

Printed 12/6/2023

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 23

Summary for Pond 14P: CB#8

Inflow Area = 4.328 ac, 9.12% Impervious, Inflow Depth > 0.85" for 2-YEAR event

Inflow = 2.62 cfs @ 12.27 hrs, Volume= 0.31 af

Outflow = 2.62 cfs @ 12.27 hrs, Volume= 0.31 af, Atten= 0%, Lag= 0.0 min

Primary = 2.62 cfs @ 12.27 hrs, Volume= 0.31 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 385.31' @ 12.27 hrs

Flood Elev= 388.51'

Device	Routing	Invert	Outlet Devices
#1	Primary	384.49'	15.0" Round Culvert L= 165.7' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 384.49' / 380.35' S= 0.0250 '/' Cc= 0.900 n= 0.013, Flow Area= 1.23 sf

Primary OutFlow Max=2.62 cfs @ 12.27 hrs HW=385.31' TW=381.26' (Dynamic Tailwater)
—1=Culvert (Inlet Controls 2.62 cfs @ 3.08 fps)

Summary for Pond 16P: CB#10

Inflow Area = 3.078 ac, 10.13% Impervious, Inflow Depth > 0.87" for 2-YEAR event

Inflow = 1.85 cfs @ 12.29 hrs, Volume= 0.22 af

Outflow = 1.85 cfs @ 12.29 hrs, Volume= 0.22 af, Atten= 0%, Lag= 0.0 min

Primary = 1.85 cfs @ 12.29 hrs, Volume= 0.22 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 392.86' @ 12.29 hrs

Flood Elev= 397.31'

Device	Routing	Invert	Outlet Devices
#1	Primary	392.11'	12.0" Round Culvert
			L= 165.5' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 392.11 / 383.83' S= 0.0500 '/' Cc= 0.900
			n= 0.013, Flow Area= 0.79 sf

Primary OutFlow Max=1.85 cfs @ 12.29 hrs HW=392.86' TW=385.31' (Dynamic Tailwater) 1=Culvert (Inlet Controls 1.85 cfs @ 2.94 fps)

Summary for Pond 17P: CB#100

Inflow Area = 0.183 ac, 70.27% Impervious, Inflow Depth > 2.01" for 2-YEAR event

Inflow = 0.43 cfs @ 12.09 hrs, Volume= 0.03 af

Outflow = 0.43 cfs @ 12.09 hrs, Volume= 0.03 af, Atten= 0%, Lag= 0.0 min

Primary = 0.43 cfs @ 12.09 hrs, Volume= 0.03 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 393.57' @ 12.09 hrs

Flood Elev= 397.31'

2304141-POST DEVELOPMENT

Type III 24-hr 2-YEAR Rainfall=2.95"

Prepared by Keach-Nordstrom Associates, Inc.

Printed 12/6/2023

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 24

Device	Routing	Invert	Outlet Devices
#1	Primary	393.25'	12.0" Round Culvert
			L= 26.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 393.25' / 392.73' S= 0.0200 '/' Cc= 0.900
			n= 0.013, Flow Area= 0.79 sf

Primary OutFlow Max=0.43 cfs @ 12.09 hrs HW=393.57' TW=392.66' (Dynamic Tailwater) 1=Culvert (Inlet Controls 0.43 cfs @ 1.94 fps)

Summary for Pond 18P: CB#11

Inflow Area = 1.524 ac, 2.68% Impervious, Inflow Depth > 0.74" for 2-YEAR event Inflow = 0.79 cfs @ 12.33 hrs, Volume= 0.09 af

Outflow = 0.79 cfs @ 12.33 hrs, Volume= 0.09 af, Atten= 0%, Lag= 0.0 min

Primary = 0.79 cfs @ 12.33 hrs, Volume= 0.09 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 403.25' @ 12.33 hrs

Flood Elev= 406.88'

Primary OutFlow Max=0.79 cfs @ 12.33 hrs HW=403.25' TW=392.85' (Dynamic Tailwater) 1=Culvert (Inlet Controls 0.79 cfs @ 2.29 fps)

Summary for Pond 20P: DMH#14

Inflow Area = 3.830 ac, 17.06% Impervious, Inflow Depth > 0.97" for 2-YEAR event

Inflow = 2.54 cfs @ 12.35 hrs, Volume= 0.31 af

Outflow = 2.54 cfs @ 12.35 hrs, Volume= 0.31 af, Atten= 0%, Lag= 0.0 min

Primary = 2.54 cfs @ 12.35 hrs, Volume= 0.31 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 375.94' @ 12.35 hrs

Flood Elev= 383.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	375.20'	18.0" Round Culvert
			L= 15.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 375.20' / 374.00' S= 0.0800 '/' Cc= 0.900
			n= 0.013, Flow Area= 1.77 sf

Primary OutFlow Max=2.54 cfs @ 12.35 hrs HW=375.94' TW=367.93' (Dynamic Tailwater) 1=Culvert (Inlet Controls 2.54 cfs @ 2.93 fps)

Type III 24-hr 2-YEAR Rainfall=2.95"

Prepared by Keach-Nordstrom Associates, Inc.

Printed 12/6/2023

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 25

Summary for Pond 21P: DMH#15

Inflow Area = 3.830 ac, 17.06% Impervious, Inflow Depth > 0.97" for 2-YEAR event

Inflow = 2.54 cfs @ 12.35 hrs, Volume= 0.31 af

Outflow = 2.54 cfs @ 12.35 hrs, Volume= 0.31 af, Atten= 0%, Lag= 0.0 min

Primary = 2.54 cfs @ 12.35 hrs, Volume= 0.31 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 382.20' @ 12.35 hrs

Flood Elev= 387.85'

Device	Routing	Invert	Outlet Devices
#1	Primary	381.46'	18.0" Round Culvert L= 30.7' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 381.46' / 379.00' S= 0.0801 '/' Cc= 0.900 n= 0.013, Flow Area= 1.77 sf

Primary OutFlow Max=2.54 cfs @ 12.35 hrs HW=382.20' TW=375.94' (Dynamic Tailwater)
—1=Culvert (Inlet Controls 2.54 cfs @ 2.93 fps)

Summary for Pond 22P: DMH#16

Inflow Area = 3.830 ac, 17.06% Impervious, Inflow Depth > 0.97" for 2-YEAR event

Inflow = 2.54 cfs @ 12.35 hrs, Volume= 0.31 af

Outflow = 2.54 cfs @ 12.35 hrs, Volume= 0.31 af, Atten= 0%, Lag= 0.0 min

Primary = 2.54 cfs @ 12.35 hrs, Volume= 0.31 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 393.66' @ 12.35 hrs

Flood Elev= 400.42'

Device	Routing	Invert	Outlet Devices
#1	Primary	392.92'	18.0" Round Culvert
	-		L= 111.5' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 392.92' / 384.00' S= 0.0800 '/' Cc= 0.900
			n= 0.013, Flow Area= 1.77 sf

Primary OutFlow Max=2.54 cfs @ 12.35 hrs HW=393.66' TW=382.20' (Dynamic Tailwater)
—1=Culvert (Inlet Controls 2.54 cfs @ 2.93 fps)

Summary for Pond 23P: DMH#17

Inflow Area = 3.830 ac, 17.06% Impervious, Inflow Depth > 0.97" for 2-YEAR event

Inflow = 2.54 cfs @ 12.35 hrs, Volume= 0.31 af

Outflow = 2.54 cfs @ 12.35 hrs, Volume= 0.31 af, Atten= ()%, Lag= 0.0 min

Primary = 2.54 cfs @ 12.35 hrs, Volume= 0.31 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 401.87' @ 12.35 hrs

Flood Elev= 409.83'

2304141-POST DEVELOPMENT

Type III 24-hr 2-YEAR Rainfall=2.95"

Prepared by Keach-Nordstrom Associates, Inc.

Printed 12/6/2023

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 26

Device	Routing	Invert	Outlet Devices
#1	Primary		18.0" Round Culvert L= 45.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 401.13' / 397.53' S= 0.0800 '/' Cc= 0.900 n= 0.013, Flow Area= 1.77 sf

Primary OutFlow Max=2.54 cfs @ 12.35 hrs HW=401.87' TW=393.66' (Dynamic Tailwater) 1=Culvert (Inlet Controls 2.54 cfs @ 2.93 fps)

Summary for Pond 24P: CB#18

Inflow Area = 3.830 ac, 17.06% Impervious, Inflow Depth > 0.97" for 2-YEAR event

Inflow = 2.54 cfs @ 12.35 hrs, Volume= 0.31 af

Outflow = 2.54 cfs @ 12.35 hrs, Volume= 0.31 af, Atten= 0%, Lag= 0.0 min

Primary = 2.54 cfs @ 12.35 hrs, Volume= 0.31 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 403.61' @ 12.35 hrs

Flood Elev= 407.95'

Device	Routing	Invert	Outlet Devices
#1	Primary	402.87'	18.0" Round Culvert
			L= 164.2' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 402.87' / 401.23' S= 0.0100 '/' Cc= 0.900
			n= 0.013, Flow Area= 1.77 sf

Primary OutFlow Max=2.54 cfs @ 12.35 hrs HW=403.61' TW=401.87' (Dynamic Tailwater) 1=Culvert (Inlet Controls 2.54 cfs @ 2.93 fps)

Summary for Pond 25P: CB#19

Inflow Area = 2.259 ac, 23.36% Impervious, Inflow Depth > 1.07" for 2-YEAR event

Inflow = 1.66 cfs @ 12.35 hrs, Volume= 0.20 af

Outflow = 1.66 cfs @ 12.35 hrs, Volume= 0.20 af, Atten= 0%, Lag= 0.0 min

Primary = 1.66 cfs @ 12.35 hrs, Volume= 0.20 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 404.05' @ 12.36 hrs

Flood Elev= 408.04'

Device	Routing	Invert	Outlet Devices
#1	Primary		15.0" Round Culvert
			L= 36.5' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 403.34' / 402.97' S= 0.0101 '/' Cc= 0.900
			n= 0 013, Flow Area= 1.23 sf

Primary OutFlow Max=1.66 cfs @ 12.35 hrs HW=404.05' TW=403.61' (Dynamic Tailwater) 1=Culvert (Outlet Controls 1.66 cfs @ 3.36 fps)

2304141-POST DEVELOPMENT

Type III 24-hr 2-YEAR Rainfall=2.95"

Prepared by Keach-Nordstrom Associates, Inc.

Printed 12/6/2023

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 27

Summary for Pond 26P: CB#20

0.702 ac, 34.65% Impervious, Inflow Depth > 1.24" for 2-YEAR event Inflow Area =

0.68 cfs @ 12.28 hrs, Volume= 0.07 af Inflow

0.68 cfs @ 12.28 hrs, Volume= 0.07 af. Atten= 0%, Lag= 0.0 min Outflow =

Primary 0.68 cfs @ 12.28 hrs, Volume= 0.07 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 404.24' @ 12.33 hrs

Flood Elev= 407.78'

Device	Routing	Invert	Outlet Devices
#1	Primary	403.70'	12.0" Round Culvert
			L= 26.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 403.70' / 403.44' S= 0.0100 '/' Cc= 0.900
			n= 0.013, Flow Area= 0.79 sf

Primary OutFlow Max=0.67 cfs @ 12.28 hrs HW=404.23' TW=404.02' (Dynamic Tailwater) -1=Culvert (Outlet Controls 0.67 cfs @ 2.30 fps)

Summary for Pond PP: POCKET POND

Inflow Area =	11.270 ac, 12.00% Impervious, Inflow De	epth > 0.92" for 2-YEAR event
Inflow =	7.14 cfs @ 12.28 hrs, Volume=	0.86 af
Outflow =	0.26 cfs @ 20.93 hrs, Volume=	0.24 af, Atten= 96%, Lag= 519.4 min
Primary =	0.26 cfs @ 20.93 hrs, Volume=	0.24 af
Secondary =	0.00 cfs @ 0.00 hrs, Volume=	0.00 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Starting Elev= 367.00' Surf.Area= 9,015 sf Storage= 7,477 cf Peak Elev= 369.49' @ 20.93 hrs Surf.Area= 13,714 sf Storage= 34,886 cf (27,409 cf above start) Flood Elev= 374.00' Surf.Area= 24,964 sf Storage= 118,753 cf (111,276 cf above start)

Plug-Flow detention time= 639.5 min calculated for 0.07 af (8% of inflow) Center-of-Mass det. time= 225.2 min (1,096.3 - 871.2)

Volume	Invert	Avail.Storage	Storage Description
#1	364.00'	118,753 cf	Custom Stage Data (Prismatic) Listed below (Recalc)
#2	369.00'	0 cf	Sediment Forebay (No Storage) (Prismatic) Listed below (Recalc)
			1,678 cf Overall x 0.0% Voids
		118 753 cf	Total Available Storage

118.753 cf - Total Available Storage

Type III 24-hr 2-YEAR Rainfall=2.95"

Prepared by Keach-Nordstrom Associates, Inc.

Printed 12/6/2023-

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 28

Elevation	Surf.Area	Inc.Store	Cum.Store
(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)
364.00	145	0	0
366.00	1,883	2,028	2,028
367.00	9,015	5,449	7,477
368.00	10,563	9,789	17,266
369.00	12,205	11,384	28,650
370.00	13,939	13,072	41,722
371.00	17,278	15,609	57,331
372.00	19,350	18,314	75,645
374.00	23,758	43,108	118,753
Elevation	Surf.Area	Inc.Store	Cum.Store
(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)
369.00	472	0	0
371.00	1,206	1,678	1,678

Device	Routing	Invert	Outlet Devices
#1	Primary	365.00'	15.0" Round Culvert
			L= 190.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 365.00' / 349.80' S= 0.0800 '/' Cc= 0.900
			n= 0.013, Flow Area= 1.23 sf
#2	Device 1	367.00'	2.5" Vert. Orifice C= 0.600
#3	Device 1	369.45'	3.2" Vert. Orifice C= 0.600
#4	Device 1	371.50'	3.0" Vert. Orifice C= 0.600
#5	Device 1	373.70'	48.0" x 48.0" Horiz. Grate C= 0.600
			Limited to weir flow at low heads
#6	Secondary	373.80'	10.0' long x 27.0' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60
			Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63

Primary OutFlow Max=0.26 cfs @ 20.93 hrs HW=369.49' TW=349.93' (Dynamic Tailwater)
1=Culvert (Passes 0.26 cfs of 11.62 cfs potential flow)

—2=Orifice (Orifice Controls 0.25 cfs @ 7.44 fps)

-3=Orifice (Orifice Controls 0.00 cfs @ 0.71 fps)

4=Orifice (Controls 0.00 cfs)

-5=Grate (Controls 0.00 cfs)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=367.00' TW=348.50' (Dynamic Tailwater) 6=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Link A: BARRETTS HILL CROSSING

Inflow Area = 41.139 ac, 8.14% Impervious, Inflow Depth > 0.41" for 2-YEAR event

Inflow = 5.83 cfs @ 12.62 hrs, Volume= 1.39 af

Primary = 5.83 cfs @ 12.62 hrs, Volume= 1.39 af, Atten= 0%, Lag= 0 0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

2304141-POST DEVELOPMENT

Type III 24-hr 2-YEAR Rainfall=2.95"

Prepared by Keach-Nordstrom Associates, Inc.

Printed 12/6/2023

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 29

Summary for Link B: LOT 57

Inflow Area = 2.097 ac, 14.39% Impervious, Inflow Depth > 0.96" for 2-YEAR event

Inflow = 1.52 cfs @ 12.29 hrs, Volume= 0.17 af

Primary = 1.52 cfs @ 12.29 hrs, Volume= 0.17 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Summary for Link C: LOT 11

Inflow Area = 2.594 ac, 9.49% Impervious, Inflow Depth > 0.85" for 2-YEAR event

Inflow = 1.57 cfs @ 12.32 hrs, Volume= 0.18 af

Primary = 1.57 cfs @ 12.32 hrs, Volume= 0.18 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

2304141-POST DEVELOPMENT

Type III 24-hr 10-YEAR Rainfall=4.47" Printed 12/6/2023

Prepared by Keach-Nordstrom Associates, Inc.

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 30

Time span=0.00-24.00 hrs, dt=0.01 hrs, 2401 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment 1S: SUBCATCHMENT	Runoff Area=961,091 sf 4.08% Impervious Runoff Depth>1.27" Flow Length=2,203' Tc=38.1 min CN=64.6 Runoff=15.46 cfs 2.34 af
Subcatchment 2S: SUBCATCHMENT	Runoff Area=113,081 sf 12.76% Impervious Runoff Depth>1.08" Flow Length=683' Tc=20.1 min CN=61.4 Runoff=1.94 cfs 0.23 af
Subcatchment 3S: SUBCATCHMENT	Runoff Area=124,041 sf 7.14% Impervious Runoff Depth>1.09" Flow Length=1,189' Tc=23.1 min CN=61.5 Runoff=2.03 cfs 0.26 af
Subcatchment 4S: SUBCATCHMENT	Runoff Area=86,429 sf 21.84% Impervious Runoff Depth>1.69" Tc=6.0 min CN=70.6 Runoff=3.83 cfs 0.28 af
Subcatchment 5S: SUBCATCHMENT	Runoff Area=16,441 sf 34.30% Impervious Runoff Depth>1.92" Flow Length=389' Tc=8.7 min CN=73.7 Runoff=0.77 cfs 0.06 af
Subcatchment 6S: SUBCATCHMENT	Runoff Area=91,362 sf 14.39% Impervious Runoff Depth>2.07" Flow Length=692' Tc=19.5 min CN=75.7 Runoff=3.45 cfs 0.36 af
Subcatchment 7S: SUBCATCHMENT	Runoff Area=112,988 sf 9.49% Impervious Runoff Depth>1.90" Flow Length=1,021' Tc=20.7 min CN=73.5 Runoff=3.80 cfs 0.41 af
Subcatchment 8S: SUBCATCHMENT	Runoff Area=30,577 sf 34.65% Impervious Runoff Depth>2.48" Flow Length=719' Tc=20.3 min CN=80.6 Runoff=1.38 cfs 0.14 af
Subcatchment 9S: SUBCATCHMENT	Runoff Area=67,824 sf 18.27% Impervious Runoff Depth>2.12" Flow Length=1,293' Tc=26.9 min CN=76.4 Runoff=2.31 cfs 0.28 af
Subcatchment 10S: SUBCATCHMENT	Runoff Area=68,434 sf 8.00% Impervious Runoff Depth>1.86" Flow Length=676' Tc=23.0 min CN=73.0 Runoff=2.16 cfs 0.24 af
Subcatchment 11S: SUBCATCHMENT	Runoff Area=66,396 sf 2.68% Impervious Runoff Depth>1.74" Flow Length=690' Tc=20.7 min CN=71.4 Runoff=2.03 cfs 0.22 af
Subcatchment 12S: SUBCATCHMENT	Runoff Area=59,686 sf 10.36% Impervious Runoff Depth>1.93" Flow Length=753' Tc=18.7 min CN=73.9 Runoff=2.13 cfs 0.22 af
Subcatchment 13S: SUBCATCHMENT	Runoff Area=54,469 sf 6.63% Impervious Runoff Depth>1.84" Flow Length=680' Tc=16.3 min CN=72.6 Runoff=1.94 cfs 0.19 af
Subcatchment 14S: SUBCATCHMENT	Runoff Area=38,021 sf 9.74% Impervious Runoff Depth>1.92" Flow Length=731' Tc=17.2 min CN=73.7 Runoff=1.39 cfs 0.14 af
Subcatchment 15S: SUBCATCHMENT	Runoff Area=14,603 sf 40.57% Impervious Runoff Depth>2.76" Tc=6.0 min CN=83.7 Runoff=1.08 cfs 0.08 af
Subcatchment 16S: SUBCATCHMENT	Runoff Area=7,988 sf 70.27% Impervious Runoff Depth>3.45" Tc=6.0 min CN=90.9 Runoff=0.72 cfs 0.05 af

2304141-POST DEVELOI Prepared by Keach-Nordsti HydroCAD® 10.00-26 s/n 0327	
Subcatchment 17S: SUBCAT	Runoff Area=82,926 sf 4.39% Impervious Runoff Depth>1.96" Flow Length=230' Tc=13.2 min CN=74.2 Runoff=3.44 cfs 0.31 af
Reach 1R: EXISTING ROADV	VAY DITCH Avg. Flow Depth=0.62' Max Vel=3.92 fps Inflow=4.56 cfs 0.34 af n=0.022 L=230.0' S=0.0173 '/' Capacity=16.21 cfs Outflow=4.51 cfs 0.34 af
Reach 2R: EXISTING ROADV	VAY DITCH Avg. Flow Depth=0.30' Max Vel=1.68 fps Inflow=0.71 cfs 0.66 af n=0.069 L=915.0' S=0.0621 '/' Capacity=12.04 cfs Outflow=0.71 cfs 0.65 af
Pond 1P: EXISTING CB	Inflow=15.46 cfs 2.34 af Primary=15.46 cfs 2.34 af
Pond 2P: EXISTING DRIVEW	YAY CULVERT Peak Elev=293.20' Inflow=1.94 cfs 0.23 af 12.0" Round Culvert n=0.013 L=21.0' S=0.0143 '/' Outflow=1.94 cfs 0.23 af
Pond 3P: EXISTING DROP IN	Peak Elev=291.40' Inflow=19.97 cfs 3.82 af 24.0" Round Culvert n=0.011 L=92.1' S=0.0433 '/' Outflow=19.97 cfs 3.82 af
Pond 4P: EXISTING CB	Peak Elev=300.20' Inflow=4.56 cfs 0.34 af Primary=4.43 cfs 0.34 af Secondary=0.13 cfs 0.00 af Outflow=4.56 cfs 0.34 af
Pond 5P: EXISTING CB	Peak Elev=300.28' Inflow=0.77 cfs 0.06 af Primary=0.77 cfs 0.06 af Secondary=0.00 cfs 0.00 af Outflow=0.77 cfs 0.06 af
Pond 6P: EXISTING CB	Peak Elev=365.03' Inflow=3.45 cfs 0.36 af 12.0" Round Culvert n=0.025 L=18.0' S=0.0689 '/' Outflow=3.45 cfs 0.36 af
Pond 7P: EXISTING CB	Peak Elev=378.09' Inflow=3.80 cfs 0.41 af 12.0" Round Culvert n=0.011 L=31.9' S=0.0091 '/' Outflow=3.80 cfs 0.41 af
Pond 8P: DMH#2	Peak Elev=350.09' Inflow=0.71 cfs 0.66 af 15.0" Round Culvert n=0.013 L=15.0' S=0.0800 '/' Outflow=0.71 cfs 0.66 af
Pond 9P: DMH#3	Peak Elev=376.15' Inflow=8.26 cfs 0.90 af 18.0" Round Culvert n=0.013 L=30.0' S=0.0073 '/' Outflow=8.26 cfs 0.90 af
Pond 10P: DMH#4	Peak Elev=377.38' Inflow=8.26 cfs 0.90 af 18.0" Round Culvert n=0.013 L=121.7' S=0.0100 '/' Outflow=8.26 cfs 0.90 af
Pond 11P: DMH#5	Peak Elev=379.63' Inflow=8.26 cfs 0.90 af 18.0" Round Culvert n=0.013 L=226.1' S=0.0100 '/' Outflow=8.26 cfs 0.90 af
Pond 12P: CB#6	Peak Elev=381.14' Inflow=8.26 cfs 0.90 af 18.0" Round Culvert n=0.013 L=141.2' S=0.0100 '/' Outflow=8.26 cfs 0.90 af
Pond 13P: CB#7	Peak Elev=382.86' Inflow=7.73 cfs 0.83 af 15.0" Round Culvert ri=0.013 L=30.3' S=0.0099 '/' Outflow=7.73 cfs 0.83 af
Pond 14P: CB#8	Peak Elev=386.27' Inflow=6.35 cfs 0.69 af 15.0" Round Culvert n=0.013 L=165.7' S=0.0250 '/' Outflow=6.35 cfs 0.69 af

Inflow=3.80 cfs 0.41 af Primary=3.80 cfs 0.41 af

2304141-POST DEVELO Prepared by Keach-Nords HydroCAD® 10.00-26 s/n 032		Type III 24-hr 10-YEAR R Printe	ainfall=4.47" ed 12/6/2023 Page 32
Pond 16P: CB#10	12.0" Round Culvert n=0.013 L	Peak Elev=393.99' Inflow=4. -=165.5' S=0.0500'/' Outflow=4.	45 cfs 0.49 af 45 cfs 0.49 af
Pond 17P: CB#100	12.0" Round Culvert n=0.013	Peak Elev=394.01' Inflow=0.1 L=26.0' S=0.0200 '/' Outflow=0.1	72 cfs 0.05 af 72 cfs 0.05 af
Pond 18P: CB#11	12.0" Round Culvert n=0.013 L	Peak Elev=403.59' Inflow=2.0 =167.8' S=0.0600 '/' Outflow=2.0	03 cfs 0.22 af 03 cfs 0.22 af
Pond 20P: DMH#14	18.0" Round Culvert n=0.013	Peak Elev=376.41' Inflow=5.7 L=15.0' S=0.0800'/' Outflow=5.7	74 cfs 0.66 af 74 cfs 0.66 af
Pond 21P: DMH#15	18.0" Round Culvert n=0.013	Peak Elev=382.67' Inflow=5.7 L=30.7' S=0.0801 '/' Outflow=5.7	74 cfs 0.66 af 74 cfs 0.66 af
Pond 22P: DMH#16	18.0" Round Culvert n=0.013 L	Peak Elev=394.13' Inflow=5.7 =111.5' S=0.0800'/' Outflow=5.7	'4 cfs 0.66 af '4 cfs 0.66 af
Pond 23P: DMH#17	18.0" Round Culvert n=0.013	Peak Elev=402.34' Inflow=5.7 L=45.0' S=0.0800 '/' Outflow=5.7	'4 cfs 0.66 af '4 cfs 0.66 af
Pond 24P: CB#18	18.0" Round Culvert n=0.013 L	Peak Elev=404.08' Inflow=5.7 =164.2' S=0.0100 '/' Outflow=5.7	74 cfs 0.66 af 74 cfs 0.66 af
Pond 25P: CB#19	15.0" Round Culvert n=0.013	Peak Elev=404.55' Inflow=3.5 L=36.5' S=0.0101 '/' Outflow=3.5	8 cfs 0.42 af 8 cfs 0.42 af
Pond 26P: CB#20	12.0" Round Culvert n=0.013	Peak Elev=404.70' Inflow=1.3 L=26.0' S=0.0100 '/' Outflow=1.3	8 cfs 0.14 af 8 cfs 0.14 af
Pond PP: POCKET POND	Peak Elev=371.43 Primary=0.71 cfs 0.66 af Seconda	3' Storage=64,996 cf Inflow=16.6 ary=0.00 cfs 0.00 af Outflow=0.7	8 cfs 1.88 af 1 cfs 0.66 af
Link A: BARRETTS HILL CR	ROSSING	Inflow=19.9 Primary=19.9	7 cfs 3.82 af 7 cfs 3.82 af
Link B: LOT 57			5 cfs 0.36 af 5 cfs 0.36 af
Link C: LOT 11		Inflow=3.8	0 cfs 0.41 af

Total Runoff Area = 45.830 ac Runoff Volume = 5.82 af Average Runoff Depth = 1.52" 91.49% Pervious = 41.932 ac 8.51% Impervious = 3.898 ac

Type III 24-hr 10-YEAR Rainfall=4.47"

Prepared by Keach-Nordstrom Associates, Inc.

Printed 12/6/2023

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 33

Summary for Subcatchment 1S: SUBCATCHMENT

Runoff = 15.46

15.46 cfs @ 12.58 hrs, Volume=

2.34 af, Depth> 1.27"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 10-YEAR Rainfall=4.47"

	Α	rea (sf)	CN	Description	n			
		7,670	30.0	Woods, G	ood, HSG	A		
	3	63,085	55.0	Woods, G	ood, HSG	В		
	4	27,890	70.0	Woods, G	Good, HSG	C		
		6,684	39.0	>75% Gra	ass cover, (Good, HSG A		
		61,618	61.0	>75% Gra	ass cover, (Good, HSG B		
		54,927	74.0	>75% Gra	ass cover, (Good, HSG C		
*		21,604	98.0	Roofs				
*		17,613	98.0	Paved pa	rking			
	9	61,091	64.6	Weighted	Average			
	921,874 63.2			95.92% Pervious Area				
		39,217	98.0	4.08% lm	pervious Ai	rea		
	Tc	Length	Slope	Velocity	Capacity	Description		
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
	17.3	100	0.0400	0.10		Sheet Flow,		
						Woods: Light underbrush n= 0.400 P2= 2.84"		
	19.7	1,862	0.0988	1.57		Shallow Concentrated Flow,		
						Woodland Kv= 5.0 fps		
	0.3	30	0.0800	1.98		Shallow Concentrated Flow,		
						Short Grass Pasture Kv= 7.0 fps		
	8.0	211	0.0430	4.21		Shallow Concentrated Flow,		
						Paved Kv= 20.3 fps		
	38.1	2,203	Total					

Summary for Subcatchment 2S: SUBCATCHMENT

Runoff =

1.94 cfs @ 12.32 hrs, Volume=

0.23 af, Depth> 1.08"

	Area (sf)	CN	Description
	28,414	30.0	Woods, Good, HSG A
	11,383	55.0	Woods, Good, HSG B
	33,808	70.0	Woods, Good, HSG C
	4,719	39.0	>75% Grass cover, Good, HSG A
	242	61.0	>75% Grass cover, Good, HSG B
	20,087	74.0	>75% Grass cover, Good, HSG C
*	3,849	98.0	Roofs
*	10,579	98.0	Paved parking
-	113,081	61.4	Weighted Average
	98,653	56.1	87.24% Pervious Area
	14,428	98.0	12.76% Impervious Area

2304141-POST DEVELOPMENT

Type III 24-hr 10-YEAR Rainfall=4.47"

Prepared by Keach-Nordstrom Associates, Inc.

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Printed 12/6/2023

Page 34

Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
12.0	100	0.1000	0.14		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 2.84"
3.9	318	0.0750	1.37		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
0.7	48	0.0300	1.21		Shallow Concentrated Flow,
					Short Grass Pasture Kv= 7.0 fps
3.6	217	0.0410	1.01		Shallow Concentrated Flow,
-					Woodland Kv= 5.0 fps
20.1	683	Total			

Summary for Subcatchment 3S: SUBCATCHMENT

Runoff = 2.03 cfs @ 12.37 hrs, Volume=

0.26 af, Depth> 1.09"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 10-YEAR Rainfall=4.47"

	rea (sf)	CN	Description	on					
	5,732	30.0	Woods, G	Voods, Good, HSG A					
	49,946	55.0	Woods, Good, HSG B						
	30,651	70.0	Woods, G	Good, HSG	C				
	4,799	39.0	>75% Gra	ass cover, (Good, HSG A				
20,628 61.0 >75% Grass cover, Good, HSG B					Good, HSG B				
3,428 74.0 >75% Grass cover, Good, HSG C				Good, HSG C					
*	2,833	98.0	Roofs						
* 6,024 98.0 Paved parking				rking					
124,041 61.5			Weighted Average						
•	115,184 58.7			ervious Are	e a				
	8,857	98.0	7.14% lm	pervious Ai	rea				
Tc	Length	Slope	Velocity	Capacity	Description				
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
9.9	100	0.1600	0.17		Sheet Flow,				
					Woods: Light underbrush n= 0.400 P2= 2.84"				
13.2	1,089	0.0760	1.38		Shallow Concentrated Flow,				
					Woodland Kv= 5.0 fps				
23.1	1,189	Total							

Summary for Subcatchment 4S: SUBCATCHMENT

Runoff = 3.83 cfs @ 12.09 hrs, Volume=

0.28 af, Depth> 1.69"

Type III 24-hr 10-YEAR Rainfall=4.47"

Prepared by Keach-Nordstrom Associates, Inc.

Printed 12/6/2023

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 35

	Area (sf)	CN	Description					
	10,510	55.0	Woods, Good, HSG B					
	6,756	70.0	Woods, Good, HSG C					
	40,259	61.0	>75% Grass cover, Good, HSG B					
	10,029	74.0	>75% Grass cover, Good, HSG C					
	16,327	98.0	Paved parking, HSG C					
*	2,548	98.0	Roofs					
1-	86,429	70.6	Weighted Average					
	67,554	62.9	78.16% Pervious Area					
	18,875	98.0	21.84% Impervious Area					
Tc (min)	10.	Slope (ft/ft)	Velocity Capacity Description (ft/sec) (cfs)					
6.0			Direct Entry,					

Summary for Subcatchment 5S: SUBCATCHMENT

Runoff = 0.77 cfs @ 12.13 hrs, Volume=

Depositation

CNI

A--- /-5

0.06 af, Depth> 1.92"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 10-YEAR Rainfall=4.47"

	Area (sf) CN Description										
	10,80)1	61.0	>75% Gra	75% Grass cover, Good, HSG B						
*	5,64	10	98.0	Paved par	aved parking						
	16,44	11	73.7	Weighted	Average						
	10,80)1	61.0	65.70% P	ervious Are	ea e e e e e e e e e e e e e e e e e e					
	5,64	10	98.0	34.30% lr	npervious /	Area					
- (mi	Γc Leng n) (fe	gth et)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description					
6	.3 1	00	0.0700	0.26		Sheet Flow,					
						Grass: Short n= 0.150 P2= 2.84"					
0	.1	16	0.0400	4.06		Shallow Concentrated Flow,					
						Paved Kv= 20.3 fps					
1	.5 1	62	0.0680	1.83		Shallow Concentrated Flow,					
_	_					Short Grass Pasture Kv= 7.0 fps					
0	.1	20	0.0400	4.06		Shallow Concentrated Flow,					
		0.4	0.0770	4.04		Paved Kv= 20.3 fps					
0	.8	91	0.0770	1.94		Shallow Concentrated Flow,					
						Short Grass Pasture Kv= 7.0 fps					
8	.7 3	889	Total								

Summary for Subcatchment 6S: SUBCATCHMENT

Runoff = 3.45 cfs @ 12.27 hrs, Volume=

0.36 af, Depth> 2.07"

2304141-POST DEVELOPMENT

Type III 24-hr 10-YEAR Rainfall=4.47"

Prepared by Keach-Nordstrom Associates, Inc.

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Printed 12/6/2023

Page 36

_	Α	rea (sf)	CN	Description	n	
		42,274	C			
42,274 70.0 Woods, Good, HSG C 35,721 74.0 >75% Grass cover, Good, HSG C						
*		8,350	98.0	Paved pa		,
*		4,800	98.0	Roofs		
		217	96.0	Gravel su	rface, HSG	S C
		91,362	75.7	Weighted	Average	
		78,212	71.9		ervious Are	ea
		13,150	98.0	14.39% Ir	npervious A	Area
					•	
	Tc	Length	Slope	Velocity	Capacity	Description
(n	nin)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
1	5.8	100	0.0500	0.11		Sheet Flow,
						Woods: Light underbrush n= 0.400 P2= 2.84"
	1.0	80	0.0750	1.37		Shallow Concentrated Flow,
						Woodland Kv= 5.0 fps
	0.2	18	0.0400	1.40		Shallow Concentrated Flow,
						Short Grass Pasture Kv= 7.0 fps
	0.2	12	0.0400	1.00		Shallow Concentrated Flow,
						Woodland Kv= 5.0 fps
	0.1	15	0.3000	3.83		Shallow Concentrated Flow,
						Short Grass Pasture Kv= 7.0 fps
	2.2	467	0.0300	3.52		Shallow Concentrated Flow,
						Paved Kv= 20.3 fps
1	9.5	692	Total			

Summary for Subcatchment 7S: SUBCATCHMENT

Runoff = 3.80 cfs @ 12.29 hrs, Volume=

0.41 af, Depth> 1.90"

	Area (sf)	CN	Description			
	83,250	70.0	Woods, Good, HSG C			
	18,246	74.0	>75% Grass cover, Good, HSG C			
*	5,928	98.0	Paved parking			
	764	96.0	Gravel surface, HSG C			
*	4,800	98.0	Roofs			
	112,988	73.5	Weighted Average			
	102,260	70.9	90.51% Pervious Area			
	10,728	98.0	9.49% Impervious Area			

2304141-POST DEVELOPMENT

Type III 24-hr 10-YEAR Rainfall=4.47"

Prepared by Keach-Nordstrom Associates, Inc.

Printed 12/6/2023

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 37

Tc.	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
12.5	100	0.0900	0.13		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 2.84"
7.5	755	0.1130	1.68		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
0.0	6	0.3000	3.83		Shallow Concentrated Flow,
					Short Grass Pasture Kv= 7.0 fps
0.7	160	0.0375	3.93		Shallow Concentrated Flow,
					Paved Kv= 20.3 fps
20.7	1,021	Total			

Summary for Subcatchment 8S: SUBCATCHMENT

Runoff = 1.38 cfs @ 12.28 hrs, Volume=

0.14 af, Depth> 2.48"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 10-YEAR Rainfall=4.47"

Area (sf) CN Description									
		13,046	70.0	Woods, Good, HSG C					
		6,935	74.0	>75% Gra	Good, HSG C				
* 10,596 98.0 Paved parking									
		30,577	80.6	Weighted	Average				
		19,981	71.4		ervious Are	ea			
		10,596	98.0	34.65% In	npervious /	Area			
		,							
	Tc	Length	Slope	Velocity	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
	15.8	100	0.0500	0.11		Sheet Flow,			
						Woods: Light underbrush n= 0.400 P2= 2.84"			
	2.9	329	0.1480	1.92		Shallow Concentrated Flow,			
						Woodland Kv= 5.0 fps			
	0.1	33	0.3330	4.04		Shallow Concentrated Flow,			
						Short Grass Pasture Kv= 7.0 fps			
	1.5	257	0.0200	2.87		Shallow Concentrated Flow,			
						Paved Kv= 20.3 fps			
8	20.3	719	Total						

Summary for Subcatchment 9S: SUBCATCHMENT

Runoff = 2.31 cfs @ 12.38 hrs, Volume=

0.28 af, Depth> 2.12"

2304141-POST DEVELOPMENT

Type III 24-hr 10-YEAR Rainfall=4.47"

Prepared by Keach-Nordstrom Associates, Inc.

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Printed 12/6/2023

Page 38

9	<i>P</i>	rea (sf)	CN	Description	Description						
		34,147	70.0	Woods, Good, HSG C							
		21,287	74.0			Good, HSG C					
	*	7,590	98.0		Paved parking						
	*	4,800	98.0	Roofs							
		67,824	76.4	Weighted	Average						
		55,434	71.5		ervious Are	ea					
	12,390 98.0 18.27% Impervious A					Area					
					,						
	Tc	Length	Slope	Velocity	Capacity	Description					
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)						
	19.4	100	0.0300	0.09		Sheet Flow,					
						Woods: Light underbrush n= 0.400 P2= 2.84"					
	2.6	350	0.1940	2.20		Shallow Concentrated Flow,					
						Woodland Kv= 5.0 fps					
	0.2	43	0.3330	4.04		Shallow Concentrated Flow,					
						Short Grass Pasture Kv= 7.0 fps					
	4.6	800	0.0200	2.87		Shallow Concentrated Flow,					
						Paved Kv= 20.3 fps					
	26.9	1,293	Total			•					

Summary for Subcatchment 10S: SUBCATCHMENT

Runoff = 2.16 cfs @ 12.34 hrs, Volume=

0.24 af, Depth> 1.86"

		rea (sf)	CN	Description	on						
		49,642	70.0	Woods, G	C						
		13,315	74.0			Good, HSG C					
*		3,077	98.0		Paved parking						
*		2,400	98.0	Roofs							
		68,434	73.0	Weighted	eighted Average						
		62,957	70.8		ervious Are	ea					
		5,477	98.0	8.00% Im	pervious Ai	rea					
	Тс	Length	Slope	Velocity	Capacity	Description					
-	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)						
	17.3	100	0.0400	0.10		Sheet Flow,					
						Woods: Light underbrush n= 0.400 P2= 2.84"					
	4.0	479	0.1610	2.01		Shallow Concentrated Flow,					
						Woodland Kv= 5.0 fps					
	1.5	65	0.0100	0.70		Shallow Concentrated Flow,					
						Short Grass Pasture Kv= 7.0 fps					
	0.2	32	0.0200	2.87		Shallow Concentrated Flow,					
						Paved Kv= 20.3 fps					
	23.0	676	Total								

2304141-POST DEVELOPMENT

Type III 24-hr 10-YEAR Rainfall=4.47"

Prepared by Keach-Nordstrom Associates, Inc. HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC Printed 12/6/2023

Page 39

Summary for Subcatchment 11S: SUBCATCHMENT

Runoff = 2.03 cfs @ 12.30 hrs, Volume=

0.22 af, Depth> 1.74"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 10-YEAR Rainfall=4.47"

	Area (sf)	CN	Description	n							
	54,267	70.0	Woods, G	Voods, Good, HSG C							
	10,350	74.0	>75% Gra	75% Grass cover, Good, HSG C							
*	1,779	98.0	Paved pa	aved parking							
	66,396	71.4	Weighted	Veighted Average							
	64,617	70.6	97.32% P	ervious Are	ea						
	1,779	98.0	2.68% Im	pervious Ai	rea						
To	Length	Slope	Velocity	Capacity	Description						
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)							
15.8	100	0.0500	0.11		Sheet Flow,						
					Woods: Light underbrush n= 0.400 P2= 2.84"						
4.2	485	0.1510	1.94		Shallow Concentrated Flow,						
					Woodland Kv= 5.0 fps						
0.6	73	0.0800	1.98		Shallow Concentrated Flow,						
					Short Grass Pasture Kv= 7.0 fps						
0.1	32	0.0600	4.97		Shallow Concentrated Flow,						
					Paved Kv= 20.3 fps						
20.7	690	Total									

Summary for Subcatchment 12S: SUBCATCHMENT

Runoff = 2.13 cfs @ 12.27 hrs, Volume=

0.22 af, Depth> 1.93"

	Area (sf)	CN	Description
	38,779	70.0	Woods, Good, HSG C
	14,722	74.0	>75% Grass cover, Good, HSG C
*	3,785	98.0	Paved parking
*	2,400	98.0	Roofs
	59,686	73.9	Weighted Average
	53,501	71.1	89.64% Pervious Area
	6,185	98.0	10.36% Impervious Area

2304141-POST DEVELOPMENT

Type III 24-hr 10-YEAR Rainfall=4.47"

Prepared by Keach-Nordstrom Associates, Inc.

Printed 12/6/2023

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 40

_	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	13.8	100	0.0700	0.12		Sheet Flow,
	4.0	462	0.1470	1.92		Woods: Light underbrush n= 0.400 P2= 2.84" Shallow Concentrated Flow, Woodland Kv= 5.0 fps
	0.4	55	0.1100	2.32		Shallow Concentrated Flow,
	0.5	136	0.0600	4.97		Short Grass Pasture Kv= 7.0 fps Shallow Concentrated Flow, Paved Kv= 20.3 fps
-	18.7	753	Total			•

Summary for Subcatchment 13S: SUBCATCHMENT

Runoff = 1.94 cfs @ 12.23 hrs, Volume=

0.19 af, Depth> 1.84"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 10-YEAR Rainfall=4.47"

	A	rea (sf)	CN	Description	n							
		40,261	70.0	Woods, G	Woods, Good, HSG C							
		10,598	74.0	>75% Gra	>75% Grass cover, Good, HSG C							
*		3,610	98.0	Paved pa	Paved parking							
		54,469 72.6 Weighted Average										
		50,859	70.8	93.37% P	ervious Are	ea						
		3,610	98.0	6.63% lm	pervious A	rea						
	Tc	Length	Slope	Velocity	Capacity	Description						
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)							
	12.0	100	0.1000	0.14		Sheet Flow,						
						Woods: Light underbrush n= 0.400 P2= 2.84"						
	3.8	425	0.1410	1.88		Shallow Concentrated Flow,						
						Woodland Kv= 5.0 fps						
	0.3	61	0.3300	4.02		Shallow Concentrated Flow,						
	0.0	0.4				Short Grass Pasture Kv= 7.0 fps						
	0.3	94	0.0600	4.97		Shallow Concentrated Flow,						
						Paved Kv= 20.3 fps						
	16.3	680	Total									

Summary for Subcatchment 14S: SUBCATCHMENT

Runoff = 1.39 cfs @ 12.24 hrs, Volume=

0.14 af, Depth> 1.92"

2304141-POST DEVELOPMENT

Type III 24-hr 10-YEAR Rainfall=4.47"

Prepared by Keach-Nordstrom Associates, Inc.

Printed 12/6/2023

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 41

A	Area (sf)	CN	Description	n							
	25,189	70.0	Woods, G	Voods, Good, HSG C							
	9,128	74.0	>75% Gra	75% Grass cover, Good, HSG C							
*	3,704	98.0	Paved pa	aved parking							
\ <u></u>	38,021	73.7	Weighted	Average							
	34,317	71.1	90.26% P	ervious Are	e a						
	3,704	98.0	9.74% lm	pervious A	rea						
Tc	Length	Slope	Velocity	Capacity	Description						
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)							
11.1	100	0.1200	0.15		Sheet Flow,						
					Woods: Light underbrush n= 0.400 P2= 2.84"						
5.5	529	0.1020	1.60		Shallow Concentrated Flow,						
					Woodland Kv= 5.0 fps						
0.2	46	0.3300	4.02	·							
					Short Grass Pasture Kv= 7.0 fps						
0.3	56	0.0200	2.87		Shallow Concentrated Flow,						
					Paved Kv= 20.3 fps						
17.2	731	Total									

Summary for Subcatchment 15S: SUBCATCHMENT

Runoff = 1.08 cfs @ 12.09 hrs, Volume=

0.08 af, Depth> 2.76"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 10-YEAR Rainfall=4.47"

	Area (sf)	CN	Description	Description					
	8,678	74.0	>75% Gra	75% Grass cover, Good, HSG C					
*	5,925	98.0	Paved pa	Paved parking					
	14,603	83.7	Weighted	Weighted Average					
	8,678	74.0	59.43% P	59.43% Pervious Area					
	5,925	98.0	40.57% lr	40.57% Impervious Area					
	Tc Length	Slope	Velocity	Capacity	Description				
(m	in) (feet)	(ft/ft)	(ft/sec)	(cfs)					
6	3.0				Direct Entry				

Summary for Subcatchment 16S: SUBCATCHMENT

Runoff = 0.72 cfs @ 12.09 hrs, Volume= 0.05 af, Depth> 3.45"

2304141-POST DEVELOPMENT

Type III 24-hr 10-YEAR Rainfall=4.47"

Prepared by Keach-Nordstrom Associates, Inc.

Printed 12/6/2023

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 42

	Area	(sf)	CN	Description						
	2,3	375	74.0	>75% Gra	>75% Grass cover, Good, HSG C					
*	5,6	313	98.0	Paved pa	Paved parking					
	7,9	88	90.9	Weighted	Weighted Average					
	2,3	375	74.0	29.73% Pervious Area						
	5,6	313	98.0	70.27% Impervious Area						
	To Lou	o orth	Clana	Malaaltu	0	December				
/		ngth	Slope	Velocity	Capacity	Description				
		eet)	(ft/ft)	(ft/sec)	(cfs)					
1	6.0					Direct Entry,				

Summary for Subcatchment 17S: SUBCATCHMENT

Runoff = 3.44 cfs @ 12.18 hrs, Volume=

0.31 af, Depth> 1.96"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 10-YEAR Rainfall=4.47"

		Area (sf)	CN	Description	n							
		26,296	70.0	Woods, C	Woods, Good, HSG C							
		51,457	74.0			Good, HSG C						
*		1,238	98.0	Paved pa		•						
*		2,400	98.0	Roofs	•							
		1,535	96.0	Gravel su	rface, HSG	G C						
		82,926	74.2	Weighted	Average							
		79,288	73.1	95.61% P	ervious Are	ea						
		3,638	98.0	4.39% Im	4.39% Impervious Area							
	Tc		Slope	Velocity	Capacity	Description						
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)							
	12.0	100	0.1000	0.14		Sheet Flow,						
						Woods: Light underbrush n= 0.400 P2= 2.84"						
	1.0	90	0.0860	1.47		Shallow Concentrated Flow,						
						Woodland Kv= 5.0 fps						
	0.2	40	0.3300	4.02		Shallow Concentrated Flow,						
						Short Grass Pasture Kv= 7.0 fps						
	13.2	230	Total									

Summary for Reach 1R: EXISTING ROADWAY DITCH

Inflow Area = 2.362 ac, 23.83% Impervious, Inflow Depth > 1.73" for 10-YEAR event

Inflow = 4.56 cfs @ 12.10 hrs, Volume= 0.34 af

Outflow = 4.51 cfs @ 12.11 hrs, Volume= 0.34 af, Atten= 1%, Lag= 0.8 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Max. Velocity= 3.92 fps, Min. Travel Time= 1.0 min Avg. Velocity = 1.70 fps, Avg. Travel Time= 2.2 min

Type III 24-hr 10-YEAR Rainfall=4.47"

Prepared by Keach-Nordstrom Associates, Inc.

Printed 12/6/2023

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 43

Peak Storage= 264 cf @ 12.11 hrs Average Depth at Peak Storage= 0.62' Bank-Full Depth= 1.00' Flow Area= 3.0 sf, Capacity= 16.21 cfs

0.00' x 1.00' deep channel, n= 0.022 Earth, clean & straight Side Slope Z-value= 3.0 '/' Top Width= 6.00' Length= 230.0' Slope= 0.0173 '/' Inlet Invert= 295.64', Outlet Invert= 291.66'



Summary for Reach 2R: EXISTING ROADWAY DITCH

Inflow Area = 11.270 ac, 12.00% Impervious, Inflow Depth > 0.71" for 10-YEAR event

Inflow = 0.71 cfs @ 17.78 hrs, Volume= 0.66 af

Outflow = 0.71 cfs @ 17.89 hrs, Volume= 0.65 af, Atten= 0%, Lag= 6.7 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Max. Velocity= 1.68 fps, Min. Travel Time= 9.1 min Avg. Velocity = 1.34 fps, Avg. Travel Time= 11.4 min

Peak Storage= 385 cf @ 17.89 hrs Average Depth at Peak Storage= 0.30' Bank-Full Depth= 1.00' Flow Area= 3.5 sf, Capacity= 12.04 cfs

0.50' x 1.00' deep channel, n= 0.069 Riprap, 6-inch Side Slope Z-value= 3.0 '/' Top Width= 6.50' Length= 915.0' Slope= 0.0621 '/' Inlet Invert= 348.50', Outlet Invert= 291.66'



Summary for Pond 1P: EXISTING CB

Inflow Area = 22.064 ac. 4.08% Impervious, Inflow Depth > 1.27" for 10-YEAR event

Inflow = 15.46 cfs @ 12.58 hrs, Volume= 2.34 af

Primary = 15.46 cfs @ 12.58 hrs, Volume= 2.34 af, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Type III 24-hr 10-YEAR Rainfall=4.47"

Prepared by Keach-Nordstrom Associates, Inc.

Printed 12/6/2023

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 44

Summary for Pond 2P: EXISTING DRIVEWAY CULVERT

Inflow Area = 2.596 ac, 12.76% Impervious, Inflow Depth > 1.08" for 10-YEAR event

Inflow = 1.94 cfs @ 12.32 hrs, Volume= 0.23 af

Outflow = 1.94 cfs @ 12.32 hrs, Volume= 0.23 af, Atten= 0%, Lag= 0.0 min

Primary = 1.94 cfs @ 12.32 hrs. Volume= 0.23 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 293.20' @ 12.32 hrs

Flood Elev= 294.00'

Primary OutFlow Max=1.94 cfs @ 12.32 hrs HW=293.20' TW=290.85' (Dynamic Tailwater) 1=Culvert (Barrel Controls 1.94 cfs @ 3.94 fps)

Summary for Pond 3P: EXISTING DROP INLET

Inflow Area = 41.139 ac, 8.14% Impervious, Inflow Depth > 1.12" for 10-YEAR event

Inflow = 19.97 cfs @ 12.53 hrs, Volume= 3.82 af

Outflow = 19.97 cfs @ 12.53 hrs, Volume= 3.82 af, Atten= 0%, Lag= 0.0 min

Primary = 19.97 cfs @ 12.53 hrs. Volume= 3.82 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 291.40' @ 12.53 hrs

Flood Elev= 293.75'

Device	Routing	Invert	Outlet Devices			
#1	Primary	288.66'	24.0" Round Culvert			
			L= 92.1' RCP, square edge headwall, Ke= 0.500			
			Inlet / Outlet Invert= 288.66' / 284.67' S= 0.0433 '/' Cc= 0.900			
			n= 0.011 Concrete pipe, straight & clean, Flow Area= 3.14 sf			

Primary OutFlow Max=19.96 cfs @ 12.53 hrs HW=291.40' TW=0.00' (Dynamic Tailwater)

1=Culvert (Inlet Controls 19.96 cfs @ 6.35 fps)

Summary for Pond 4P: EXISTING CB

Inflow Area =	2.362 ac, 23.83% Impervious, Inflow	Depth > 1.73" for 10-YEAR event
Inflow =	4.56 cfs @ 12.10 hrs, Volume=	0.34 af
Outflow =	4.56 cfs @ 12.10 hrs, Volume=	0.34 af, Atten= 0%, Lag= 0.0 min
Primary =	4.43 cfs @ 12.10 hrs, Volume=	0.34 af
Secondary =	0.13 cfs @ 12.10 hrs, Volume=	0.00 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Type III 24-hr 10-YEAR Rainfall=4.47"

Prepared by Keach-Nordstrom Associates, Inc.
HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Printed 12/6/2023

Page 45

Peak Elev= 300.20' @ 12.10 hrs

Flood Elev= 300.17'

Device	Routing	Invert	Outlet Devices
#1	Primary	297.00'	12.0" Round Culvert
	•		L= 49.1' CMP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 297.00' / 295.64' S= 0.0277 '/' Cc= 0.900
			n= 0.025 Corrugated metal, Flow Area= 0.79 sf
#2	Secondary	300.17'	24.0" x 24.0" Horiz. Orifice/Grate C= 0.600
	•		Limited to weir flow at low heads

Primary OutFlow Max=4.43 cfs @ 12.10 hrs HW=300.20' TW=296.26' (Dynamic Tailwater)
1=Culvert (Barrel Controls 4.43 cfs @ 5.64 fps)

Secondary OutFlow Max=0.12 cfs @ 12.10 hrs HW=300.20' TW=296.26' (Dynamic Tailwater) —2=Orifice/Grate (Weir Controls 0.12 cfs @ 0.54 fps)

Summary for Pond 5P: EXISTING CB

Inflow Area =	0.377 ac, 34.	30% Impervious, Infl	ow Depth > 1.92"	for 10-YEAR event
Inflow =	0.77 cfs @ 12	2.13 hrs, Volume=	0.06 af	
Outflow =	0.77 cfs @ 12	2.13 hrs, Volume=	0.06 af, Atte	n= 0%, Lag= 0.0 min
Primary =	0.77 cfs @ 1	2.13 hrs, Volume=	0.06 af	
Secondary =	0.00 cfs @	0.00 hrs, Volume=	0.00 af	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 300.28' @ 12.11 hrs Flood Elev= 301.16'

Device	Routing	Invert	Outlet Devices
#1	Primary	297.70'	12.0" Round Culvert
	-		L= 34.1' CMP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 297.70' / 297.30' S= 0.0117 '/' Cc= 0.900
			n= 0.025 Corrugated metal, Flow Area= 0.79 sf
#2	Secondary	301.16'	24.0" x 24.0" Horiz. Orifice/Grate
	-		Limited to weir flow at low heads

Primary OutFlow Max=1.36 cfs @ 12.13 hrs HW=300.25' TW=300.00' (Dynamic Tailwater)
—1=Culvert (Outlet Controls 1.36 cfs @ 1.73 fps)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=297.70' TW=0.00' (Dynamic Tailwater) 2=Orifice/Grate (Controls 0.00 cfs)

Summary for Pond 6P: EXISTING CB

Inflow Area	a =	2.097 ac,	14.39% Impe	rvious, li	nflow Depth >	2.07" for	10-YEAR event
Inflow	=	3.45 cfs @	12.27 hrs, \	Volume=	0.36 af		
Outflow	=	3.45 cfs @	12.27 hrs, \	Volume=	0.36 af	f, Atten= 09	%, Lag= 0.0 min
Primary	=	3.45 cfs @	12.27 hrs, \	Volume=	0.36 af	:	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

2304141-POST DEVELOPMENT

Type III 24-hr 10-YEAR Rainfall=4.47"

Prepared by Keach-Nordstrom Associates, Inc.

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Printed 12/6/2023

Page 46

Peak Elev= 365.03' @ 12.27 hrs

Flood Elev= 366.35'

Device	Routing	Invert	Outlet Devices
#1	Primary	363.70'	12.0" Round Culvert
			L= 18.0' CMP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 363.70' / 362.46' S= 0.0689 '/' Cc= 0.900
			n= 0.025 Corrugated metal Flow Area= 0.79 sf

Primary OutFlow Max=3.45 cfs @ 12.27 hrs HW=365.03' TW=0.00' (Dynamic Tailwater) 1=Culvert (Inlet Controls 3.45 cfs @ 4.40 fps)

Summary for Pond 7P: EXISTING CB

Inflow Area = 2.594 ac, 9.49% Impervious, Inflow Depth > 1.90" for 10-YEAR event

Inflow = 3.80 cfs @ 12.29 hrs, Volume= 0.41 af

Outflow = 3.80 cfs @ 12.29 hrs, Volume= 0.41 af, Atten= 0%, Lag= 0.0 min

Primary = 3.80 cfs @ 12.29 hrs, Volume= 0.41 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 378.09' @ 12.29 hrs

Flood Elev= 380.49'

Device	Routing	Invert	Outlet Devices
#1	Primary	376.57'	12.0" Round Culvert
			L= 31.9' RCP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 376.57' / 376.28' S= 0.0091 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf

Primary OutFlow Max=3.80 cfs @ 12.29 hrs HW=378.09' TW=0.00' (Dynamic Tailwater)

1=Culvert (Barrel Controls 3.80 cfs @ 4.84 fps)

Summary for Pond 8P: DMH#2

Inflow Area = 11.270 ac, 12.00% Impervious, Inflow Depth > 0.71" for 10-YEAR event

Inflow = 0.71 cfs @ 17.78 hrs, Volume= 0.66 af

Outflow = 0.71 cfs @ 17.78 hrs, Volume= 0.66 af, Atten= 0%, Lag= 0.0 min

Primary = 0.71 cfs @ 17.78 hrs, Volume= 0.66 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 350.09' @ 17.78 hrs

Flood Elev= 353.05'

Device	Routing	Invert	Outlet Devices
#1	Primary	349.70'	15.0" Round Culvert
			L= 15.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 349.70' / 348.50' S= 0.0800 '/' Cc= 0.900
			n= 0.013, Flow Area= 1.23 sf

Primary OutFlow Max=0.71 cfs @ 17.78 hrs HW=350.09' TW=348.80' (Dynamic Tailwater)
-1=Culvert (Inlet Controls 0.71 cfs @ 2.14 fps)

Type III 24-hr 10-YEAR Rainfall=4.47"

Prepared by Keach-Nordstrom Associates, Inc.

Printed 12/6/2023

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 47

Summary for Pond 9P: DMH#3

Inflow Area = 5.536 ac, 11.12% Impervious, Inflow Depth > 1.96" for 10-YEAR event

Inflow = 8.26 cfs @ 12.25 hrs, Volume= 0.90 af

Outflow = 8.26 cfs @ 12.25 hrs, Volume= 0.90 af, Atten= 0%, Lag= 0.0 min

Primary = 8.26 cfs @ 12.25 hrs, Volume= 0.90 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 376.15' @ 12.25 hrs

Flood Elev= 377.79'

Device	Routing	Invert	Outlet Devices
#1	Primary	374.26'	18.0" Round Culvert
			L= 30.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 374.26' / 374.04' S= 0.0073 '/' Cc= 0.900
			n= 0.013, Flow Area= 1.77 sf

Primary OutFlow Max=8.26 cfs @ 12.25 hrs HW=376.15' TW=368.83' (Dynamic Tailwater)
—1=Culvert (Barrel Controls 8.26 cfs @ 4.78 fps)

Summary for Pond 10P: DMH#4

Inflow Area = 5.536 ac, 11.12% Impervious, Inflow Depth > 1.96" for 10-YEAR event

Inflow = 8.26 cfs @ 12.25 hrs, Volume= 0.90 af

Outflow = 8.26 cfs @ 12.25 hrs, Volume= 0.90 af, Atten= 0%, Lag= 0.0 min

Primary = 8,26 cfs @ 12.25 hrs, Volume= 0.90 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 377.38' @ 12.25 hrs

Flood Elev= 381.66'

Device	Routing	Invert	Outlet Devices
#1	Primary	375.58'	18.0" Round Culvert
			L= 121.7' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 375.58' / 374.36' S= 0.0100 '/' Cc= 0.900
			n= 0.013. Flow Area= 1.77 sf

Primary OutFlow Max=8.25 cfs @ 12.25 hrs HW=377.38' TW=376.15' (Dynamic Tailwater)
1=Culvert (Outlet Controls 8.25 cfs @ 4.93 fps)

Summary for Pond 11P: DMH#5

Inflow Area = 5.536 ac, 11.12% Impervious, Inflow Depth > 1.96" for 10-YEAR event

Inflow = 8.26 cfs @ 12.25 hrs, Volume= 0.90 af

Outflow = 8.26 cfs @ 12.25 hrs, Volume= 0.90 af, Atten= 0%, Lag= 0.0 min

Primary = 8.26 cfs @ 12.25 hrs, Volume= 0.90 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 379.63' @ 12.25 hrs

Flood Elev= 384.00'

2304141-POST DEVELOPMENT

Type III 24-hr 10-YEAR Rainfall=4.47"

Prepared by Keach-Nordstrom Associates, Inc.

Printed 12/6/2023

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 48

Device	Routing	Invert	Outlet Devices
#1	Primary	377.94'	18.0" Round Culvert
			L= 226.1' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 377.94' / 375.68' S= 0.0100 '/' Cc= 0.900
			n= 0.013, Flow Area= 1.77 sf

Primary OutFlow Max=8.26 cfs @ 12.25 hrs HW=379.63' TW=377.38' (Dynamic Tailwater)
1=Culvert (Inlet Controls 8.26 cfs @ 4.67 fps)

Summary for Pond 12P: CB#6

Inflow Area = 5.536 ac, 11.12% Impervious, Inflow Depth > 1.96" for 10-YEAR event

Inflow = 8.26 cfs @ 12.25 hrs, Volume= 0.90 af

Outflow = 8.26 cfs @ 12.25 hrs, Volume= 0.90 af, Atten= 0%, Lag= 0.0 min

Primary = 8.26 cfs @ 12.25 hrs, Volume= 0.90 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 381.14' @ 12.25 hrs

Flood Elev= 384.30'

Primary OutFlow Max=8.26 cfs @ 12.25 hrs HW=381.14' TW=379.63' (Dynamic Tailwater) 1=Culvert (Inlet Controls 8.26 cfs @ 4.67 fps)

Summary for Pond 13P: CB#7

Inflow Area = 5.201 ac, 9.22% Impervious, Inflow Depth > 1.91" for 10-YEAR event

Inflow = 7.73 cfs @ 12.26 hrs, Volume= 0.83 af

Outflow = 7.73 cfs @ 12.26 hrs, Volume= 0.83 af, Atten= 0%, Lag= 0.0 min

Primary = 7.73 cfs @ 12.26 hrs, Volume= 0.83 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 382.86' @ 12.26 hrs

Flood Elev= 384.30'

Device	Routing	Invert	Outlet Devices
#1	Primary	380.25'	15.0" Round Culvert
			L= 30.3' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 380 25' / 379.95' S= 0.0099 '/' Cc= 0.900
			n= 0.013, Flow Area= 1.23 sf

Primary OutFlow Max=7.73 cfs @ 12.26 hrs HW=382.85' TW=381.14' (Dynamic Tailwater) 1=Culvert (Inlet Controls 7.73 cfs @ 6.30 fps)

Type III 24-hr 10-YEAR Rainfall=4.47"

Prepared by Keach-Nordstrom Associates, Inc.

Printed 12/6/2023

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 49

Summary for Pond 14P: CB#8

Inflow Area = 4.328 ac, 9.12% Impervious, Inflow Depth > 1.90" for 10-YEAR event

Inflow = 6.35 cfs @ 12.26 hrs, Volume= 0.69 af

Outflow = 6.35 cfs @ 12.26 hrs, Volume= 0.69 af, Atten= 0%, Lag= 0.0 min

Primary = 6.35 cfs @ 12.26 hrs, Volume= 0.69 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 386.27' @ 12.26 hrs

Flood Elev= 388.51'

Device	Routing	Invert	Outlet Devices
#1	Primary	384.49'	15.0" Round Cuivert L= 165.7' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 384.49' / 380.35' S= 0.0250 '/' Cc= 0.900
			n= 0.013, Flow Area= 1.23 sf

Primary OutFlow Max=6.35 cfs @ 12.26 hrs HW=386.27' TW=382.85' (Dynamic Tailwater)
1=Culvert (Inlet Controls 6.35 cfs @ 5.17 fps)

Summary for Pond 16P: CB#10

Inflow Area = 3.078 ac, 10.13% Impervious, Inflow Depth > 1.93" for 10-YEAR event

Inflow = 4.45 cfs @ 12.27 hrs, Volume= 0.49 af

Outflow = 4.45 cfs @ 12.27 hrs, Volume= 0.49 af, Atten= 0%, Lag= 0.0 min

Primary = 4.45 cfs @ 12.27 hrs, Volume= 0.49 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 393.99' @ 12.27 hrs

Flood Elev= 397.31'

Device	Routing	Invert	Outlet Devices
#1	Primary	392.11'	12.0" Round Culvert
	•		L= 165.5' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 392.11' / 383.83' S= 0.0500 '/' Cc= 0.900
			n= 0.013. Flow Area= 0.79 sf

Primary OutFlow Max=4.45 cfs @ 12.27 hrs HW=393.99' TW=386.26' (Dynamic Tailwater) 1=Culvert (Inlet Controls 4.45 cfs @ 5.66 fps)

Summary for Pond 17P: CB#100

Inflow Area = 0.183 ac, 70.27% Impervious, Inflow Depth > 3.45" for 10-YEAR event

Inflow = 0.72 cfs @ 12.09 hrs, Volume= 0.05 af

Outflow = 0.72 cfs @ 12.09 hrs, Volume= 0.05 af, Atten= 0%, Lag= 0.0 min

Primary = 0.72 cfs @ 12.09 hrs, Volume= 0.05 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 394.01' @ 12.28 hrs

Flood Elev= 397.31'

2304141-POST DEVELOPMENT

Type III 24-hr 10-YEAR Rainfall=4.47"

Prepared by Keach-Nordstrom Associates, Inc.

Printed 12/6/2023

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 50

Device	Routing	Invert	Outlet Devices
#1	Primary		12.0" Round Culvert L= 26.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 393.25' / 392.73' S= 0.0200 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

Primary OutFlow Max=0.72 cfs @ 12.09 hrs HW=393.68' TW=393.17' (Dynamic Tailwater) 1=Culvert (Inlet Controls 0.72 cfs @ 2.23 fps)

Summary for Pond 18P: CB#11

Inflow Area = 1.524 ac, 2.68% Impervious, Inflow Depth > 1.74" for 10-YEAR event lnflow = 2.03 cfs @ 12.30 hrs, Volume= 0.22 af Outflow = 2.03 cfs @ 12.30 hrs, Volume= 0.22 af, Atten= 0%, Lag= 0.0 min Primary = 2.03 cfs @ 12.30 hrs, Volume= 0.22 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 403.59' @ 12.30 hrs Flood Elev= 406.88'

Device Routing Invert Outlet Devices

#1 Primary

402.80' 12.0" Round Culvert

L= 167.8' CPP, square edge headwall, Ke= 0.500
Inlet / Outlet Invert= 402.80' / 392.73' S= 0.0600 '/' Cc= 0.900

n= 0.013. Flow Area= 0.79 sf

Primary OutFlow Max=2.03 cfs @ 12.30 hrs HW=403.59' TW=393.97' (Dynamic Tailwater) 1=Culvert (Inlet Controls 2.03 cfs @ 3.03 fps)

Summary for Pond 20P: DMH#14

Inflow Area = 3.830 ac, 17.06% Impervious, Inflow Depth > 2.08" for 10-YEAR event Inflow = 0.66 af

Outflow = 5.74 cfs @ 12.33 hrs, Volume= 0.66 af, Atten= 0%, Lag= 0.0 min

Primary = 5.74 cfs @ 12.33 hrs, Volume= 0.66 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 376.41' @ 12.33 hrs

Flood Elev= 383.00'

<u>Device</u>	Routing	Invert	Outlet Devices
#1	Primary	375.20'	18.0" Round Culvert
			L= 15.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 375.20' / 374.00' S= 0.0800 '/' Cc= 0.900
			n= 0.013, Flow Area= 1.77 sf

Primary OutFlow Max=5.74 cfs @ 12.33 hrs HW=376.41' TW=369.23' (Dynamic Tailwater)
-1=Culvert (Inlet Controls 5.74 cfs @ 3.75 fps)

Type III 24-hr 10-YEAR Rainfall=4.47"

Prepared by Keach-Nordstrom Associates, Inc.

Printed 12/6/2023

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 51

Summary for Pond 21P: DMH#15

Inflow Area =

3.830 ac. 17.06% Impervious. Inflow Depth > 2.08" for 10-YEAR event

Inflow

5.74 cfs @ 12.33 hrs, Volume=

0.66 af, Atten= 0%, Lag= 0.0 min

Outflow

5.74 cfs @ 12.33 hrs, Volume=

Primary

5.74 cfs @ 12.33 hrs, Volume=

0.66 af

0.66 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 382.67' @ 12.33 hrs

Flood Elev= 387.85'

Device Routing Invert Outlet Devices

#1 Primary 381.46 18.0" Round Culvert

L= 30.7' CPP, square edge headwall, Ke= 0.500

Inlet / Outlet Invert= 381.46' / 379.00' S= 0.0801 '/' Cc= 0.900

n= 0.013. Flow Area= 1.77 sf

Primary OutFlow Max=5.74 cfs @ 12.33 hrs HW=382.67' TW=376.41' (Dynamic Tailwater)

1=Culvert (Inlet Controls 5.74 cfs @ 3.75 fps)

Summary for Pond 22P: DMH#16

Inflow Area =

3.830 ac, 17.06% Impervious, Inflow Depth > 2.08" for 10-YEAR event

Inflow

5.74 cfs @ 12.33 hrs, Volume= 0.66 af

0.66 af, Atten= 0%, Lag= 0.0 min

Outflow = Primary

5.74 cfs @ 12.33 hrs, Volume= 5.74 cfs @ 12.33 hrs, Volume=

0.66 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 394.13' @ 12.33 hrs

Flood Elev= 400.42'

Device Routing

Invert Outlet Devices

#1 Primary 392.92'

18.0" Round Culvert L= 111.5' CPP, square edge headwall, Ke= 0.500

Inlet / Outlet Invert= 392.92' / 384.00' S= 0.0800 '/' Cc= 0.900

n= 0.013, Flow Area= 1.77 sf

Primary OutFlow Max=5.74 cfs @ 12.33 hrs HW=394.13' TW=382.67' (Dynamic Tailwater)

1=Culvert (Inlet Controls 5.74 cfs @ 3.75 fps)

Summary for Pond 23P: DMH#17

Inflow Area =

3.830 ac, 17.06% Impervious, Inflow Depth > 2.08" for 10-YEAR event

Inflow

5.74 cfs @ 12.33 hrs, Volume=

0.66 af

Outflow = 5.74 cfs @ 12.33 hrs, Volume=

0.66 af, Atten= 0%, Lag= 0.0 min

Primary

5.74 cfs @ 12.33 hrs, Volume=

0.66 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 402.34' @ 12.33 hrs

Flood Elev= 409.83'

2304141-POST DEVELOPMENT

Type III 24-hr 10-YEAR Rainfall=4.47"

Prepared by Keach-Nordstrom Associates, Inc.

Printed 12/6/2023

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 52

Device	Routing	Invert	Outlet Devices
#1	Primary	401.13'	18.0" Round Culvert L= 45.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 401.13' / 397.53' S= 0.0800 '/' Cc= 0.900 n= 0.013, Flow Area= 1.77 sf

Primary OutFlow Max=5.74 cfs @ 12.33 hrs HW=402.34' TW=394.13' (Dynamic Tailwater) 1=Culvert (Inlet Controls 5.74 cfs @ 3.75 fps)

Summary for Pond 24P: CB#18

Inflow Area = 3.830 ac, 17.06% Impervious, Inflow Depth > 2.08" for 10-YEAR event

Inflow = 5.74 cfs @ 12.33 hrs, Volume= 0.66 af

Outflow = 5.74 cfs @ 12.33 hrs, Volume= 0.66 af, Atten= 0%, Lag= 0.0 min

Primary = 5.74 cfs @ 12.33 hrs, Volume= 0.66 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 404.08' @ 12.33 hrs

Flood Elev= 407.95'

Device	Routing	Invert	Outlet Devices
#1	Primary	402.87'	18.0" Round Culvert
			L= 164.2' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 402.87' / 401.23' S= 0.0100 '/' Cc= 0.900
			n= 0.013, Flow Area= 1.77 sf

Primary OutFlow Max=5.74 cfs @ 12.33 hrs HW=404.08' TW=402.34' (Dynamic Tailwater) 1=Culvert (Inlet Controls 5.74 cfs @ 3.75 fps)

Summary for Pond 25P: CB#19

Inflow Area = 2.259 ac, 23.36% Impervious, Inflow Depth > 2.23" for 10-YEAR event

Inflow = 3.58 cfs @ 12.33 hrs, Volume= 0.42 af

Outflow = 3.58 cfs @ 12.33 hrs, Volume= 0.42 af, Atten= 0%, Lag= 0.0 min

Primary = 3.58 cfs @ 12.33 hrs. Volume= 0.42 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 404.55' @ 12.34 hrs

Flood Elev= 408.04'

Device	Routing	Invert	Outlet Devices
#1	Primary	403.34'	15.0" Round Culvert L= 36.5' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 403.34' / 402.97' S= 0.0101 '/' Cc= 0.900 n= 0.013, Flow Area= 1.23 sf

Primary OutFlow Max=3.58 cfs @ 12.33 hrs HW=404.55' TW=404.08' (Dynamic Tailwater) 1=Culvert (Outlet Controls 3.58 cfs @ 3.74 fps)

2304141-POST DEVELOPMENT

Type III 24-hr 10-YEAR Rainfall=4.47"

Prepared by Keach-Nordstrom Associates, Inc.

Printed 12/6/2023

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 53

Summary for Pond 26P: CB#20

Inflow Area = 0.702 ac, 34.65% Impervious, Inflow Depth > 2.48" for 10-YEAR event

Inflow = 1.38 cfs @ 12.28 hrs, Volume= 0.14 af

Outflow = 1.38 cfs @ 12.28 hrs, Volume= 0.14 af, Atten= 0%, Lag= 0.0 min

Primary = 1.38 cfs @ 12.28 hrs, Volume= 0.14 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 404.70' @ 12.33 hrs

Flood Elev= 407.78'

Device	Routing	Invert	Outlet Devices
#1	Primary	403.70'	12.0" Round Culvert
	-		L= 26.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 403.70' / 403.44' S= 0.0100 '/' Cc= 0.900
			n= 0.013, Flow Area= 0.79 sf

Primary OutFlow Max=1.32 cfs @ 12.28 hrs HW=404.67' TW=404.52' (Dynamic Tailwater) 1=Culvert (Outlet Controls 1.32 cfs @ 2.15 fps)

Summary for Pond PP: POCKET POND

Inflow Area =	11.270 ac, 12.00% Impervious, Inflow Depth > 2.00" for 10-YEAR even	∍nt
Inflow =	16.68 cfs @ 12.26 hrs, Volume= 1.88 af	
Outflow =	0.71 cfs @ 17.78 hrs, Volume= 0.66 af, Atten= 96%, Lag= 331	.3 min
Primary =	0.71 cfs @ 17.78 hrs, Volume= 0.66 af	
Secondary =	0.00 cfs @ 0.00 hrs, Volume= 0.00 af	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs
Starting Elev= 367.00' Surf.Area= 9,015 sf Storage= 7,477 cf
Peak Elev= 371.43' @ 17.78 hrs Surf.Area= 19,380 sf Storage= 64,996 cf (57,519 cf above start)
Flood Elev= 374.00' Surf.Area= 24,964 sf Storage= 118,753 cf (111,276 cf above start)

Plug-Flow detention time= 470.3 min calculated for 0.49 af (26% of inflow) Center-of-Mass det. time= 241.8 min (1.090.7 - 848.9)

Volume	Invert	Avail.Storage	Storage Description
#1	364.00'	118,753 cf	Custom Stage Data (Prismatic) Listed below (Recalc)
#2	369.00'	0 cf	Sediment Forebay (No Storage) (Prismatic) Listed below (Recalc)
			1,678 cf Overall x 0.0% Voids
		440.750 - [Total Accellable Otenson

118,753 cf Total Available Storage

Type III 24-hr 10-YEAR Rainfall=4.47"

Prepared by Keach-Nordstrom Associates, Inc.

Printed 12/6/2023

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 54

Elevation	Surf.Area	Inc.Store	Cum.Store
(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)
364.00	145	0	0
366.00	1,883	2,028	2,028
367.00	9,015	5,449	7,477
368.00	10,563	9,789	17,266
369.00	12,205	11,384	28,650
370.00	13,939	13,072	41,722
371.00	17,278	15,609	57,331
372.00	19,350	18,314	75,645
374.00	23,758	43,108	118,753
Elevation	Surf.Area	Inc.Store	Cum.Store
(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)
369.00	472	0	0
371.00	1,206	1,678	1,678

Device	Routing	Invert	Outlet Devices
#1	Primary	365.00'	15.0" Round Culvert
			L= 190.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 365.00' / 349.80' S= 0.0800 '/' Cc= 0.900
			n= 0.013, Flow Area= 1.23 sf
#2	Device 1	367.00'	2.5" Vert. Orifice C= 0.600
#3	Device 1	369.45'	3.2" Vert. Orifice C= 0.600
#4	Device 1	371.50'	3.0" Vert. Orifice C= 0.600
#5	Device 1	373.70'	48.0" x 48.0" Horiz. Grate C= 0.600
			Limited to weir flow at low heads
#6	Secondary	373.80'	10.0' long x 27.0' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60
			Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63

Primary OutFlow Max=0.71 cfs @ 17.78 hrs HW=371.43' TW=350.09' (Dynamic Tailwater)

-1=Culvert (Passes 0.71 cfs of 14.24 cfs potential flow)

-2=Orifice (Orifice Controls 0.34 cfs @ 10.02 fps)

-3=Orifice (Orifice Controls 0.37 cfs @ 6.55 fps)

4=Orifice (Controls 0.00 cfs) **5=Grate** (Controls 0.00 cfs)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=367.00' TW=348.50' (Dynamic Tailwater) 6=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Link A: BARRETTS HILL CROSSING

Inflow Area = 41.139 ac, 8.14% Impervious, Inflow Depth > 1.12" for 10-YEAR event

Inflow = 19.97 cfs @ 12.53 hrs, Volume= 3.82 af

Primary = 19.97 cfs @ 12.53 hrs, \foliame = 3.82 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

2304141-POST DEVELOPMENT

Type III 24-hr 10-YEAR Rainfall=4.47"

Prepared by Keach-Nordstrom Associates, Inc.

Printed 12/6/2023

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 55

Summary for Link B: LOT 57

Inflow Area = 2.097 ac, 14.39% Impervious, Inflow Depth > 2.07" for 10-YEAR event

Inflow = 3.45 cfs @ 12.27 hrs, Volume= 0.36 af

Primary = 3.45 cfs @ 12.27 hrs, Volume= 0.36 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Summary for Link C: LOT 11

Inflow Area = 2.594 ac, 9.49% Impervious, Inflow Depth > 1.90" for 10-YEAR event

Inflow = 3.80 cfs @ 12.29 hrs, Volume= 0.41 af

Primary = 3.80 cfs @ 12.29 hrs, Volume= 0.41 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

2304141-POST DEVELOPMENT

Type III 24-hr 25-YEAR Rainfall=5.66"

Prepared by Keach-Nordstrom Associates, Inc. HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Printed 12/6/2023

Page 56

Time span=0.00-24.00 hrs, dt=0.01 hrs, 2401 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment 1S: SUBCATCHMENT	Runoff Area=961,091 sf 4.08% Impervious Runoff Depth>2.05"
	Flow Length=2,203' Tc=38.1 min CN=64.6 Runoff=26.13 cfs 3.77 af
Subcatchment 2S: SUBCATCHMENT	Runoff Area=113,081 sf 12.76% Impervious Runoff Depth>1.80" Flow Length=683' Tc=20.1 min CN=61.4 Runoff=3.49 cfs 0.39 af
Subcatchment 3S: SUBCATCHMENT	Runoff Area=124,041 sf 7.14% Impervious Runoff Depth>1.81" Flow Length=1,189' Tc=23.1 min CN=61.5 Runoff=3.64 cfs 0.43 af
Subcatchment 4S: SUBCATCHMENT	Runoff Area=86,429 sf 21.84% Impervious Runoff Depth>2.58" Tc=6.0 min CN=70.6 Runoff=5.98 cfs 0.43 af
Subcatchment 5S: SUBCATCHMENT	Runoff Area=16,441 sf 34.30% Impervious Runoff Depth>2.87" Flow Length=389' Tc=8.7 min CN=73.7 Runoff=1.16 cfs 0.09 af
Subcatchment 6S: SUBCATCHMENT	Runoff Area=91,362 sf 14.39% Impervious Runoff Depth>3.04" Flow Length=692' Tc=19.5 min CN=75.7 Runoff=5.12 cfs 0.53 af
Subcatchment 7S: SUBCATCHMENT	Runoff Area=112,988 sf 9.49% Impervious Runoff Depth>2.84" Flow Length=1,021' Tc=20.7 min CN=73.5 Runoff=5.76 cfs 0.61 af
Subcatchment 8S: SUBCATCHMENT	Runoff Area=30,577 sf 34.65% Impervious Runoff Depth>3.52" Flow Length=719' Tc=20.3 min CN=80.6 Runoff=1.95 cfs 0.21 af
Subcatchment 9S: SUBCATCHMENT	Runoff Area=67,824 sf 18.27% Impervious Runoff Depth>3.11" Flow Length=1,293' Tc=26.9 min CN=76.4 Runoff=3.39 cfs 0.40 af
Subcatchment 10S: SUBCATCHMENT	Runoff Area=68,434 sf 8.00% Impervious Runoff Depth>2.80" Flow Length=676' Tc=23.0 min CN=73.0 Runoff=3.28 cfs 0.37 af
Subcatchment 11S: SUBCATCHMENT	Runoff Area=66,396 sf 2.68% Impervious Runoff Depth>2.65" Flow Length=690' Tc=20.7 min CN=71.4 Runoff=3.14 cfs 0.34 af
Subcatchment 12S: SUBCATCHMENT	Runoff Area=59,686 sf 10.36% Impervious Runoff Depth>2.88" Flow Length=753' Tc=18.7 min CN=73.9 Runoff=3.21 cfs 0.33 af
Subcatchment 13S: SUBCATCHMENT	Runoff Area=54,469 sf 6.63% Impervious Runoff Depth>2.77" Flow Length=680' Tc=16.3 min CN=72.6 Runoff=2.96 cfs 0.29 af
Subcatchment 14S: SUBCATCHMENT	Runoff Area=38,021 sf 9.74% Impervious Runoff Depth>2.86" Flow Length=731' Tc=17.2 min CN=73.7 Runoff=2.10 cfs 0.21 af
Subcatchment 15S: SUBCATCHMENT	Runoff Area=14,603 sf 40.57% Impervious Runoff Depth>3.85" Tc=6.0 min CN=83.7 Runoff=1.50 cfs 0.11 af
Subcatchment 16S: SUBCATCHMENT	Runoff Area=7,988 sf 70.27% Impervious Runoff Depth>4.61" Tc=6.0 min CN=90.9 Runoff=0.94 cfs 0.07 af

Type III 24-hr 25-YEAR Rainfall=5.66"

Prepared by Keach-Nords HydroCAD® 10.00-26 s/n 032	rom Associates, Inc. 73 © 2020 HydroCAD Software Solutions LLC Printed 12/6/2023 Page 57
Subcatchment 17S: SUBCA	TCHMENT Runoff Area=82,926 sf 4.39% Impervious Runoff Depth>2.91" Flow Length=230' Tc=13.2 min CN=74.2 Runoff=5.17 cfs 0.46 af
Reach 1R: EXISTING ROAD	WAY DITCH Avg. Flow Depth=0.73' Max Vel=4.38 fps Inflow=7.07 cfs 0.52 af n=0.022 L=230.0' S=0.0173 '/' Capacity=16.21 cfs Outflow=7.01 cfs 0.52 af
Reach 2R: EXISTING ROAD	WAY DITCH Avg. Flow Depth=0.37' Max Vel=1.90 fps Inflow=1.13 cfs 1.05 af n=0.069 L=915.0' S=0.0621 '/' Capacity=12.04 cfs Outflow=1.13 cfs 1.04 af
Pond 1P: EXISTING CB	Inflow=26.13 cfs 3.77 af Primary=26.13 cfs 3.77 af
Pond 2P: EXISTING DRIVEY	VAY CULVERT Peak Elev=295.21' Inflow=3.49 cfs 0.39 af 12.0" Round Culvert n=0.013 L=21.0' S=0.0143 '/' Outflow=3.49 cfs 0.39 af
Pond 3P: EXISTING DROP I	Peak Elev=294.71' Inflow=33.99 cfs 6.15 af 24.0" Round Culvert n=0.011 L=92.1' S=0.0433 '/' Outflow=33.99 cfs 6.15 af
Pond 4P: EXISTING CB	Peak Elev=300.38' Inflow=7.07 cfs 0.52 af Primary=4.54 cfs 0.49 af Secondary=2.53 cfs 0.02 af Outflow=7.07 cfs 0.52 af
Pond 5P: EXISTING CB	Peak Elev=300.56' Inflow=1.16 cfs 0.09 af Primary=1.16 cfs 0.09 af Secondary=0.00 cfs 0.00 af Outflow=1.16 cfs 0.09 af
Pond 6P: EXISTING CB	Peak Elev=366.04' Inflow=5.12 cfs 0.53 af 12.0" Round Culvert n=0.025 L=18.0' S=0.0689 '/' Outflow=5.12 cfs 0.53 af
Pond 7P: EXISTING CB	Peak Elev=379.39' Inflow=5.76 cfs 0.61 af 12.0" Round Culvert n=0.011 L=31.9' S=0.0091 '/' Outflow=5.76 cfs 0.61 af
Pond 8P: DMH#2	Peak Elev=350.21' Inflow=1.13 cfs 1.05 af 15.0" Round Culvert n=0.013 L=15.0' S=0.0800 '/' Outflow=1.13 cfs 1.05 af
Pond 9P: DMH#3	Peak Elev=377.15' Inflow=12.46 cfs 1.34 af 18.0" Round Culvert n=0.013 L=30.0' S=0.0073 '/' Outflow=12.46 cfs 1.34 af
Pond 10P: DMH#4	Peak Elev=380.03' Inflow=12.46 cfs 1.34 af 18.0" Round Culvert n=0.013 L=121.7' S=0.0100 '/' Outflow=12.46 cfs 1.34 af
Pond 11P: DMH#5	Peak Elev=384.37' Inflow=12.46 cfs 1.34 af 18.0" Round Culvert n=0.013 L=226.1' S=0.0100 '/' Outflow=12.46 cfs 1.34 af
Pond 12P: CB#6	Peak Elev=387.50' Inflow=12.46 cfs 1.34 af 18.0" Round Culvert n=0.013 L=141.2' S=0.0100 '/' Outflow=12.46 cfs 1.34 af
Pond 13P: CB#7	Peak Elev=391.42' Inflow=11.74 cfs 1.23 af 15.0" Round Culvert n=0.013 L=30.3' S=0.0099 '/' Outflow=11.74 cfs 1.23 af
Pond 14ℙ: CB#8	Feak Elev=396.52' Inflow=9.64 cfs 1.02 af 15.0" Round Culvert n=0.013 L=165.7' S=0.0250 '/' Outflow=9.64 cfs 1.02 af

2304141-POST DEVELOPMENT

Inflow=5.76 cfs 0.61 af Primary=5.76 cfs 0.61 af

2304141-POST DEVELO Prepared by Keach-Nords HydroCAD® 10.00-26 s/n 032		Type III 24-hr 25-YEAR Rainfall=5.66" Printed 12/6/2023 utions LLC Page 58
Pond 16P: CB#10	12.0" Round Culvert n=0.013 L=	Peak Elev=404.14' Inflow=6.74 cfs 0.74 af =165.5' S=0.0500'/' Outflow=6.74 cfs 0.74 af
Pond 17P: CB#100	12.0" Round Culvert n=0.013 L	Peak Elev=404.15' Inflow=0.94 cfs 0.07 af L=26.0' S=0.0200 '/' Outflow=0.94 cfs 0.07 af
Pond 18P: CB#11	12.0" Round Culvert n=0.013 L=	Peak Elev=405.82' Inflow=3.14 cfs 0.34 af =167.8' S=0.0600 '/' Outflow=3.14 cfs 0.34 af
Pond 20P: DMH#14	18.0" Round Culvert n=0.013 L	Peak Elev=376.95' Inflow=8.49 cfs 0.98 af L=15.0' S=0.0800 '/' Outflow=8.49 cfs 0.98 af
Pond 21P: DMH#15	18.0" Round Culvert n=0.013 L	Peak Elev=383.21' Inflow=8.49 cfs 0.98 af L=30.7' S=0.0801 '/' Outflow=8.49 cfs 0.98 af
Pond 22P: DMH#16	18.0" Round Culvert n=0.013 L=	Peak Elev=394.67' Inflow=8.49 cfs 0.98 af =111.5' S=0.0800 '/' Outflow=8.49 cfs 0.98 af
Pond 23P: DMH#17	18.0" Round Culvert n=0.013 L	Peak Elev=402.88' Inflow=8.49 cfs 0.98 af _=45.0' S=0.0800 '/' Outflow=8.49 cfs 0.98 af
Pond 24P: CB#18	18.0" Round Culvert n=0.013 L=	Peak Elev=404.62' Inflow=8.49 cfs 0.98 af =164.2' S=0.0100 '/' Outflow=8.49 cfs 0.98 af
Pond 25P: CB#19	15.0" Round Culvert n=0.013 L	Peak Elev=405.39' Inflow=5.21 cfs 0.61 af =36.5' S=0.0101 '/' Outflow=5.21 cfs 0.61 af
Pond 26P: CB#20		Peak Elev=405.63' Inflow=1.95 cfs 0.21 af .=26.0' S=0.0100 '/' Outflow=1.95 cfs 0.21 af
Pond PP: POCKET POND	Primary=1.13 cfs 1.05 af Seconda	' Storage=92,034 cf Inflow=25.01 cfs 2.78 af ary=0.00 cfs 0.00 af Outflow=1.13 cfs 1.05 af
Link A: BARRETTS HILL CF	ROSSING	Inflow=33.99 cfs 6.15 af Primary=33.99 cfs 6.15 af
Link B: LOT 57		Inflow=5.12 cfs 0.53 af Primary=5.12 cfs 0.53 af

Total Runoff Area = 45.830 ac Runoff Volume = 9.03 af Average Runoff Depth = 2.37" 91.49% Pervious = 41.932 ac 8.51% Impervious = 3.898 ac

Link C: LOT 11

Type III 24-hr 25-YEAR Rainfall=5.66"

Prepared by Keach-Nordstrom Associates, Inc.

Printed 12/6/2023

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 59

Summary for Subcatchment 1S: SUBCATCHMENT

Runoff = 26.13 cfs @ 12.57 hrs, Volume=

3.77 af, Depth> 2.05"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 25-YEAR Rainfall=5.66"

Α	rea (sf)	CN	Description	n				
	7,670	30.0	Woods, G	Good, HSG	A			
3	363,085	55.0	Woods, G	Woods, Good, HSG B				
4	27,890	70.0	Woods, G	Good, HSG	C			
	6,684	39.0	>75% Gra	ass cover, (Good, HSG A			
	61,618	61.0	>75% Gra	ass cover, (Good, HSG B			
	54,927	74.0	>75% Gra	ass cover, (Good, HSG C			
*	21,604	98.0	Roofs					
*	17,613	98.0	Paved pa	rking				
ç	61,091	64.6	Weighted	Average				
ç	921,874 63.2			95.92% Pervious Area				
	39,217 98.0			4.08% Impervious Area				
Tc	Length	Slope	Velocity	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
17.3	100	0.0400	0.10		Sheet Flow,			
					Woods: Light underbrush n= 0.400 P2= 2.84"			
19.7	1,862	0.0988	1.57		Shallow Concentrated Flow,			
					Woodland Kv= 5.0 fps			
0.3	30	0.0800	1.98		Shallow Concentrated Flow,			
					Short Grass Pasture Kv= 7.0 fps			
0.8	211	0.0430	4.21		Shallow Concentrated Flow,			
					Paved Kv= 20.3 fps			
38.1	2.203	Total						

Summary for Subcatchment 2S: SUBCATCHMENT

Runoff = 3.49 cfs @ 12.29 hrs, Volume=

0.39 af, Depth> 1.80"

	Area (sf)	CN	Description				
	28,414	30.0	Woods, Good, HSG A				
	11,383	55.0	Woods, Good, HSG B				
	33,808	70.0	Woods, Good, HSG C				
	4,719	39.0	>75% Grass cover, Good, HSG A				
	242	61.0	>75% Grass cover, Good, HSG B				
	20,087	74.0	>75% Grass cover, Good, HSG C				
*	3,849	98.0	Roofs				
*	10,579	98.0	Paved parking				
	113,081	61.4	Weighted Average				
	98,653	56.1	87.24% Pervious Area				
	14,428	98.0	12.76% Impervious Area				

2304141-POST DEVELOPMENT

20.1

683

Total

Type III 24-hr 25-YEAR Rainfall=5.66"

Prepared by Keach-Nordstrom Associates, Inc. HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Printed 12/6/2023 Page 60

Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs) 12.0 100 0.1000 0.14 Sheet Flow, Woods: Light underbrush n= 0.400 P2= 2.84" 3.9 318 0.0750 1.37 Shallow Concentrated Flow. Woodland Kv= 5.0 fps 0.7 48 0.0300 1.21 **Shallow Concentrated Flow.** Short Grass Pasture Kv= 7.0 fps 3.6 217 0.0410 1.01 Shallow Concentrated Flow.

Summary for Subcatchment 3S: SUBCATCHMENT

Woodland Kv= 5.0 fps

Runoff = 3.64 cfs @ 12.34 hrs, Volume=

0.43 af, Depth> 1.81"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 25-YEAR Rainfall=5.66"

,	Area (sf)	CN	Description							
	5,732	30.0	Woods, G	Woods, Good, HSG A						
	49,946	55.0	Woods, Good, HSG B							
	30,651	70.0	Woods, G	Noods, Good, HSG C						
	4,799	39.0	>75% Gra	ass cover, (Good, HSG A					
	20,628	61.0	>75% Gra	>75% Grass cover, Good, HSG B						
	3,428	74.0	>75% Gra	ass cover, (Good, HSG C					
*	2,833	98.0	Roofs							
*	<u>* 6,024 98.0 F</u>			Paved parking						
124,041 61.5			Weighted Average							
	115,184 58.7 8,857 98.0			92.86% Pervious Area						
				7.14% Impervious Area						
_										
Tc		Slope	Velocity	Capacity	Description					
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)						
9.9	100	0.1600	0.17		Sheet Flow,					
					Woods: Light underbrush n= 0.400 P2= 2.84"					
13.2	1,089	0.0760	1.38		Shallow Concentrated Flow,					
					Woodland Kv= 5.0 fps					
23.1	1,189	Total								

Summary for Subcatchment 4S: SUBCATCHMENT

Runoff =: 5.98 cfs @ 12.09 hrs, Volume=

0.43 af, Depth> 2.58"

2304141-POST DEVELOPMENT

Type III 24-hr 25-YEAR Rainfall=5.66"

Prepared by Keach-Nordstrom Associates, Inc.

Printed 12/6/2023

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 61

	Area (sf)	CN	Description					
	10,510	55.0	Woods, Good, HSG B					
	6,756	70.0	Woods, Good, HSG C					
	40,259	61.0	>75% Grass cover, Good, HSG B					
	10,029	74.0	75% Grass cover, Good, HSG C					
	16,327	98.0	Paved parking, HSG C					
*	2,548	98.0	Roofs					
	86,429	70.6	Weighted Average					
	67,554	62.9	78.16% Pervious Area					
	18,875	98.0	21.84% Impervious Area					
To (min)		Slope (ft/ft)	70 To 10 To					
6.0)		Direct Entry,					

Summary for Subcatchment 5S: SUBCATCHMENT

Runoff = 1.16 cfs @ 12.12 hrs, Volume=

Description

CNI

Auna (af)

0.09 af, Depth> 2.87"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 25-YEAR Rainfall=5.66"

	Area (sf) CN Description										
		10,801	61.0	>75% Gra	75% Grass cover, Good, HSG B						
*		5,640	98.0	Paved pa	Paved parking						
0.		16,441	73.7	Weighted Average							
		10,801	61.0	эа							
		5,640	98.0	34.30% Ir	65.70% Pervious Area 34.30% Impervious Area						
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description					
	6.3	100	0.0700	0.26		Sheet Flow,					
						Grass: Short n= 0.150 P2= 2.84"					
	0.1	16	0.0400	4.06		Shallow Concentrated Flow,					
						Paved Kv= 20.3 fps					
	1.5	162	0.0680	1.83		Shallow Concentrated Flow,					
						Short Grass Pasture Kv= 7.0 fps					
	0.1	20	0.0400	4.06		Shallow Concentrated Flow,					
		0.4	0.0770	4.04		Paved Kv= 20.3 fps					
	0.8	91	0.0770	1.94		Shallow Concentrated Flow,					
-						Short Grass Pasture Kv= 7.0 fps					
	8.7	389	Total								

Summary for Subcatchment 6S: SUBCATCHMENT

Runoff = 5.12 cfs @ 12.27 hrs, Volume=

0.53 af, Depth> 3.04"

2304141-POST DEVELOPMENT

Type III 24-hr 25-YEAR Rainfall=5.66"

Prepared by Keach-Nordstrom Associates, Inc.

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Printed 12/6/2023 Page 62

	rea (sf)	CN	Description	n				
	42,274	70.0	Woods, Good, HSG C					
	35,721	74.0		75% Grass cover, Good, HSG C				
*	8,350	98.0	Paved pa		,			
*	4,800	98.0	Roofs	Ü				
	217	96.0	Gravel su	rface, HSG	S C			
	91,362	75.7	Weighted	Average				
	78,212	71.9		ervious Are	ea			
	13,150	98.0	14.39% Ir	npervious A	Area			
				•				
Tc	Length	Slope	Velocity	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
15.8	100	0.0500	0.11		Sheet Flow,			
					Woods: Light underbrush n= 0.400 P2= 2.84"			
1.0	80	0.0750	1.37		Shallow Concentrated Flow,			
					Woodland Kv= 5.0 fps			
0.2	18	0.0400	1.40		Shallow Concentrated Flow,			
					Short Grass Pasture Kv= 7.0 fps			
0.2	12	0.0400	1.00		Shallow Concentrated Flow,			
0.4	4=				Woodland Kv= 5.0 fps			
0.1	15	0.3000	3.83		Shallow Concentrated Flow,			
0.0	407	0.0000	0.50		Short Grass Pasture Kv= 7.0 fps			
2.2	467	0.0300	3.52		Shallow Concentrated Flow,			
- 10.5					Paved Kv= 20.3 fps			
19.5	692	Total						

Summary for Subcatchment 7S: SUBCATCHMENT

Runoff = 5.76 cfs @ 12.29 hrs, Volume=

0.61 af, Depth> 2.84"

_	Area (sf)	CN	Description						
	83,250	70.0	Voods, Good, HSG C						
	18,246	74.0	>75% Grass cover, Good, HSG C						
*	5,928	98.0	Paved parking						
	764	96.0	Gravel surface, HSG C						
*	4,800	98.0	Roofs						
	112,988	73.5	Weighted Average						
	102,260	70.9	90.51% Pervious Area						
	10,728	98.0	9.49% Impervious Area						

2304141-POST DEVELOPMENT

Type III 24-hr 25-YEAR Rainfall=5.66"

Prepared by Keach-Nordstrom Associates, Inc.

Printed 12/6/2023

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 63

	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	12.5	100	0.0900	0.13		Sheet Flow,
						Woods: Light underbrush n= 0.400 P2= 2.84"
	7.5	755	0.1130	1.68		Shallow Concentrated Flow,
						Woodland Kv= 5.0 fps
	0.0	6	0.3000	3.83		Shallow Concentrated Flow,
						Short Grass Pasture Kv= 7.0 fps
	0.7	160	0.0375	3.93		Shallow Concentrated Flow,
						Paved Kv= 20.3 fps
-	20.7	1,021	Total			

Summary for Subcatchment 8S: SUBCATCHMENT

Runoff = 1.95 cfs @ 12.28 hrs, Volume=

0.21 af, Depth> 3.52"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 25-YEAR Rainfall=5.66"

A	rea (sf)	CN	Description						
	13,046	70.0	Woods, G	Woods, Good, HSG C					
	6,935	74.0	>75% Gra	iss cover, (Good, HSG C				
*	10,596	98.0	Paved parking						
).	30,577	80.6	Weighted	Average					
	19,981	71.4	65.35% P	ervious Are	ea				
	10,596	98.0	34.65% Ir	npervious A	Area				
Tc	Length	Slope	Velocity	Capacity	Description				
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
15.8	100	0.0500	0.11		Sheet Flow,				
					Woods: Light underbrush n= 0.400 P2= 2.84"				
2.9	329	0.1480	1.92		Shallow Concentrated Flow,				
					Woodland Kv= 5.0 fps				
0.1	33	0.3330	4.04		Shallow Concentrated Flow,				
					Short Grass Pasture Kv= 7.0 fps				
1.5	257	0.0200	2.87		Shallow Concentrated Flow,				
					Paved Kv= 20.3 fps				
20.3	719	Total							

Summary for Subcatchment 9S: SUBCATCHMENT

Runoff = 3.39 cfs @ 12.38 hrs, Volume=

0.40 af, Depth> 3.11"

2304141-POST DEVELOPMENT

Type III 24-hr 25-YEAR Rainfall=5.66"

Prepared by Keach-Nordstrom Associates, Inc.

Printed 12/6/2023

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 64

A	rea (sf)	CN	Description	n			
	34,147	70.0	Woods, G	Woods, Good, HSG C			
	21,287	74.0	>75% Gra	ass cover, (Good, HSG C		
*	7,590	98.0	Paved pa				
*	4,800	98.0	Roofs	Ü			
	67,824	76.4	Weighted	Weighted Average			
	55,434	71.5		81.73% Pervious Area			
	12,390	98.0	18.27% In	18.27% Impervious Area			
				•			
Tc	Length	Slope	Velocity	Capacity	Description		
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	·		
19.4	100	0.0300	0.09	· · · · · · · · · · · · · · · · · · ·	Sheet Flow,		
					Woods: Light underbrush n= 0.400 P2= 2.84"		
2.6	350	0.1940	2.20		Shallow Concentrated Flow,		
					Woodland Kv= 5.0 fps		
0.2	43	0.3330	4.04		Shallow Concentrated Flow,		
					Short Grass Pasture Kv= 7.0 fps		
4.6	800	0.0200	2.87		Shallow Concentrated Flow,		
					Paved Kv= 20.3 fps		
26.9	1,293	Total					

Summary for Subcatchment 10S: SUBCATCHMENT

Runoff = 3.28 cfs @ 12.33 hrs, Volume=

0.37 af, Depth> 2.80"

	Α	rea (sf)	CN	Description	on				
		49,642	70.0	Woods, C	Voods, Good, HSG C				
		13,315	74.0			Good, HSG C			
*		3,077	98.0	Paved pa		· ·			
*		2,400	98.0	Roofs					
	68,434 73.0 Weighted Average			Weighted	Average				
	62,957 70.8 92.00% Pervious Are			92.00% P	ervious Are	ea			
		5,477	98.0 8.00% Impervious A			rea			
	Тс	Length	Slope	Velocity	Capacity	Description			
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
	17.3	100	0.0400	0.10		Sheet Flow,			
						Woods: Light underbrush n= 0.400 P2= 2.84"			
	4.0	479	0.1610	2.01		Shallow Concentrated Flow,			
						Woodland Kv= 5.0 fps			
	1.5	65	0.0100	0.70		Shallow Concentrated Flow,			
						Short Grass Pasture Kv= 7.0 fps			
	0.2	32	0.0200	2.87		Shallow Concentrated Flow,			
_	_					Paved Kv= 20.3 fps			
	23.0	676	Total						

2304141-POST DEVELOPMENT

Type III 24-hr 25-YEAR Rainfall=5.66"

Prepared by Keach-Nordstrom Associates, Inc. HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC Printed 12/6/2023

Page 65

Summary for Subcatchment 11S: SUBCATCHMENT

Runoff = 3.14 cfs @ 12.29 hrs, Volume=

0.34 af, Depth> 2.65"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 25-YEAR Rainfall=5.66"

	Α	rea (sf)	CN	Description	n					
		54,267	70.0	Woods, G	Voods, Good, HSG C					
		10,350	74.0	>75% Gra	ass cover, (Good, HSG C				
*		1,779	98.0	Paved pa	rking					
		66,396	71.4	Weighted	Average					
		64,617	70.6	97.32% P	ervious Are	ea				
		1,779	98.0	2.68% lm	pervious Ai	rea				
	Тс	Length	Slope		Capacity	Description				
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
	15.8	100	0.0500	0.11		Sheet Flow,				
						Woods: Light underbrush n= 0.400 P2= 2.84"				
	4.2	485	0.1510	1.94		Shallow Concentrated Flow,				
						Woodland Kv= 5.0 fps				
	0.6	73	0.0800	1.98		Shallow Concentrated Flow,				
						Short Grass Pasture Kv= 7.0 fps				
	0.1	32	0.0600	4.97		Shallow Concentrated Flow,				
_						Paved Kv= 20.3 fps				
	20.7	690	Total							

Summary for Subcatchment 12S: SUBCATCHMENT

Runoff = 3.21 cfs @ 12.26 hrs, Volume=

0.33 af, Depth> 2.88"

	Area (sf)	CN	Description
	38,779	70.0	Woods, Good, HSG C
	14,722	74.0	>75% Grass cover, Good, HSG C
*	3,785	98.0	Paved parking
*	2,400	98.0	Roofs
	59,686	73.9	Weighted Average
	53,501	71.1	89.64% Pervious Area
	6,185	98.0	10.36% Impervious Area

2304141-POST DEVELOPMENT

Type III 24-hr 25-YEAR Rainfall=5.66"

Prepared by Keach-Nordstrom Associates, Inc.

Printed 12/6/2023

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 66

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
13.8	100	0.0700	0.12		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 2.84"
4.0	462	0.1470	1.92		Shallow Concentrated Flow,
0.4		0.4400	0.00		Woodland Kv= 5.0 fps
0.4	55	0.1100	2.32		Shallow Concentrated Flow,
0.5	400	0.0000	4.07		Short Grass Pasture Kv= 7.0 fps
0.5	136	0.0600	4.97		Shallow Concentrated Flow,
					Paved Kv= 20.3 fps
18.7	753	Total			

Summary for Subcatchment 13S: SUBCATCHMENT

Runoff = 2.96 cfs @ 12.23 hrs, Volume=

0.29 af, Depth> 2.77"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 25-YEAR Rainfall=5.66"

	A	rea (sf)	CN	Description	Description					
		40,261	70.0	Woods, G	Woods, Good, HSG C					
		10,598	74.0	>75% Gra	ass cover, (Good, HSG C				
	*	3,610	98.0	Paved pa	rking	•				
		54,469	72.6	Weighted	Average					
		50,859	70.8		ervious Are	ea				
		3,610	98.0	6.63% lm	pervious A	rea				
	Тс	Length	Slope	Velocity	Capacity	Description				
32	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
	12.0	100	0.1000	0.14		Sheet Flow,				
						Woods: Light underbrush n= 0.400 P2= 2.84"				
	3.8	425	0.1410	1.88		Shallow Concentrated Flow,				
						Woodland Kv= 5.0 fps				
	0.3	61	0.3300	4.02		Shallow Concentrated Flow,				
	0.0	0.4	0.0000	4.0=		Short Grass Pasture Kv= 7.0 fps				
	0.3	94	0.0600	4.97		Shallow Concentrated Flow,				
						Paved Kv= 20.3 fps				
	16.3	680	Total							

Summary for Subcatchment 14S: SUBCATCHMENT

Runoff = 2.10 cfs @ 12.23 hrs, Volume=

0.21 af, Depth> 2.86"

Type III 24-hr 25-YEAR Rainfall=5.66"

Prepared by Keach-Nordstrom Associates, Inc.

Printed 12/6/2023

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 67

	Α	rea (sf)	CN	Description	n				
		25,189	70.0	Woods, G	Woods, Good, HSG C				
		9,128	74.0	>75% Gra	ss cover, (Good, HSG C			
*		3,704	98.0	Paved par	rking				
		38,021	73.7	Weighted	Average				
		34,317	71.1		ervious Are	ea			
		3,704	98.0	9.74% Im	pervious Ai	rea			
		·			'				
	Tc	Length	Slope	Velocity	Capacity	Description			
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
	11.1	100	0.1200	0.15		Sheet Flow,			
						Woods: Light underbrush n= 0.400 P2= 2.84"			
	5.5	529	0.1020	1.60		Shallow Concentrated Flow,			
						Woodland Kv= 5.0 fps			
	0.2	46	0.3300	4.02		Shallow Concentrated Flow,			
						Short Grass Pasture Kv= 7.0 fps			
	0.3	56	0.0200	2.87		Shallow Concentrated Flow,			
						Paved Kv= 20.3 fps			
	17.2	731	Total	·					

Summary for Subcatchment 15S: SUBCATCHMENT

Runoff = 1.50 cfs @ 12.09 hrs, Volume=

0.11 af, Depth> 3.85"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 25-YEAR Rainfall=5.66"

	Α	rea (sf)	CN	Description	n					
		8,678	74.0	>75% Gra	>75% Grass cover, Good, HSG C					
k		5,925	98.0	Paved par	Paved parking					
		14,603	83.7	Weighted	Weighted Average					
		8,678	74.0	59.43% Pervious Area						
		5,925	98.0	40.57% In	40.57% Impervious Area					
	Тс	Length	Slope	Velocity	Capacity	Description				
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
	6.0					Direct Entry.				

Summary for Subcatchment 16S: SUBCATCHMENT

Runoff = 0.94 cfs @ 12.08 hrs, Volume=

0.07 af, Depth> 4.61"

2304141-POST DEVELOPMENT

Type III 24-hr 25-YEAR Rainfall=5.66"

Prepared by Keach-Nordstrom Associates, Inc.

Printed 12/6/2023

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 68

_	A	rea (sf)	CN	Description	n					
		2,375	74.0	>75% Gra	>75% Grass cover, Good, HSG C					
*		5,613	98.0	Paved pa	rking					
		7,988	90.9	Weighted	Average					
		2,375	74.0	29.73% Pervious Area						
		5,613	98.0	70.27% In	70.27% Impervious Area					
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description				
	6.0					Direct Entry,				

Summary for Subcatchment 17S: SUBCATCHMENT

Runoff = 5.17 cfs @ 12.18 hrs, Volume=

0.46 af, Depth> 2.91"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 25-YEAR Rainfall=5.66"

		Area (sf)	CN	Description	n			
		26,296	70.0	Woods, G	Good, HSG	C		
		51,457	74.0	>75% Gra	75% Grass cover, Good, HSG C			
*		1,238	98.0	Paved pa		•		
*		2,400	98.0	Roofs	Ü			
_		1,535	96.0	Gravel su	Gravel surface, HSG C			
		82,926	74.2	Weighted	Veighted Average			
		79,288	73.1		05.61% Pervious Area			
		3,638	98.0		4.39% Impervious Area			
	Tc	Length	Slope	Velocity	Capacity	Description		
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	<u>'</u>		
	12.0	100	0.1000	0.14		Sheet Flow,		
						Woods: Light underbrush n= 0.400 P2= 2.84"		
	1.0	90	0.0860	1.47		Shallow Concentrated Flow,		
						Woodland Kv= 5.0 fps		
	0.2	40	0.3300	4.02		Shallow Concentrated Flow,		
_						Short Grass Pasture Kv= 7.0 fps		
	13.2	230	Total					

Summary for Reach 1R: EXISTING ROADWAY DITCH

Inflow Area = 2.362 ac, 23.83% Impervious, Inflow Depth > 2.63" for 25-YEAR event

Inflow = 7.07 cfs @ 12.10 hrs, Volume= 0.52 af

Outflow = 7.01 cfs @ 12.11 hrs, Volume= 0.52 af, Atten= 1%, Lag= 0.7 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Max. Velocity= 4.38 fps, Min. Travel Time= 0.9 min Avg. Velocity = 1.85 fps, Avg. Travel Time= 2.1 min

2304141-POST DEVELOPMENT

Type III 24-hr 25-YEAR Rainfall=5.66"

Prepared by Keach-Nordstrom Associates, Inc.

Printed 12/6/2023

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 69

Peak Storage= 368 cf @ 12.11 hrs Average Depth at Peak Storage= 0.73'

Bank-Full Depth= 1.00' Flow Area= 3.0 sf, Capacity= 16.21 cfs

0.00' x 1.00' deep channel, n= 0.022 Earth, clean & straight

Side Slope Z-value= 3.0 '/' Top Width= 6.00'

Length= 230.0' Slope= 0.0173 '/'

Inlet Invert= 295.64', Outlet Invert= 291.66'



Summary for Reach 2R: EXISTING ROADWAY DITCH

Inflow Area = 11.270 ac, 12.00% Impervious, Inflow Depth > 1.12" for 25-YEAR event

Inflow = 1.13 cfs @ 17.14 hrs, Volume= 1.05 af

Outflow = 1.13 cfs @ 17.25 hrs, Volume= 1.04 af, Atten= 0%, Lag= 6.3 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Max. Velocity= 1.90 fps, Min. Travel Time= 8.0 min Avg. Velocity = 1.46 fps, Avg. Travel Time= 10.4 min

Peak Storage= 546 cf @ 17.25 hrs Average Depth at Peak Storage= 0.37'

Bank-Full Depth= 1.00' Flow Area= 3.5 sf, Capacity= 12.04 cfs

0.50' x 1.00' deep channel, n= 0.069 Riprap, 6-inch

Side Slope Z-value= 3.0 '/' Top Width= 6.50'

Length= 915.0' Slope= 0.0621 '/'

Inlet Invert= 348.50', Outlet Invert= 291.66'



Summary for Pond 1P: EXISTING CB

Inflow Area = 22.064 ac, 4.08% Impervious, Inflow Depth > 2.05" for 25-YEAR event

Inflow = 2.6.13 cfs @ 12.57 hrs, Volume= 3.77 af

Primary = 26.13 cfs @ 12.57 hrs, Volume= 3.77 af, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Type III 24-hr 25-YEAR Rainfall=5.66"

Prepared by Keach-Nordstrom Associates, Inc.

Printed 12/6/2023

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 70

Summary for Pond 2P: EXISTING DRIVEWAY CULVERT

Inflow Area = 2.596 ac, 12.76% Impervious, Inflow Depth > 1.80" for 25-YEAR event

Inflow = 3.49 cfs @ 12.29 hrs, Volume= 0.39 af

Outflow = 3.49 cfs @ 12.29 hrs, Volume= 0.39 af, Atten= 0%, Lag= 0.0 min

Primary = 3.49 cfs @ 12.29 hrs, Volume= 0.39 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 295.21' @ 12.50 hrs

Flood Elev= 294.00'

Primary OutFlow Max=3.27 cfs @ 12.29 hrs HW=293.79' TW=293.04' (Dynamic Tailwater) 1=Culvert (Inlet Controls 3.27 cfs @ 4.17 fps)

Summary for Pond 3P: EXISTING DROP INLET

Inflow Area = 41.139 ac, 8.14% Impervious, Inflow Depth > 1.80" for 25-YEAR event

Inflow = 33.99 cfs @ 12.49 hrs, Volume= 6.15 af

Outflow = 33.99 cfs @ 12.49 hrs, Volume= 6.15 af, Atten= 0%, Lag= 0.0 min

Primary = 33.99 cfs @ 12.49 hrs, Volume= 6.15 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 294.71' @ 12.49 hrs

Flood Elev= 293.75'

Device	Routing	Invert	Outlet Devices
#1	Primary	288.66'	24.0" Round Culvert
			L= 92.1' RCP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 288.66' / 284.67' S= 0.0433 '/' Cc= 0.900
			n= 0.011 Concrete pipe, straight & clean. Flow Area= 3.14 sf

Primary OutFlow Max=33.98 cfs @ 12.49 hrs HW=294.71' TW=0.00' (Dynamic Tailwater) 1=Culvert (Inlet Controls 33.98 cfs @ 10.82 fps)

Summary for Pond 4P: EXISTING CB

Inflow Area =	2.362 ac, 23.83% Impervious, Inflow	v Depth > 2.63" for 25-YEAR event
Inflow =	7.07 cfs @ 12.10 hrs, Volume=	0.52 af
Outflow =	7.07 cfs @ 12.10 hrs, Volume=	0.52 af, Atten= 0%, Lag= 0.0 min
Primary =	4.54 cfs @ 12.10 hrs, Volume=	0.49 af
Secondary =	2.53 cfs @ 12.10 hrs, Volume=	0.02 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Type III 24-hr 25-YEAR Rainfall=5.66"

Prepared by Keach-Nordstrom Associates, Inc. HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC Printed 12/6/2023 Page 71

Peak Elev= 300.38' @ 12.10 hrs

Flood Elev= 300.17'

Device	Routing	Invert	Outlet Devices
#1	Primary	297.00'	12.0" Round Culvert
	•		L= 49.1' CMP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 297.00' / 295.64' S= 0.0277 '/' Cc= 0.900
			n= 0.025 Corrugated metal, Flow Area= 0.79 sf
#2	Secondary	300.17'	24.0" x 24.0" Horiz. Orifice/Grate C= 0.600
			Limited to weir flow at low heads

Primary OutFlow Max=4.54 cfs @ 12.10 hrs HW=300.38' TW=296.37' (Dynamic Tailwater) 1=Culvert (Barrel Controls 4.54 cfs @ 5.78 fps)

Secondary OutFlow Max=2.52 cfs @ 12.10 hrs HW=300.38' TW=296.37' (Dynamic Tailwater) 2=Orifice/Grate (Weir Controls 2.52 cfs @ 1.50 fps)

Summary for Pond 5P: EXISTING CB

Inflow Area =	0.377 ac, 34.30% Impervious, Inflow	Depth > 2.87" for 25-YEAR event
Inflow =	1.16 cfs @ 12.12 hrs, Volume=	0.09 af
Outflow =	1.16 cfs @ 12.12 hrs, Volume=	0.09 af, Atten= 0%, Lag= 0.0 min
Primary =	1.16 cfs @ 12.12 hrs, Volume=	0.09 af
Secondary =	0.00 cfs @ 0.00 hrs, Volume=	0.00 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 300.56' @ 12.11 hrs Flood Elev= 301.16'

Device	Routing	Invert	Outlet Devices
#1	Primary	297.70'	12.0" Round Culvert
	,		L= 34.1' CMP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 297.70' / 297.30' S= 0.0117 '/' Cc= 0.900
			n= 0.025 Corrugated metal, Flow Area= 0.79 sf
#2	Secondary	301.16'	24.0" x 24.0" Horiz. Orifice/Grate C= 0.600
			Limited to weir flow at low heads

Primary OutFlow Max=1.19 cfs @ 12.12 hrs HW=300.55' TW=300.36' (Dynamic Tailwater) 1=Culvert (Outlet Controls 1.19 cfs @ 1.52 fps)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=297.70' TW=0.00' (Dynamic Tailwater) 2=Orifice/Grate (Controls 0.00 cfs)

Summary for Pond 6P: EXISTING CB

Inflow Area =	2.097 ac, 14.39% Impervious, Inflow	[,] Depth > ∃.04" for 25-YEAR event
Inflow =	5.12 cfs @ 12.27 hrs, Volume=	0.53 af
Outflow =	5.12 cfs @ 12.27 hrs, Volume=	0.53 af, Atten= 0%, Lag= 0.0 min
Primary =	5.12 cfs @ 12.27 hrs, Volume=	0.53 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

2304141-POST DEVELOPMENT

Type III 24-hr 25-YEAR Rainfall=5.66"

Prepared by Keach-Nordstrom Associates, Inc.
HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Printed 12/6/2023

Page 72

Peak Elev= 366.04' @ 12.27 hrs

Flood Elev= 366.35'

Device	Routing	Invert	Outlet Devices
#1	Primary	363.70'	12.0" Round Culvert
			L= 18.0' CMP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 363.70' / 362.46' S= 0.0689 '/' Cc= 0.900
			n= 0.025 Corrugated metal, Flow Area= 0.79 sf

Primary OutFlow Max=5.12 cfs @ 12.27 hrs HW=366.03' TW=0.00' (Dynamic Tailwater)

1=Culvert (Inlet Controls 5.12 cfs @ 6.52 fps)

Summary for Pond 7P: EXISTING CB

Inflow Area = 2.594 ac, 9.49% Impervious, Inflow Depth > 2.84" for 25-YEAR event

Inflow = 5.76 cfs @ 12.29 hrs, Volume= 0.61 af

Outflow = 5.76 cfs @ 12.29 hrs, Volume= 0.61 af, Atten= 0%, Lag= 0.0 min

Primary = 5.76 cfs @ 12.29 hrs, Volume= 0.61 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 379.39' @ 12.29 hrs

Flood Elev= 380.49'

Device	Routing	Invert	Outlet Devices
#1	Primary	376.57'	12.0" Round Culvert
			L= 31.9' RCP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 376.57' / 376.28' S= 0.0091 '/' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf

Primary OutFlow Max=5.76 cfs @ 12.29 hrs HW=379.39' TW=0.00' (Dynamic Tailwater) 1=Culvert (Inlet Controls 5.76 cfs @ 7.33 fps)

Summary for Pond 8P: DMH#2

Inflow Area = 11.270 ac, 12.00% Impervious, Inflow Depth > 1.12" for 25-YEAR event

Inflow = 1.13 cfs @ 17.15 hrs, Volume= 1.05 af

Outflow = 1.13 cfs @ 17.14 hrs, Volume= 1.05 af, Atten= 0%, Lag= 0.0 min

Primary = 1.13 cfs @ 17.14 hrs, Volume= 1.05 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 350.21' @ 17.14 hrs

Flood Elev= 353.05'

<u>Device</u>	Routing	Invert	Outlet Devices
#1	Primary	349.7'0'	15.0" Round Culvert
			L= 15.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 349.70' / 348.50' S= 0.0800 '/' Cc= 0.900
			n= 0.013, Flow Area= 1.23 sf

Primary OutFlow Max=1.13 cfs @ 17.14 hrs HW=350.21' TW=348.87' (Dynamic Tailwater) 1=Culvert (Inlet Controls 1.13 cfs @ 2.42 fps)

Type III 24-hr 25-YEAR Rainfall=5.66"

Prepared by Keach-Nordstrom Associates, Inc.

Printed 12/6/2023

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 73

Summary for Pond 9P: DMH#3

Inflow Area = 5.536 ac, 11.12% Impervious, Inflow Depth > 2.90" for 25-YEAR event

Inflow = 12.46 cfs @ 12.24 hrs, Volume= 1.34 af

Outflow = 12.46 cfs @ 12.24 hrs, Volume= 1.34 af, Atten= 0%, Lag= 0.0 min

Primary = 12.46 cfs @ 12.24 hrs, Volume= 1.34 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 377.15' @ 12.24 hrs

Flood Elev= 377.79'

Device	Routing	Invert	Outlet Devices
#1	Primary	374.26'	18.0" Round Culvert L= 30.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 374.26' / 374.04' S= 0.0073 '/' Cc= 0.900 n= 0.013, Flow Area= 1.77 sf

Primary OutFlow Max=12.46 cfs @ 12.24 hrs HW=377.15' TW=369.81' (Dynamic Tailwater) 1=Culvert (Inlet Controls 12.46 cfs @ 7.05 fps)

Summary for Pond 10P: DMH#4

Inflow Area = 5.536 ac, 11.12% Impervious, Inflow Depth > 2.90" for 25-YEAR event

Inflow = 12.46 cfs @ 12.24 hrs, Volume= 1.34 af

Outflow = 12.46 cfs @ 12.24 hrs, Volume= 1.34 af, Atten= 0%, Lag= 0.0 min

Primary = 12.46 cfs @ 12.24 hrs, Volume= 1.34 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 380.03' @ 12.25 hrs

Flood Elev= 381.66'

Device	Routing	Invert	Outlet Devices
#1	Primary	375.58'	18.0" Round Culvert
	-		L= 121.7' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 375.58' / 374.36' S= 0.0100 '/' Cc= 0.900
			n= 0.013, Flow Area= 1.77 sf

Primary OutFlow Max=12.44 cfs @ 12.24 hrs HW=380.02' TW=377.15' (Dynamic Tailwater) 1=Culvert (Outlet Controls 12.44 cfs @ 7.04 fps)

Summary for Pond 11P: DMH#5

Inflow Area = 5.536 ac, 11.12% Impervious, Inflow Depth > 2.90" for 25-YEAR event

Inflow = 12.46 cfs @ 12.24 hrs, Volume= 1.34 af

Outflow = 12.46 cfs @ 12.24 hrs, Volume= 1.34 af, Atten= 0%, Lag= 0.0 min

Primary = 12.46 cfs @ 12.24 hrs, Volume= 1.34 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 384.37' @ 12.25 hrs

Flood Elev= 384.00'

2304141-POST DEVELOPMENT

Type III 24-hr 25-YEAR Rainfall=5.66"

Prepared by Keach-Nordstrom Associates, Inc.
HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Printed 12/6/2023

Page 74

Primary OutFlow Max=12.42 cfs @ 12.24 hrs HW=384.34' TW=380.02' (Dynamic Tailwater) 1=Culvert (Outlet Controls 12.42 cfs @ 7.03 fps)

Summary for Pond 12P: CB#6

Inflow Area = 5.536 ac, 11.12% Impervious, Inflow Depth > 2.90" for 25-YEAR event

Inflow = 12.46 cfs @ 12.24 hrs, Volume= 1.34 af

Outflow = 12.46 cfs @ 12.24 hrs, Volume= 1.34 af, Atten= 0%, Lag= 0.0 min

Primary = 12.46 cfs @ 12.24 hrs, Volume= 1.34 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 387.50' @ 12.26 hrs

Flood Elev= 384.30'

| Device Routing | Invert Outlet Devices | 379.45' | 18.0" Round Culvert | L= 141.2' CPP, square edge headwall, Ke= 0.500 | Inlet / Outlet Invert= 379.45' / 378.04' S= 0.0100 '/' Cc= 0.900 | n= 0.013, Flow Area= 1.77 sf

Primary OutFlow Max=12.32 cfs @ 12.24 hrs HW=387.43' TW=384.34' (Dynamic Tailwater) 1=Culvert (Outlet Controls 12.32 cfs @ 6.97 fps)

Summary for Pond 13P: CB#7

Inflow Area = 5.201 ac, 9.22% Impervious, Inflow Depth > 2.84" for 25-YEAR event

Inflow = 11.74 cfs @ 12.25 hrs, Volume= 1.23 af

Outflow = 11.74 cfs @ 12.25 hrs, Volume= 1.23 af, Atten= 0%, Lag= 0.0 min

Primary = 11.74 cfs @ 12.25 hrs, Volume= 1.23 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 391.42' @ 12.26 hrs

Flood Elev= 384.30'

Device	Routing	Invert	Outlet Devices
#1	Primary	380.25'	15.0" Round Culvert
			L= 30.3' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 380.25' / 379.95' S= 0.0099 '/' Cc= 0.900
			n= 0.013, Flow Area= 1.23 sf

Primary OutFlow Max=11.61 cfs @ 12.25 hrs HW=391.34' TW=387.48' (Dynamic Tailwater)

1::Culvert (Inlet Controls 11.61 cfs @ 9.46 fps)

Type III 24-hr 25-YEAR Rainfall=5.66"

Prepared by Keach-Nordstrom Associates, Inc.

Printed 12/6/2023

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 75

Summary for Pond 14P: CB#8

Inflow Area = 4.328 ac, 9.12% Impervious, Inflow Depth > 2.84" for 25-YEAR event

Inflow = 9.64 cfs @ 12.25 hrs, Volume= 1.02 af

Outflow = 9.64 cfs @ 12.25 hrs, Volume= 1.02 af, Atten= 0%, Lag= 0.0 min

Primary = 9.64 cfs @ 12.25 hrs, Volume= 1.02 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 396.52' @ 12.27 hrs

Flood Elev= 388.51'

| Device Routing | Invert Outlet Devices | 384.49' | 15.0" | Round Culvert | L= 165.7' | CPP, square edge headwall, Ke= 0.500 | Inlet / Outlet Invert= 384.49' / 380.35' | S= 0.0250 '/' | Cc= 0.900 | n= 0.013. | Flow Area= 1.23 sf

Primary OutFlow Max=9.50 cfs @ 12.25 hrs HW=396.35' TW=391.36' (Dynamic Tailwater)
1=Culvert (Outlet Controls 9.50 cfs @ 7.74 fps)

Summary for Pond 16P: CB#10

Inflow Area = 3.078 ac, 10.13% Impervious, Inflow Depth > 2.87" for 25-YEAR event

Inflow = 6.74 cfs @ 12.27 hrs, Volume= 0.74 af

Outflow = 6.74 cfs @ 12.27 hrs, Volume= 0.74 af, Atten= 0%, Lag= 0.0 min

Primary = 6.74 cfs @ 12.27 hrs, Volume= 0.74 af

Routing by Dyn-Stor-Ind method. Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 404.14' @ 12.28 hrs

Flood Elev= 397.31'

Device	Routing	Invert	Outlet Devices
#1	Primary	392.11'	12.0" Round Culvert
	-		L= 165.5' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 392.11' / 383.83' S= 0.0500 '/' Cc= 0.900
			n= 0.013, Flow Area= 0.79 sf

Primary OutFlow Max=6.70 cfs @ 12.27 hrs HW=404.08' TW=396.51' (Dynamic Tailwater) 1=Culvert (Outlet Controls 6.70 cfs @ 8.53 fps)

Summary for Pond 17P: CB#100

Inflow Area = 0.183 ac, 70.27% Impervious, Inflow Depth > 4.61" for 25-YEAR event

Inflow = 0.94 cfs @ 12.08 hrs, Volume= 0.07 af

Outflow = 0.94 cfs @ 12.08 hrs, Volume= 0.07 af, Atten= 0%, Lag= 0.0 min

Primary = 0.94 cfs @ 12.08 hrs, Volume= 0.07 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 404.15' @ 12.29 hrs

Flood Elev= 397.31'

2304141-POST DEVELOPMENT

Type III 24-hr 25-YEAR Rainfall=5.66"

Prepared by Keach-Nordstrom Associates, Inc.
HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Printed 12/6/2023

Page 76

Device Routing Invert Outlet Devices

#1 Primary 393.25' 12.0" Round Culvert

L= 26.0' CPP, square edge headwall, Ke= 0.500
Inlet / Outlet Invert= 393.25' / 392.73' S= 0.0200 '/' Cc= 0.900

Primary OutFlow Max=0.53 cfs @ 12.08 hrs HW=393.97' TW=393.92' (Dynamic Tailwater) 1=Culvert (Outlet Controls 0.53 cfs @ 1.22 fps)

n= 0.013, Flow Area= 0.79 sf

Summary for Pond 18P: CB#11

Inflow Area = 1.524 ac, 2.68% Impervious, Inflow Depth > 2.65" for 25-YEAR event

Inflow = 3.14 cfs @ 12.29 hrs, Volume= 0.34 af

Outflow = 3.14 cfs @ 12.29 hrs, Volume= 0.34 af, Atten= 0%, Lag= 0.0 min

Primary = 3.14 cfs @ 12.29 hrs, Volume= 0.34 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 405.82' @ 12.29 hrs

Flood Elev= 406.88'

| Device | Routing | Invert | Outlet Devices | | 402.80' | 12.0" | Round Culvert | L= 167.8' | CPP, square edge headwall, Ke= 0.500 | Inlet / Outlet Invert= 402.80' / 392.73' | S= 0.0600 '/' | Cc= 0.900 | n= 0.013, Flow Area= 0.79 sf

Primary OutFlow Max=3.29 cfs @ 12.29 hrs HW=405.77' TW=403.92' (Dynamic Tailwater) 1=Culvert (Outlet Controls 3.29 cfs @ 4.19 fps)

Summary for Pond 20P: DMH#14

Inflow Area = 3.830 ac, 17.06% Impervious, Inflow Depth > 3.06" for 25-YEAR event

Inflow = 8.49 cfs @ 12.33 hrs, Volume= 0.98 af

Outflow = 8.49 cfs @ 12.33 hrs, Volume= 0.98 af, Atten= 0%, Lag= 0.0 min

Primary = 8.49 cfs @ 12.33 hrs. Volume= 0.98 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs. dt= 0.01 hrs.

Peak Elev= 376.95' @ 12.33 hrs

Flood Elev= 383.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	375.20'	18.0" Round Culvert
			L= 15.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 375.20' / 374.00' S= 0.0800 '/' Cc= 0.900
			n= 0.013. Flow Area= 1.77 sf

Primary OutFlow Max=8.49 cfs @ 12.33 hrs HW=376.95' TW=370.32' (Dynamic Tailwater)
1=Culvert (Inlet Controls 8.49 cfs @ 4.80 fps)

Type III 24-hr 25-YEAR Rainfall=5.66"

Prepared by Keach-Nordstrom Associates, Inc.

Printed 12/6/2023

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 77

Summary for Pond 21P: DMH#15

Inflow Area = 3.830 ac, 17.06% Impervious, Inflow Depth > 3.06" for 25-YEAR event

Inflow = 8.49 cfs @ 12.33 hrs, Volume= 0.98 af

Outflow = 8.49 cfs @ 12.33 hrs, Volume= 0.98 af, Atten= 0%, Lag= 0.0 min

Primary = 8.49 cfs @ 12.33 hrs, Volume= 0.98 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 383.21' @ 12.33 hrs

Flood Elev= 387.85'

Device	Routing	Invert	Outlet Devices
#1	Primary	381.46'	18.0" Round Culvert L= 30.7' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 381.46' / 379.00' S= 0.0801 '/' Cc= 0.900 n= 0.013, Flow Area= 1.77 sf

Primary OutFlow Max=8.49 cfs @ 12.33 hrs HW=383.21' TW=376.95' (Dynamic Tailwater) 1=Culvert (Inlet Controls 8.49 cfs @ 4.80 fps)

Summary for Pond 22P: DMH#16

Inflow Area = 3.830 ac, 17.06% Impervious, Inflow Depth > 3.06" for 25-YEAR event

Inflow = 8.49 cfs @ 12.33 hrs, Volume= 0.98 af

Outflow = 8.49 cfs @ 12.33 hrs, Volume= 0.98 af, Atten= 0%, Lag= 0.0 min

Primary = 8.49 cfs @ 12.33 hrs, Volume= 0.98 af

Routing by Dvn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 394.67' @ 12.33 hrs

Flood Elev= 400.42'

Device	Routing	Invert	Outlet Devices	
#1	Primary	392.92'	18.0" Round Culvert	
	_		L= 111.5' CPP, square edge headwall, Ke= 0.500	
			Inlet / Outlet Invert= 392.92' / 384.00' S= 0.0800 '/' Cc= 0.900	
			n= 0.013, Flow Area= 1.77 sf	

Primary OutFlow Max=8.49 cfs @ 12.33 hrs HW=394.67' TW=383.21' (Dynamic Tailwater) 1=Culvert (Inlet Controls 8.49 cfs @ 4.80 fps)

Summary for Pond 23P: DMH#17

Inflow Area = 3.830 ac, 17.06% Impervious, Inflow Depth > 3.06" for 25-YEAR event

Inflow = 8.49 cfs @ 12.33 hrs, Volume= 0.98 af

Outflow = 8.49 cfs @ 12.33 hrs, Volume= 0.98 af, Atten= 0%, Lag= 0.0 min

Primary = 8.49 cfs @ 12.33 hrs, Volume= 0.98 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 402.88' @ 12.33 hrs

Flood Elev= 409.83'

2304141-POST DEVELOPMENT

Type III 24-hr 25-YEAR Rainfall=5.66"

Prepared by Keach-Nordstrom Associates, Inc. HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Printed 12/6/2023

Page 78

Device	Routing	Invert	Outlet Devices
#1	Primary	401.13'	18.0" Round Culvert
			L= 45.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 401.13' / 397.53' S= 0.0800 '/' Cc= 0.900
			n= 0.013, Flow Area= 1.77 sf

Primary OutFlow Max=8.49 cfs @ 12.33 hrs HW=402.88' TW=394.67' (Dynamic Tailwater) 1=Culvert (Inlet Controls 8.49 cfs @ 4.80 fps)

Summary for Pond 24P: CB#18

Inflow Area = 3.830 ac, 17.06% Impervious, Inflow Depth > 3.06" for 25-YEAR event Inflow = 8.49 cfs @ 12.33 hrs, Volume= 0.98 af

Outflow = 8.49 cfs @ 12.33 hrs, Volume= 0.98 af, Atten= 0%, Lag= 0.0 min

Primary = 8.49 cfs @ 12.33 hrs, Volume= 0.98 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 404.62' @ 12.33 hrs

Flood Elev= 407.95'

Primary OutFlow Max=8.49 cfs @ 12.33 hrs HW=404.62' TW=402.88' (Dynamic Tailwater) 1=Culvert (Inlet Controls 8.49 cfs @ 4.80 fps)

Summary for Pond 25P: CB#19

Inflow Area = 2.259 ac, 23.36% Impervious, Inflow Depth > 3.24" for 25-YEAR event

Inflow = 5.21 cfs @ 12.33 hrs, Volume= 0.61 af

Outflow = 5.21 cfs @ 12.33 hrs, Volume= 0.61 af, Atten= 0%, Lag= 0.0 min

Primary = 5.21 cfs @ 12.33 hrs, Volume= 0.61 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 405.39' @ 12.33 hrs

Flood Elev= 408.04'

Device	Routing	Invert	Outlet Devices
#1	Primary	403.34'	15.0" Round Culvert
			L= 36.5' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 403.34' / 402.97' S= 0.0101 '/' Cc= 0.900
			n= 0.013, Flow Area= 1.23 sf

Primary OutFlow Max=5.20 cfs @ 12.33 hrs HW=405.39' TW=404.62' (Dynamic Tailwater)
1=Culvert (Inlet Controls 5.20 cfs @ 4.24 fps)

2304141-POST DEVELOPMENT

Type III 24-hr 25-YEAR Rainfall=5.66"

Prepared by Keach-Nordstrom Associates, Inc.

Printed 12/6/2023

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 79

Summary for Pond 26P: CB#20

Inflow Area = 0.702 ac, 34.65% Impervious, Inflow Depth > 3.52" for 25-YEAR event

Inflow = 1.95 cfs @ 12.28 hrs, Volume= 0.21 af

Outflow = 1.95 cfs @ 12.28 hrs, Volume= 0.21 af, Atten= 0%, Lag= 0.0 min

Primary = 1.95 cfs @ 12.28 hrs, Volume= 0.21 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 405.63' @ 12.34 hrs

Flood Elev= 407.78'

Device	Routing	Invert	Outlet Devices	
#1	Primary	403.70'	2.0" Round Culvert	
	-		L= 26.0' CPP, square edge headwall, Ke= 0.500	
			Inlet / Outlet Invert= 403.70' / 403.44' S= 0.0100 '/' Cc= 0.900	
			n= 0.013, Flow Area= 0.79 sf	

Primary OutFlow Max=1.78 cfs @ 12.28 hrs HW=405.49' TW=405.27' (Dynamic Tailwater) 1=Culvert (Inlet Controls 1.78 cfs @ 2.26 fps)

Summary for Pond PP: POCKET POND

Inflow Area =	11.270 ac, 12.00% Impervious, Inflow De	epth > 2.96" for 25-YEAR event
Inflow =	25.01 cfs @ 12.25 hrs, Volume=	2.78 af
Outflow =	1.13 cfs @ 17.15 hrs, Volume=	1.05 af, Atten= 95%, Lag= 293.8 min
Primary =	1.13 cfs @ 17.15 hrs, Volume=	1.05 af
Secondary =	0.00 cfs @ 0.00 hrs, Volume=	0.00 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs
Starting Elev= 367.00' Surf.Area= 9,015 sf Storage= 7,477 cf
Peak Elev= 372.81' @ 17.15 hrs Surf.Area= 22,341 sf Storage= 92,034 cf (84,557 cf above start)
Flood Elev= 374.00' Surf.Area= 24,964 sf Storage= 118,753 cf (111,276 cf above start)

Plug-Flow detention time= 437.0 min calculated for 0.88 af (32% of inflow) Center-of-Mass det. time= 248.0 min (1,086.1 - 838.0)

Volume	Invert	Avail.Storage	Storage Description
#1	364.00'	118,753 cf	Custom Stage Data (Prismatic) Listed below (Recalc)
#2	369.00'	0 cf	Sediment Forebay (No Storage) (Prismatic) Listed below (Recalc)
2			1,678 cf Overall x 0.0% Voids
		110 752 of	Total Available Storage

118,753 cf Total Available Storage

Type III 24-hr 25-YEAR Rainfall=5.66"

Prepared by Keach-Nordstrom Associates, Inc.

Printed 12/6/2023

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 80

Elevation	Surf.Area	Inc.Store	Cum.Store
(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)
364.00	145	0	0
366.00	1,883	2,028	2,028
367.00	9,015	5,449	7,477
368.00	10,563	9,789	17,266
369.00	12,205	11,384	28,650
370.00	13,939	13,072	41,722
371.00	17,278	15,609	57,331
372.00	19,350	18,314	75,645
374.00	23,758	43,108	118,753
Elevation	Surf.Area	Inc.Store	Cum.Store
(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)
369.00	472	0	0
371.00	1,206	1,678	1,678

Device	Routing	Invert	Outlet Devices	
#1	Primary	365.00'	15.0" Round Culvert	
			L= 190.0' CPP, square edge headwall, Ke= 0.500	
			Inlet / Outlet Invert= 365.00' / 349.80' S= 0.0800 '/' Cc= 0.900	
			n= 0.013, Flow Area= 1.23 sf	
#2	Device 1	367.00'	2.5" Vert. Orifice C= 0.600	
#3	Device 1	369.45'	3.2" Vert. Orifice C= 0.600	
#4	Device 1	371.50'	3.0" Vert. Orifice C= 0.600	
#5	Device 1	373.70'	48.0" x 48.0" Horiz. Grate C= 0.600	
			Limited to weir flow at low heads	
#6	Secondary	373.80'	10.0' long x 27.0' breadth Broad-Crested Rectangular Weir	
			Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60	
			Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63	

Primary OutFlow Max=1.13 cfs @ 17.15 hrs HW=372.81' TW=350.21' (Dynamic Tailwater) -1=Culvert (Passes 1.13 cfs of 15.84 cfs potential flow)

-2=Orifice (Orifice Controls 0.39 cfs @ 11.50 fps) -3=Orifice (Orifice Controls 0.48 cfs @ 8.65 fps)

-4=Orifice (Orifice Controls 0.26 cfs @ 5.24 fps)

-5=Grate (Controls 0.00 cfs)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=367.00' TW=348.50' (Dynamic Tailwater) 6=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Link A: BARRETTS HILL CROSSING

Inflow Area = 41.139 ac, 8.14% Impervious, Inflow Depth > 1.80" for 25-YEAR event

Inflow 33.99 cfs @ 12.49 hrs, Volume= 6.15 af

Primary 33.99 cfs @ 12.49 hrs, Volume= 6.15 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

2304141-POST DEVELOPMENT

Type III 24-hr 25-YEAR Rainfall=5.66"

Prepared by Keach-Nordstrom Associates, Inc.

Printed 12/6/2023

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 81

Summary for Link B: LOT 57

Inflow Area = 2.097 ac, 14.39% Impervious, Inflow Depth > 3.04" for 25-YEAR event

Inflow = 5.12 cfs @ 12.27 hrs, Volume= 0.53 af

Primary = 5.12 cfs @ 12.27 hrs, Volume= 0.53 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Summary for Link C: LOT 11

Inflow Area = 2.594 ac, 9.49% Impervious, Inflow Depth > 2.84" for 25-YEAR event

Inflow = 5.76 cfs @ 12.29 hrs, Volume= 0.61 af

Primary = 5.76 cfs @ 12.29 hrs, Volume= 0.61 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

2304141-POST DEVELOPMENT

Type III 24-hr 50-YEAR Rainfall=6.77"

Prepared by Keach-Nordstrom Associates, Inc.

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Printed 12/6/2023 Page 82

Time span=0.00-24.00 hrs, dt=0.01 hrs, 2401 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment 1S: SUBCATCHMENT	Runoff Area=961,091 sf 4.08% Impervious Runoff Depth>2.86" Flow Length=2,203' Tc=38.1 min CN=64.6 Runoff=37.01 cfs 5.26 af
Subcatchment 2S: SUBCATCHMENT	Runoff Area=113,081 sf 12.76% Impervious Runoff Depth>2.56" Flow Length=683' Tc=20.1 min CN=61.4 Runoff=5.12 cfs 0.55 af
Subcatchment 3S: SUBCATCHMENT	Runoff Area=124,041 sf 7.14% Impervious Runoff Depth>2.57" Flow Length=1,189' Tc=23.1 min CN=61.5 Runoff=5.32 cfs 0.61 af
Subcatchment 4S: SUBCATCHMENT	Runoff Area=86,429 sf 21.84% Impervious Runoff Depth>3.48" Tc=6.0 min CN=70.6 Runoff=8.11 cfs 0.58 af
Subcatchment 5S: SUBCATCHMENT	Runoff Area=16,441 sf 34.30% Impervious Runoff Depth>3.80" Flow Length=389' Tc=8.7 min CN=73.7 Runoff=1.54 cfs 0.12 af
Subcatchment 6S: SUBCATCHMENT	Runoff Area=91,362 sf 14.39% Impervious Runoff Depth>4.00" Flow Length=692' Tc=19.5 min CN=75.7 Runoff=6.74 cfs 0.70 af
Subcatchment 7S: SUBCATCHMENT	Runoff Area=112,988 sf 9.49% Impervious Runoff Depth>3.77" Flow Length=1,021' Tc=20.7 min CN=73.5 Runoff=7.67 cfs 0.82 af
Subcatchment 8S: SUBCATCHMENT	Runoff Area=30,577 sf 34.65% Impervious Runoff Depth>4.53" Flow Length=719' Tc=20.3 min CN=80.6 Runoff=2.50 cfs 0.27 af
Subcatchment 9S: SUBCATCHMENT	Runoff Area=67,824 sf 18.27% Impervious Runoff Depth>4.07" Flow Length=1,293' Tc=26.9 min CN=76.4 Runoff=4.44 cfs 0.53 af
Subcatchment 10S: SUBCATCHMENT	Runoff Area=68,434 sf 8.00% Impervious Runoff Depth>3.72" Flow Length=676' Tc=23.0 min CN=73.0 Runoff=4.37 cfs 0.49 af
Subcatchment 11S: SUBCATCHMENT	Runoff Area=66,396 sf 2.68% Impervious Runoff Depth>3.55" Flow Length=690' Tc=20.7 min CN=71.4 Runoff=4.24 cfs 0.45 af
Subcatchment 12S: SUBCATCHMENT	Runoff Area=59,686 sf 10.36% Impervious Runoff Depth>3.82" Flow Length=753' Tc=18.7 min CN=73.9 Runoff=4.27 cfs 0.44 af
Subcatchment 13S: SUBCATCHMENT	Runoff Area=54,469 sf 6.63% Impervious Runoff Depth>3.69" Flow Length=680' Tc=16.3 min CN=72.6 Runoff=3.97 cfs 0.38 af
Subcatchment 14S: SUBCATCHMENT	Runoff Area=38,021 sf 9.74% Impervious Runoff Depth>3.80" Flow Length=731' Tc=17.2 min CN=73.7 Runoff=2.80 cfs 0.28 af
Subcatchment 15S: SUBCATCHMENT	Runoff Area=14,603 sf 40.57% Impervious Runoff Depth>4.89' Tc=6.0 min CN=83.7 Runoff=1.88 cfs 0.14 af

2304141-POST DEVELO Prepared by Keach-Nords HydroCAD® 10.00-26 s/n 032		Type III 24-hr 50-YEAR Rainfall=6.77" Printed 12/6/2023 ions LLC Page 83
Subcatchment 17S: SUBCA		26 sf 4.39% Impervious Runoff Depth>3.85" 13.2 min CN=74.2 Runoff=6.86 cfs 0.61 af
Reach 1R: EXISTING ROAD		2' Max Vel=4.72 fps Inflow=9.56 cfs 0.70 af Capacity=16.21 cfs Outflow=9.48 cfs 0.69 af
Reach 2R: EXISTING ROAD		4' Max Vel=2.36 fps Inflow=2.71 cfs 1.49 af Capacity=12.04 cfs Outflow=2.70 cfs 1.48 af
Pond 1P: EXISTING CB		Inflow=37.01 cfs 5.26 af Primary=37.01 cfs 5.26 af
Pond 2P: EXISTING DRIVEY		Peak Elev=300.88' Inflow=5.12 cfs 0.55 af =21.0' S=0.0143 '/' Outflow=5.12 cfs 0.55 af
Pond 3P: EXISTING DROP I		Peak Elev=299.86' Inflow=48.31 cfs 8.59 af 92.1' S=0.0433 '/' Outflow=48.31 cfs 8.59 af
Pond 4P: EXISTING CB	Primary=4.61 cfs 0.63 af Secondar	Peak Elev=300.50' Inflow=9.56 cfs 0.70 af ry=4.94 cfs 0.06 af Outflow=9.56 cfs 0.70 af
Pond 5P: EXISTING CB	Primary=1.54 cfs 0.12 af Secondar	Peak Elev=300.82' Inflow=1.54 cfs 0.12 af ry=0.00 cfs 0.00 af Outflow=1.54 cfs 0.12 af
Pond 6P: EXISTING CB	12.0" Round Culvert n=0.025 L	Peak Elev=367.57' Inflow=6.74 cfs 0.70 af =18.0' S=0.0689 '/' Outflow=6.74 cfs 0.70 af
Pond 7P: EXISTING CB	12.0" Round Culvert n=0.011 L	Peak Elev=381.18' Inflow=7.67 cfs 0.82 af =31.9' S=0.0091 '/' Outflow=7.67 cfs 0.82 af
Pond 8P: DMH#2	15.0" Round Culvert n=0.013 L	Peak Elev=350.54' Inflow=2.71 cfs 1.49 af =15.0' S=0.0800 '/' Outflow=2.71 cfs 1.49 af
Pond 9P: DMH#3	18.0" Round Culvert n=0.013 L=	Peak Elev=378.80' Inflow=16.57 cfs 1.77 af 30.0' S=0.0073 '/' Outflow=16.57 cfs 1.77 af
Pond 10P: DMH#4	18.0" Round Culvert n=0.013 L=1	Peak Elev=383.88' Inflow=16.57 cfs 1.77 af 21.7' S=0.0100 '/' Outflow=16.57 cfs 1.77 af
Pond 11P: DMH#5	18.0" Round Culvert n=0.013 L=2	Peak Elev=391.55' Inflow=16.57 cfs 1.77 af 26.1' S=0.0100 '/' Outflow=16.57 cfs 1.77 af
Pond 12P: CB#6	18.0" Round Culvert n=0.013 L=1	Peak Elev=397.09' Inflow=16.57 cfs 1.77 af 41.2' S=0.0100 '/' Outflow=16.57 cfs 1.77 af
Pond 13P: CB#7	15.0" Round Culvert n=0.013 L=	Peak Elev=404.06' Inflow=15.66 cfs 1.63 af 30.3' S=0.0099 '/' Outflow=15.66 cfs 1.63 af
Pond 14P: CB#8	15.0" Round Culvert n=0.013 L=1	Peak Elev=413.14' Inflow=12.87 cfs 1.36 af 65.7' S=0.0250 '/' Outflow=12.87 cfs 1.36 af

Inflow=6.74 cfs 0.70 af Primary=6.74 cfs 0.70 af

Inflow=7.67 cfs 0.82 af Primary=7.67 cfs 0.82 af

2304141-POST DEVELOPTED Prepared by Keach-Nords HydroCAD® 10.00-26 s/n 03		Type III 24-hr 50-YEAR Rainfall=6.77° Printed 12/6/2023 utions LLC Page 84
Pond 16P: CB#10		Peak Elev=426.70' Inflow=8.98 cfs 0.97 at =165.5' S=0.0500 '/' Outflow=8.98 cfs 0.97 at
Pond 17P: CB#100	12.0" Round Culvert n=0.013	Peak Elev=426.71' Inflow=1.15 cfs 0.09 at L=26.0' S=0.0200 '/' Outflow=1.15 cfs 0.09 at
Pond 18P: CB#11	12.0" Round Culvert n=0.013 L	Peak Elev=429.75' Inflow=4.24 cfs 0.45 at .=167.8' S=0.0600 '/' Outflow=4.24 cfs 0.45 at
Pond 20P: DMH#14	18.0" Round Culvert n=0.013 L	Peak Elev=377.67' Inflow=11.16 cfs 1.28 at .=15.0' S=0.0800 '/' Outflow=11.16 cfs 1.28 at
Pond 21P: DMH#15	18.0" Round Culvert n=0.013 L	Peak Elev=383.93' Inflow=11.16 cfs 1.28 af =30.7' S=0.0801 '/' Outflow=11.16 cfs 1.28 af
Pond 22P: DMH#16	18.0" Round Culvert n=0.013 L=	Peak Elev=395.39' Inflow=11.16 cfs 1.28 af 111.5' S=0.0800 '/' Outflow=11.16 cfs 1.28 af
Pond 23P: DMH#17	18.0" Round Culvert n=0.013 L	Peak Elev=403.60' Inflow=11.16 cfs 1.28 af =45.0' S=0.0800 '/' Outflow=11.16 cfs 1.28 af
Pond 24P: CB#18	18.0" Round Culvert n=0.013 L=	Peak Elev=406.38' Inflow=11.16 cfs 1.28 af 164.2' S=0.0100 '/' Outflow=11.16 cfs 1.28 af
Pond 25P: CB#19	15.0" Round Culvert n=0.013	Peak Elev=407.69' Inflow=6.78 cfs 0.79 af L=36.5' S=0.0101 '/' Outflow=6.78 cfs 0.79 af
Pond 26P: CB#20	12.0" Round Culvert n=0.013	Peak Elev=408.08' Inflow=2.50 cfs 0.27 af L=26.0' S=0.0100 '/' Outflow=2.50 cfs 0.27 af
Pond PP: POCKET POND		Storage=113,786 cf Inflow=33.13 cfs 3.66 af ary=0.00 cfs 0.00 af Outflow=2.71 cfs 1.49 af
Link A: BARRETTS HILL CI	ROSSING	Inflow=48.31 cfs 8.59 af Primary=48.31 cfs 8.59 af

Total Runoff Area = 45.830 ac Runoff Volume = 12.29 af Average Runoff Depth = 3.22" 91.49% Pervious = 41.932 ac 8.51% Impervious = 3.898 ac

Link B: LOT 57

Link C: LOT 11

Type III 24-hr 50-YEAR Rainfall=6.77"

Prepared by Keach-Nordstrom Associates, Inc.

Printed 12/6/2023

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 85

Summary for Subcatchment 1S: SUBCATCHMENT

Runoff = 37.01 cfs @ 12.57 hrs, Volume=

5.26 af, Depth> 2.86"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 50-YEAR Rainfall=6.77"

-	A	rea (sf)	CN	Description	Description					
		7,670	30.0	Woods, G	Good, HSG	A				
	3	63,085	55.0	Woods, G	Good, HSG	В				
	4	27,890	70.0	Woods, G	Good, HSG	C				
		6,684	39.0	>75% Gra	ass cover, (Good, HSG A				
		61,618	61.0	>75% Gra	75% Grass cover, Good, HSG B					
		54,927	74.0	>75% Gra	ass cover, (Good, HSG C				
*		21,604	98.0	Roofs						
*		17,613	98.0	Paved pa	rking					
	9	61,091	64.6	Weighted	Average					
	921,874 63.2			95.92% Pervious Area						
		39,217	98.0	4.08% Impervious Area						
	Tc	Length	Slope	Velocity	Capacity	Description				
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
	17.3	100	0.0400	0.10		Sheet Flow,				
						Woods: Light underbrush n= 0.400 P2= 2.84"				
	19.7	1,862	0.0988	1.57		Shallow Concentrated Flow,				
						Woodland Kv= 5.0 fps				
	0.3	30	0.0800	1.98		Shallow Concentrated Flow,				
						Short Grass Pasture Kv= 7.0 fps				
	8.0	211	0.0430	4.21		Shallow Concentrated Flow,				
						Paved Kv= 20.3 fps				
	38.1	2,203	Total							

IOlai

Summary for Subcatchment 2S: SUBCATCHMENT

Runoff = 5.12 cfs @ 12.29 hrs, Volume=

0.55 af, Depth> 2.56"

	Area (sf)	CN	Description
	28,414	30.0	Woods, Good, HSG A
	11,383	55.0	Woods, Good, HSG B
	33,808	70.0	Woods, Good, HSG C
	4,719	39.0	>75% Grass cover, Good, HSG A
	242	61.0	>75% Grass cover, Good, HSG B
	20,087	74.0	>75% Grass cover, Good, HSG C
*	3,849	98.0	Roofs
*	10,579	98.0	Paved parking
	113,081	61.4	Weighted Average
	98,653	56.1	87.24% Pervious Area
	14,428	98.0	12.76% Impervious Area

2304141-POST DEVELOPMENT

Type III 24-hr 50-YEAR Rainfall=6.77"

Prepared by Keach-Nordstrom Associates, Inc.

Printed 12/6/2023

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 86

Tc	Length	Slope	Velocity	Capacity	Description
(min)_	(feet)	(ft/ft)	(ft/sec)	(cfs)	
12.0	100	0.1000	0.14		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 2.84"
3.9	318	0.0750	1.37		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
0.7	48	0.0300	1.21		Shallow Concentrated Flow,
	0.4-				Short Grass Pasture Kv= 7.0 fps
3.6	217	0.0410	1.01		Shallow Concentrated Flow,
-					Woodland Kv= 5.0 fps
20.1	683	Total			

Summary for Subcatchment 3S: SUBCATCHMENT

Runoff = 5.32 cfs @ 12.34 hrs, Volume=

0.61 af, Depth> 2.57"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 50-YEAR Rainfall=6.77"

	Area (sf)	CN	Description	Description						
5,732 30.0 Woods, Good, HSG A					A					
	49,946	55.0		Good, HSG						
	30,651	70.0	Woods, G	Good, HSG	C					
	4,799	39.0	>75% Gra	ass cover, (Good, HSG A					
	20,628	61.0			Good, HSG B					
	3,428	74.0	>75% Gra	ass cover, (Good, HSG C					
*	2,833	98.0	Roofs							
*	6,024	98.0	Paved pa	rking						
•	124,041 61.5			Weighted Average						
	115,184	58.7	92.86% Pervious Area							
	8,857	98.0	7.14% Impervious Area							
Тс	Length	Slope	\/alaaitr	Canasitu	Description					
(min)	(feet)	9 1 3	Velocity	Capacity	Description					
		(ft/ft)	(ft/sec)	(cfs)						
9.9	100	0.1600	0.17		Sheet Flow,					
40.0	4.000	0.0700			Woods: Light underbrush n= 0.400 P2= 2.84"					
13.2	1,089	0.0760	1.38		Shallow Concentrated Flow,					
					Woodland Kv= 5.0 fps					
23.1	1.189	Total								

Summary for Subcatchment 4S: SUBCATCHMENT

Runoff = 8.11 cfs @ 12.09 hrs, Volume=

0.58 af, Depth> 3.48"

2304141-POST DEVELOPMENT

Type III 24-hr 50-YEAR Rainfall=6.77"

Prepared by Keach-Nordstrom Associates, Inc. HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Printed 12/6/2023

Page 87

Α	rea (sf)	CN	Description						
	10,510	55.0	Woods, Good, HSG B						
	6,756	70.0	Woods, Good, HSG C						
	40,259	61.0	>75% Grass cover, Good, HSG B						
	10,029	74.0	75% Grass cover, Good, HSG C						
	16,327	98.0	Paved parking, HSG C						
*	2,548	98.0	Roofs						
	86,429	70.6	Weighted Average						
	67,554	62.9	78.16% Pervious Area						
	18,875	98.0	21.84% Impervious Area						
Tc	Length	Slope	Velocity Capacity Description						
(min)	(feet)	(ft/ft)	(ft/sec) (cfs)						
6.0			Direct Entry,						

Summary for Subcatchment 5S: SUBCATCHMENT

Runoff = 1.54 cfs @ 12.12 hrs, Volume=

0.12 af, Depth> 3.80"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 50-YEAR Rainfall=6.77"

-	Α	rea (sf)	CN	Description	n						
		10,801	61.0	>75% Gra	75% Grass cover, Good, HSG B						
*		5,640	98.0	Paved pa	aved parking						
-		16,441	73.7	Weighted	Average						
		10,801	61.0	65.70% P	ervious Are	ea					
		5,640	98.0	34.30% Ir	npervious A	Area					
	Tc	Length	Slope	Velocity	Capacity	Description					
8=	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)						
	6.3	100	0.0700	0.26		Sheet Flow,					
						Grass: Short n= 0.150 P2= 2.84"					
	0.1	16	0.0400	4.06		Shallow Concentrated Flow,					
						Paved Kv= 20.3 fps					
	1.5	162	0.0680	1.83		Shallow Concentrated Flow,					
						Short Grass Pasture Kv= 7.0 fps					
	0.1	20	0.0400	4.06		Shallow Concentrated Flow,					
						Paved Kv= 20.3 fps					
	0.8	91	0.0770	1.94		Shallow Concentrated Flow,					
0						Short Grass Pasture Kv= 7.0 fps					
	8.7	389	Total								

Summary for Subcatchment 6S: SUBCATCHMENT

Runoff = 6.74 cfs @ 12.26 hrs, Volume=

0.70 af, Depth> 4.00"

2304141-POST DEVELOPMENT

Type III 24-hr 50-YEAR Rainfall=6.77"

Prepared by Keach-Nordstrom Associates, Inc.

Printed 12/6/2023

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 88

_	Α	rea (sf)	CN	Description	on						
		42,274	70.0	Woods, Good, HSG C							
		35,721	74.0	>75% Gra	75% Grass cover, Good, HSG C						
*		8,350	98.0	Paved pa	rking						
*		4,800	98.0	Roofs							
		217	96.0	Gravel su	rface, HSG	G C					
		91,362	75.7	Weighted	Average						
		78,212	71.9		ervious Are	ea					
		13,150	98.0	14.39% Ir	npervious A	Area					
	Tc	Length	Slope	Velocity	Capacity	Description					
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)						
	15.8	100	0.0500	0.11		Sheet Flow,					
						Woods: Light underbrush n= 0.400 P2= 2.84"					
	1.0	80	0.0750	1.37		Shallow Concentrated Flow,					
						Woodland Kv= 5.0 fps					
	0.2	18	0.0400	1.40		Shallow Concentrated Flow,					
						Short Grass Pasture Kv= 7.0 fps					
	0.2	12	0.0400	1.00		Shallow Concentrated Flow,					
						Woodland Kv= 5.0 fps					
	0.1	15	0.3000	3.83		Shallow Concentrated Flow,					
						Short Grass Pasture Kv= 7.0 fps					
	2.2	467	0.0300	3.52		Shallow Concentrated Flow,					
_						Paved Kv= 20.3 fps					
	19.5	692	Total								

Summary for Subcatchment 7S: SUBCATCHMENT

Runoff 7.67 cfs @ 12.29 hrs, Volume=

0.82 af, Depth> 3.77"

	Area (sf)	CN	Description
	83,250	70.0	Woods, Good, HSG C
	18,246	74.0	>75% Grass cover, Good, HSG C
*	5,928	98.0	Paved parking
	764	96.0	Gravel surface, HSG C
*	4,800	98.0	Roofs
	112,988	73.5	Weighted Average
	102,260	70.9	90.51% Pervious Area
	10,728	98.0	9.49% Impervious Area

2304141-POST DEVELOPMENT

Type III 24-hr 50-YEAR Rainfall=6.77"

Prepared by Keach-Nordstrom Associates, Inc.

Printed 12/6/2023

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 89

	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
-	12.5	100	0.0900	0.13		Sheet Flow,
						Woods: Light underbrush n= 0.400 P2= 2.84"
	7.5	755	0.1130	1.68		Shallow Concentrated Flow,
						Woodland Kv= 5.0 fps
	0.0	6	0.3000	3.83		Shallow Concentrated Flow,
						Short Grass Pasture Kv= 7.0 fps
	0.7	160	0.0375	3.93		Shallow Concentrated Flow,
						Paved Kv= 20.3 fps
	20.7	1,021	Total			

Summary for Subcatchment 8S: SUBCATCHMENT

Runoff = 2.50 cfs @ 12.28 hrs, Volume=

0.27 af, Depth> 4.53"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 50-YEAR Rainfall=6.77"

	Aı	rea (sf)	CN	Description	n				
		С							
		6,935	74.0	>75% Grass cover, Good, HSG C					
*		10,596	98.0	Paved par	king				
		30,577	80.6	Weighted	Average				
		19,981	71.4	65.35% P	ervious Are	ea			
		10,596	98.0	34.65% In	npervious A	Area			
	т.	Longth	Clana	Volocity	Conocity	Description			
/~	Tc	Length	Slope	Velocity	Capacity (cfs)	Description			
	nin)	(feet)	(ft/ft)	(ft/sec)	(015)				
1	5.8	100	0.0500	0.11		Sheet Flow,			
						Woods: Light underbrush n= 0.400 P2= 2.84"			
	2.9	329	0.1480	1.92		Shallow Concentrated Flow,			
						Woodland Kv= 5.0 fps			
	0.1	33	0.3330	4.04		Shallow Concentrated Flow,			
						Short Grass Pasture Kv= 7.0 fps			
	1.5	257	0.0200	2.87		Shallow Concentrated Flow,			
						Paved Kv= 20.3 fps			
2	0.3	719	Total						

Summary for Subcatchment 9S: SUBCATCHMENT

Runoff = 4.44 cfs @ 12.38 hrs, Volume=

0.53 af, Depth> 4.07"

2304141-POST DEVELOPMENT

Type III 24-hr 50-YEAR Rainfall=6.77"

Prepared by Keach-Nordstrom Associates, Inc.

Printed 12/6/2023

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC Page 90

	A	rea (sf)	CN	Description	Description						
		34,147	70.0	Woods, Good, HSG C							
21,287 74.0 >75% Grass cover, Good, HSG C						Good, HSG C					
rh.		7,590	98.0	Paved pa	,						
*		4,800	98.0	Roofs	3						
		67,824	76.4	Weighted	Average						
		55,434	71.5		ervious Are	ea					
		12,390	98.0	18.27% In	npervious A	Area					
	Tc	Length	Slope	Velocity	Capacity	Description					
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	·					
	19.4	100	0.0300	0.09		Sheet Flow,					
						Woods: Light underbrush n= 0.400 P2= 2.84"					
	2.6	350	0.1940	2.20		Shallow Concentrated Flow,					
						Woodland Kv= 5.0 fps					
	0.2	43	0.3330	4.04		Shallow Concentrated Flow,					
						Short Grass Pasture Kv= 7.0 fps					
	4.6	800	0.0200	2.87		Shallow Concentrated Flow,					
_						Paved Kv= 20.3 fps					
	26.9	1,293	Total								

Summary for Subcatchment 10S: SUBCATCHMENT

Runoff 4.37 cfs @ 12.32 hrs, Volume=

0.49 af, Depth> 3.72"

	А	rea (sf)	CN	Description	on					
		49,642	70.0	Woods, G	C					
		13,315	74.0	>75% Gra	ass cover, (Good, HSG C				
*		3,077	98.0	Paved pa	Paved parking					
*		2,400	98.0	Roofs						
		68,434	73.0	Weighted	Average					
	62,957 70.8 92.00% Pervious Area				ervious Are	ea ea				
		5,477	98.0	8.00% lm	pervious A	rea				
	Tc	Length	Slope	Velocity	Capacity	Description				
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
	17.3	100	0.0400	0.10		Sheet Flow,				
						Woods: Light underbrush n= 0.400 P2= 2.84"				
	4.0	479	0.1610	2.01		Shallow Concentrated Flow,				
						Woodland Kv= 5.0 fps				
	1.5	65	0.0100	0.70		Shallow Concentrated Flow,				
						Short Grass Pasture Kv = 7.0 fps				
	0.2	32	0.0200	2.87		Shallow Concentrated Flow,				
-						Paved Kv= 20.3 fps				
	23.0	676	Total							

2304141-POST DEVELOPMENT

Type III 24-hr 50-YEAR Rainfall=6.77"

Prepared by Keach-Nordstrom Associates, Inc.

Printed 12/6/2023

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 91

Summary for Subcatchment 11S: SUBCATCHMENT

Runoff = 4.24 cfs @ 12.29 hrs, Volume=

0.45 af, Depth> 3.55"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 50-YEAR Rainfall=6.77"

Α	rea (sf)	CN	Description	n	
	54,267	70.0	Woods, G	C	
	10,350	74.0	>75% Gra	ass cover, (Good, HSG C
*	1,779 98.0 Paved parking				
	66,396 71.4 Weighted Average			Average	
64,617 70.6			97.32% Pervious Area		
	1,779 98.0 2.68% Imperv			pervious A	rea
Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
15.8	100	0.0500	0.11		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 2.84"
4.2	485	0.1510	1.94		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
0.6	73	0.0800	1.98		Shallow Concentrated Flow,
					Short Grass Pasture Kv= 7.0 fps
0.1	32	0.0600	4.97		Shallow Concentrated Flow,
					Paved Kv= 20.3 fps
20.7	690	Total			

Summary for Subcatchment 12S: SUBCATCHMENT

Runoff = 4.27 cfs @ 12.26 hrs, Volume=

0.44 af, Depth> 3.82"

	Area (sf)	CN	Description	
7	38,779	70.0	Woods, Good, HSG C	
	14,722	74.0	>75% Grass cover, Good, HSG C	
*	3,785	98.0	Paved parking	
*	2,400	98.0	Roofs	
	59,686	73.9	Weighted Average	
	53,501	71.1	89.64% Pervious Area	
	6,185	98.0	10.36% Impervious Area	

2304141-POST DEVELOPMENT

Type III 24-hr 50-YEAR Rainfall=6.77"

Prepared by Keach-Nordstrom Associates, Inc.

Printed 12/6/2023

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 92

(mi	Tc in)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
13	8.8	100	0.0700	0.12		Sheet Flow,
						Woods: Light underbrush n= 0.400 P2= 2.84"
4	.0	462	0.1470	1.92		Shallow Concentrated Flow,
_			0.4400			Woodland Kv= 5.0 fps
C	.4	55	0.1100	2.32		Shallow Concentrated Flow,
_		400	0.0000	4.07		Short Grass Pasture Kv= 7.0 fps
U	.5	136	0.0600	4.97		Shallow Concentrated Flow,
						Paved Kv= 20.3 fps
18	.7	753	Total			

Summary for Subcatchment 13S: SUBCATCHMENT

Runoff = 3.97 cfs @ 12.23 hrs, Volume=

0.38 af, Depth> 3.69"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 50-YEAR Rainfall=6.77"

-	A	rea (sf)	CN	Description				
40,261 70.0 Woods, Good, HSG C						C		
		10,598	74.0	>75% Gra	ass cover, (Good, HSG C		
*		3,610	98.0	Paved pa	rking			
		54,469	72.6	Weighted	Average			
		50,859	70.8		ervious Are	ea		
		3,610	98.0	6.63% lm	pervious A	rea		
	Tc	Length	Slope	Velocity	Capacity	Description		
-	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
	12.0	100	0.1000	0.14		Sheet Flow,		
						Woods: Light underbrush n= 0.400 P2= 2.84"		
	3.8	425	0.1410	1.88		Shallow Concentrated Flow,		
						Woodland Kv= 5.0 fps		
	0.3	61	0.3300	4.02		Shallow Concentrated Flow,		
						Short Grass Pasture Kv= 7.0 fps		
	0.3	94	0.0600	4.97		Shallow Concentrated Flow,		
-						Paved Kv= 20.3 fps		
	16.3	680	Total					

Summary for Subcatchment 14S: SUBCATCHMENT

Runoff = 2.80 cfs @ 12.23 hrs, Volume=

0.28 af, Depth> 3.80"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 50-YEAR Rainfall=6.77"

Type III 24-hr 50-YEAR Rainfall=6.77"

Prepared by Keach-Nordstrom Associates, Inc.

Printed 12/6/2023

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 93

15	Α	rea (sf)	CN	Description	n	
		25,189	70.0	Woods, G	ood, HSG	С
		9,128	74.0	>75% Gra	ss cover, (Good, HSG C
	*	3,704	98.0	Paved par	rking	
		38,021	73.7	Weighted	Average	
		34,317	71.1	90.26% P	ervious Are	ea
		3,704	98.0	9.74% lm	pervious Ai	rea
	Tc	Length	Slope	Velocity	Capacity	Description
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	11.1	100	0.1200	0.15		Sheet Flow,
						Woods: Light underbrush n= 0.400 P2= 2.84"
	5.5	529	0.1020	1.60		Shallow Concentrated Flow,
						Woodland Kv= 5.0 fps
	0.2	46	0.3300	4.02		Shallow Concentrated Flow,
						Short Grass Pasture Kv= 7.0 fps
	0.3	56	0.0200	2.87		Shallow Concentrated Flow,
-						Paved Kv= 20.3 fps
	17.2	731	Total			

Summary for Subcatchment 15S: SUBCATCHMENT

Runoff = 1.88 cfs @ 12.09 hrs, Volume=

0.14 af, Depth> 4.89"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 50-YEAR Rainfall=6.77"

	Area (sf)	CN	Description	n					
,	8,678	74.0	>75% Gra	75% Grass cover, Good, HSG C					
*	5,925	98.0	Paved par	Paved parking					
2	14,603	83.7	Weighted	Weighted Average					
	8,678	74.0	59.43% Pervious Area						
	5,925	98.0	40.57% lr	40.57% Impervious Area					
	Tc Length	n Slope	Velocity	Capacity	·				
(mi	n) (feet) (ft/ft)	(ft/sec)	(cfs)					
6	5.0				Direct Entry,				

Summary for Subcatchment 16S: SUBCATCHMENT

Runoff = 1.15 cfs @ 12.08 hrs, Volume= 0.09 af, Depth> 5.69"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 50-YEAR Rainfall=6.77"

2304141-POST DEVELOPMENT

Type III 24-hr 50-YEAR Rainfall=6.77"

Prepared by Keach-Nordstrom Associates, Inc.

Printed 12/6/2023

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 94

A	rea (sf)	CN	Description	n				
	2,375	74.0	>75% Gra	iss cover, (Good, HSG C			
*	5,613	98.0	Paved par	Paved parking				
	7,988	90.9	Weighted	Veighted Average				
	2,375	74.0	29.73% Pervious Area					
	5,613	98.0	70.27% Impervious Area					
Tc	Length	Slope	Velocity	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
6.0					Direct Entry,			

Summary for Subcatchment 17S: SUBCATCHMENT

Runoff = 6.86 cfs @ 12.18 hrs, Volume=

0.61 af, Depth> 3.85"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 50-YEAR Rainfall=6.77"

		rea (sf)	CN	Description	on					
		26,296	70.0	Woods, C	Good, HSG	C				
		51,457	74.0	>75% Gra	ass cover, (Good, HSG C				
*		1,238	98.0		Paved parking					
*		2,400	98.0	Roofs	1 0					
		1,535	96.0	Gravel su	Gravel surface, HSG C					
		82,926	74.2	Weighted	Veighted Average					
		79,288	73.1		95.61% Pervious Area					
		3,638	98.0	4.39% Im	4.39% Impervious Area					
	Tc	Length	Slope	Velocity	Capacity	Description				
(m	in)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
1:	2.0	100	0.1000	0.14		Sheet Flow,				
						Woods: Light underbrush n= 0.400 P2= 2.84"				
	1.0	90	0.0860	1.47		Shallow Concentrated Flow,				
						Woodland Kv= 5.0 fps				
(0.2	40	0.3300	4.02		Shallow Concentrated Flow,				
						Short Grass Pasture Kv= 7.0 fps				
1:	3.2	230	Total			·				

Summary for Reach 1R: EXISTING ROADWAY DITCH

Inflow Area = 2.362 ac, 23.83% Impervious, Inflow Depth > 3.53" for 50-YEAR event

Inflow = 9.56 cfs @ 12.09 hrs, Volume= 0.70 af

Outflow = 9.48 cfs @ 12.10 hrs, Volume= 0.69 af, Atten= 1%, Lag= 0.6 min

Flouting by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Max. Velocity= 4.72 fps, Min. Travel Time= 0.8 min Avg. Velocity = 1.95 fps, Avg. Travel Time= 2.0 min

Type III 24-hr 50-YEAR Rainfall=6.77"

Prepared by Keach-Nordstrom Associates, Inc.

Printed 12/6/2023

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 95

Peak Storage= 462 cf @ 12.10 hrs
Average Depth at Peak Storage= 0.82'
Bank-Full Depth= 1.00' Flow Area= 3.0 sf, Capacity= 16.21 cfs

0.00' x 1.00' deep channel, n= 0.022 Earth, clean & straight Side Slope Z-value= 3.0 '/' Top Width= 6.00' Length= 230.0' Slope= 0.0173 '/' Inlet Invert= 295.64', Outlet Invert= 291.66'



Summary for Reach 2R: EXISTING ROADWAY DITCH

Inflow Area = 11.270 ac, 12.00% Impervious, Inflow Depth > 1.59" for 50-YEAR event

Inflow = 2.71 cfs @ 14.88 hrs, Volume= 1.49 af

Outflow = 2.70 cfs @ 14.97 hrs, Volume= 1.48 af, Atten= 0%, Lag= 5.7 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Max. Velocity= 2.36 fps, Min. Travel Time= 6.5 min Avg. Velocity = 1.55 fps, Avg. Travel Time= 9.8 min

Peak Storage= 1,045 cf @ 14.97 hrs Average Depth at Peak Storage= 0.54'

Bank-Full Depth= 1.00' Flow Area= 3.5 sf, Capacity= 12.04 cfs

0.50' x 1.00' deep channel, n= 0.069 Riprap, 6-inch Side Slope Z-value= 3.0 '/' Top Width= 6.50' Length= 915.0' Slope= 0.0621 '/' Inlet Invert= 348.50', Outlet Invert= 291.66'



Summary for Pond 1P: EXISTING CB

Inflow Area = 22.064 ac, 4.08% Impervious, Inflow Depth > 2.86" for 50-YEAR event

Inflow = 37.01 cfs @ 12.57 hrs, Volume= 5.26 af

Primary = 37.01 cfs @ 12.57 hrs, Volume= 5.26 af, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Type III 24-hr 50-YEAR Rainfall=6.77"

Prepared by Keach-Nordstrom Associates, Inc.

Printed 12/6/2023

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 96

Summary for Pond 2P: EXISTING DRIVEWAY CULVERT

Inflow Area = 2.596 ac, 12.76% Impervious, Inflow Depth > 2.56" for 50-YEAR event

Inflow = 5.12 cfs @ 12.29 hrs, Volume= 0.55 af

Outflow = 5.12 cfs @ 12.29 hrs, Volume= 0.55 af, Atten= 0%, Lag= 0.0 min

Primary = 5.12 cfs @ 12.29 hrs, Volume= 0.55 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs. dt= 0.01 hrs.

Peak Elev= 300.88' @ 12.46 hrs

Flood Elev= 294.00'

Primary OutFlow Max=4.70 cfs @ 12.29 hrs HW=298.31' TW=296.77' (Dynamic Tailwater) 1=Culvert (Inlet Controls 4.70 cfs @ 5.99 fps)

Summary for Pond 3P: EXISTING DROP INLET

Inflow Area = 41.139 ac, 8.14% Impervious, Inflow Depth > 2.51" for 50-YEAR event

Inflow = 48.31 cfs @ 12.49 hrs, Volume= 8.59 af

Outflow = 48.31 cfs @ 12.49 hrs, Volume= 8.59 af, Atten= 0%, Lag= 0.0 min

Primary = 48.31 cfs @ 12.49 hrs, Volume= 8.59 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 299.86' @ 12.49 hrs

Flood Elev= 293.75'

Device	Routing	Invert	Outlet Devices
#1	Primary	288.66'	24.0" Round Culvert
			L= 92.1' RCP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 288.66' / 284.67' S= 0.0433 '/' Cc= 0.900
			n= 0.011 Concrete pipe, straight & clean, Flow Area= 3.14 sf

Primary OutFlow Max=48.30 cfs @ 12.49 hrs HW=299.86' TW=0.00' (Dynamic Tailwater) 1=Culvert (Inlet Controls 48.30 cfs @ 15.38 fps)

Summary for Pond 4P: EXISTING CB

Inflow Area =	2.362 ac, 23.83% Impervious, Inflow	Depth > 3.53" for 50-YEAR event
inflow =	9.56 cfs @ 12.09 hrs, Volume=	0.70 af
Outflow =	9.56 cfs @ 12.09 hrs, Volume=	0.70 af, Atten= 0%, Lag= 0.0 min
Primary =	4.61 cfs @ 12.09 hrs, Volume=	0.63 af
Secondary =	4.94 cfs @ 12.09 hrs, Volum⊕=	0.06 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Type III 24-hr 50-YEAR Rainfall=6.77"

Prepared by Keach-Nordstrom Associates, Inc.

Printed 12/6/2023

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 97

Peak Elev= 300.50' @ 12.09 hrs

Flood Elev= 300.17'

Device	Routing	Invert	Outlet Devices
#1	Primary	297.00'	12.0" Round Culvert
	•		L= 49.1' CMP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 297.00' / 295.64' S= 0.0277 '/' Cc= 0.900
			n= 0.025 Corrugated metal, Flow Area= 0.79 sf
#2	Secondary	300.17'	24.0" x 24.0" Horiz. Orifice/Grate C= 0.600
			Limited to weir flow at low heads

Primary OutFlow Max=4.61 cfs @ 12.09 hrs HW=300.50' TW=296.45' (Dynamic Tailwater) 1=Culvert (Barrel Controls 4.61 cfs @ 5.87 fps)

Secondary OutFlow Max=4.93 cfs @ 12.09 hrs HW=300.50' TW=296.45' (Dynamic Tailwater) 2=Orifice/Grate (Weir Controls 4.93 cfs @ 1.87 fps)

Summary for Pond 5P: EXISTING CB

Inflow Area =	0.377 ac, 34.30% Impervious, Inflow I	Depth > 3.80" for 50-YEAR event
Inflow =	1.54 cfs @ 12.12 hrs, Volume=	0.12 af
Outflow =	1.54 cfs @ 12.12 hrs, Volume=	0.12 af, Atten= 0%, Lag= 0.0 min
Primary =	1.54 cfs @ 12.12 hrs, Volume=	0.12 af
Secondary =	0.00 cfs @ 0.00 hrs, Volume=	0.00 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 300.82' @ 12.11 hrs Flood Elev= 301.16'

Device	Routing	Invert	Outlet Devices
#1	Primary	297.70'	12.0" Round Culvert
	•		L= 34.1' CMP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 297.70' / 297.30' S= 0.0117 '/' Cc= 0.900
			n= 0.025 Corrugated metal, Flow Area= 0.79 sf
#2	Secondary	301.16'	24.0" x 24.0" Horiz. Orifice/Grate C= 0.600
	•		Limited to weir flow at low heads

Primary OutFlow Max=1.56 cfs @ 12.12 hrs HW=300.81' TW=300.47' (Dynamic Tailwater) 1=Culvert (Outlet Controls 1.56 cfs @ 1.99 fps)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=297.70' TW=0.00' (Dynamic Tailwater) —2=Orifice/Grate (Controls 0.00 cfs)

Summary for Pond 6P: EXISTING CB

Inflow Area =	2.097 ac,	14.39% Impervious,	Inflow Depth > 4.00	" for 50-YEAR event
inflow =	6.74 cfs @	12.26 hrs, Volume	= 0.70 af	
Outflow =	6.74 cfs @	12.26 hrs, Volume	= 0.70 af, A	tten= 0%, Lag= 0.0 min
Primary =	6.74 cfs @	12.26 hrs, Volume	= 0.70 af	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

2304141-POST DEVELOPMENT

Type III 24-hr 50-YEAR Rainfall=6.77"

Prepared by Keach-Nordstrom Associates, Inc.
HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Printed 12/6/2023

Page 98

Peak Elev= 367.57' @ 12.26 hrs

Flood Elev= 366.35'

Device	Routing	Invert	Outlet Devices
#1	Primary	363.70'	12.0" Round Culvert
			L= 18.0' CMP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 363.70' / 362.46' S= 0.0689 '/' Cc= 0.900
			n= 0.025 Corrugated metal. Flow Area= 0.79 sf

Primary OutFlow Max=6.74 cfs @ 12.26 hrs HW=367.56' TW=0.00' (Dynamic Tailwater) 1=Culvert (Barrel Controls 6.74 cfs @ 8.58 fps)

Summary for Pond 7P: EXISTING CB

Inflow Area = 2.594 ac, 9.49% Impervious, Inflow Depth > 3.77" for 50-YEAR event

Inflow = 7.67 cfs @ 12.29 hrs, Volume= 0.82 af

Outflow = 7.67 cfs @ 12.29 hrs, Volume= 0.82 af, Atten= 0%, Lag= 0.0 min

Primary = 7.67 cfs @ 12.29 hrs, Volume= 0.82 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 381.18' @ 12.29 hrs

Flood Elev= 380.49'

Device	Routing	Invert	Outlet Devices
#1	Primary	376.57'	12.0" Round Culvert
			L= 31.9' RCP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 376.57' / 376.28' S= 0.0091 '/' Cc= 0.900
			n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf

Primary OutFlow Max=7.67 cfs @ 12.29 hrs HW=381.18' TW=0.00' (Dynamic Tailwater) 1=Culvert (Inlet Controls 7.67 cfs @ 9.76 fps)

Summary for Pond 8P: DMH#2

Inflow Area = 11.270 ac, 12.00% Impervious, Inflow Depth > 1.59" for 50-YEAR event

Inflow = 2.71 cfs @ 14.88 hrs, Volume= 1.49 af

Outflow = 2.71 cfs @ 14.88 hrs, Volume= 1.49 af, Atten= 0%, Lag= 0.0 min

Primary = 2.71 cfs @ 14.88 hrs, Volume= 1.49 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 350.54' @ 14.88 hrs

Flood Elev= 353.05'

Device	Routing	Invert	Outlet Devices
#1	Primary		15.0" Round Culvert
			L= 15.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 349.70' / 348.50' S= 0.0800 '/' Cc= 0.900
			n= 0.013, Flow Area= 1.23 sf

Primary OutFlow Max=2.71 cfs @ 14.88 hrs HW=350.54' TW=349.04' (Dynamic Tailwater) 1=Culvert (Inlet Controls 2.71 cfs @ 3.11 fps)

Type III 24-hr 50-YEAR Rainfall=6.77"

Prepared by Keach-Nordstrom Associates, Inc.

Printed 12/6/2023

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 99

Summary for Pond 9P: DMH#3

Inflow Area = 5.536 ac, 11.12% Impervious, Inflow Depth > 3.84" for 50-YEAR event

Inflow = 16.57 cfs @ 12.24 hrs, Volume= 1.77 af

Outflow = 16.57 cfs @ 12.24 hrs, Volume= 1.77 af, Atten= 0%, Lag= 0.0 min

Primary = 16.57 cfs @ 12.24 hrs, Volume= 1.77 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 378.80' @ 12.24 hrs

Flood Elev= 377.79'

Device	Routing	Invert	Outlet Devices
#1	Primary	374.26'	18.0" Round Culvert L= 30.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 374.26' / 374.04' S= 0.0073 '/' Cc= 0.900 n= 0.013, Flow Area= 1.77 sf

Primary OutFlow Max=16.57 cfs @ 12.24 hrs HW=378.80' TW=370.70' (Dynamic Tailwater)
1=Culvert (Inlet Controls 16.57 cfs @ 9.37 fps)

Summary for Pond 10P: DMH#4

Inflow Area = 5.536 ac. 11.12% Impervious, Inflow Depth > 3.84" for 50-YEAR event

Inflow = 16.57 cfs @ 12.24 hrs, Volume= 1.77 af

Outflow = 16.57 cfs @ 12.24 hrs, Volume= 1.77 af, Atten= 0%, Lag= 0.0 min

Primary = 16.57 cfs @ 12.24 hrs, Volume= 1.77 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 383.88' @ 12.25 hrs

Flood Elev= 381.66'

Device	Routing	Invert	Outlet Devices
#1	Primary	375.58'	18.0" Round Culvert
			L= 121.7' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 375.58' / 374.36' S= 0.0100 '/' Cc= 0.900
			n= 0.013, Flow Area= 1.77 sf

Primary OutFlow Max=16.55 cfs @ 12.24 hrs HW=383.87' TW=378.80' (Dynamic Tailwater) 1=Culvert (Outlet Controls 16.55 cfs @ 9.37 fps)

Summary for Pond 11P: DMH#5

Inflow Area = 5.536 ac, 11.12% Impervious, Inflow Depth > 3.84" for 50-YEAR event

Inflow = 16.57 cfs @ 12.24 hrs, Volume= 1.77 af

Outflow = 16.57 cfs @ 12.24 hrs, Volume= 1.77 af, Atten= 0%, Lag= 0.0 min

Primary = 16.57 cfs @ 12.24 hrs, Volume= 1.77 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 391.55' @ 12.25 hrs

Flood Elev= 384.00'

2304141-POST DEVELOPMENT

Type III 24-hr 50-YEAR Rainfall=6.77"

Prepared by Keach-Nordstrom Associates, Inc.

Printed 12/6/2023

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC Page 100

Device	Routing	Invert	Outlet Devices
#1	Primary		18.0" Round Culvert L= 226.1' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 377.94' / 375.68' S= 0.0100 '/' Cc= 0.900 n= 0.013, Flow Area= 1.77 sf

Primary OutFlow Max=16.51 cfs @ 12.24 hrs HW=391.52' TW=383.87' (Dynamic Tailwater) 1=Culvert (Outlet Controls 16.51 cfs @ 9.34 fps)

Summary for Pond 12P: CB#6

Inflow Area = 5.536 ac, 11.12% Impervious, Inflow Depth > 3.84" for 50-YEAR event

Inflow = 16.57 cfs @ 12.24 hrs, Volume= 1.77 af

Outflow = 16.57 cfs @ 12.24 hrs, Volume= 1.77 af, Atten= 0%, Lag= 0.0 min

Primary = 16.57 cfs @ 12.24 hrs, Volume= 1.77 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 397.09' @ 12.25 hrs

Flood Elev= 384.30'

Device	Routing	Invert	Outlet Devices
#1	Primary	379.45'	18.0" Round Culvert
			L= 141.2' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 379.45 / 378.04' S= 0.0100 '/' Cc= 0.900
			n= 0.013, Flow Area= 1.77 sf

Primary OutFlow Max=16.38 cfs @ 12.24 hrs HW=396.96' TW=391.52' (Dynamic Tailwater)

1=Culvert (Outlet Controls 16.38 cfs @ 9.27 fps)

Summary for Pond 13P: CB#7

Inflow Area = 5.201 ac, 9.22% Impervious, Inflow Depth > 3.77" for 50-YEAR event

Inflow = 15.66 cfs @ 12.25 hrs, Volume= 1.63 af

Outflow = 15.66 cfs @ 12.25 hrs, Volume= 1.63 af, Atten= 0%, Lag= 0.0 min

Primary = 15.66 cfs @ 12.25 hrs, Volume= 1.63 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 404.06' @ 12.26 hrs

Flood Elev= 384.30'

Device	Routing	Invert	Outlet Devices
#1	Primary	380.25'	15.0" Round Culvert
			L= 30.3' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 380.25' / 379.95' S= 0.0099 '/' Cc= 0.900
			n= 0.013, Flow Area= 1.23 sf

Primary OutFlow Max=15.48 cfs @ 12.25 hrs HW=403.90' TW=397.04' (Dynamic Tailwater) 1=Culvert (Inlet Controls 15.48 cfs @ 12.61 fps)

Type III 24-hr 50-YEAR Rainfall=6.77"

Prepared by Keach-Nordstrom Associates, Inc.

Printed 12/6/2023

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 101

Summary for Pond 14P: CB#8

Inflow Area = 4.328 ac, 9.12% Impervious, Inflow Depth > 3.77" for 50-YEAR event

Inflow = 12.87 cfs @ 12.25 hrs, Volume= 1.36 af

Outflow = 12.87 cfs @ 12.25 hrs, Volume= 1.36 af, Atten= 0%, Lag= 0.0 min

Primary = 12.87 cfs @ 12.25 hrs, Volume= 1.36 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 413.14' @ 12.27 hrs

Flood Elev= 388.51'

Device	Routing	Invert	Outlet Devices
#1	Primary	384.49'	15.0" Round Culvert L= 165.7' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 384.49' / 380.35' S= 0.0250 '/' Cc= 0.900
			n= 0.013, Flow Area= 1.23 sf

Primary OutFlow Max=12.69 cfs @ 12.25 hrs HW=412.88' TW=403.97' (Dynamic Tailwater) 1=Culvert (Outlet Controls 12.69 cfs @ 10.34 fps)

Summary for Pond 16P: CB#10

Inflow Area = 3.078 ac, 10.13% Impervious, Inflow Depth > 3.80" for 50-YEAR event

Inflow = 8.98 cfs @ 12.27 hrs, Volume= 0.97 af

Outflow = 8.98 cfs @ 12.27 hrs, Volume= 0.97 af, Atten= 0%, Lag= 0.0 min

Primary = 8.98 cfs @ 12.27 hrs. Volume= 0.97 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 426.70' @ 12.27 hrs

Flood Elev= 397.31'

Device	Routing	Invert	Outlet Devices
#1	Primary	392.11'	12.0" Round Culvert
			L= 165.5' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 392.11' / 383.83' S= 0.0500 '/' Cc= 0.900
			n= 0.013, Flow Area= 0.79 sf

Primary OutFlow Max=8.94 cfs @ 12.27 hrs HW=426.57' TW=413.11' (Dynamic Tailwater) 1=Culvert (Outlet Controls 8.94 cfs @ 11.38 fps)

Summary for Pond 17P: CB#100

Inflow Area = 0.183 ac, 70.27% Impervious, Inflow Depth > 5.69" for 50-YEAR event

Inflow = 1.15 cfs @ 12.08 hrs, Volume= 0.09 af

Outflow = 1.15 cfs @ 12.08 hrs, Volume= 0.09 af, Atten= 0%, Lag= 0.0 min

Primary = 1.15 cfs @ 12.08 hrs, Volume= 0.09 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 426.71' @ 12.28 hrs

Flood Elev= 397.31'

2304141-POST DEVELOPMENT

Type III 24-hr 50-YEAR Rainfall=6.77"

Prepared by Keach-Nordstrom Associates, Inc. HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Printed 12/6/2023

Page 102

Device	Routing	Invert	Outlet Devices
#1	Primary	393.25'	12.0" Round Culvert
			L= 26.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 393.25' / 392.73' S= 0.0200 '/' Cc= 0.900
			n= 0.013, Flow Area= 0.79 sf

Primary OutFlow Max=0.00 cfs @ 12.08 hrs HW=394.88' TW=395.11' (Dynamic Tailwater) 1=Culvert (Controls 0.00 cfs)

Summary for Pond 18P: CB#11

Inflow Area = 1.524 ac, 2.68% Impervious, Inflow Depth > 3.55" for 50-YEAR event
Inflow = 4.24 cfs @ 12.29 hrs, Volume= 0.45 af
Outflow = 4.24 cfs @ 12.29 hrs, Volume= 0.45 af
Primary = 4.24 cfs @ 12.29 hrs, Volume= 0.45 af
0.45 af
0.45 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 429.75' @ 12.28 hrs Flood Elev= 406.88'

| Device | Routing | Invert | Outlet Devices | | 402.80' | 12.0" | Round Culvert | L= 167.8' | CPP, square edge headwall, Ke= 0.500 | Inlet / Outlet Invert= 402.80' / 392.73' | S= 0.0600 '/' | Cc= 0.900 | n= 0.013, Flow Area= 0.79 sf

Primary OutFlow Max=4.46 cfs @ 12.29 hrs HW=429.67' TW=426.29' (Dynamic Tailwater)

1=Culvert (Outlet Controls 4.46 cfs @ 5.68 fps)

Summary for Pond 20P: DMH#14

Inflow Area = 3.830 ac, 17.06% Impervious, Inflow Depth > 4.01" for 50-YEAR event

Inflow = 11.16 cfs @ 12.32 hrs, Volume= 1.28 af

Outflow = 11.16 cfs @ 12.32 hrs, Volume= 1.28 af, Atten= 0%, Lag= 0.0 min

Primary = 11.16 cfs @ 12.32 hrs, Volume= 1.28 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 377.67' @ 12.32 hrs

Flood Elev= 383.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	375.20'	18.0" Round Culvert
			L= 15.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 375.20' / 374.00' S= 0.0800 '/' Cc= 0.900
			n= 0.013, Flow Area= 1.77 sf

Primary OutFlow Max=11.15 cfs @ 12.32 hrs HW=377.67' TW=371.26' (Dynamic Tailwater)
—1=Culvert (Inlet Controls 11.15 cfs @ 6.31 fps)

Type III 24-hr 50-YEAR Rainfall=6.77"

Prepared by Keach-Nordstrom Associates, Inc.

Printed 12/6/2023

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 103

Summary for Pond 21P: DMH#15

Inflow Area = 3.830 ac, 17.06% Impervious, Inflow Depth > 4.01" for 50-YEAR event

Inflow = 11.16 cfs @ 12.32 hrs, Volume= 1.28 af

Outflow = 11.16 cfs @ 12.32 hrs, Volume= 1.28 af, Atten= 0%, Lag= 0.0 min

Primary = 11.16 cfs @ 12.32 hrs, Volume= 1.28 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 383.93' @ 12.32 hrs

Flood Elev= 387.85'

Device	Routing	Invert	Outlet Devices
#1	Primary	381.46'	18.0" Round Culvert
			L= 30.7' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 381.46' / 379.00' S= 0.0801 '/' Cc= 0.900
			n= 0.013, Flow Area= 1.77 sf

Primary OutFlow Max=11.15 cfs @ 12.32 hrs HW=383.93' TW=377.67' (Dynamic Tailwater) 1=Culvert (Inlet Controls 11.15 cfs @ 6.31 fps)

Summary for Pond 22P: DMH#16

Inflow Area = 3.830 ac, 17.06% Impervious, Inflow Depth > 4.01" for 50-YEAR event

Inflow = 11.16 cfs @ 12.32 hrs, Volume= 1.28 af

Outflow = 11.16 cfs @ 12.32 hrs, Volume= 1.28 af, Atten= 0%, Lag= 0.0 min

Primary = 11.16 cfs @ 12.32 hrs, Volume= 1.28 af

Routing by Dvn-Stor-Ind method. Time Span= 0.00-24.00 hrs. dt= 0.01 hrs

Peak Elev= 395.39' @ 12.32 hrs

Flood Elev= 400.42'

Device	Routing	Invert	Outlet Devices
#1	Primary	392.92'	18.0" Round Culvert
	_		L= 111.5' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 392.92' / 384.00' S= 0.0800 '/' Cc= 0.900
			n= 0.013. Flow Area= 1.77 sf

Primary OutFlow Max=11.15 cfs @ 12.32 hrs HW=395.39' TW=383.93' (Dynamic Tailwater)

1=Culvert (Inlet Controls 11.15 cfs @ 6.31 fps)

Summary for Pond 23P: DMH#17

Inflow Area = 3.830 ac, 17.06% Impervious, Inflow Depth > 4.01" for 50-YEAR event

Inflow = 11.16 cfs @ 12.32 hrs, Volume= 1.28 af

Outflow = 11.16 cfs @ 12.32 hrs, Volume= 1.28 af, Atten= 0%, Lag= 0.0 min

Primary = 11.16 cfs @ 12.32 hrs, Volume= 1.28 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 403.60' @ 12.32 hrs

Flood Elev= 409.83'

2304141-POST DEVELOPMENT

Type III 24-hr 50-YEAR Rainfall=6.77"

Prepared by Keach-Nordstrom Associates, Inc. HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Printed 12/6/2023

Page 104

Device	Routing	Invert	Outlet Devices
#1	Primary	401.13'	18.0" Round Culvert L= 45.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 401.13' / 397.53' S= 0.0800 '/' Cc= 0.900 n= 0.013, Flow Area= 1.77 sf

Primary OutFlow Max=11.15 cfs @ 12.32 hrs HW=403.60' TW=395.39' (Dynamic Tailwater) 1=Culvert (Inlet Controls 11.15 cfs @ 6.31 fps)

Summary for Pond 24P: CB#18

Inflow Area = 3.830 ac, 17.06% Impervious, Inflow Depth > 4.01" for 50-YEAR event

Inflow = 11.16 cfs @ 12.32 hrs, Volume= 1.28 af

Outflow = 11.16 cfs @ 12.32 hrs, Volume= 1.28 af, Atten= 0%, Lag= 0.0 min

Primary = 11.16 cfs @ 12.32 hrs, Volume= 1.28 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 406.38' @ 12.33 hrs

Flood Elev= 407.95'

Device	Routing	Invert	Outlet Devices
#1	Primary	402.87'	18.0" Round Culvert
			L= 164.2' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 402.87' / 401.23' S= 0.0100 '/' Cc= 0.900
			n= 0.013, Flow Area= 1.77 sf

Primary OutFlow Max=11.14 cfs @ 12.32 hrs HW=406.38' TW=403.60' (Dynamic Tailwater) 1=Culvert (Outlet Controls 11.14 cfs @ 6.30 fps)

Summary for Pond 25P: CB#19

Inflow Area = 2.259 ac, 23.36% Impervious, Inflow Depth > 4.21" for 50-YEAR event

Inflow = 6.78 cfs @ 12.32 hrs, Volume= 0.79 af

Outflow = 6.78 cfs @ 12.32 hrs, Volume= 0.79 af, Atten= 0%, Lag= 0.0 min

Primary = 6.78 cfs @ 12.32 hrs, Volume= 0.79 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 407.69' @ 12.34 hrs

Flood Elev= 408.04'

Device	Routing	Invert	Outlet Devices
#1	Primary	403.34'	15.0" Round Culvert
			L= 36.5' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 403.34' / 402.97' S= 0.0101 '/' Cc= 0.900
			n= 0.013, Flow Area= 1.23 sf

Frimary OutFlow Max=6.72 cfs @ 12.32 hrs HW=407.67' TW=406.38' (Dynamic Tailwater)
—1=Culvert (Inlet Controls 6.72 cfs @ 5.48 fps)

2304141-POST DEVELOPMENT

Type III 24-hr 50-YEAR Rainfall=6.77"

Prepared by Keach-Nordstrom Associates, Inc.

Printed 12/6/2023

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 105

Summary for Pond 26P: CB#20

Inflow Area = 0.702 ac, 34.65% Impervious, Inflow Depth > 4.53" for 50-YEAR event

Inflow = 2.50 cfs @ 12.28 hrs, Volume= 0.27 af

Outflow = 2.50 cfs @ 12.28 hrs, Volume= 0.27 af, Atten= 0%, Lag= 0.0 min

Primary = 2.50 cfs @ 12.28 hrs, Volume= 0.27 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 408.08' @ 12.34 hrs

Flood Elev= 407.78'

Device	Routing	Invert	Outlet Devices
#1	Primary	403.70'	12.0" Round Culvert
	-		L= 26.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 403.70' / 403.44' S= 0.0100 '/' Cc= 0.900
			n= 0.013. Flow Area= 0.79 sf

Primary OutFlow Max=1.99 cfs @ 12.28 hrs HW=407.53' TW=407.26' (Dynamic Tailwater) 1=Culvert (Inlet Controls 1.99 cfs @ 2.54 fps)

Summary for Pond PP: POCKET POND

Inflow Area =	11.270 ac, 12.00% Impervious,	Inflow Depth > 3.90" for 50-YEAR event
Inflow =	33.13 cfs @ 12.25 hrs, Volume	e= 3.66 af
Outflow =	2.71 cfs @ 14.88 hrs, Volume	e= 1.49 af, Atten= 92%, Lag= 158.0 min
Primary =	2.71 cfs @ 14.88 hrs, Volume	e= 1.49 af
Secondary =	0.00 cfs @ 0.00 hrs, Volume	e= 0.00 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs
Starting Elev= 367.00' Surf.Area= 9,015 sf Storage= 7,477 cf
Peak Elev= 373.79' @ 14.88 hrs Surf.Area= 24,499 sf Storage= 113,786 cf (106,309 cf above start)
Flood Elev= 374.00' Surf.Area= 24,964 sf Storage= 118,753 cf (111,276 cf above start)

Plug-Flow detention time= 390.6 min calculated for 1.32 af (36% of inflow) Center-of-Mass det. time= 226.1 min (1,056.5 - 830.4)

Volume	Invert	Avail.Storage	Storage Description
#1	364.00'	118,753 cf	Custom Stage Data (Prismatic) Listed below (Recalc)
#2	369.00'	0 cf	Sediment Forebay (No Storage) (Prismatic) Listed below (Recalc)
Ø			1,678 cf Overall x 0.0% Voids
		119 753 cf	Total Available Storage

118,753 cf Total Available Storage

Type III 24-hr 50-YEAR Rainfall=6.77"

Prepared by Keach-Nordstrom Associates, Inc.

Printed 12/6/2023

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 106

Elevation	Surf.Area	Inc.Store	Cum.Store
(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)
364.00	145	0	0
366.00	1,883	2,028	2,028
367.00	9,015	5,449	7,477
368.00	10,563	9,789	17,266
369.00	12,205	11,384	28,650
370.00	13,939	13,072	41,722
371.00	17,278	15,609	57,331
372.00	19,350	18,314	75,645
374.00	23,758	43,108	118,753
Elevation	Surf.Area	Inc.Store	Cum.Store
(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)
369.00	472	0	0
371.00	1,206	1,678	1,678

Device	Routing	Invert	Outlet Devices
#1	Primary	365.00'	15.0" Round Culvert
			L= 190.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 365.00' / 349.80' S= 0.0800 '/' Cc= 0.900
			n= 0.013, Flow Area= 1.23 sf
#2	Device 1	367.00'	2.5" Vert. Orifice C= 0.600
#3	Device 1	369.45'	3.2" Vert. Orifice C= 0.600
#4	Device 1	371.50'	3.0" Vert. Orifice C= 0.600
#5	Device 1	373.70'	48.0" x 48.0" Horiz. Grate C= 0.600
			Limited to weir flow at low heads
#6	Secondary	373.80'	10.0' long x 27.0' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60
			Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63

Primary OutFlow Max=2.71 cfs @ 14.88 hrs HW=373.79' TW=350.54' (Dynamic Tailwater)

-1=Culvert (Passes 2.71 cfs of 16.88 cfs potential flow)
-2=Orifice (Orifice Controls 0.42 cfs @ 12.45 fps)

3=Orifice (Orifice Controls 0.42 cfs @ 12.43 fps)

-4=Orifice (Orifice Controls 0.35 cfs @ 7.08 fps)

-5=Grate (Weir Controls 1.39 cfs @ 0.98 fps)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=367.00' TW=348.50' (Dynamic Tailwater) 6=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Link A: BARRETTS HILL CROSSING

Inflow Area = 41.139 ac, 8.14% Impervious, Inflow Depth > 2.51" for 50-YEAR event

Inflow = 48.31 cfs @ 12.49 hrs, Volume= 8.59 af

Primary = 48.31 cfs @ 12.49 hrs, Volume= 8.59 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

2304141-POST DEVELOPMENT

Type III 24-hr 50-YEAR Rainfall=6.77"

Prepared by Keach-Nordstrom Associates, Inc.

Printed 12/6/2023

HydroCAD® 10.00-26 s/n 03273 © 2020 HydroCAD Software Solutions LLC

Page 107

Summary for Link B: LOT 57

Inflow Area =

2.097 ac, 14.39% Impervious, Inflow Depth > 4.00" for 50-YEAR event

Inflow = Primary =

6.74 cfs @ 12.26 hrs, Volume= 6.74 cfs @ 12.26 hrs, Volume= 0.70 af 0.70 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Summary for Link C: LOT 11

Inflow Area =

2.594 ac, 9.49% Impervious, Inflow Depth > 3.77" for 50-YEAR event

Inflow =

Primary

7.67 cfs @ 12.29 hrs, Volume= 7.67 cfs @ 12.29 hrs, Volume=

0.82 af 0.82 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

17. RIP RAP APRON CALCULATIONS

KEACH-NORDSTROM ASSOCIATES, INC.

RIP RAP OUTLET PROTECTION APRON CALCULATIONS Barrett Hill, LLC 23-0414-1 Date: 12/6/2023

The purpose of this spreadsheet is to calculate the dimensions of inlet/Outlet Protection apron (fiprap) required during the SCS/NRCS 10-year type III 24-hr storm event. The spllway weir(s) inlet/outlet apron protection will be sized for the SCS/NRCS 10-year type III 24-hr storm event. Required in

Depending on the tall water conditions, either column 1 or column 2 is used for calculations ¥ D ⊡ diameter in feet of outlet or width of channel tall water at end of apron

peak flow in CFS

Width of Apron at outfall W1 = 3*Do W2 = 3*Do + La Length of Apron La = (1.8Q/Do^3/2)+7Do Column Chis winers Tw<1/2Do La = 3*Q/Do^3/2+7Do Column Two where Tw>1/2Do

If defined channel, then use channel width for W1 and W2

W1 = 3*DoW2 = 3*Do+0.4*La

Rock Rip Rap Size:

d50 = (0.02*Q^4/3)/(Tw*Da)

Calculation Summary Table:

Input to Chart
Description (Optional)
HV#1 Pond Outlet
HV#4 Pond inlet
HW#13 Pond inlet

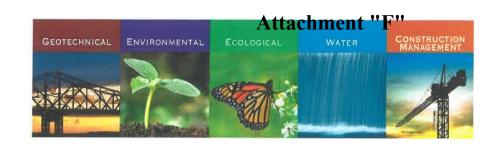
RIRAP GRADATION ENVELOPE

2		b	5	30	s		6	4		o	0	c		0.00	4.0	-	1			
14	a	42	5		-	-		-	-		0	,		20.5	0	21	'n	16	0.45	J.SO
		2	40	10	J		50	4		o	a	a	4	2.00	0.0				2	700
13	*	10	-		,	1		-	1		9	0		320	00	14	5	24	. 13	
		10	5	10	N		6	4	,	ď	0		1	0.70	24.			2	3	20
7.	17.	-						1	4	,	0	2	_	0 70	2	13	4	2	0.17	62.1
	•	•	3	5	5	5	ŝ	5	5	5	5	=	uov III.	000	41.000	011			7	S I
244	LAA	Cellan	Delotte	ocpu.	Ġ						_	ï	250 25	250	450 #	no channel	×1	La	(JL) AA	20 (11)
1	10/4	modh	Danis.	4606	3	FROM	5	FROM	d	FROM	c	TACK	COL			7.44	adout		1	-
	ń	00			1		1	1			H	1	-			CVM		Calculated O		
					Oi.	0.1	d50		d85	a	0100	0								

^{*} Center Apron with Headwall and Outlet Pipe (All Cases)

^{*} Line Apron with 6.0 oz. Geotextile Fabric (All Cases)

18. SITE SPECIFIC SOIL SURVEY REPORT





SITE-SPECIFIC SOIL MAPPING REPORT 75 Barretts Hill Road Tax Map 151, Lot 59 Hudson, New Hampshire

October 2023 File No. 04.0191600.00



PREPARED FOR:

Keach-Nordstrom Associates, Inc. Bedford, New Hampshire

GZA GeoEnvironmental, Inc.

5 Commerce Park North, Suite 201 | Bedford, NH 03110-6984 603-623-3600

Offices Nationwide www.gza.com

Copyright© 2023 GZA GeoEnvironmental, Inc.



VIA EMAIL

October 4, 2023 File No. 04.0191600.00

Mr. Paul Chisholm, PE Keach – Nordstrom Associates, Inc. 10 Commerce Park North, Suite 3 Bedford, New Hampshire 03110

Re: Site-Specific Soil Mapping Report

75 Barretts Hill Road Tax Map 151, Lot 59 Hudson, New Hampshire

Dear Mr. Chisholm:

This report presents the findings of a Site-Specific Soil Survey completed by GZA GeoEnvironmental, Inc. (GZA) for the parcel identified as Tax Map 151, Lot 59, located on Barretts Hill Road in Hudson, New Hampshire ("Site"). This report summarizes the results of fieldwork completed on August 22, 2023, to identify Site soils and develop mapping.

Should you have any questions, please feel free to contact Ms. Lindsey White at 603-232-8753 or lindsey.white@gza.com.

Deborah M. Zarta Gier, CNRP

Consultant/Reviewer

James Long, CWS, CSS

Certified Soil Scientist

Very truly yours,

GZA GEOENVIRONMENTAL, INC.

Lindsey White, CPSS Project Manager

Tracy L. Tarr, CWS, CWB, CESSWI Associate Principal

LEW/DMZ/TLT/JHL: jkm

\\gzabedford\jobs\04jobs\0191600s\04.0191600.00\work\sssm\draft 04.0191600.00 sssm report.dock

Attachment: Site-Specific Soil Mapping Report

Suite 201
Bedford, NH 03210
T: 603.623.3600
F 603.624.9463



Attachment "F" October 4, 2023

Site-Specific Soil Mapping Report

04.0191600.00

TOC | i

TABLE OF CONTENTS

1.0	INTRO	DUCTION	.1
2.0		DDOLOGY	
3.0	RESULT	TS	.2
	3.1	SITE DESCRIPTION	.2
	3.2	SOIL MAP UNIT DESCRIPTIONS	.2
		44B - Montauk, 3 to 8 percent slopes	3
		44C - Montauk, 8 to 15 percent slopes	
		44D - Montauk, 15 to 25 percent slopes	3
		448B - Scituate, 3 to 8 percent slopes	4
		448C - Scituate, 8 to 15 percent slopes	4
		448D - Scituate, 15 to 25 percent slopes	4
	3.3	HYDROLOGIC SOIL GROUP CORRELATION	4
4.0	FINDIN	GS AND CONCLUSIONS	.5



Attachment "F" October 4, 2023

Site-Specific Soil Mapping Report

04.0191600.00 TOC | ii

FIGURE

FIGURE 1

TAX MAP

FIGURE 2

AERIAL OVERVIEW MAP

FIGURE 3

SITE-SPECIFIC SOIL MAP

APPENDICES

APPENDIX A

NATURAL RESOURCE LIMITATIONS

APPENDIX B

PHOTOGRAPH LOG



October 3, 2023 Site-Specific Soil Mapping Report 04.0191600.00 Page | 1

1.0 INTRODUCTION

This report presents the findings of Site-Specific Soil Mapping fieldwork conducted by GZA GeoEnvironmental, Inc. (GZA) on Barretts Hill Road in Hudson, New Hampshire (Tax Map 151, Lot 59) (see Figure 1- Tax Map) during August, 2023. GZA was retained by Keach — Nordstrom Associates, Inc. (KNA) to complete Site-Specific Soil mapping. Wetland delineation was completed separately by KNA. Based on assessing data, the Tax Map 151, Lot 59 is approximately 35-acres in size. Site-Specific Soil Mapping was requested on 24 acres, extending southerly from Barretts Hill Road to the utility line easement as outlined on the attached figure (i.e. the "Site") (see Figure 2 — Aerial Overview Plan). The Site is located in the northern portion of the Town of Hudson, New Hampshire, and is bordered to the north by Barretts Hill Road and to the east and west by residential properties. An existing maintained electric transmission utility easement is located on the southern boundary of the Site. The Site is primarily forested, with a mix of forested and shrub vegetation located in the central and northern portions of the Site. There are no building or other structures on the property, and appears undeveloped. Access is directly off Barretts Hill Road.

GZA understands the Site is currently owned by Helen Stabler. GZA also understands a Site-Specific Soil Map is required to support a New Hampshire Department of Environmental Services Alteration of Terrain Permitting Application, to be completed by KNA (see **Figure 3 – Site-Specific Soil Map**). This report is subject to the Limitations in **Appendix A**.

2.0 METHODOLOGY

The soil mapping of the Site was conducted in accordance with the standards set forth in the Society of Soil Scientists of Northern New England (SSSNNE) Publication No. 3 "Site-Specific Soil Mapping Standards for New Hampshire and Vermont, Version 7.0" dated July 2021 by New Hampshire Certified Soil Scientist James H. Long (CSS #15). The Site-Specific Soil Mapping Standards are based on a universally recognized taxonomic system of soil classification and are supported by national soil mapping standards established by the USDA National Cooperative Soil Survey.

This mapping has been completed based on a combination of publicly available databases and Site-specific data collected by on-Site observations. This report provides soil information including soil drainage classification, physical characteristics, and depth to bedrock (if encountered). GZA assessed soil characteristics through the evaluation of hand-dug soil evaluation units conducted during field investigations on August 22, 2023. In addition, GZA utilized 19 test pit logs provided by KNA for depths to bedrock. GZA completed hand-dug test pits with a tile spade and soil auger. Test pits were dug to a minimum depth of 40 inches for the purpose of evaluating and identifying the soil's characteristics. Locations were selected when changes in slope, vegetation, or soil surface were observed. When changes were noted from one hole to the next involving soil drainage or parent material, a soil boundary was placed on the map between the holes to reflect the transition between the soils as it occurs on the landscape. The slopes of the soil map units were measured in the field using a clinometer and augmented by the topography shown on the Existing Conditions Plan prepared by KNA dated July 26, 2023 (see Figure 1 – Site-Specific Soil Map). For purposes of this report, GZA considered the minimum size delineation area of a Site-Specific Soil Survey map unit as 2,000 square feet.



October 3, 2023 Site-Specific Soil Mapping Report 04.0191600.00

Page | 2

GZA used the following resources during data collection to supplement on-Site observations:

- Natural Resource Conservation Service (NRCS) Web Soil Survey;¹ and
- New Hampshire Statewide Geographic Information System Clearinghouse (NH GRANIT) LIDAR-based Bare Earth Hillshade of the Site².

The NRCS Web Soil Survey (WSS) provides soil data and information produced by the National Cooperative Soil Survey. The WSS data was used to gather information prior to field work to use as a baseline of soil units that may be observed during field investigations. Use of the online resource NH GRANIT LIDAR-Based Bare Earth Hillshade of the project area provided imagery to assist in soil unit delineation, to identify changes in topography to help identify ideal locations to dig auger holes.

The on-Site investigation was conducted on August 22, 2023 using a base plan provided by KNA with a 1:100 scale and 2-foot topographic contours. In accordance with the Site-Specific Soil Mapping standards, the identified individual soil map units were correlated to the New Hampshire State-Wide Numerical Soils Legend maintained by the New Hampshire State Office of the NRCS. Soil characteristics for each of the units comply with the Range in Characteristics described in the Official Series Descriptions for each map unit. In addition, GZA has provided High-Intensity Soil Survey (HISS) soil unit correlations in the event local or State agencies request this data.

3.0 RESULTS

3.1 SITE DESCRIPTION

Based on field observations, the Site is primarily forested in the southern portion of the Site, and primarily dominated by a mix of forested and shrub vegetation in the central and northern portion of the Site. The northern portion of the Site is dominated by Scituate soils (Soil Unit #448). The southern portion of the Site is dominated by Montauk (Soil Unit #44) (see Figure 1 – Site-Specific Soil Map). No human-disturbed soil units observed on Site.

3.2 SOIL MAP UNIT DESCRIPTIONS

Individual soil map units are summarized in Table 1 - Soil Map Units below:

Table 1 – Site-Specific Soil Map Soil Types

Soil ID (SSSM)	Soil Type	Soil ID (HISS)	
44	Montauk (well drained)	223	
448	Scituate (moderately well drained)	323	

¹ www.websoilsurvey.sc.egov.usda.gov/App/HomePage.htm

² https://granitview.unh.edu/html5viewer/index.html?viewer=granit_view



October 3, 2023
Site-Specific Soil Mapping Report

04.0191600.00

Page | 3

Slope designations are itemized below in Table 2.

Table 2 - Site-Specific Soil Map Slope Classifications

Slope Class	SSSM	
Α	0-3%	
В	3-8%	
С	8-15%	
D	15-25%	
E	25-50%	
F	>50%	

The individual soil map unit descriptions of the soils identified on the Site and summarized in **Table 1** are as follows:

44B - Montauk, 3 to 8 percent slopes

This map unit consists of well drained loamy soils formed in lodgment till. They are very deep to bedrock and moderately deep to a densic contact. These soils are typically located on upland hills and moraines.

Typically, the fine sandy loam surface layer is dark brown and about four inches thick. The subsoil is dark yellowish brown and about 30 inches thick. The substratum, to a depth of 40 inches or more, is yellowish brown loamy sand.

Included with this mapping are small areas of slopes less than 3 percent and greater than 8 percent and may contain moderately well-drained Scituate soils. These inclusions make up as much as 5 percent of the map unit.

44C - Montauk, 8 to 15 percent slopes

This map unit consists of well-drained loamy soils formed in lodgment till. They are very deep to bedrock and moderately deep to a densic contact. These soils are typically located on upland hills and moraines.

Typically, the fine sandy loam surface layer is dark brown and about four inches thick. The subsoil is dark yellowish brown and about 30 inches thick. The substratum, to a depth of 40 inches or more, is yellowish brown loamy sand.

Included with this mapping are small areas of slopes less than 8 percent and greater than 15 percent; and may contain moderately well-drained Scituate soils. These inclusions make up as much as 5 percent of the map unit.

44D - Montauk, 15 to 25 percent slopes

This map unit consists of well drained loamy soils formed in lodgment till. They are very deep to bedrock and moderately deep to a densic contact. These soils are typically located on upland hills and moraines.

Typically, the fine sandy loam surface layer is dark brown and about four inches thick. The subsoil is dark yellowish brown and about 30 inches thick. The substratum, to a depth of 40 inches or more, is yellowish brown loamy sand.

Included with this mapping are small areas of slopes less than 15 percent and greater than 25 percent and may contain moderately well-drained Scituate soils. These inclusions make up as much as 5 percent of the map unit.



October 3, 2023 Site-Specific Soil Mapping Report 04.0191600.00 Page | 4

448B - Scituate, 3 to 8 percent slopes

This map unit consists of moderately well drained loamy soils formed in lodgment till. They are very deep to bedrock and moderately deep to a densic contact. These soils are nearly level through moderately steep soils on glaciated uplands.

Typically, the fine sandy loam surface layer is a very dark greyish brown to dark brown about four inches thick. The subsoil is strong brown fine sandy loam and up to 27 inches thick. The substratum, to a depth of 40 inches or more, is yellowish brown loamy sand.

Included with this mapping are small areas of slopes less than 3 percent and greater than 8 percent; and may contain well drained Montauk soils. These inclusions make up as much as 10 percent of the map unit.

448C - Scituate, 8 to 15 percent slopes

This map unit consists of moderately well drained loamy soils formed in lodgment till. They are very deep to bedrock and moderately deep to a densic contact. These soils are nearly level through moderately steep soils on glaciated uplands.

Typically, the fine sandy loam surface layer is a very dark greyish brown to dark brown about four inches thick. The subsoil is strong brown fine sandy loam and up to 27 inches thick. The substratum, to a depth of 40 inches or more, is yellowish-brown loamy sand.

Included with this mapping are small areas of slopes less than 8 percent and greater than 15 percent; and may contain well drained Montauk soils. These inclusions make up as much as 10 percent of the map unit.

448D - Scituate, 15 to 25 percent slopes

This map unit consists of moderately well drained loamy soils formed in lodgment till. They are very deep to bedrock and moderately deep to a densic contact. These soils are nearly level through moderately steep soils on glaciated uplands.

Typically, the fine sandy loam surface layer is a very dark greyish brown to dark brown about four inches thick. The subsoil is strong brown fine sandy loam and up to 27 inches thick. The substratum, to a depth of 40 inches or more, is yellowish brown loamy sand.

Included with this mapping are small areas of slopes less than 15 percent and greater than 25 percent and may contain well-drained Montauk soils. These inclusions make up as much as 10 percent of the map unit.

3.3 HYDROLOGIC SOIL GROUP CORRELATION

In order to correlate the soil map units identified, as part of this soil survey, to the appropriate hydrologic soil group, we referenced the SSSNNE "Ksat Values for New Hampshire Soils, Special Publication No. 5, September 2009." Below Table 3 – Hydrologic Soil Group Correlation provides the correlation of the identified soil map units to the appropriate hydrologic soil group. Identification of correlating hydrologic soil group provides context for infiltration rates for stormwater management planning.

³ www.sssnne.org/publications.html



October 3, 2023 **Site-Specific Soil Mapping Report** 04.0191600.00

Page | 5

Table 3 - Hydrologic Soil Group and KSat Values

Soil ID (SSSM)	Soil Type	Hydrologic Soil Group	Ksat Value (low C) Inch/Hour
44	Montauk	С	0.06 in/hr
448	Scituate	С	0.06 in/hr

4.0 **FINDINGS AND CONCLUSIONS**

GZA has completed Site-Specific Soil Mapping of the Site in support of the proposed residential subdivision project permitting being prepared by KNA. The following is a summary of our findings and conclusions:

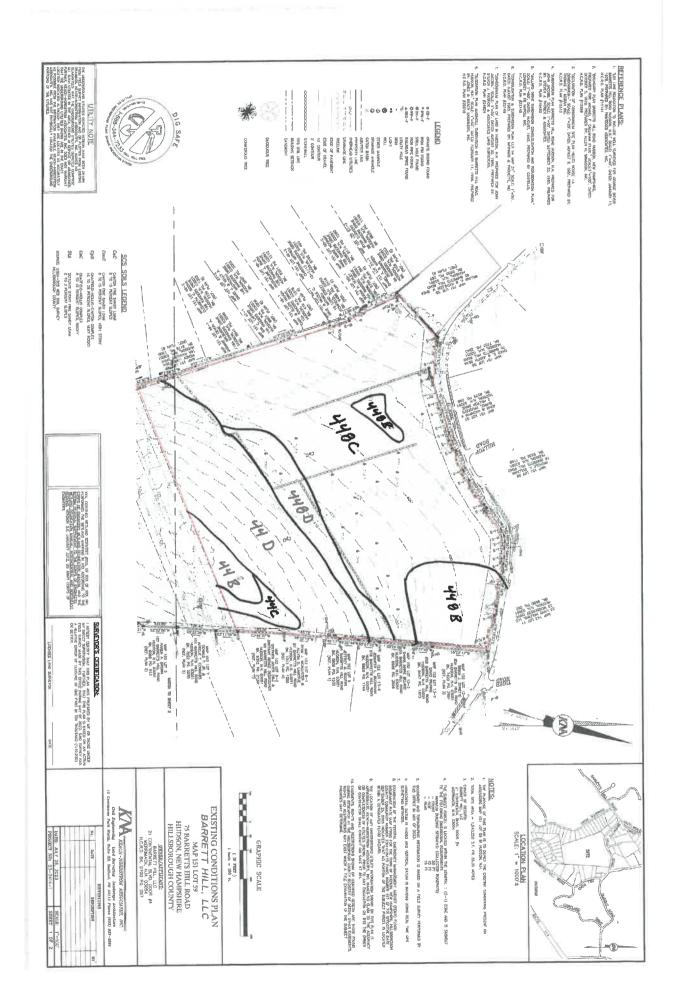
- The Site consists of well drained and moderately well drained soils;
- Moderately well drained Scituate soils were dominant in the northern portion of the Site and well drained Montauk soils were dominant in the southern part of the Site; and
- No poorly drained or very poorly drained soils (i.e. hydric soils) were identified on Site.

\gzabedford\jobs\04jobs\0191600s\04.0191600.00\work\sssm\final 04.0191600.00 sssm report 100323.docx



Figures







Appendix A - Natural Resource Limitations



NATURAL RESOURCE SURVEY AND ASSESSMENT LIMITATIONS

04.0191600.00 Page | 1 June 2023

USE OF REPORT

1. GZA GeoEnvironmental, Inc. (GZA) has prepared this report on behalf of, and for the exclusive use of Main Street Franklin Solar, LLC ("Client") for the stated purpose(s) and location(s) identified in the report. Use of this report, in whole or in part, at other locations, or for other purposes, may lead to inappropriate conclusions; and we do not accept any responsibility for the consequences of such use(s). Further, reliance by any party not identified in the agreement, for any use, without our prior written permission, shall be at that party's risk, and without any liability to GZA.

STANDARD OF CARE

- 2. GZA's findings and conclusions are based on the work conducted as part of the Scope of Services set forth in the Report and/or proposal, and reflect our professional judgment. These findings and conclusions must be considered not as scientific or engineering certainties, but rather as our professional opinions concerning the data gathered and observations made during the course of our work. Conditions other than described in this report may be found at the subject location(s).
- GZA's services were performed using the degree of skill and care ordinarily exercised by qualified professionals performing
 the same type of services, at the same time, under similar conditions, at the same or a similar property. No warranty,
 expressed or implied, is made.

LIMITS TO OBSERVATIONS

- 4. Natural resource characteristics are inherently variable. Biological community composition and diversity can be affected by seasonal, annual or anthropogenic influences. In addition, soil conditions are reflective of subsurface geologic materials, the composition and distribution of which vary spatially.
- 5. The observations described in this report were made on the dates referenced and under the conditions stated therein. Conditions observed and reported by GZA reflect the conditions that could be reasonably observed based upon the visual observations of surface conditions and/or a limited observation of subsurface conditions at the specific time of observation. Such conditions are subject to environmental and circumstantial alteration and may not reflect conditions observable at another time.
- 6. The conclusions and recommendations contained in this report are based upon the data obtained from a limited number of surveys performed during the course of our work on the site, as described in the Report. There may be variations between these surveys and other past or future surveys due to inherent environmental and circumstantial variability.

RELIANCE ON INFORMATION FROM OTHERS

7. Preparation of this Report may have relied upon information made available by Federal, state and local authorities; and/or work products prepared by other professionals as specified in the report. Unless specifically stated, GZA did not attempt to independently verify the accuracy or completeness of that information.

COMPLIANCE WITH REGULATIONS AND CODES

8. GZA's services were performed to render an opinion on the presence and/or condition of natural resources as described in the Report. Standards used to identify or assess these resources as well as regulatory jurisdiction, if any, are stated in the Report. Standards for identification of jurisdictional resources and regulatory control over them may vary between governmental agencies at Federal, state and local levels and are subject to change over time which may affect the conclusions and findings of this report.

GZN

Attachment "F"

NATURAL RESOURCE SURVEY AND ASSESSMENT LIMITATIONS

04.0191600.00 Page | 2 June 2023

NEW INFORMATION

9. In the event that the Client or others authorized to use this report obtain information on environmental regulatory compliance issues at the site not contained in this report, such information shall be brought to GZA's attention forthwith. GZA will evaluate such information and, on the basis of this work, may modify the conclusions stated in this report.

ADDITIONAL SERVICES

10. GZA recommends that we be retained to provide further investigation, if necessary, which would allow GZA to (1) observe compliance with the concepts and recommendations contained herein; (2) evaluate whether the manner of implementation creates a potential new finding; and (3) evaluate whether the manner of implementation affects or changes the conditions on which our opinions were made.



Appendix B - Photograph Log

PHOTOGRAPH LOG Barretts Hill Road Hudson, New Hampshire Photos Taken: August 22, 2023



Photograph No. 1: View facing easterly of landscape in southeastern portion of the Site.



Photograph No. 2: View facing northeasterly of landscape in southwestern portion of the Site.

PHOTOGRAPH LOG Barretts Hill Road Hudson, New Hampshire Photos Taken: August 22, 2023



Photograph No. 3: View of landscape in northwestern portion of the Site.



Photograph No. 4: View of Scituate soils from the center portion of the Site.

PHOTOGRAPH LOG Barretts Hill Road Hudson, New Hampshire Photos Taken: August 22, 2023



Photograph No. 5: View of Montauk soils from the southern portion of the Site.



19. OPERATION and MAINTENANCE PLAN with CHECKLIST

STORMWATER OPERATION & MAINTENANCE PLAN

Barrett Hill Subdivision 75 Barretts Hill Road Hudson, New Hampshire

Map 151; Lot 59

Date: December 6, 2023



TABLE OF CONTENTS:

I. General

Introduction

General Maintenance Requirements

II. Supporting Documents

Annual Inspection & Maintenance Reporting Form
Long-Term Inspection & Maintenance Plan Checklist
Long-Term Inspection & Maintenance Log
Anti-Icing Route Data Form

III. Control of Invasive Plants

Invasive Plant Guide

IV. Stormwater Practice Location Plan

11"x17" "Grading and Drainage Plan"

I. General

Introduction

The project owner or their assigned heirs will maintain the stormwater treatment facilities after construction is completed. The applicant of the project is Barrett Hill, LLC located at 21 Continental Blvd. Door #4, Merrimack, NH. Upon completion of construction, the Homeowner's Association will assume ownership of long-term maintenance.

The subject property is referenced on Map 151; Lot 59 in Hudson, New Hampshire. Any transfer of responsibility for inspection and maintenance activities or transfer of ownership shall be documented to the New Hampshire Department of Environmental Services and the town of Hudson in writing. The contract documents will require the contractor to designate a person responsible for maintenance of the sedimentation control features during construction. Long-term operation and maintenance for the stormwater management facilities are presented below.

Maintenance will be performed as described and required in the Alteration of Terrain Permit unless and until the system is formally accepted by a municipality or quasi-municipal district or is placed under the jurisdiction of a legally created association that will be responsible for the maintenance of the system.

Post Construction:

The following standards will be met after construction is complete:

Documentation:

A maintenance log will be kept summarizing inspections, maintenance, and any corrective actions taken. The log will include the date on which each inspection or maintenance task was performed, a description of the inspection findings or maintenance completed, and the name of the inspector or maintenance personnel performing the task. If a maintenance task requires the clean out of any sediments or debris, the location where the sediment and debris was disposed after removal will be indicated. The log will be made accessible to department and/or Hudson staff and a copy provided upon request.

Maintenance Requirements

Pocket Ponds:

- Systems should be inspected at least twice annually, and following any rainfall event exceeding 2.5 inches in a 24-hour period, with maintenance or rehabilitation conducted as warranted by such inspection.
- System embankments should be mowed periodically to maintain grass cover and any other vegetation found on the embankment should be removed at each inspection.
- Trash and debris found within the pond or in the outlet structure should be removed at each inspection.
- Removal of accumulated sediment
- Inspection and repair of embankments, inlet and outlet structures, and appurtenances

Sediment Forebays:

- Forebays help reduce the sediment load to downstream BMP's, and will therefore require more frequent cleaning.
- Systems should be inspected at least annually.
- Conduct periodic mowing of embankments (generally two times per year) to control growth of woody vegetation.
- Trash and debris should be removed at each inspection.
- Accumulated sediment should be removed as warranted by such inspection.
- Install and maintain a staff gage or other measuring device, to indicate depth of sediment accumulation and level at which clean-out is required.

Catch Basins and Closed Drainage Network:

- Catch basins may require frequent maintenance. This may require several cleanings of the sumps each year. At a minimum, it is recommended that catch basins be inspected at least twice annually.
- Sediment should be removed when it approaches half of the sump depth.
- If floating hydrocarbons are observed during an inspection, the material should be removed immediately by skimming, absorbent materials, or other methods and disposed in conformance with the applicable state and federal regulations.

Outlet Protection:

 Inspect the outlet protection annually for damage and deterioration. Repair damages immediately.

General:

- If any invasive species begin to grow in the stormwater management practices the species shall be disposed of in an appropriate manner that will not allow the pest to survive or spread. The disposal of such species shall be witnessed or approved by a state inspector. Methods for disposal may include, but not be limited to:
 - o Encapsulating the plant(s) in plastic bags and disposing of the plant material in one of the following ways:
 - Trash pickup;
 - Discarding;
 - Open burning;
 - Incineration; or
 - Burial of infested nursery.

II. Supporting Documents

Annual Inspection and Maintenance Reporting Form for Barrett Hill Subdivision Hudson, New Hampshire

Date:	
To: Barrett Hill, LLC	
Re: Certification of Inspection and Ma	intenance; Submittal of Forms
Property Name:	
Property Address:	
Contact Name:	
Contact Phone #:	
Contact Email Address:	
I verify that the required stormwater facility in have been completed in accordance with associated with the above referenced proper. The required Long-Term Inspection & Main this form.	the <u>Operation & Maintenance Plan</u> rty.
Name of Party Responsible for Inspection & Maintenance	Property Owner
Authorized Signature	Signature

Long-Term Inspection & Maintenance Plan Checklist Barrett Hill Subdivision – Hudson, NH

Current Owner Name:	Date:		
Business Address:	Inspector:		
Weather:			
Date of Last Rainfall:	ate of Last Rainfall: Amount: Inches:		
Best Management Practice	I.		
Pocket Pond	Reason for Inspection		
	Spring		
Maintenance Required? Corrective Action Needed & Notes:	Yes □ No □		
Sideslopes & berms need repair? Clean inlet & outlet structures?	Yes No Yes No		
Sediment Forebays	Reason for Inspection		
	Spring ☐ Fall/Yearly ☐ After Major Storm		
Maintenance Required? Corrective Action Needed & Notes:	Yes No		
Catch Basins & Closed Drainage	Reason for Inspection		
Network	Spring		
Maintenance Required? Corrective Action Needed & Notes:	Yes No		
Outlet Protection	Reason for Inspection		
	Spring ☐ Fall/Yearly ☐ After Major Storm		
Maintenance Required? Corrective Action Needed & Notes:	Yes No		

Attachment "F"

	Fall/Yearly	☐ After Major Storm ☐
Ν	lo 🗆	
	Ν	No 🗆

Long-Term Inspection & Maintenance Log Barrett Hill Subdivision – Hudson, NH

Date	Inspection (Yes or No)	Maintenance (Yes or No)	List BMPs Inspected and/or Provide Comments	Inspected By:

Anti-icing Route Data Form Barrett Hill Subdivision – Hudson, NH

Truck Station:				
Date:				
Temperature:	Pavement Temperature:	Relative Humidity:	Dew Point:	Sky:
Reason For Ap	plying:			
Route:				
Chemical:				
Application Tim	e:			
Application Amo	ount:			
Observation (fir	st day):			
Observation (af	ter event):			
Observation (be	fore next application):			
Name:				

III. Control of Invasive Plants

Invasive plants are introduced, alien, or non-native plants, which have been moved by people from their native habitat to a new area. Some Exotic plants are imported for human use such as landscaping, erosion control, or food crops. They also can arrive as "hitchhikers" among shipments of other plants, seeds, packing materials, or fresh produce. Some exotic plants become invasive and cause harm by:

- becoming weedy and overgrown;
- killing established shade trees;
- obstructing pipes and drainage systems;
- forming dense beds in water;
- lowering water levels in lakes, streams, and wetlands;
- destroying natural communities;
- promoting erosion on stream banks and hillsides; and
- resisting control except by hazardous chemical.

During maintenance activities, check for the presence of invasive plants and suitably remove according to the methods provided in the table below. The following table, based on the "Control of Invasive Plants" published by the New Hampshire Department of Agriculture, describes the most common invasive plants in this region and proper methods of disposal.

Name	Description	Invasive Qualities	Control Methods
		Invasive Trees	
Norway Maple	- Large leaves - Will exude milky white sap when leaves are broken - Leaves turn color in Late October (fall foliage is yellow)	- Suppresses growth of grass, garden plants, and forest understory -Wind-borne seeds can germinate and grow in deep shade	 Pull seedlings and small or shallow-rooted plants when soil is moist. Dig out plants, including the root systems. Use a forked spade or weed wrench. Cut down the tree. Grind out the stump, or clip off re-growth. Girdle¹ Frill² Cut stem/ cut stump with glyphosate. Follow label directions for cut stump application. Clip off sucker sprouts or paint with glyphosate.* Foliar spray with glyphosate ^{3*} (mid-October to early November).
Tree of Heaven	- Long compound leaves with 11-25 lance shaped leaflets - Smell like peanut butter or burnt coffee when crushed	- Tough, can grow in poor conditions - Produces large quantities of wind-borne seeds - Grows rapidly - Secretes a toxin that kills other plants - Cannot be removed by mechanical means alone	 Pull seedlings when soil is moist. Frill² (no more than 1" gap between cuts). Use Garlon 3a herbicide. Cut stem/ cut stump with Garlon 3a. Follow label directions for cut stump application. Clip off sucker sprouts or paint with Garlon 3a.* Foliar spray³* (on regrowth) Paint bottom 12" of bark with Garlon 4 Ultra (February/March). Use maximum strength specified on label for all herbicide applications.
	di-		
		Invasive Shrubs	Doll dl' l II
Autumn Olive	- Formerly recommended for erosion control and wildlife value	- Highly invasive, diminishes the overall quality of wildlife habitat	 Pull seedlings and small or shallow-rooted plants when soil is moist. Dig out larger plants, including the root systems. Use a forked spade or weed wrench for trees or shrubs (up to 4" diameter trunks). Cut down the tree. Grind out the stump, or clip off re-growth. Cut stem/ cut stump with glyphosate. Follow label directions for cut stump application. Clip off sucker sprouts or paint with glyphosate.* Bury stump Do not mow

	Invas	ive Shrubs (continued)	
Multiflora Rose	 Formerly recommended for erosion control, hedges, and wildlife habitat Covered in white flowers in June Very hard, curved thorns Fringed edge to leaf stalk 	- Huge shrub that chokes out all other vegetation - Too dense for most birds to nest in - Grows up trees like a vine in Shade	 Pull seedlings and small or shallow-rooted plants when soil is moist. Dig out larger plants, including the root systems (at least 6" from the crown and 6" down). Use a forked spade or weed wrench for trees or shrubs. Controlled burning⁴ (on extensive infestations) Cut stem/ cut stump with glyphosate. Follow label directions for cut stump application. Clip off sucker sprouts or paint with glyphosate.* Foliar spray^{3*} (mix Rodeo with extra sticker-spreader, or use Roundup Sure Shot Foam on small plants) Herbicide may be applied in winter when other plants are dormant.
Bush Honeysuckles	- Includes Belle, Amur, Morrow's, and Tatarian Honeysuckle	- Creates dense shade reducing plant diversity and eliminating nest sites in forest interior spaces	 Deadhead to prevent spread of seeds (on ornamentals). Cut off seeds or fruits before they ripen. Bag and burn, or send to a landfill. Pull seedlings and small or shallow-rooted plants when soil is moist. Dig out larger plants, including the root systems. Use a forked spade or weed wrench for trees or shrubs. Mow or cutting at least 4 times a season to deplete plants' store of nutrients and carbohydrates, reduce seed formation, and kill or minimize spread of plants. If necessary, repeat each year (on shady sites only, brush cut in early spring and fall). Controlled burning⁴ (during growing season) Cut down the tree. Grind out the stump, or clip off re-growth. Cut stem/ cut stump with Glyphosate (late in the growing season). Follow label directions for cut stump application. Clip off sucker sprouts or paint with glyphosate.*

	Invas	ive Shrubs (continued)	
Blunt-Leaved Privet	- Medium sized shrub - Simple, oblong, dark green leaves 1-2" in length - Fragrant white flowers (spring) - Blackish-purple fruit (late summer)	- Toxic to mammals - Loss of valuable habitat	 Pull seedlings and small or shallow-rooted plants when soil is moist. Dig out larger plants, including the root systems. Use a forked spade or weed wrench for trees or shrubs. Cut down the tree. Grind out the stump, or clip off re-growth. Cut stem/ cut stump with Glyphosate. Follow label directions for cut stump application. Clip off sucker sprouts or paint with glyphosate.* Trim off all flowers Do not cut back or mow
Burning Bush, Winged Euonymus	- Wide, corky wings on the Branches - Brilliant red autumn leaves - Fruit	- High seed production	 Pull seedlings and small or shallow-rooted plants when soil is moist. Dig out larger plants, including the root systems. Use a forked spade or weed wrench for trees or shrubs. Cut down the tree. Grind out the stump, or clip off re-growth. Cut stem/ cut stump with Glyphosate. Follow label directions for cut stump application. Clip off sucker sprouts or paint with glyphosate.* Trim off all flowers
Japanese Barberry	- Spiny deciduous shrub - Small leaves	 Very dense, displaces native plants Can change chemistry of soil 	 Pull seedlings and small or shallow-rooted plants when soil is moist. Dig out larger plants, including the root systems. Use a forked spade or weed wrench for trees or shrubs. Cut down the tree. Grind out the stump, or clip off re-growth. Cut stem/ cut stump with Glyphosate. Follow label directions for cut stump application. Clip off sucker sprouts or paint with glyphosate.* Trim off all flowers

	Ir	nvasive Woody Vines	
Japanese Honeysuckle	- Gold and White flowers - Heavy scent and sweet nectar in June	- Shade shrubs and young trees of the forest understory, eventually killing them, and changing the open structure of the forest into a dense tangle - Rampant grower - Spirals around trees, often strangling them	 Pull seedlings and small or shallow-rooted plants when soil is moist. Dig out larger plants, including the root systems. Use a forked spade or weed wrench for trees or shrubs. Mow or cutting at least 4 times a season to deplete plants' store of nutrients and carbohydrates, reduce seed formation, and kill or minimize spread of plants. If necessary, repeat each year. Cut stem/ cut stump with Glyphosate. Follow label directions for cut stump application. Clip off sucker sprouts or paint with glyphosate.* Foliar spray^{3*} (fall or early spring when native vegetation is dormant) Plan to re-treat repeatedly
Oriental Bittersweet	- Bright orange seed capsules in clusters all along the stem - Flowers	- Shade shrubs and young trees of the forest understory, eventually killing them, and changing the open structure of the forest into a dense tangle	 Pull seedlings and small or shallow-rooted plants when soil is moist. Dig out larger plants, including the root systems. Use a forked spade or weed wrench for trees or shrubs. Keep ornamental plants cut back, remove all fruits as soon as they open, and bag or burn fruits. Cut stem/ cut stump with Garlon 3a. Follow label directions for cut stump application. Clip off sucker sprouts or paint with Garlon 3a.*
Japanese Knotweed, Mexican Bamboo	- The stems have knotty joints, similar to bamboo - Grows 6-10' tall - Large, pointed oval or triangular leaves	- Shade shrubs and young trees of the forest understory, eventually killing them, and changing the open structure of the forest into a dense tangle - Can grow in shade	- Cut stem/ cut stump with Glyphosate (at least 3 times each during growing season). Follow label directions for cut stump application. Clip off sucker sprouts or paint with glyphosate.* - Foliar spray ^{3*} - Treat with Rodeo - In gardens, heavy mulch or dense shade may kill it.

	Invas	ive Herbaceous Plants	
Garlic Mustard	- White-flowered biennial - Rough scalloped leaves (kidney, heart, or arrow shaped) - Garlic smell, mustard taste when its leaves are crushed	- Shade shrubs and young trees of the forest understory, eventually killing them, and changing the open structure of the forest into a dense tangle - Rampant grower - Spirals around trees, often strangling them	 Pull seedlings and small or shallow-rooted plants when soil is moist (before it flowers in spring). Dig out larger plants, including the crown and root systems. Use a forked spade or weed wrench for trees or shrubs. Tamp down soil afterwards. Deadhead to prevent spread of seeds. Cut off seeds or fruits before they ripen. Bag and burn or send to a landfill. Foliar spray^{3*} (may be appropriate in some settings)
Japanese Stilt Grass	- Lime green color - Line of silvery hairs down the middle of the 2-3" long blade	- Tolerates sun or dense shade -Quickly invades areas left bare or disturbed by tilling or flooding - Builds a large seed bank in the soil	 Pull seedlings and small or shallow-rooted plants when soil is moist (pulled easily in early to midsummer). Dig out larger plants, including root systems. Use a forked spade or weed wrench for trees or shrubs. Be sure to pull before it goes to seed. If seeds have formed, bag and burn or send to a landfill. Mow or cutting at least 4 times a season to deplete plants' store of nutrients and carbohydrates, reduce seed formation, and kill or minimize spread of plants. If necessary, repeat each year. Mowing weekly or when it has just begun to flower may prevent it from setting seed. Foliar spray³* (use glyphosate or herbicidal soap on large infestations. Use a corn-based pre-emergence herbicide on annual weeds (spring). This product is also an organic fertilizer, i.e., it can stimulate growth of existing plants, including weeds, so it is appropriate for lawns and gardens but may not be appropriate in woodlands.

	Invasive He	erbaceous Plants (continu	ued)
Mile-A-Minute Vine, Devil's Tail Tearthumb	- Triangular leaves - Barbed stems - Turquoise berries	- Rapid growth - Quickly covers and shades out herbaceous plants	 Pull seedlings and small or shallow-rooted plants when soil is moist (pulled easily in early to midsummer). Dig out larger plants, including root systems. Use a forked spade or weed wrench for trees or shrubs. Be sure to pull before it goes to seed. If seeds have formed, bag and burn or send to a landfill. Mow or cutting at least 4 times a season to deplete plants' store of nutrients and carbohydrates, reduce seed formation, and kill or minimize spread of plants. If necessary, repeat each year. Mowing weekly or when it has just begun to flower may prevent it from setting seed. Foliar spray³* (use glyphosate or herbicidal soap on large infestations. Use a corn-based pre-emergence herbicide on annual weeds (spring). This product is also an organic fertilizer, i.e., it can stimulate growth of existing plants, including weeds, so it is appropriate for lawns and gardens but may not be appropriate in woodlands.
Spotted Knapweed	- Thistle-like flowers	- Dense, crowds out native species	 Do not pull unless the plant is young and the ground is very soft. The root will break and produce several new plants. Wear sturdy gloves Deadhead to prevent spread of seeds. Cut off seeds or fruits before they ripen. Bag and burn, or send to a landfill. In lawns, spot treat with broad-leaf weed killer. Good lawn care practices (test soil; use lime and fertilizer only when soil test shows a need; mow high and frequently; leave clippings on lawn) reduce weed infestations. Cut stem/ cut stump with Glyphosate. Follow label directions for cut stump application. Clip off sucker sprouts or paint with glyphosate.* Foliar spray^{3*}

<u>¹Girdle:</u> Cut through the bark and growing layer all around the trunk, about 6" above the ground. Girdling is most effective in spring (when the sap is rising) & middle-late summer (when the tree is sending food to the roots). Clip off sucker sprouts.

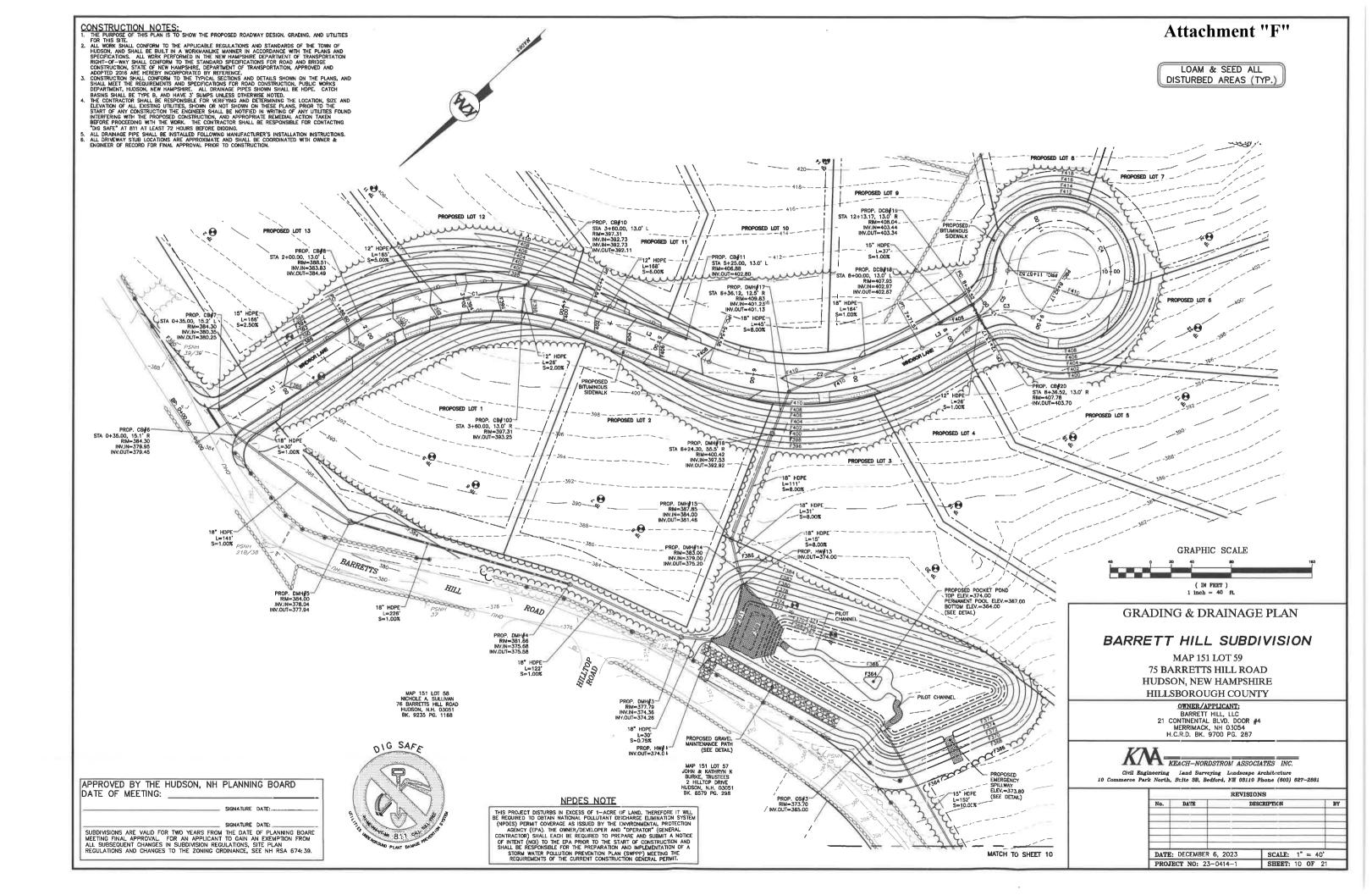
²<u>Frill:</u> Using a machete, hatchet, or similar device, hack scars (several holes in larger trees) downward into the growing layer, and squirt in glyphosate (or triclopyr if specified in table). Follow label directions for injection and frill applications. This is most effective from middle to late summer. Clip off any sucker sprouts or treat with glyphosate.

<u>³Foliar Spray:</u> Use a backpack or garden sprayer or mist blower, following label directions. Avoid overspray and/or dripping onto non-target plants, because glyphosate kills most plants except moss. If it rolls off waxy or grass-like foliage, use additional sticker-spreader. Deciduous trees, shrubs, and perennials move nutrients down to the roots in late summer. Glyphosate is particularly effective at this time and when plants have just gone out of flowering. Several invasive species retain their foliage after native plants have lost theirs, and resume growth earlier in spring than most natives. This allows you to treat them without harming the natives. However, the plant must be actively growing for the herbicide to work. Retreatments may be necessary the following year if suckering occurs or the plant hasn't been entirely killed.

<u>4Controlled Burning:</u> Burning during the spring (repeated over several years) will allow native vegetation to compete more effectively with the invasive species. This requires a permit. Spot treatment with glyphosate in late fall can be used to make this method more effective

*Herbicides: It is highly recommended that small populations try to be controlled using non-chemical methods where feasible. However, for large infestations, and for a few plants herbicide use is essential. Apply herbicides carefully to avoid non-target plants, glyphosate is the least environmentally damaging herbicide in most cases. Add food coloring for visibility, and a soap-based sticker such as Cide-Kick. Glyphosate is ineffective on some plants; for these, triclopyr or Garlon 3a may be indicated. When using herbicides read the entire label and observe all precautions listed, including proper disposal. If in doubt, call your local Cooperative Extension Service.

IV. Stormwater Practice Location Plan

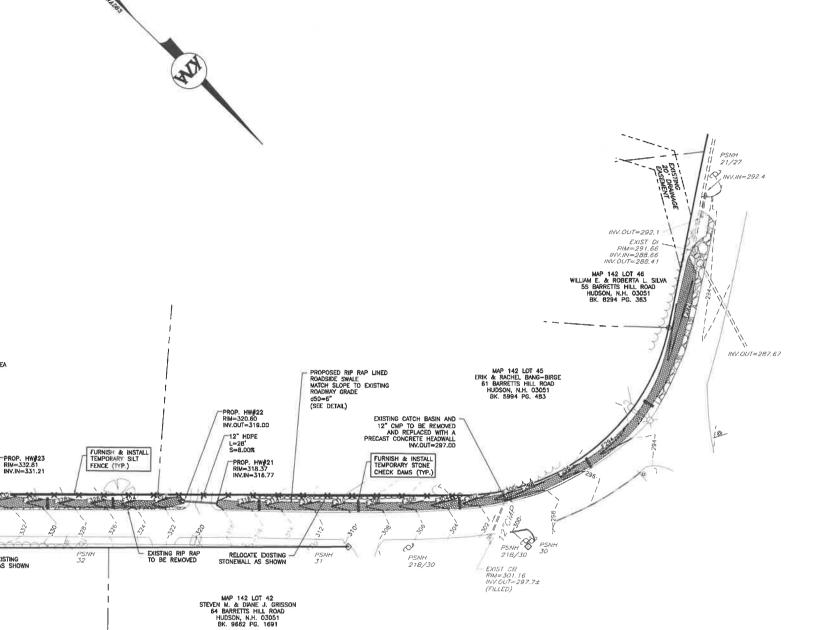




LOAM & SEED ALL DISTURBED AREAS (TYP.)

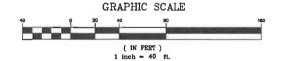
SEE SHEET 10 FOR NOTES

SEE SHEET 12 FOR EROSION CONTROL NOTES & LEGEND



LEGEND

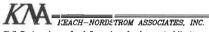
GRANITE BOUND FOUND IRON PIN FOUND DRILL HOLE FOUND @ IPIN-F O IPP-F IRON PIPE FOUND RAILROAD SPIKE FOUND UTILITY POLE LIGHT ((() WELL SEWER MANHOLE DRAINAGE MANHOLE CATCH BASIN ABUTTER LINE PROPERTY LINE STREAM OVERHEAD UTILITIES ====== DRAINAGE LINE TREELINE ______ 10' CONTOUR STONEWALL
BUILDING SETBACK EASEMENT PROPOSED WATER VALVE PROPOSED HYDRANT PROPOSED WELL PROPOSED DRAINAGE MANHOLE PROPOSED CATCH BASIN PROPOSED PROPERTY LINE PROPOSED GUARDRAIL 1160 ----PROPOSED UNDERGROUND UTILITIES PROPOSED GAS LINE PROPOSED WATER LINE PROPOSED DRAINAGE LINE . PROPOSED TREELINE PROPOSED EDGE OF PAVEMENT PROPOSED SLOPED GRANITE CURB PROPOSED 2' CONTOUR



OFFSITE GRADING, DRAINAGE & EROSION CONTROL PLAN BARRETT HILL SUBDIVISION

MAP 151 LOT 59 75 BARRETTS HILL ROAD HUDSON, NEW HAMPSHIRE HILLSBOROUGH COUNTY

OWNER/APPLICANT:
BARRETT HILL, LLC
21 (:ONTINENTAL BLVD. DOOR #4
MERRIMACK, IIH 03054
H.C.R.D. BK. 9700 PG. 287



Civil Engineering Land Surveying Landscape Architecture 10 Commerce Park North, Suite 3B, Bedford, NH 03110 Phone (603) 627-2881

		REVISI	ONS	
No.	io. DATE	DESCRIPTION		
DATE:	DECEMBER	6, 2023	SCALE: 1" = 40'	=
PROJ	ECT NO: 23	3-0414-1	SHEET: 11 OF 21	



12" HDPE-L=22" S=8.00%

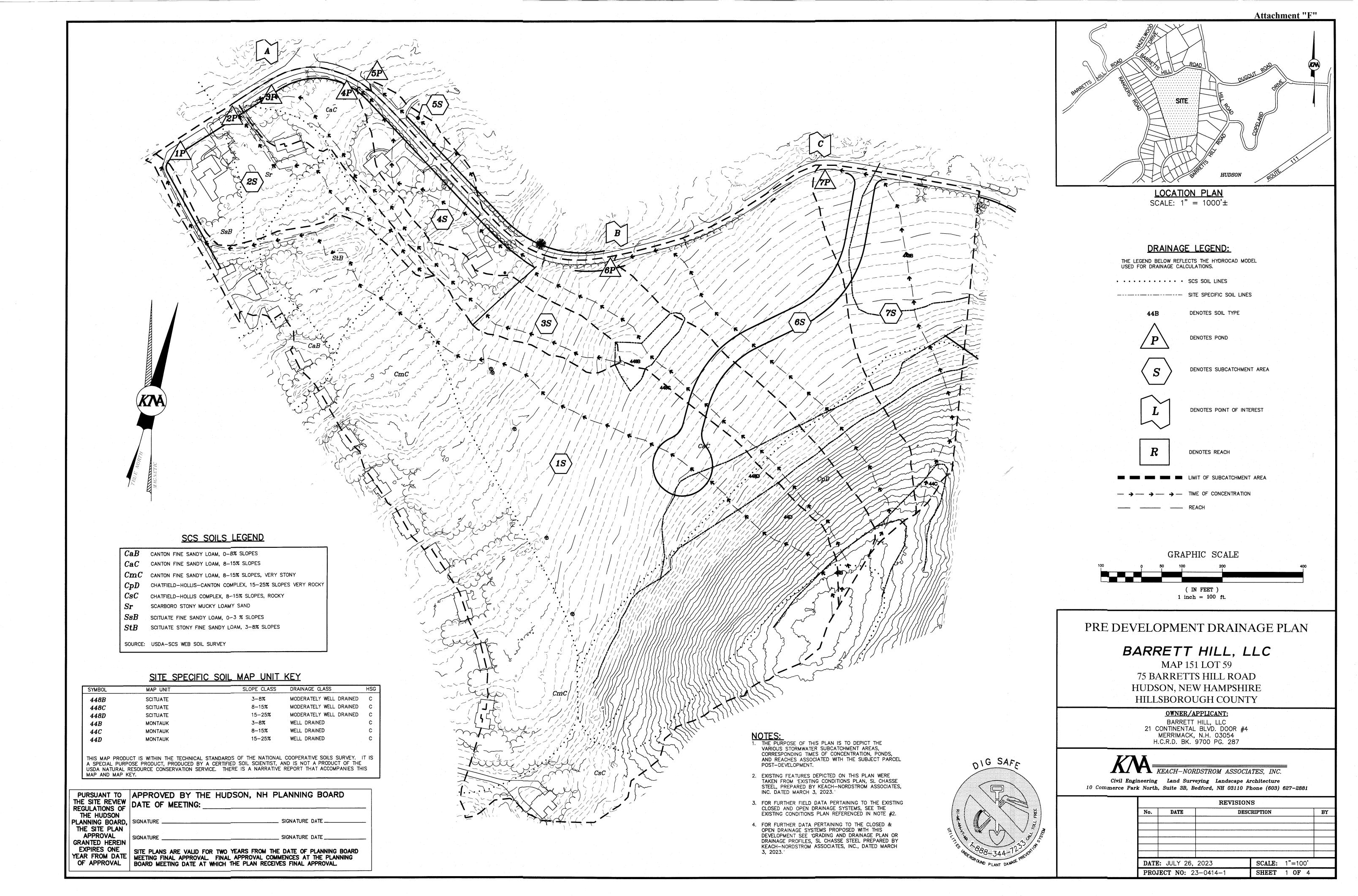
218/33

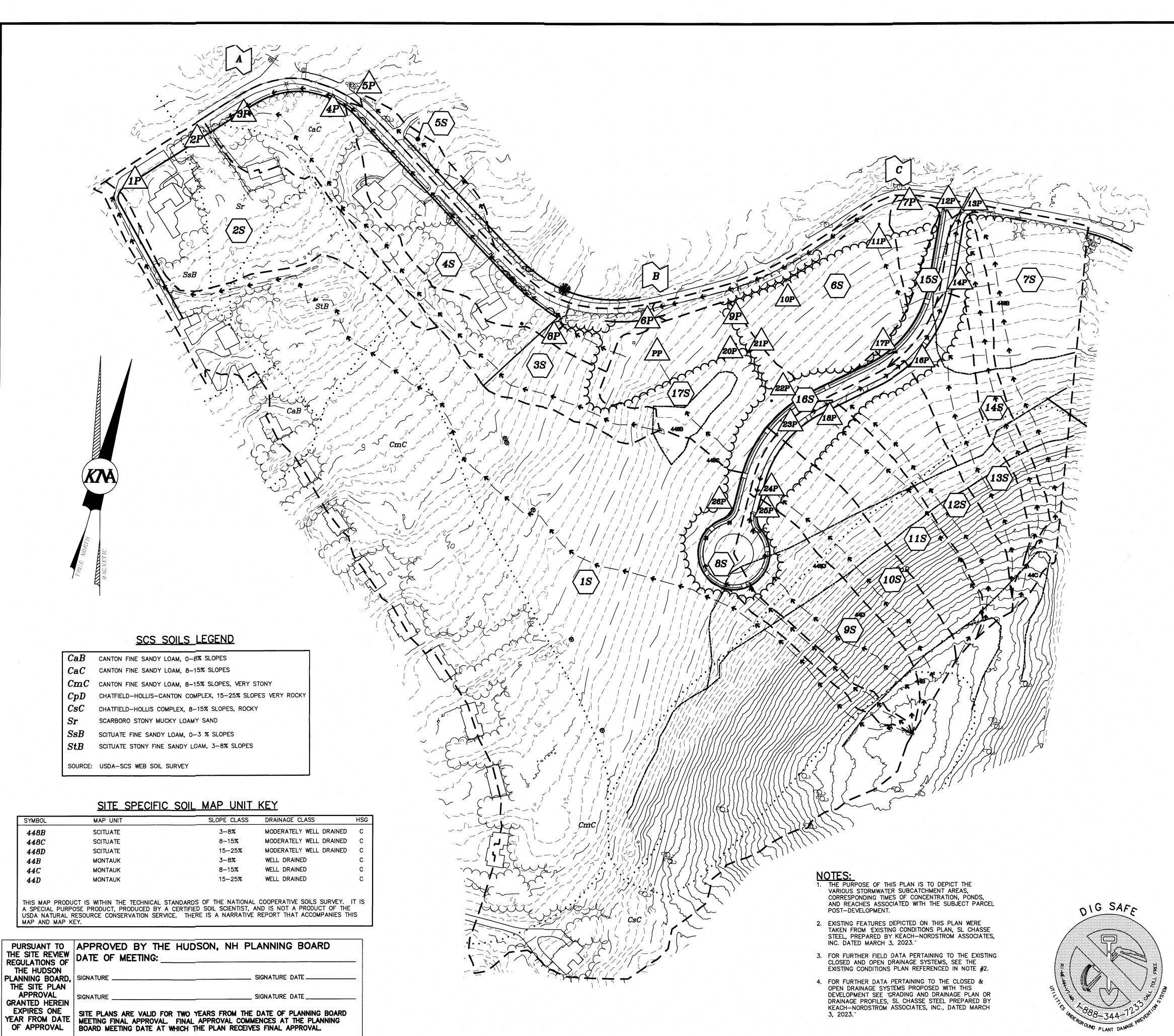
 RELOCATE EXISTING STONEWALL AS SHOWN

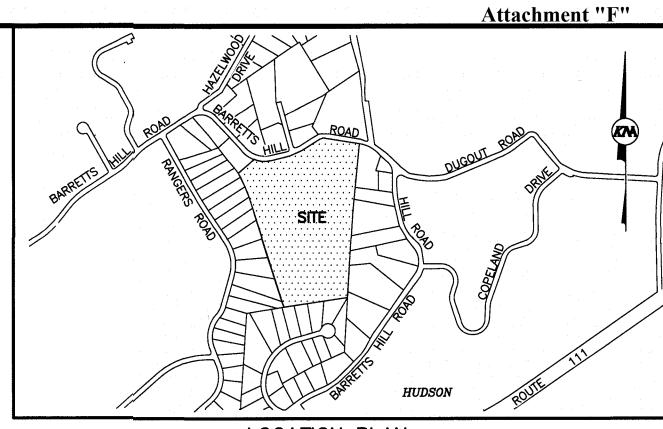
NPDES_NOTE

THIS PROJECT DISTURBS IN EXCESS OF 1-ACRE OF LAND. THEREFORE IT WILL BE REQUIRED TO OBTAIN NATIONAL POLLUTANT DISCHARGE ELIMINATION SYSTEM (NPDES) PERMIT COVERAGE AS ISSUED BY THE ENVIRONMENTAL PROTECTION AGENCY (EPA). THE DWINER/DEVELOPER AND "OPERATOR" (GENERAL CONTRACTOR) SHALL EACH BE REQUIRED TO PREPARE AND SUBMIT A NOTICE OF INTENT (NO) TO THE EPA PRIOR TO THE STATA OF CONSTRUCTION AND SHALL BE RESPONSIBLE FOR THE PREPARATION AND IMPLEMENTATION OF A STORM WATER POLLUTION PEACH (SWPPP) MEETING THE REQUIREMENTS OF THE CURRENT CONSTRUCTION GENERAL PERMIT.

SUBDIVISIONS ARE VALID FOR TWO YEARS FROM THE DATE OF PLANNING BOARD MEETING FINAL APPROVAL. FOR AN APPLICANT TO GAIN AN EXEMPTION FROM ALL SUBSEQUENT CHANGES IN SUBDIVISION REGULATIONS, SITE PLAN REGULATIONS AND CHANGES TO THE ZONING ORDINANCE, SEE NH RSA 674:39.







LOCATION PLAN

SCALE: $1" = 1000' \pm$

DRAINAGE LEGEND:

THE LEGEND BELOW REFLECTS THE HYDROCAD MODEL USED FOR DRAINAGE CALCULATIONS. · · · · · · · · · · · · SCS SOIL LINES

DENOTES SOIL TYPE

DENOTES POND



DENOTES SUBCATCHMENT AREA



DENOTES POINT OF INTEREST

DENOTES REACH

 \rightarrow \rightarrow \rightarrow \rightarrow TIME OF CONCENTRATION

GRAPHIC SCALE

(IN FEET) 1 inch = 100 ft.

POST DEVELOPMENT DRAINAGE PLAN

BARRETT HILL, LLC

MAP 151 LOT 59 75 BARRETTS HILL ROAD HUDSON, NEW HAMPSHIRE HILLSBOROUGH COUNTY

OWNER/APPLICANT:

BARRETT HILL, LLC 21 CONTINENTAL BLVD. DOOR #4 MERRIMACK, N.H. 03054 H.C.R.D. BK. 9700 PG. 287

KEACH-NORDSTROM ASSOCIATES, INC. Civil Engineering Land Surveying Landscape Architecture 10 Commerce Park North, Suite 3B, Bedford, NH 03110 Phone (603) 627-2881

		REVI	SIONS	
No.	DATE	DESCRIPTION		
DATE	JULY 26,	2023	SCALE: 1"=100)'.
PROJ	ECT NO: 2	3-0414-1	SHEET 2 OF	4



TOWN OF HUDSON



Planning Board

Timothy Malley, Chairman

12 School Street · Hudson, New Hampshire 03051 · Tel: 603-886-6008 · Fax: 603-594-1142

CAP FEE WORKSHEET - 2024

Project Na	me: SB# 08	-23 Barrett Hill LLC OSD	475 Barretts Hill Rd. Subdivision
Proposed I	TE Use #1:	Single Fami	ly
Proposed Building Area (square footage):			N/A S.F.
CAP FEES	S: (ONE CHEC	K NEEDED)	
1.	(Bank 09) 2070-701	Traffic Improvements	\$ 2,216.00
2.	(Bank 09) 2050-182	Recreation	<u>\$ 400.00</u>
3.	(Bank 09) 2080-051	School	\$ 3,578.00
		Total CAP Fee	\$ 6,194.00
		ble to the Town of Hudson.	